



# **DRAINAGE STRATEGY**

**ISSUE 2**

**September 2026**

**The Seven Stars**

**Gores Lane**

**Bottlesford**

**Pewsey**

**SN9 6LW**

## Document Control

|                                 |                             |              |            |
|---------------------------------|-----------------------------|--------------|------------|
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| <b>Report title</b>             | Drainage Strategy           |              |            |
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| <b>Prepared by</b>              | A.Dixon BSc(Hons) CEng MICE |              |            |
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#### APPENDIX

Appendix A – Drawings by DC

#### TITLE

Proposed  
Strategy

Drainage

#### PRODUCED BY

Dixon Civils

Appendix B – Drawings by others

Site Plan

Milton Architects

Location Plan

Milton Architects

Topographical Survey

Dando Surveying

Appendix C – Sewer Records

Appendix D – BRE 365 Infiltration Test Results

Appendix E – Infodrainage Calculations

## 1.0 Introduction

This drainage strategy report has been prepared on behalf of Meeru Property Services for the proposed redevelopment and extension of the Seven Stars pub in Bottlesford, Pewsey. This report will introduce the foul and surface water design and demonstrate how the appropriate means of sustainable drainage (SuDS) will be achieved for the development site in line with national guidance.

Refer to the Location Plan (2601-1\_001-P1) and Proposed Site Plan (2601-3\_101-P1) prepared by Milton Architects within Appendix B this report.

## 2.0 The Existing Site

The application site covers 4690m<sup>2</sup> (0.469ha) of land occupied by the existing pub and associated gardens and car park.

The site is centred on Ordnance Survey grid reference SU 11260 59228 and co-ordinates:

E411260, N159228. The post code for the site is SN9 6LW.

Vehicular access to the site is gained from the south via an existing entrance off Gores Lane.

The site is located within flood zone 1 and is bounded to the north by open fields and Moonraker Farm, to the east by residential properties, to the west by an unnamed watercourse and to the south by Gores Lane and agricultural fields beyond. Land use in the vicinity is predominantly residential and agricultural. The site falls from northeast to southwest, toward the watercourse along the western boundary. Existing ground levels within the application site vary from approximately 113.80mAOD at the site's northeastern extent, to 108.50mAOD at its southwestern extent.

A topographical survey by Dando Surveying is included in Appendix B.

### 2.1 Existing ground conditions

British Geological Survey (BGS) mapping indicates that the site is underlain by the Upper Greensand Formation, comprising glauconitic sandstone.

Defra's Multi-Agency Geographic Information for the Countryside (MAGIC) mapping indicates that the site lies within the River Avon System SSSI. The underlying geology is classified as a Secondary A aquifer. The site is not located within a groundwater Source Protection Zone but is identified as having high groundwater vulnerability

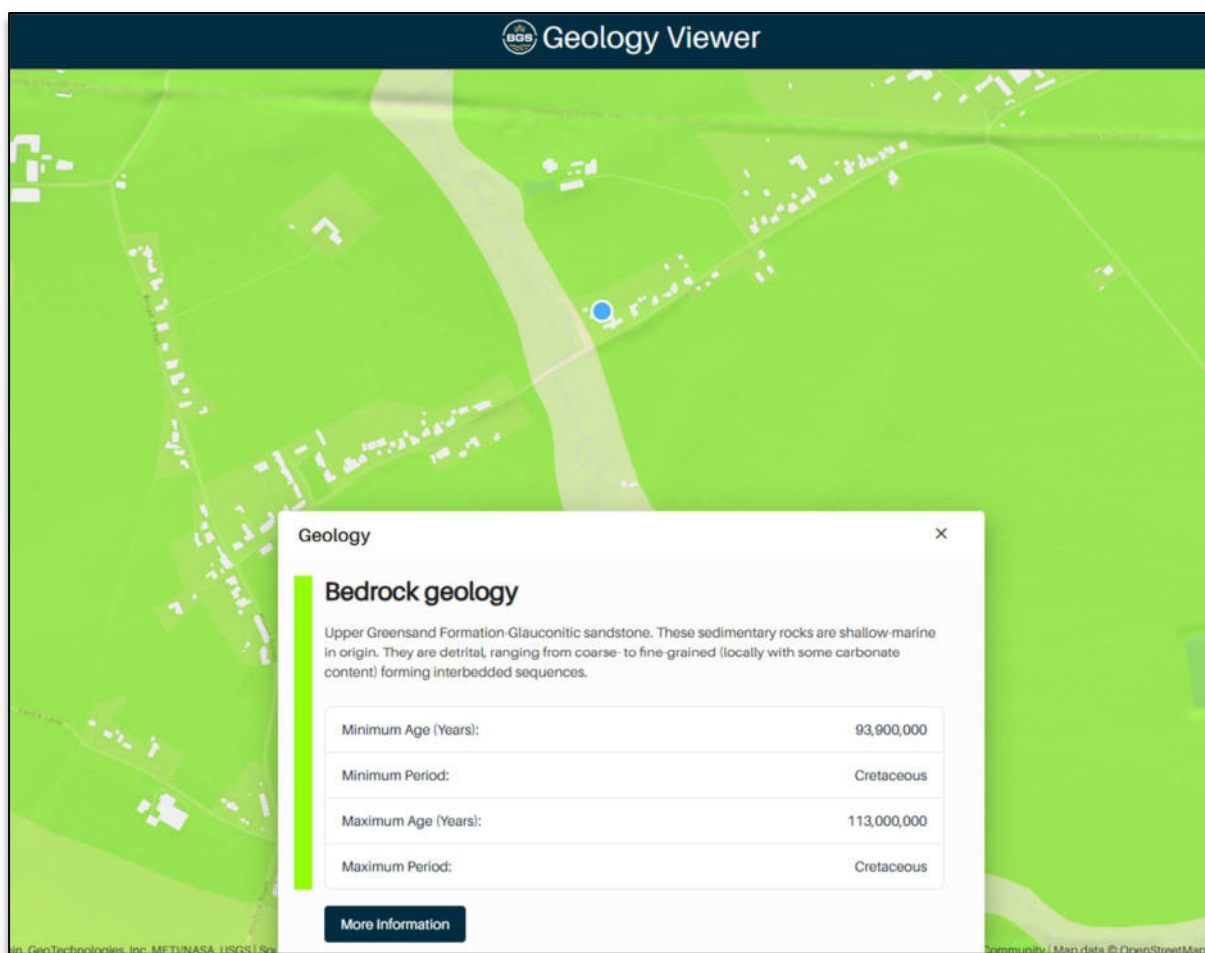


**Figure 1: Bedrock Geology - Extract from British Geological Survey Geology of Britain Viewer**  
Source: <https://www.bgs.ac.uk/map-viewers/bgs-geology-viewer>

## 2.2 Intrusive Site Investigations

An intrusive site investigation was undertaken in June 2026, comprising four trial pits excavated to depths of between 1.5 m and 2.0 m below ground level. Infiltration testing was undertaken within each trial pit in accordance with BRE 365. The recorded infiltration rates are summarised below, with the raw test data included within Appendix D.

The investigation confirmed that the site is underlain by greensand soils with varying clay content. Groundwater was not encountered within the trial pits.

| Test Number          | Recorded Infiltration Rate (m/s) |
|----------------------|----------------------------------|
| Trial Pit 1 – Test 1 | 3.086 x 10-6                     |
| Trial Pit 2 – Test 1 | 3.018 x 10-6                     |
| Trial Pit 2 – Test 2 | 5.279 x 10-6                     |
| Trial Pit 3 – Test 1 | Failed                           |
| Trial Pit 4 – Test 1 | 1.891 x 10-5                     |
| Trial Pit 4 – Test 2 | 3.468 x 10-5                     |



### **2.3 Existing Watercourses and Drainage Features**

The western boundary of the site is bordered by a watercourse flowing from north to south. The watercourse passes beneath Gores Lane before continuing southwards and ultimately discharging to the River Avon approximately 2.5 km southeast of the site.

Asset records obtained from Wessex Water indicate the presence of public gravity foul water sewers within Gores Lane to the south of the site, which discharge towards a Wessex Water pumping station adjacent to the southwestern corner of the site. The existing pub currently drains via private foul drainage within the site before connecting to the public sewer network.

Refer to the Wessex Water sewer records included within Appendix C

### **2.4 Existing surface water drainage arrangement**

Roof runoff from the existing building currently discharges onto the surrounding external surfaces, from where runoff follows the natural site topography towards areas of soft landscaping, the adjacent highway and the watercourse.

The existing site does not incorporate formal SuDS measures or attenuation, with surface water runoff therefore draining from the site largely unrestricted.

### 3.0 Summary of Flood Risk

| Source of Potential Flooding to the Development Site | Flood Risk | Mitigation/Comments                                                                                                                                                                                                                                                                                                                                                                                                                                                                                                 |
|------------------------------------------------------|------------|---------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------------|
| <b>Fluvial</b>                                       | Low        | EA mapping indicates that the site is located within Flood Zone 1 (Low Risk).                                                                                                                                                                                                                                                                                                                                                                                                                                       |
| <b>Public Sewers / Drains</b>                        | Low        | Wessex Water records indicate the present of sewers within Gores Lane at a lower level than the site. Any sewer flooding would be contained within the kerbed highway falling away from the site and not result in flooding of the building.                                                                                                                                                                                                                                                                        |
| <b>Overland Flow</b>                                 | Low        | EA mapping indicates the site is at Low risk of flooding from this source.                                                                                                                                                                                                                                                                                                                                                                                                                                          |
| <b>Groundwater</b>                                   | Low        | <p>Wiltshire Council's SFRA Data Explorer indicates the site to be in a zone at risk of groundwater flooding to surface and subsurface assets with the possibility of groundwater emerging at the surface locally.</p> <p>Intrusive testing did not encounter groundwater to a depth of 2m below ground level. Should groundwater emerge, water would follow the topography to the south away from the existing and proposed buildings.</p> <p>Flooding from groundwater emergence is therefore considered low.</p> |
| <b>Artificial Sources</b>                            | Low        | Wiltshire Council's SFRA Data Explorer indicates that the site is not located within an area at risk of flooding from this source.                                                                                                                                                                                                                                                                                                                                                                                  |

### 4.0 Surface Water Drainage Proposals

Refer to DC Drawing 1242-01-01 [*Drainage Strategy*] in Appendix A.

This section of the report addresses how flood risk generated by developing the site will be managed using sustainable control measures to ensure that flood risks are not increased on and off the site and how surface water is managed post development.

#### 4.1 Drainage Hierarchy

The surface water drainage strategy for the site has been prepared according to the drainage discharge hierarchy from CIRIA C753 The SuDS Manual, where the first available option will be utilised as follows.

| Technique                                                            | Summary                                                                                                   |
|----------------------------------------------------------------------|-----------------------------------------------------------------------------------------------------------|
| 1 Infiltration to the maximum extent that is practical               | Proposed solution – Greensand soils enable the use of infiltration devices, confirmed by on-site testing. |
| 2 To surface water body                                              | Not required.                                                                                             |
| 3 To a surface water sewer, highway drain or another drainage system | Not required – No surface water sewers within the vicinity of the site.                                   |
| 4 To a combined sewer                                                | Not required.                                                                                             |

## 4.2 Surface Water Design

Infiltration testing has confirmed that the underlying greensand soils provide sufficient infiltration potential to enable the use of infiltration-based SuDS.

Due to the relatively steep topography of the site, the potential to provide significant attenuation storage within permeable pavement sub-bases or open SuDS features is limited. It is therefore proposed to utilise below-ground cellular soakaways beneath the car park to provide attenuation and infiltration of surface water runoff from the proposed development.

Where levels and proposed construction allow, permeable surfacing will be utilised to manage runoff at source, reducing the volume and rate of runoff conveyed to the cellular soakaways.

Hydraulic calculations indicate that approximately 230.5 m<sup>3</sup> of attenuation storage is required to accommodate runoff from the proposed development for storm events up to and including the 1 in 100-year event, including a 45% allowance for climate change.

Runoff from the access and parking areas will be intercepted by permeable surfacing, providing treatment and infiltration at source, with connection to the cellular soakaways providing additional storage and infiltration capacity.

Refer to the hydraulic calculations included within Appendix E

## 4.3 Design Exceedance

Should the drainage system become overwhelmed or experience blockage during an extreme rainfall event, exceedance flows will follow the natural site topography from northeast to southwest, across the car park and towards the highway and adjacent watercourse. This broadly reflects the existing surface water flow regime and directs exceedance flows away from the proposed buildings.

## 4.4 Water Quality

Roof runoff is considered to present a very low pollution hazard. Runoff from parking areas has a greater potential to contain suspended solids, metals and hydrocarbons. This runoff will pass through the proposed permeable pavement construction, where filtration through the pavement and underlying aggregate provides treatment prior to infiltration to ground or onward discharge to the cellular soakaways.

Water quality has been assessed in line with the Simple Index approach from Chapter 26 of CIRIA C753 The SuDS Manual:

1. Step 1 – Allocate suitable pollution hazard indices for the proposed land use.
2. Step 2 – Select SuDS with a total pollution mitigation index that equals or exceeds the pollution hazard index.

### Pollution Hazard Indices

Below figures are obtained from Table 26.2 CIRIA C753 The SuDS Manual '*Pollution hazard indices for different land use classifications*'

| Land use                        | Pollution hazard level | Total suspended solids (TSS) | Metals | Hydrocarbons |
|---------------------------------|------------------------|------------------------------|--------|--------------|
| Roofs                           | Very Low               | 0.2                          | 0.2    | 0.05         |
| Car parks and low traffic roads | Low                    | 0.5                          | 0.4    | 0.4          |

### Pollution Mitigation Indices

Below figures are taken from Table 26.3 & 26.4 CIRIA C753 The SuDS Manual '*Indicative SuDS mitigation indices for discharge to surface waters and groundwater*'

| SuDS Feature       | Total suspended solids (TSS) | Metals | Hydrocarbons |
|--------------------|------------------------------|--------|--------------|
| Permeable Pavement | 0.7                          | 0.6    | 0.7          |

The pollution mitigation indices for the proposed drainage system therefore exceed the pollution hazard indices and provide suitable pollutant removal.

## 4.5 Adoption

The onsite drainage components will remain private and be the maintenance responsibility of the occupiers.

## 4.6 Maintenance

Regular inspection and maintenance are important for the effective operation of the SuDS systems as designed. The regular maintenance of SuDS should be carried out in line with C753 The SUDS Manual (CIRIA, November 2015) or as per manufacturer's specification.

### Pipework and Catchpits

It is not envisaged that silt build up within the pipework systems will require rigorous maintenance. Notwithstanding this, a suitable maintenance regime for the systems will comprise of routine inspection (every six months) and silt removal as necessary.

### Permeable Paving/Surfacing

Permeable surfaces need to be regularly cleaned of silt and other sediments to preserve the

infiltration capability of the surface. Suggested brushing regime would be at the end of winter to collect winter debris, mid-summer to collect dust, vegetation and grass-type deposits and after autumn leaf fall.

#### Cellular Soakaway Tank

The design of the soakaway tank should include monitoring points where the water level in the system can be observed or measured. This can either be via an inspection well or upstream catch pit

The useful life and effective operation of an soakaway component is related to the frequency of maintenance and the risk of sediment being introduced into the system.

It is not envisaged that silt build up within the cellular crate systems will require a rigorous maintenance regime so long as silt is removed from upstream catch pits and inspection chambers on a regular basis. Camera access can be gained via inspection chambers and inlet pipework to the soakaway. Silt removal can be achieved by jetting the inspection tunnels, where a standard jet head with rear facing nozzles should be used. The head should be fed to the far end of the crate tunnel via the nearest inspection chamber, activated and retracted. As the nozzle is removed, debris will be swept back into the inspection chamber where it can then be removed with the use of a standard gully sucker.

### **5.0 Foul Water Drainage Proposals**

Refer to DC Drawing 1242-01-01 [*Drainage Strategy*] in Appendix A.

#### **5.1 Proposal**

Foul water flows generated by the proposed development will discharge to the existing private foul drainage network within the site, which subsequently connects to the Wessex Water public foul sewer within Gores Lane to the south.

The existing private drainage arrangement will be retained and modified as necessary to accommodate the proposed development.

#### **5.2 Sewer Capacity**

A pre-planning capacity enquiry has been submitted to Wessex Water to confirm available network capacity, and a response is currently awaited.

#### **5.3 Adoption**

The foul water drainage components will remain private and be the maintenance responsibility of the property owners.

## **6.0 Conclusion**

The proposed redevelopment and extension of the Seven Stars will result in an increase in impermeable area compared with the existing development.

Surface water runoff from the proposed development will be managed through an infiltration-based sustainable drainage system. Runoff from the proposed building will discharge to cellular soakaways located beneath the car park, while permeable surfacing will be utilised where practicable to provide treatment, storage and infiltration at source.

The proposed SuDS system has been designed to accommodate storm events up to and including the 1 in 100-year event, including a 45% allowance for climate change. Infiltration testing undertaken on site has confirmed that the underlying soils provide suitable infiltration potential for the proposed drainage strategy.

Foul water generated by the development will discharge to the existing private foul drainage network within the site, which connects to the Wessex Water public foul sewer within Gores Lane. Confirmation of available capacity within the public sewer network is currently awaited from Wessex Water.

The proposed foul and surface water drainage systems will remain private and will be maintained by the property owner or appointed management company.

The proposed drainage strategy demonstrates that the development can be accommodated without increasing flood risk to the site or surrounding area, while providing an improvement over the existing surface water drainage arrangement through the introduction of formal SuDS, attenuation, treatment and controlled infiltration.

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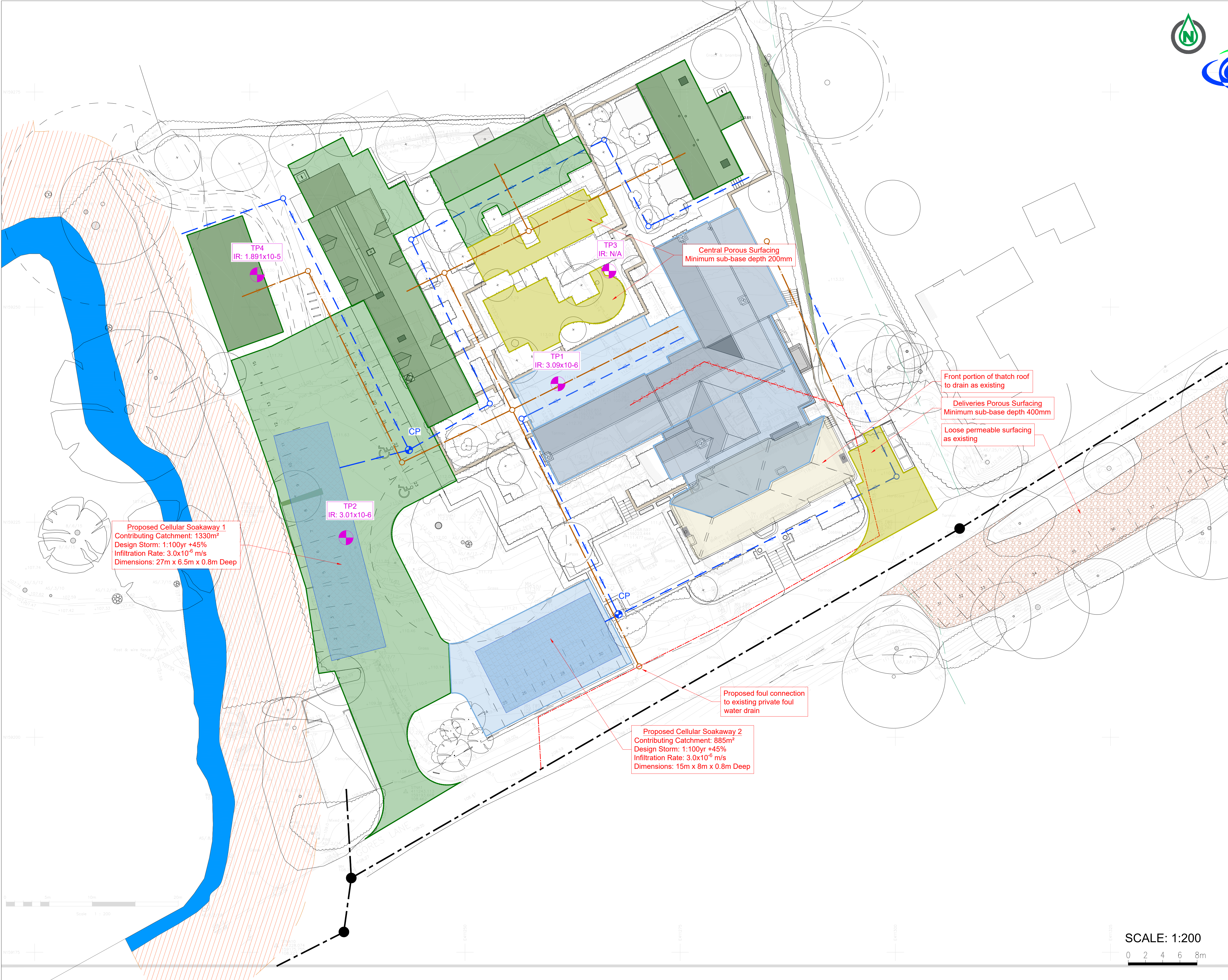
Issue 2 - September 2026

# APPENDIX A

## Drawings by Dixon Civils

Drawing ref. 1242-01-01

Drainage Strategy



#### KEY:

- SI Test Locations
- Proposed Private Surface Water Drain
- Proposed Surface Water Catchpit Chamber
- Proposed Cellular Soakaway
- Permeable Surfacing
- Soakaway 1 Catchment
- Soakaway 2 Catchment
- Existing Foul Water Sewer
- Existing Private Foul Water Drain
- Proposed Private Foul Water Drain

#### GENERAL NOTES:

- DO NOT SCALE FROM THIS DRAWING. WORK ONLY FROM STATED DIMENSIONS OR SETTING OUT COORDINATES.
- ALL DIMENSIONS AND LEVELS ARE IN METRES AND RELATE TO AN ASSUMED LOCAL DATUM.
- THIS DRAWING SHALL BE READ IN CONJUNCTION WITH ALL RELEVANT SCHEME DRAWINGS AND SPECIFICATIONS.
- ANY WORKS CONSTRUCTED PRIOR TO THE ISSUE OF LOCAL AUTHORITY APPROVAL IS ENTIRELY AT THE RISK OF THE CONTRACTOR/CLIENT.
- THIS DRAWING IS FOR PLANNING APPROVAL PURPOSES ONLY.

|     |                |    |          |
|-----|----------------|----|----------|
| A   | Layout updated | AD | 03.09.26 |
| -   | FIRST ISSUE    | AD | 10.08.26 |
| Rev | Description    | Dm | Date     |

#### REVISIONS

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CLIENT  
**Meeru Property Services**

JOB TITLE  
**The Seven Stars  
Bottlesford  
Pewsey**

DRAWING TITLE  
**Drainage Strategy**

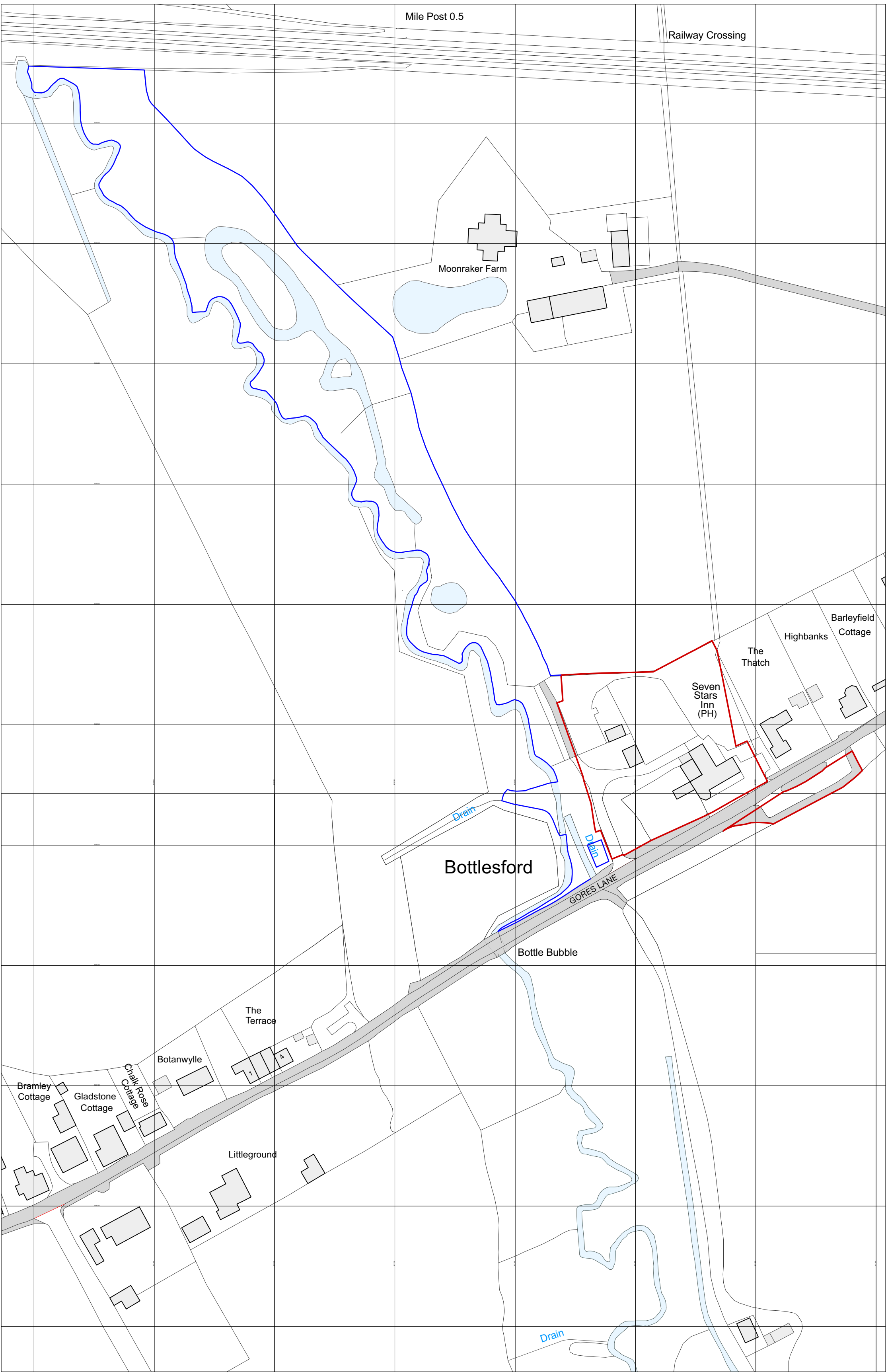
DATE: JUNE 2026 DRAWN: AD  
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# APPENDIX B

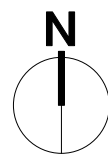
## Drawings by others

|                                        |                                        |
|----------------------------------------|----------------------------------------|
| Drawing ref. 2601-1_001-P1             | Location Plan (Milton Architects)      |
| Drawing ref. 2601-3_101-P1             | Proposed Site Plan (Milton Architects) |
| Topographical Survey (Dando Surveying) |                                        |



**Key**

- Application Boundary
- Other Land within Applicant's Ownership



0 10 20 30 40 50 M



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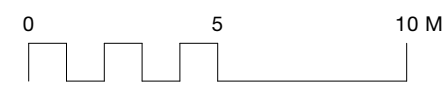
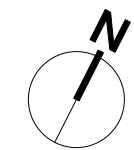
Project : The Seven Stars  
Gores Lane, Bottlesford  
Extension and alterations

Contents: EXISTING - Location Plan

Date : July 2026

Scale : 1:1250 Size : ISO A1

Drawing : **2601 / 1\_001** **P1**



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Project : The Seven Stars  
Gores Lane, Bottlesford  
Extension and alterations  
Contents: PROPOSED - Site Plan

Date : July 2026

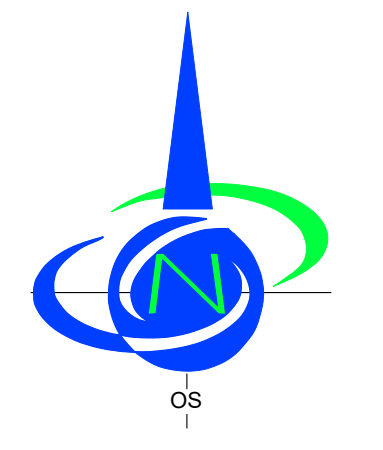
Scale : 1:200 Size : ISO A1

Drawing : 2601 / 3\_101 P1



- Key**
1. Main Entrance
  2. Pub
  3. Kitchens
  4. Restaurant
  5. Dining Terrace
  6. Courtyard Garden
  7. Multi-purpose Space
  8. Guest Rooms: East
  9. Plant Room
  10. Guest Rooms: West
  11. Housekeeping & Plant
  12. Yoga Barn
  13. Parking - 30 Spaces
  14. Parking - 10 Spaces
  15. Deliveries & Refuse
  16. Cart Shed
  17. Play Area
  18. Wessex Water property
  19. 10m SSSI Protection Zone
  20. Commemorative trees and plaque
  21. EV Charger
  22. Staff Cycle Parking
  23. Guest Cycle Parking

- Key to Elements**
- 10m SSSI Protection Zone
  - Permeable paving - Pedestrian
  - Permeable paving - Car Park
  - Soft Landscaping
  - Proposed trees
  - Existing trees



| Tree Abbreviations |                           |
|--------------------|---------------------------|
| ASH Ash            | LP London Plane           |
| BAY Bay            | MAP Maple                 |
| BEE Beech          | OAK Oak                   |
| CB Cooper Beech    | POP Poplar                |
| CED Cedar          | RM Red Maple              |
| CH Cherry          | RW Redwood                |
| CON Coniferous     | SB Silver Birch           |
| EUC Eucalyptus     | SER Service Tree          |
| FR Fruit           | SP Scots Pine             |
| HAW Hawthorn       | SYC Sycamore              |
| HAZ Hazel          | WAL Walnut                |
| HC Horse Chestnut  | WIL Willow                |
| HOL Holly          | WP Weeping Willow         |
| LAB Laburnum       | YEW Yew                   |
| LAU Laurel         | *MB Multiple stems/trunks |
| LI Lime            | Overall dia. shown        |

| Key                         |                      |
|-----------------------------|----------------------|
| AV Air Valve                | LP Lamp Post         |
| BL Bollard                  | M Meter              |
| BM Bench Mark               | MH Manhole Cover     |
| BP Boundary Point           | MK Marker            |
| BT BT Cover                 | MS Milestone         |
| CATV Cable Television Cover | RG Ridge Level       |
| CAB Cabinet                 | RS Road Sign         |
| CL Cover Level              | rwp Rain Water Pipe  |
| EP Electric Power Pole      | SC Storm Water Sewer |
| EV Eaves Level (ult fascia) | Sr Strainer          |
| FL Fire Hydrant             | SV Stop Valve        |
| FH Floor Level              | swp Soil Vent Pipe   |
| FWS Flood Water Sewer       | TH Threshold Level   |
| G Gully                     | TL Traffic Light     |
| GP Gate Post                | TP Telephone Pole    |
| GV Gas Valve                | UTL Unable To Lift   |
| IC Inspection Chamber       | VP Vent Pipe         |
| I Invert Level              | WM Water Meter       |
| KO Kerb Outlet              | WO Wash Out          |

| Linetypes               |     |
|-------------------------|-----|
| Bank                    | --- |
| Building                | --- |
| Drop kerb               | --- |
| Channel                 | --- |
| Electric                | --- |
| Fence                   | --- |
| Foliage                 | --- |
| FWS (Flood Water Sewer) | --- |
| Grd Cross               | --- |
| Hedge                   | --- |
| Kerb                    | --- |
| Manhole                 | --- |
| Pipe                    | --- |
| Ridge                   | --- |
| SWS (Storm Water Sewer) | --- |
| Telephone               | --- |
| Track/Path              | --- |
| Tree Canopy             | --- |
| Verge                   | --- |
| Water line              | --- |
| Wall                    | --- |

**Coordinate System**

This survey has been oriented to the O.S. National Grid (OSGB36) via a global position system and the O.S. Active Network (OS Net) OSGB36. True OSGB36 coordinates have been established at STN1 E 411306.288 N 159219.966 ELEV 110.135 and a further OSGB36 point established to create a true O.S. bearing for angle orientation. All levels are based on O.S. Datum. A scale factor of 1:1 has been applied to the survey and therefore the coordinate system must be considered arbitrary.



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Project Name:  
**The Seven Stars Inn, Bettleford**  
Pewsey, SN9 6LW

Drawing Title:  
**Topographical Survey**

Client:  
**Acorn Construction**  
Agent:

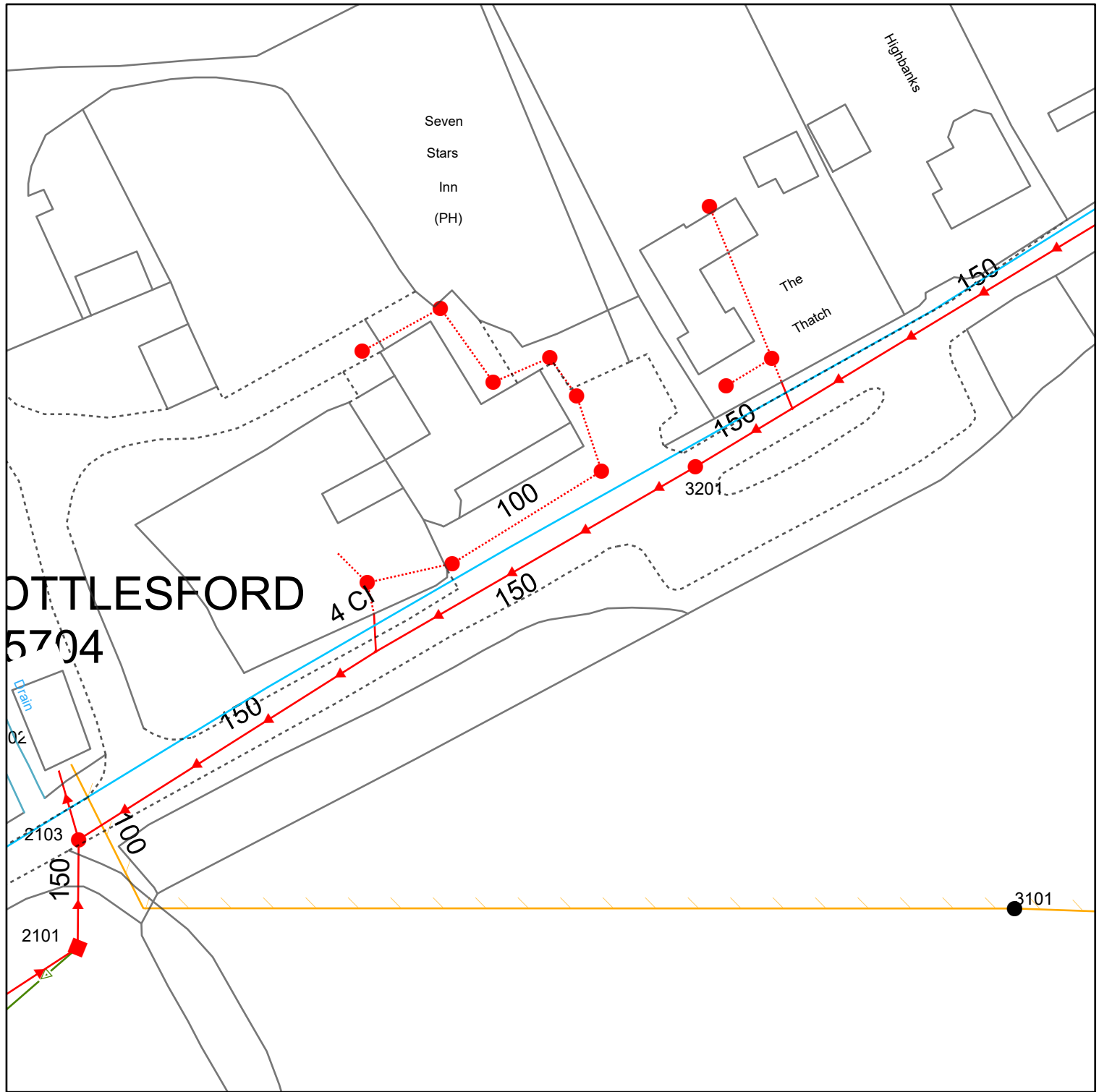
|                             |                            |
|-----------------------------|----------------------------|
| Date: 7th November 2025     | Scale: 1:200 @ A1          |
| Grid Orientation: North GPS | Sheet ID: Abbreviated TOPO |
| Site Level Datum: OS GPS    | Surveyed By: AMCH          |
|                             | Drawn By: AMCH             |



# APPENDIX C

## Wessex Water Sewer Records

# Wessex Water Network Map



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| WATER MAINS |              | SEWERS     |                          | STRATEGIC |                             | PUBLIC |  | PRIVATE |  | SECTION 104 |  | OTHER WESSEX PIPES |                       | NON-WESSEX / UNKNOWN |                       |
|-------------|--------------|------------|--------------------------|-----------|-----------------------------|--------|--|---------|--|-------------|--|--------------------|-----------------------|----------------------|-----------------------|
|             | Distribution |            | Foul                     |           |                             |        |  |         |  |             |  |                    | Rising Mains          |                      | Private Rising Mains  |
|             | Washout      |            | Surface                  |           |                             |        |  |         |  |             |  |                    | Standby Rising Mains  |                      | Culverted Watercourse |
|             | Raw Water    |            | Combined                 |           |                             |        |  |         |  |             |  |                    | EDM Effluent Disposal |                      | Highway Drain         |
|             | Abandoned    |            | Abandoned                |           |                             |        |  |         |  |             |  |                    | Overflow              |                      | Use Unknown           |
|             | Private      |            |                          |           |                             |        |  |         |  |             |  |                    | Syphon                |                      | Status Unknown        |
| FITTINGS    |              | STRUCTURES |                          |           |                             |        |  |         |  |             |  | OTHER STRUCTURES   |                       |                      |                       |
|             | Hydrant      |            | Manhole - Foul           |           | Pumping Station - Surface   |        |  |         |  |             |  |                    | Attenuation Tank      |                      | Chamber               |
|             | Other        |            | Manhole - Surface        |           | Pumping Stn - Foul/Combined |        |  |         |  |             |  |                    | Storage Tank          |                      | Tunnel                |
|             |              |            | Manhole - Combined       |           | Gully                       |        |  |         |  |             |  |                    |                       |                      | Interceptor           |
|             |              |            | Inlet                    |           | Vent Column                 |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            | Outfall                  |           | Rodding Eye                 |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            | Lamphole                 |           | Catchpit                    |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            | Bifurcation - Foul       |           | Flushing Chamber            |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            | Bifurcation - Surface    |           | Soakaway                    |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            | Bifurcation - Combined   |           | Non Return Valve            |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            | Combined Sewage Overflow |           | Air Valve                   |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            |                          |           | Hatch Box                   |        |  |         |  |             |  |                    |                       |                      |                       |
|             |              |            |                          |           | Washout                     |        |  |         |  |             |  |                    |                       |                      |                       |

Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526333 or e-mail: [developer.enquiries@wessexwater.co.uk](mailto:developer.enquiries@wessexwater.co.uk) to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.

**Date:** 04/02/2026  
**Centre:** 411292E, 159216N  
**Scale:** 1:625  
(when printed at A4 size)



# APPENDIX D

## BRE 365 Infiltration Test Results



4

3

1

2

## TEST RECORD

Test Pit Number: 1 (by portaloo)

Pit dimensions: 2m Long x 1m Wide x 1.5m Deep

26.5.26.

|        | Reading<br>(% full) | Depth of water<br>from base | Time    |
|--------|---------------------|-----------------------------|---------|
| Test 1 | 100%                | 1m                          | 11.26am |
|        | <del>75%</del>      | 0.92m                       | 11.46am |
|        | 50%                 | 0.88m                       | 12.06pm |
|        | 25%                 | 0.84m                       | 12.26pm |
| Test 2 | 100%                | 0.53m                       | 5.01pm  |
|        | <del>75%</del>      |                             |         |
|        | 50%                 | 27.5.26                     |         |
|        | 25%                 | 0.38m                       | 6.30am  |
| Test 3 | 100%                |                             |         |
|        | <del>75%</del>      |                             |         |
|        | 50%                 |                             |         |
|        | 25%                 |                             |         |

Calculated Infiltration Rate =

## TEST RECORD

Test Pit Number: 2 (rear driveway)

Pit dimensions: 1.5m Long x 1m Wide x 1.5m Deep

26.5.26.

|        | Reading<br>(% full) | Depth of water<br>from base | Time   |
|--------|---------------------|-----------------------------|--------|
| Test 1 | 100%                | 1.38m                       | 12:40  |
|        | 75%                 | 1.13m                       | 1:00   |
|        | 50%                 | 1m                          | 1:20   |
|        | 25%                 | 0.9m                        | 1:40   |
| Test 2 | 100%                | 0.54m                       | 4:57pm |
|        | 75%                 |                             |        |
|        | 50%                 | 27.5.26                     |        |
|        | 25%                 | 0.26                        | 6:30am |
| Test 3 | 100%                |                             |        |
|        | 75%                 |                             |        |
|        | 50%                 |                             |        |
|        | 25%                 |                             |        |

Calculated Infiltration Rate =

27.5.26  
**TEST RECORD**

Test Pit Number: 2

Pit dimensions: 1.5 Long x 1 Wide x 1.5 Deep

|               | Reading<br>(% full) | Depth of water<br>from base | Time  |
|---------------|---------------------|-----------------------------|-------|
| <b>Test 1</b> | <b>100%</b>         | 1.26                        | 7.10  |
|               | <b>75%</b>          | 1.04                        | 7.40  |
|               | <b>50%</b>          | 0.87                        | 8.10  |
|               | <b>25%</b>          | 0.76                        | 8.40  |
| <b>Test 2</b> | <b>100%</b>         | 0.67                        | 9.10  |
|               | <b>75%</b>          | 0.60                        | 9.40  |
|               | <b>50%</b>          | 0.56                        | 10.10 |
|               | <b>25%</b>          | 0.52                        | 10.40 |
| <b>Test 3</b> | <b>100%</b>         | 0.50                        | 11.10 |
|               | <b>75%</b>          | 0.49                        | 11.40 |
|               | <b>50%</b>          | 0.49                        | 12.10 |
|               | <b>25%</b>          | 0.48                        | 12.40 |

0.27  
2.45  
4.45  
6.50

Calculated Infiltration Rate =

27.8<sup>5</sup>.26.  
**TEST RECORD**

Test Pit Number: 3

Pit dimensions: 1.8 Long x 1 Wide x 2 Deep

|               | Reading<br>(% full) | Depth of water<br>from base <sub>m</sub> | Time               |
|---------------|---------------------|------------------------------------------|--------------------|
| <b>Test 1</b> | 100%                | 1.54                                     | 8.30               |
|               | 75%                 | 1.41                                     | 9.00               |
|               | 50%                 | 1.35                                     | 9.30               |
|               | 25%                 | 1.3                                      | <del>9.54</del> 10 |
| <b>Test 2</b> | 100%                | 1.24                                     | 10.30              |
|               | 75%                 | 1.21                                     | 11                 |
|               | 50%                 | 1.175                                    | 11.30              |
|               | 25%                 | 1.145                                    | 12                 |
| <b>Test 3</b> | 100%                | 1.110                                    | 12:30              |
|               | 75%                 | 1.080                                    | 1                  |
|               | 50%                 | 1.055                                    | 1:30               |
|               | 25%                 | 1.035                                    | 2                  |

Calculated Infiltration Rate =

1.030      2.30  
1.030      3  
1.015      4.45

27-5-26

**TEST RECORD**

Test Pit Number: 4

Pit dimensions: 1.75 Long x 0.75 Wide x 1.8 Deep

|               | Reading<br>(% full) | Depth of water<br>from base | Time  |
|---------------|---------------------|-----------------------------|-------|
| <b>Test 1</b> | <b>100%</b>         | 1.520                       | 7.20  |
|               | <b>75%</b>          | 1.25                        | 7.50  |
|               | <b>50%</b>          | 0.99                        | 8.20  |
|               | <b>25%</b>          | 0.79                        | 8.50  |
| <b>Test 2</b> | <b>100%</b>         | 0.61                        | 9.20  |
|               | <b>75%</b>          | 0.48                        | 9.50  |
|               | <b>50%</b>          | 0.38                        | 10.20 |
|               | <b>25%</b>          | 0.325                       | 10.50 |
| <b>Test 3</b> | <b>100%</b>         | 0.275                       | 11.20 |
|               | <b>75%</b>          | 0.225                       | 11.50 |
|               | <b>50%</b>          | 0.170                       | 12.20 |
|               | <b>25%</b>          | 0.130                       | 12.50 |

0.090 1.20  
0.045 1.50  
0.00 2.20

Calculated Infiltration Rate =

28.5.26

**TEST RECORD**

**Test Pit Number:** 4

**Pit dimensions:** 1.75 Long x 0.75 Wide x 1.8 Deep

|               | Reading<br>(% full) | Depth of water<br>from base | Time  |
|---------------|---------------------|-----------------------------|-------|
| <b>Test 1</b> | <b>100%</b>         | 1.47                        | 8.00  |
|               | <b>75%</b>          | 1.18                        | 8.30  |
|               | <b>50%</b>          | 0.93                        | 9.00  |
|               | <b>25%</b>          | 0.69                        | 9.30  |
| <b>Test 2</b> | <b>100%</b>         | 0.53                        | 10.00 |
|               | <b>75%</b>          | 0.4                         | 10.30 |
|               | <b>50%</b>          | 0.29                        | 11.00 |
|               | <b>25%</b>          | 0.21                        | 11.30 |
| <b>Test 3</b> | <b>100%</b>         | 0.145                       | 12.00 |
|               | <b>75%</b>          | 0.095                       | 12.30 |
|               | <b>50%</b>          | 0.055                       | 1.00  |
|               | <b>25%</b>          | 0.025                       | 1.30  |

0.00 2.00

**Calculated Infiltration Rate =**

**Site: The Seven Stars, Pewsey**  
**Test Number 1**

**SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365**

**Soil infiltration rate ( $f$ )** =  $V_{p75-25}/a_{p50} \times t_{p75-25}$

where

$V_{p75-25}$  = the effective storage volume of water in the trial pit between 75% and 25% effective depth

$a_{p50}$  = the internal surface area of the trial pit up to 50% effective depth and including base area

$t_{p75-25}$  = the time for the water level to fall from 75% to 25% effective depth

**Soakaway Pit Dimensions**

|        |       |
|--------|-------|
| Length | 2 m   |
| Width  | 1 m   |
| Depth  | 1.5 m |

**Depth to Water Level**

|          |        |
|----------|--------|
| 75% full | 0.75 m |
| 50% full | 1 m    |
| 25% full | 1.25 m |

**Depth to Water Level**

|               |     |
|---------------|-----|
| Start of test | 0.5 |
| End of test   | 1.5 |

**Time to empty**

|           |          |
|-----------|----------|
| $t_{p25}$ | 120 min  |
| $t_{p50}$ | 330 min  |
| $t_{p75}$ | 1200 min |

**Effective Storage Volume and Area**

$V_{p75-25}$                       1  
 $a_{p50}$                               5

|                                                               |
|---------------------------------------------------------------|
| <b>Soil Infiltration R 3.086E-06 m/s</b><br><b>0.011 m/hr</b> |
|---------------------------------------------------------------|

**Site: The Seven Stars, Pewsey**  
**Test Pit 2 -Test 1**

**SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365**

**Soil infiltration rate ( $f$ )=  $V_{p75-25}/a_{p50} \times t_{p75-25}$**

where

$V_{p75-25}$  = the effective storage volume of water in the trial pit between 75% and 25% effective depth

$a_{p50}$  = the internal surface area of the trial pit up to 50% effective depth and including base area

$t_{p75-25}$  = the time for the water level to fall from 75% to 25% effective depth

**Soakaway Pit Dimensions**

|        |       |
|--------|-------|
| Length | 1.5 m |
| Width  | 1 m   |
| Depth  | 1.5 m |

**Depth to Water Level**

|          |         |
|----------|---------|
| 75% full | 0.345 m |
| 50% full | 0.69 m  |
| 25% full | 1.035 m |

**Depth to Water Level**

|               |      |
|---------------|------|
| Start of test | 0.12 |
| End of test   | 1.5  |

**Time to empty**

|           |          |
|-----------|----------|
| $t_{p25}$ | 50 min   |
| $t_{p50}$ | 280 min  |
| $t_{p75}$ | 1080 min |

**Effective Storage Volume and Area**

$V_{p75-25}$  1.035

$a_{p50}$  5.55

|                                                               |
|---------------------------------------------------------------|
| <b>Soil Infiltration R 3.018E-06 m/s</b><br><b>0.011 m/hr</b> |
|---------------------------------------------------------------|

**Site: The Seven Stars, Pewsey**  
**Test Pit 2 -Test 2**

**SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365**

**Soil infiltration rate ( $f$ )=  $V_{p75-25}/a_{p50} \times t_{p75-25}$**

where

$V_{p75-25}$  = the effective storage volume of water in the trial pit between 75% and 25% effective depth

$a_{p50}$  = the internal surface area of the trial pit up to 50% effective depth and including base area

$t_{p75-25}$  = the time for the water level to fall from 75% to 25% effective depth

**Soakaway Pit Dimensions**

|        |       |
|--------|-------|
| Length | 1.5 m |
| Width  | 1 m   |
| Depth  | 1.5 m |

**Depth to Water Level**

|          |         |
|----------|---------|
| 75% full | 0.315 m |
| 50% full | 0.63 m  |
| 25% full | 0.945 m |

**Depth to Water Level**

|               |      |
|---------------|------|
| Start of test | 0.24 |
| End of test   | 1.5  |

**Time to empty**

|           |         |
|-----------|---------|
| $t_{p25}$ | 50 min  |
| $t_{p50}$ | 140 min |
| $t_{p75}$ | 560 min |

**Effective Storage Volume and Area**

$V_{p75-25}$  0.945

$a_{p50}$  5.85

|                                                               |
|---------------------------------------------------------------|
| <b>Soil Infiltration R 5.279E-06 m/s</b><br><b>0.019 m/hr</b> |
|---------------------------------------------------------------|

**Site: The Seven Stars, Pewsey**  
**Test Pit 3 -Test 1**

**SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365**

**Soil infiltration rate ( $f$ )** =  $V_{p75-25}/a_{p50} \times t_{p75-25}$

where

$V_{p75-25}$  = the effective storage volume of water in the trial pit between 75% and 25% effective depth

$a_{p50}$  = the internal surface area of the trial pit up to 50% effective depth and including base area

$t_{p75-25}$  = the time for the water level to fall from 75% to 25% effective depth

**Soakaway Pit Dimensions**

|        |       |
|--------|-------|
| Length | 1.8 m |
| Width  | 1 m   |
| Depth  | 2 m   |

**Depth to Water Level**

|          |         |
|----------|---------|
| 75% full | 0.385 m |
| 50% full | 0.77 m  |
| 25% full | 1.155 m |

**Depth to Water Level**

|               |      |
|---------------|------|
| Start of test | 0.26 |
| End of test   | 2    |

**Time to empty**

|           |         |
|-----------|---------|
| $t_{p25}$ | 210 min |
| $t_{p50}$ | NA min  |
| $t_{p75}$ | NA min  |

**Effective Storage Volume and Area**

$V_{p75-25}$  1.386

$a_{p50}$  8.688

|                                                               |
|---------------------------------------------------------------|
| <b>Soil Infiltration R #VALUE! m/s</b><br><b>#VALUE! m/hr</b> |
|---------------------------------------------------------------|

**Site: The Seven Stars, Pewsey**  
**Test Pit 4 -Test 1**

**SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365**

**Soil infiltration rate ( $f$ )** =  $V_{p75-25}/a_{p50} \times t_{p75-25}$

where

$V_{p75-25}$  = the effective storage volume of water in the trial pit between 75% and 25% effective depth

$a_{p50}$  = the internal surface area of the trial pit up to 50% effective depth and including base area

$t_{p75-25}$  = the time for the water level to fall from 75% to 25% effective depth

**Soakaway Pit Dimensions**

|        |        |
|--------|--------|
| Length | 1.75 m |
| Width  | 0.75 m |
| Depth  | 1.8 m  |

**Depth to Water Level**

|          |        |
|----------|--------|
| 75% full | 0.38 m |
| 50% full | 0.76 m |
| 25% full | 1.14 m |

**Depth to Water Level**

|               |      |
|---------------|------|
| Start of test | 0.28 |
| End of test   | 1.8  |

**Time to empty**

|           |         |
|-----------|---------|
| $t_{p25}$ | 45 min  |
| $t_{p50}$ | 100 min |
| $t_{p75}$ | 180 min |

**Effective Storage Volume and Area**

$V_{p75-25}$       0.998

$a_{p50}$          6.513

|                                                                 |
|-----------------------------------------------------------------|
| <b>Soil Infiltration R   1.891E-05 m/s</b><br><b>0.068 m/hr</b> |
|-----------------------------------------------------------------|

**Site: The Seven Stars, Pewsey**  
**Test Pit 4 -Test 1**

## SOIL INFILTRATION RATE IN ACCORDANCE WITH BRE DIGEST 365

**Soil infiltration rate ( $f$ ) =  $V_{p75-25}/a_{p50} \times t_{p75-25}$**

where

$V_{p75-25}$  = the effective storage volume of water in the trial pit between 75% and 25% effective depth

$a_{p50}$  = the internal surface area of the trial pit up to 50% effective depth and including base area

$t_{p75-25}$  = the time for the water level to fall from 75% to 25% effective depth

## Soakaway Pit Dimensions

|        |        |
|--------|--------|
| Length | 1.75 m |
| Width  | 0.75 m |
| Depth  | 1.8 m  |

### Depth to Water Level

|          |         |
|----------|---------|
| 75% full | 0.368 m |
| 50% full | 0.736 m |
| 25% full | 1.104 m |

## Depth to Water Level

|               |      |
|---------------|------|
| Start of test | 0.33 |
| End of test   | 1.8  |

## Time to empty

|                  |         |
|------------------|---------|
| t <sub>p25</sub> | 35 min  |
| t <sub>p50</sub> | 80 min  |
| t <sub>p75</sub> | 105 min |

## Effective Storage Volume and Area

**V<sub>p75-25</sub>** 0.966

**a<sub>p50</sub>** 6.633

|                     |               |
|---------------------|---------------|
| Soil Infiltration R | 3.468E-05 m/s |
|                     | 0.125 m/hr    |

# APPENDIX E

## Infodrainage Soakaway Calculations

|                                                        |                               |             |              |
|--------------------------------------------------------|-------------------------------|-------------|--------------|
| Project:<br>The Seven Stars<br>Pewsey                  | Date:<br>03/09/2026           |             |              |
|                                                        | Designed by:<br>AD            | Checked by: | Approved By: |
| Report Details:<br>Type: Inflows<br>Storm Phase: Phase | Design:<br>SW Soakaway Design |             |              |





SA1 Catchment

Type : Catchment Area

|           |       |
|-----------|-------|
| Area (ha) | 0.133 |
|-----------|-------|

Dynamic Sizing

|                                      |                       |
|--------------------------------------|-----------------------|
| Runoff Method                        | Time of Concentration |
| Summer Volumetric Runoff             | 0.750                 |
| Winter Volumetric Runoff Coefficient | 0.840                 |
| Time of Concentration (mins)         | 5                     |
| Percentage Impervious (%)            | 100                   |



SA2 Catchment

Type : Catchment Area

|           |       |
|-----------|-------|
| Area (ha) | 0.089 |
|-----------|-------|

Dynamic Sizing

|                                      |                       |
|--------------------------------------|-----------------------|
| Runoff Method                        | Time of Concentration |
| Summer Volumetric Runoff             | 0.750                 |
| Winter Volumetric Runoff Coefficient | 0.840                 |
| Time of Concentration (mins)         | 5                     |
| Percentage Impervious (%)            | 100                   |



Central Porous Catchment

Type : Catchment Area

|           |       |
|-----------|-------|
| Area (ha) | 0.008 |
|-----------|-------|

Dynamic Sizing

|                                      |                       |
|--------------------------------------|-----------------------|
| Runoff Method                        | Time of Concentration |
| Summer Volumetric Runoff             | 0.750                 |
| Winter Volumetric Runoff Coefficient | 0.840                 |
| Time of Concentration (mins)         | 5                     |
| Percentage Impervious (%)            | 100                   |



Deliveries Porous Catchment

Type : Catchment Area

|           |       |
|-----------|-------|
| Area (ha) | 0.007 |
|-----------|-------|

Dynamic Sizing

|                                      |                       |
|--------------------------------------|-----------------------|
| Runoff Method                        | Time of Concentration |
| Summer Volumetric Runoff             | 0.750                 |
| Winter Volumetric Runoff Coefficient | 0.840                 |
| Time of Concentration (mins)         | 5                     |
| Percentage Impervious (%)            | 100                   |

|                                                                    |                               |             |              |
|--------------------------------------------------------------------|-------------------------------|-------------|--------------|
| Project:<br>The Seven Stars<br>Pewsey                              | Date:<br>03/09/2026           |             |              |
|                                                                    | Designed by:<br>AD            | Checked by: | Approved By: |
| Report Details:<br>Type: Stormwater Controls<br>Storm Phase: Phase | Design:<br>SW Soakaway Design |             |              |





### Central Porous Paving

Type : Porous Paving

#### Dimensions

|                             |          |
|-----------------------------|----------|
| Exceedance Level (m)        | 113.000  |
| Depth (m)                   | 0.280    |
| Base Level (m)              | 112.720  |
| Paving Layer Depth (mm)     | 80       |
| Membrane Percolation (m/hr) | 1000.0   |
| Porosity (%)                | 30       |
| Length (m)                  | 8.000    |
| Long. Slope (1:X)           | 10000.00 |
| Width (m)                   | 10.000   |
| Total Volume (m³)           | 4.800    |

#### Inlets

##### Inlet

|                    |                          |
|--------------------|--------------------------|
| Inlet Type         | Lateral Inflow           |
| Incoming Item(s)   | Central Porous Catchment |
| Bypass Destination | (None)                   |
| Capacity Type      | No Restriction           |

#### Advanced

|                               |        |
|-------------------------------|--------|
| Base Infiltration Rate (m/hr) | 0.0108 |
| Side Infiltration Rate (m/hr) | 0.0108 |
| Safety Factor                 | 3.0    |
| Conductivity (m/hr)           | 1000.0 |



### Deliveries Porous Paving

Type : Porous Paving

#### Dimensions

|                             |         |
|-----------------------------|---------|
| Exceedance Level (m)        | 110.100 |
| Depth (m)                   | 0.530   |
| Base Level (m)              | 109.570 |
| Paving Layer Depth (mm)     | 80      |
| Membrane Percolation (m/hr) | 1000.0  |
| Porosity (%)                | 30      |
| Length (m)                  | 10.000  |
| Long. Slope (1:X)           | 20.00   |
| Width (m)                   | 7.000   |
| Total Volume (m³)           | 9.450   |

#### Inlets

##### Inlet

|                    |                             |
|--------------------|-----------------------------|
| Inlet Type         | Lateral Inflow              |
| Incoming Item(s)   | Deliveries Porous Catchment |
| Bypass Destination | (None)                      |
| Capacity Type      | No Restriction              |

#### Advanced

|                               |        |
|-------------------------------|--------|
| Base Infiltration Rate (m/hr) | 0.0108 |
| Side Infiltration Rate (m/hr) | 0.0108 |
| Safety Factor                 | 1.5    |
| Conductivity (m/hr)           | 1000.0 |

|                                                                    |                               |             |              |
|--------------------------------------------------------------------|-------------------------------|-------------|--------------|
| Project:<br>The Seven Stars<br>Pewsey                              | Date:<br>03/09/2026           |             |              |
|                                                                    | Designed by:<br>AD            | Checked by: | Approved By: |
| Report Details:<br>Type: Stormwater Controls<br>Storm Phase: Phase | Design:<br>SW Soakaway Design |             |              |





Soakaway 1

Type : Cellular Storage

Dimensions

|                       |         |
|-----------------------|---------|
| Exceedance Level (m)  | 100.000 |
| Depth (m)             | 0.800   |
| Base Level (m)        | 97.000  |
| Number of Crates Long | 27      |
| Number of Crates Wide | 13      |
| Number of Crates High | 2       |
| Porosity (%)          | 95      |
| Crate Length (m)      | 1       |
| Crate Width (m)       | 0.5     |
| Crate Height (m)      | 0.4     |
| Total Volume (m³)     | 135.580 |

Inlets

Inlet

|                    |                |
|--------------------|----------------|
| Inlet Type         | Point Inflow   |
| Incoming Item(s)   | SA1 Catchment  |
| Bypass Destination | (None)         |
| Capacity Type      | No Restriction |

Advanced

|                               |        |
|-------------------------------|--------|
| Base Infiltration Rate (m/hr) | 0.0108 |
| Side Infiltration Rate (m/hr) | 0.0108 |
| Safety Factor                 | 3.0    |



Soakaway 2

Type : Cellular Storage

Dimensions

|                       |         |
|-----------------------|---------|
| Exceedance Level (m)  | 100.000 |
| Depth (m)             | 0.800   |
| Base Level (m)        | 97.000  |
| Number of Crates Long | 15      |
| Number of Crates Wide | 16      |
| Number of Crates High | 2       |
| Porosity (%)          | 95      |
| Crate Length (m)      | 1       |
| Crate Width (m)       | 0.5     |
| Crate Height (m)      | 0.4     |
| Total Volume (m³)     | 93.400  |

Inlets

Inlet

|                    |                |
|--------------------|----------------|
| Inlet Type         | Point Inflow   |
| Incoming Item(s)   | SA2 Catchment  |
| Bypass Destination | (None)         |
| Capacity Type      | No Restriction |

Advanced

|                               |        |
|-------------------------------|--------|
| Base Infiltration Rate (m/hr) | 0.0108 |
| Side Infiltration Rate (m/hr) | 0.0108 |
| Safety Factor                 | 3.0    |

|                                             |                               |             |              |                                                                                     |
|---------------------------------------------|-------------------------------|-------------|--------------|-------------------------------------------------------------------------------------|
| Project:<br>The Seven Stars<br>Pewsey       | Date:<br>03/09/2026           |             |              |  |
|                                             | Designed by:<br>AD            | Checked by: | Approved By: |                                                                                     |
| Report Title:<br>Rainfall Analysis Criteria | Design:<br>SW Soakaway Design |             |              |                                                                                     |

|                                 |                          |
|---------------------------------|--------------------------|
| Runoff Type                     | Dynamic                  |
| Output Interval (mins)          | 5                        |
| Time Step                       | Default                  |
| Urban Creep                     | Apply Global Value       |
| Urban Creep Global Value (%)    | 0                        |
| Junction Flood Risk Margin (mm) | 300                      |
| Perform No Discharge Analysis   | <input type="checkbox"/> |

#### Rainfall

|     |           |
|-----|-----------|
| FSR | Type: FSR |
|-----|-----------|

|            |                                     |
|------------|-------------------------------------|
| Region     | England And Wales                   |
| M5-60 (mm) | 19.5                                |
| Ratio R    | 0.350                               |
| Summer     | <input checked="" type="checkbox"/> |
| Winter     | <input checked="" type="checkbox"/> |

#### Return Period

| Return Period (years) | Increase Rainfall (%) |
|-----------------------|-----------------------|
| 10.0                  | 0.000                 |
| 30.0                  | 0.000                 |
| 100.0                 | 0.000                 |
| 100.0                 | 45.000                |

#### Storm Durations

| Duration (mins) | Run Time (mins) |
|-----------------|-----------------|
| 15              | 30              |
| 30              | 60              |
| 60              | 120             |
| 120             | 240             |
| 180             | 360             |
| 240             | 480             |
| 360             | 720             |
| 480             | 960             |
| 600             | 1200            |
| 720             | 1440            |
| 960             | 1920            |
| 1440            | 2880            |
| 2160            | 4320            |
| 2880            | 5760            |
| 4320            | 8640            |
| 5760            | 11520           |
| 7200            | 14400           |
| 8640            | 17280           |
| 10080           | 20160           |

|                                                                            |                               |             |              |
|----------------------------------------------------------------------------|-------------------------------|-------------|--------------|
| Project:<br>The Seven Stars<br>Pewsey                                      | Date:<br>03/09/2026           |             |              |
|                                                                            | Designed by:<br>AD            | Checked by: | Approved By: |
| Report Details:<br>Type: Stormwater Controls Summary<br>Storm Phase: Phase | Design:<br>SW Soakaway Design |             |              |





Critical Storm Per Item: Rank By: Max. Avg. Depth

| Stormwater Control       | Storm Event                              | Max. US Level (m) | Max. DS Level (m) | Max. US Depth (m) | Max. DS Depth (m) | Max. Inflow (L/s) | Max. Resident Volume (m³) | Max. Flooded Volume (m³) | Total Lost Volume (m³) | Max. Outflow (L/s) | Total Discharge Volume (m³) | Percentage Available (%) | Status |
|--------------------------|------------------------------------------|-------------------|-------------------|-------------------|-------------------|-------------------|---------------------------|--------------------------|------------------------|--------------------|-----------------------------|--------------------------|--------|
| Soakaway 1               | FSR: 100 years: +45 %: 2880 mins: Winter | 97.798            | 97.798            | 0.798             | 0.798             | 2.4               | 133.025                   | 0.000                    | 72.822                 | 0.0                | 0.000                       | 1.885                    | OK     |
| Soakaway 2               | FSR: 100 years: +45 %: 2880 mins: Winter | 97.778            | 97.778            | 0.778             | 0.778             | 1.6               | 88.732                    | 0.000                    | 49.596                 | 0.0                | 0.000                       | 4.998                    | OK     |
| Central Porous Paving    | FSR: 100 years: +45 %: 360 mins: Winter  | 112.909           | 112.910           | 0.189             | 0.190             | 0.7               | 4.539                     | 0.000                    | 3.606                  | 0.0                | 0.000                       | 5.433                    | OK     |
| Deliveries Porous Paving | FSR: 100 years: +45 %: 240 mins: Winter  | 110.070           | 109.967           | 0.000             | 0.397             | 0.9               | 4.167                     | 0.000                    | 2.759                  | 0.0                | 0.000                       | 55.906                   | OK     |



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