Flood Risk Assessment & Drainage Strategy



Chicklade Services, Phase 2 Car Park

awa
ANDREW WARING ASSOCIATES

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Offices

Romsey The Old Brewery House, Portersbridge St, Romsey, Hampshire, SO51 8DJ

Bath 2nd Floor, Unit 7, Riverside Court, Bath BA2 3DZ

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1. Introduction

Andrew Waring Associates were engaged by Bruno Skinner of Blackford Projects, on behalf of Instavolt, to provide a Flood Risk Assessment (FRA) and Drainage Strategy (DS) to support a Full Planning Application.

This FRA and DS report sets out the risk of flooding to the site and those that may arise as a result of the development. It also sets out the proposals to manage the surface water and foul water generated by the site.

This FRA and DS report has been prepared in line with the requirements of National Planning Policy Framework (NPPF) and Practice Planning Guidance (PPG). This report also takes into consideration the National Standards for Sustainable Drainage Systems (SSDS) July 2025.

2. Site Description

Location:

The proposed site is located approximately 600m west of Chicklade, directly adjacent the A303 trunk road, with the nearest postcode SP3 5ST. The application boundary is approximately 0.6245 hectares with a National Grid Reference (NGR) of ST 90367 34439.

The site is currently used as arable farm land, which borders the site to the north and west, with the A303 forming the southern boundary. To the east the site adjoins a new Electric Vehicle (EV) charging station, which is currently under construction, and to which this application is linked.



Figure 1: Site Location (Open Streetmaps, 2026)

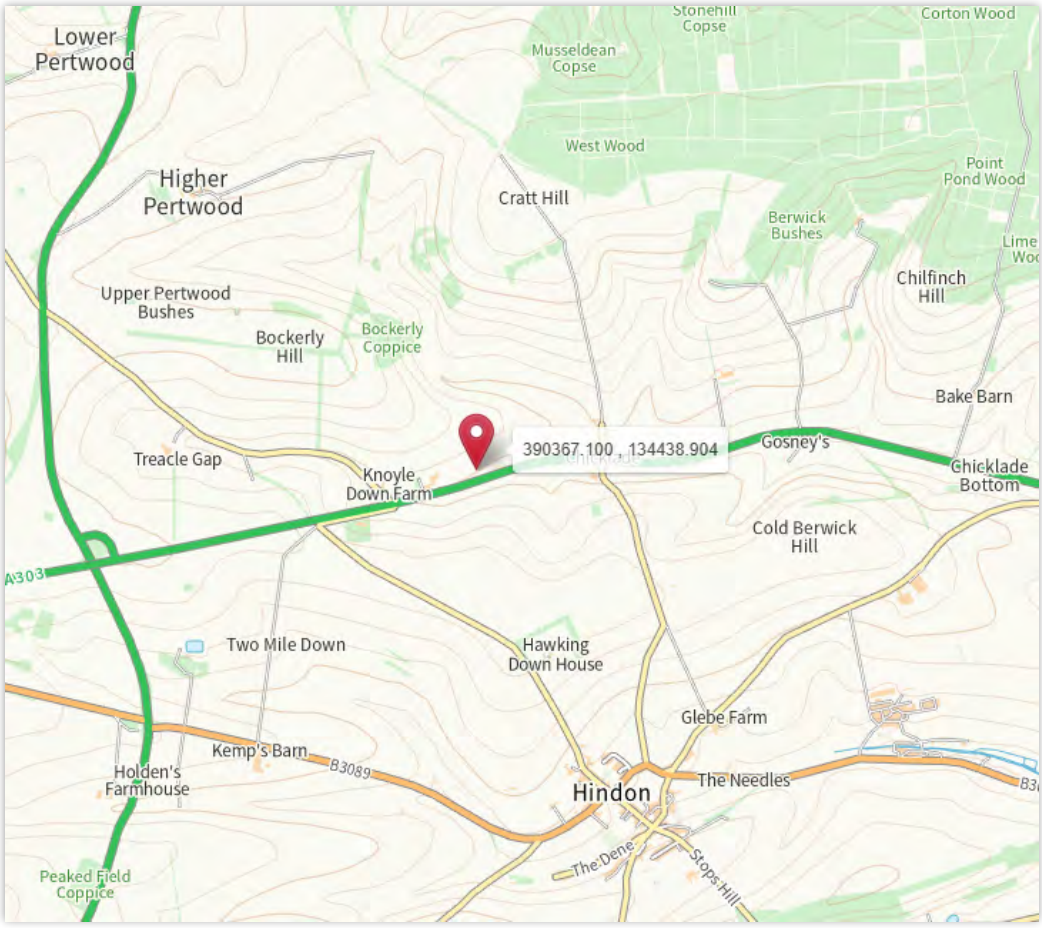


Figure 2: Wider site context (OS maps, 2026)

Topography:

A topographical survey has been undertaken at the site and the development boundary overlaid. Upon review it is indicated that the site broadly falls in a North West to South-East direction. The levels fall from a high point of 154.25m AOD to 144.40m AOD.

A reproduction of a section of the original survey is included as **Appendix A.**



Figure 3: Principal heights and slope direction (Digital Terrain Surveys)

Geology:

A site investigation was conducted in November 2025 by Ground & Water for the adjacent development A copy of the factual and interpretive reports can be found in **Appendix B.** The logs and infiltration results will be used to inform the drainage strategy for this application.

The soil conditions encountered were farmed soil (gravelly CLAY) typically 0.25 to 0.6m depth, overlaying gravelly clay (HEAD DEPOSITS). The depth of the head deposits varies, but the closest intrusive test (BH01) shows a depth of 1.7m below ground level (BGL). The bedrock below the clay is chalk (SEAFORD CHALK FORMATION) and windowless sampling proved a depth to at least 8.0m BGL.

Hydrogeology:

The Groundsure section of the site investigation report shows that the superficial head deposits constitute a **Secondary A Aquifer,** which are underlain by a **Principal Aquifer** in the Seaford Chalk bedrock, and the British Geological Survey maps note this as being highly productive. Refer to pages 30 & 31 of the Groundsure report for the relevant

mapping. The Environment Agency (EA) online mapping indicates that the site is in a drinking water safeguard zone.

The site is located within Source Protection Zone 3 (SPZ3) total catchment, and borders SPZ2, as shown in **Figure 4.**

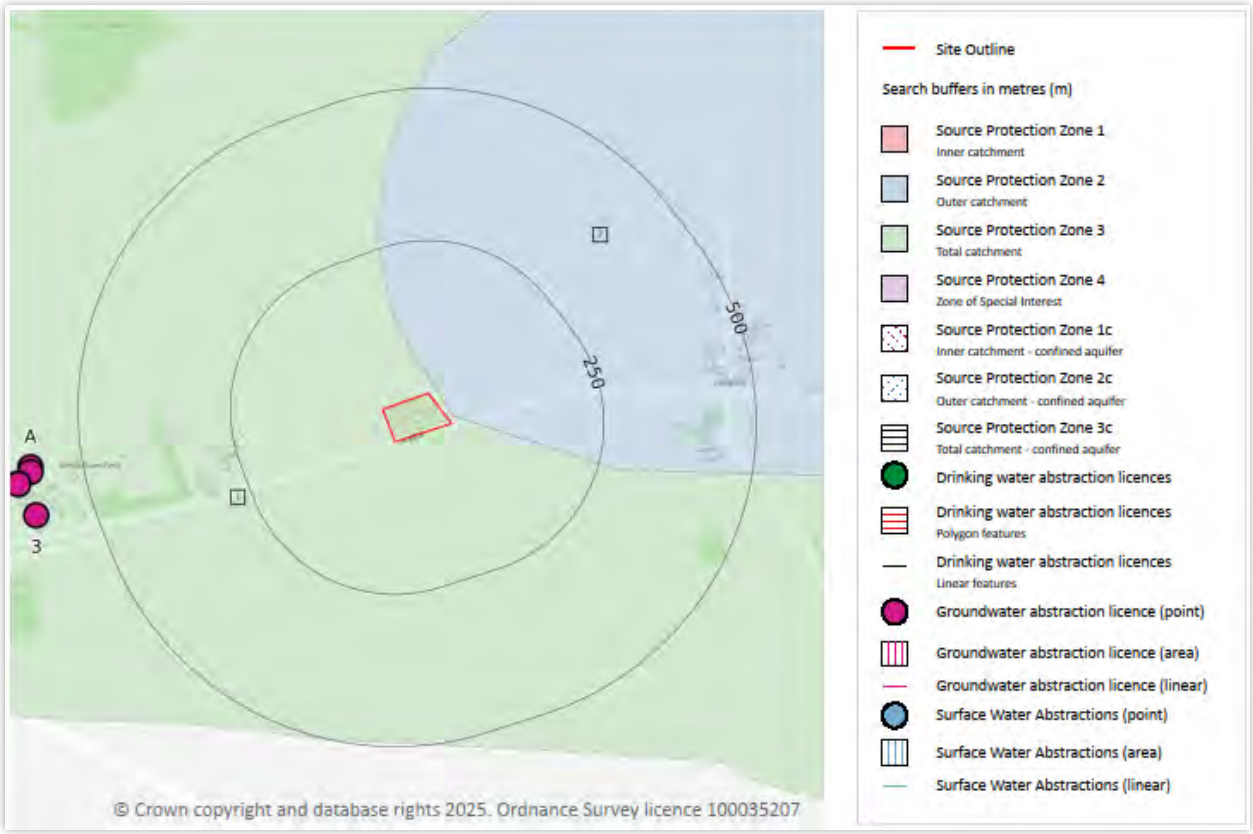


Figure 4: Source Protection Zones and Abstractions (Groundsure)

Hydrology:

The Groundsure section of the site investigation report indicates ground level surface water features adjacent to the proposed site, following the line of the A303, and these are assumed to be the highway ditch draining the A303. **Figure 5** shows there are no other water bodies in the vicinity, with the nearest stream 37m to the south-west.

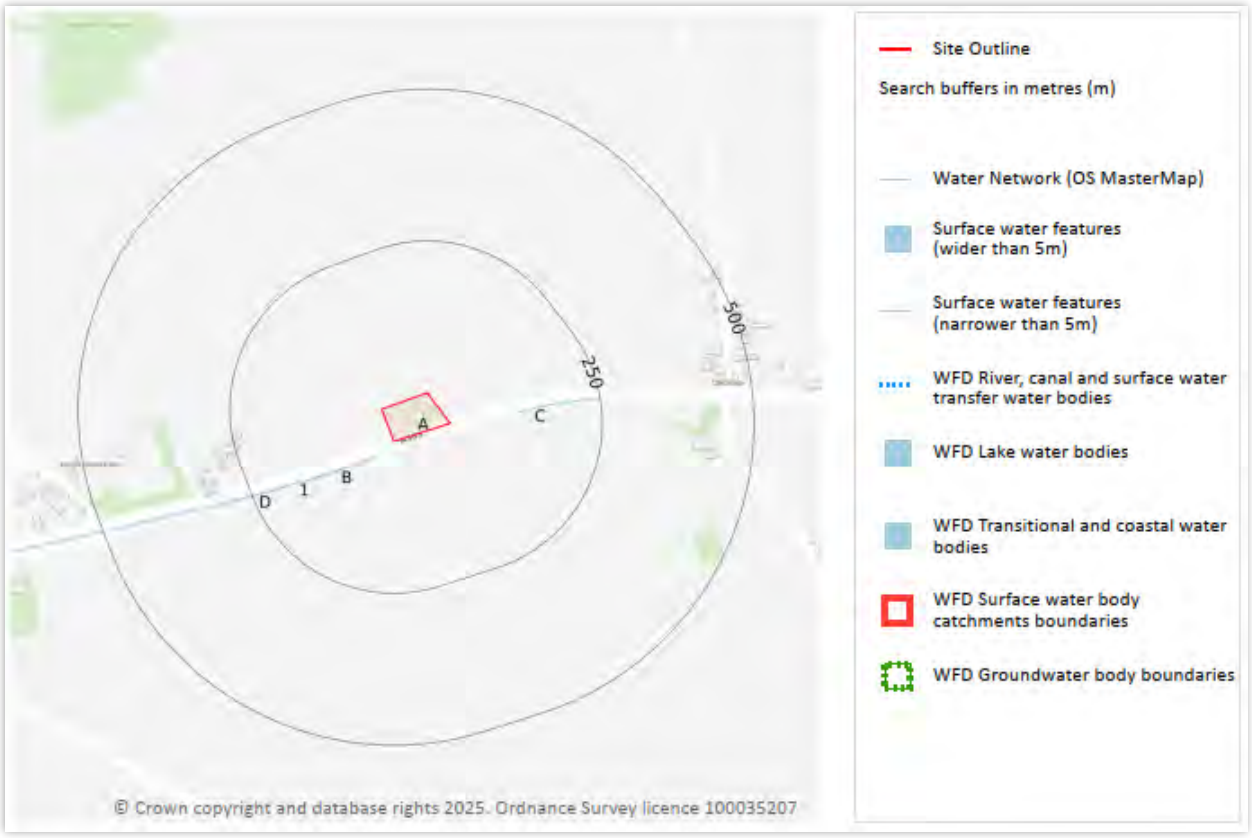


Figure 5: Water network map (Groundsure)

During site investigations, windowless samples were bored to 10.0m depth and no groundwater was encountered.

Soakage testing was undertaken during the site investigation works and results indicate favourable rates of infiltration at 1.0m BGL. Owing to the reduced ground levels proposed in the application, it is anticipated that infiltration results will, as a minimum, be equivalent, but will in all likelihood exceed the tested values. The SI report is included in **Appendix B,** the soakage test results are reproduced in **Figure 6.**

Soakage Testing Results						
Trial Hole	Test Number	Depth (m bgl)	Start Depth (m bgl)	Finish Depth (m bgl)	Time Taken (mins)	Infiltration Rate (m/sec)
TP01	1	1.00	0.60	1.00	60	4.22×10^{-5}
	2	1.00	0.18	0.88	94	3.76×10^{-5}
	3	1.00	0.40	0.93	90	4.14×10^{-5}
TP02	1	1.00	0.54	1.00	20	1.85×10^{-4}
	2	1.00	0.45	1.00	69	8.42×10^{-5}
	3	1.00	0.39	0.97	60	7.18×10^{-5}
TP03	1	1.00	0.10	1.00	50	9.90×10^{-5}
	2	1.00	0.39	0.91	70	6.02×10^{-5}
	3	1.00	0.45	0.93	90	4.36×10^{-5}
TP04	1	1.00	0.53	1.00	30	2.02×10^{-4}
	2	1.00	0.30	0.90	70	5.94×10^{-5}
	3	1.00	0.37	0.95	95	3.88×10^{-5}
TP05	1*	1.00	0.17	0.62	240	7.52×10^{-6}
	2	-	-	-	-	
	3	-	-	-	-	-
TP06	1*	1.00	0.17	0.61	240	7.06×10^{-6}
	2	-	-	-	-	-
	3	-	-	-	-	-
TP07	1*	1.00	0.14	0.69	240	9.75×10^{-6}
	2	-	-	-	-	
	3	-	-	-	-	-
TP08	1	1.00	0.20	1.00	39	7.56×10^{-5}
	2	1.00	0.26	1.00	53	5.87×10^{-5}
	3	1.00	0.24	0.81	85	2.38×10^{-5}
*Given the relatively slow infiltration rate encountered, only one test could be undertaken and it did not reach 75% mark.						

Figure 6: Infiltration results – highlighted value used for designs

Existing Drainage:

The existing local public sewer network in the area is owned by Wessex Water. It is known from the adjacent application that no sewers exist on site; the surface water for that development is infiltrated on site and the foul drainage discharges to a private treatment plant and disposed via infiltration.

Wessex Water's online maps were checked and the plans confirm there is no foul or surface water sewer in the area. As the land is currently used for agriculture there are no existing private drains on site.

Refer to **Appendix C** for a copy of the blank asset search.

3. Proposed Development

Development Description:

The proposed development is a new area of car parking to increase capacity for the service station currently under construction. The plans allow for 42 new parking spaces with aisles to circulate, areas of planting, and a new electrical substation. To enable this, the existing ground will be reduced and reshaped to the required profile.

Refer to **Appendix D** for a copy of the Architect's Site and the Landscape Architect's plans, detailing the development proposals.

Development Suitability:

The NPPF aims to direct developments to suitable areas with low probability of flooding. The table below in **Figure 7** illustrates the acceptable classification of development within each flood zone.

Flood Risk Vulnerability Classification	Essential Infrastructure	Water Compatible	Highly Vulnerable	More Vulnerable	Less Vulnerable
Flood Zone 1 (<1 in 1000)	✓	✓	✓	✓	✓
Flood Zone 2 (up to 1 in 1000)	✓	✓	Exception Test	✓	✓
Flood Zone 3a (1 in 100 fluvial) (1 in 200 tidal)	Exception Test	✓	X	Exception Test	✓
Flood Zone 3b (functional floodplain)	Exception Test	✓	X	X	X

Figure 7: NPPF Table 2

The proposed development type is considered to be classified as a 'less vulnerable' development under Table 2 of the NPPF.

As the site is in Flood Zone 1, all vulnerability classes are suitable and thus the proposed scheme is deemed sequentially appropriate within flood risk terms, with the Exception Test therefore not required.

Romsey: 01794 524447 Bath: 01225 251498
mail@awaengineers.com
www.awaengineers.com

4. Flood Risk

This section of the report explores flood risk to the site.

Tidal & Fluvial:

The Environment Agency (EA) Flood Map for Planning shows the site is located within Flood Zone 1 – Low probability in Table 1 of NPPF Planning Practice Guidance. This zone has less than 1 in 1000 year annual probability of flooding.

The NPPF Planning Practice Guidance states that all types of development are suitable for this flood zone. See below **Figure 8** for the EA map.



Figure 8: EA Flood Map

The proposed site is identified as lying outside of the tidal and fluvial flood risk zone. Given the location and elevation of the site, the risk of tidal and fluvial flooding should be considered as very low.

Surface Water Flooding:

The EA Flood Map below in **Figure 9** shows the risk of flooding from surface water up to and including the 1 in 1000 year event plus climate change allowance. The flood risk occurs to the south of the site and does not impact the proposed development.

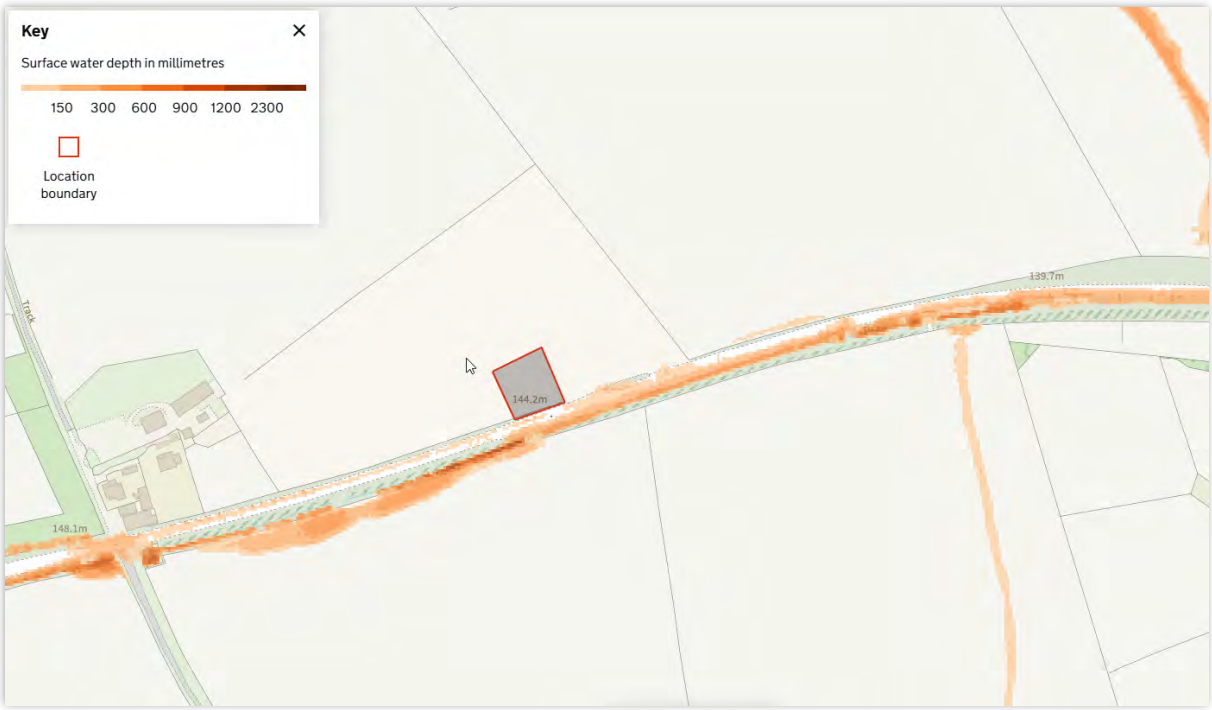


Figure 9: EA Surface Water flood risk map

The risk of flooding from surface water should be considered very low.

Groundwater Flooding:

Groundwater Emergence for the development has been checked against the Wiltshire Council *Strategic Flood Risk Assessment Data Explorer* which notes "there is a risk to flooding of subsurface assets but surface manifestation is unlikely". The development has no critical subsurface assets that would be affected. The same data explorer shows the site straddling a groundwater flood warning area relating to the South Wiltshire Downs area.

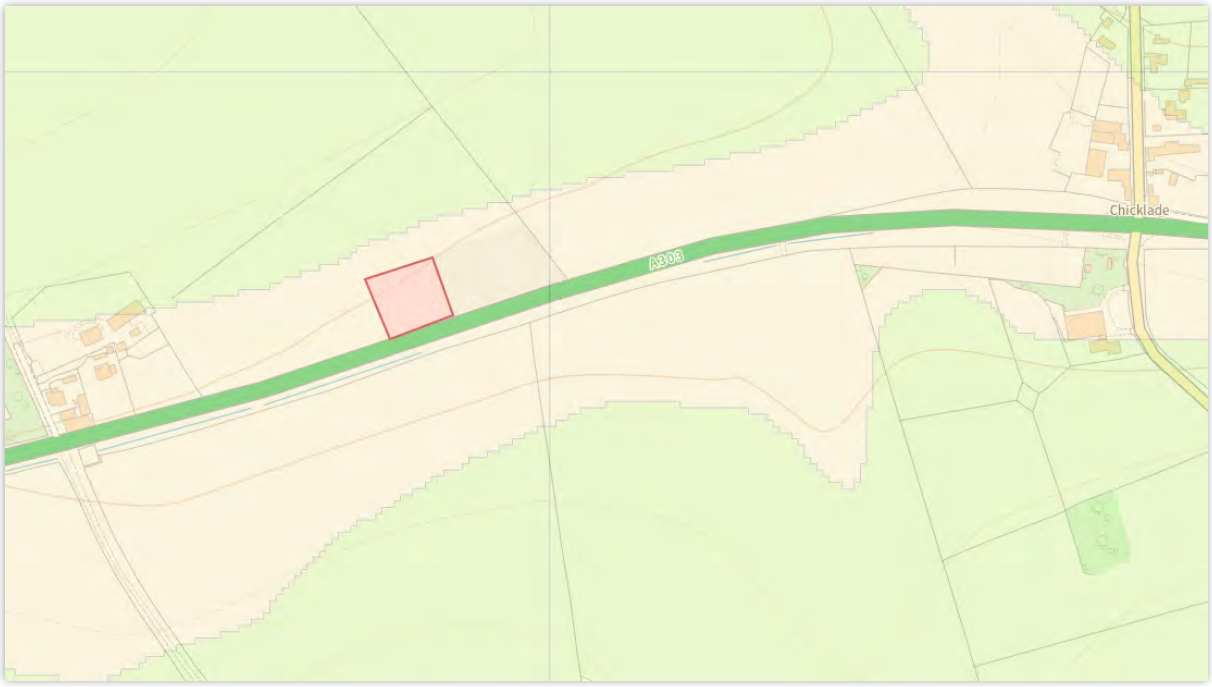


Figure 10: Groundwater emergence from *Strategic Flood Risk Assessment Data Explorer*

The risk of groundwater flooding is noted as "High" in the Groundsure report in the SI, but the same report notes no historical flood events in the preceding 80 years, on site or within 250m, inclusive of groundwater flooding.

7.2 Historical Flood Events

Records within 250m	0

Records of historic flooding from rivers, the sea, groundwater and surface water. Records began in 1946 when predecessor bodies started collecting detailed information about flooding incidents, although limited details may be included on flooding incidents prior to this date. Takes into account the presence of defences, structures, and other infrastructure where they existed at the time of flooding, and includes flood extents that may have been affected by overtopping, breaches or blockages.

This data is sourced from the Environment Agency and Natural Resources Wales.

Figure 11: Historical Flood Events (*Groundsure Report)*

The intrusive tests for the SI involved a number of boreholes carried out by windowless sampler, extending to 10.0m BGL. No water strikes were noted in any of the boreholes and trial pits. The risk of groundwater flooding is considered low.

Sewer Flooding:

There are no recorded sewers or private drains in the vicinity of the site so the risk of sewer flooding should be considered very low.

Flooding from Artificial Sources:

The EA's Flood Risk from Reservoirs map in **Figure 12** below shows that the site is not at risk of flooding from a reservoir. There are no canals or other artificial water bodies off site that would pose a risk.

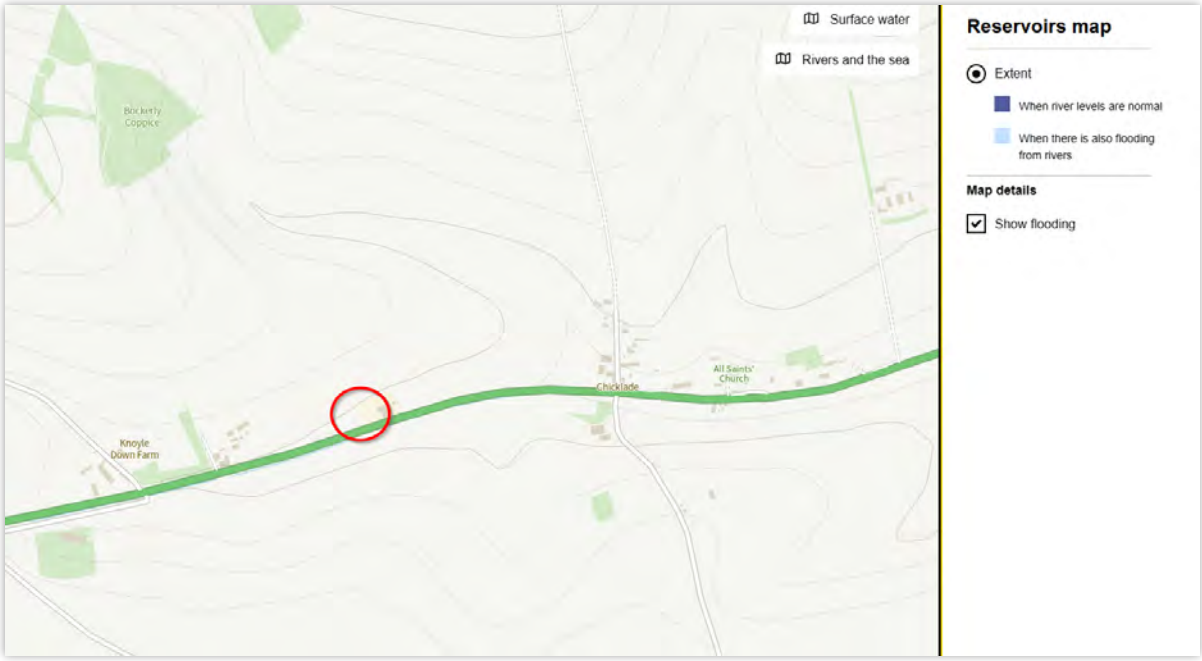


Figure 12: EA Flood Risk from Reservoirs map

The risk of flooding from artificial sources should be considered very low.

5. Drainage Strategy

Existing Drainage

The application boundary for the site equates to an area of approximately 0.6245ha and is considered as a greenfield site.

The existing site greenfield runoff rates were determined using the UKSuds online greenfield runoff rate estimation tool and the results are presented in **Appendix E.**

As the site is currently undeveloped and used for agriculture there is no existing drainage infrastructure. As previously discussed, the topography broadly broadly falls in a North West to South-East decline, ending at the southern boundary where the A303 runs parallel. There is no cut-off drain or ditch between the site and the highway.

Proposed Foul Water Drainage:

The development proposals are strictly limited to the surfacing and planting for the car park; the adjacent service station has WCs and refreshments available and thus there is no foul provision relating to this development.

Proposed Surface Water Drainage:

Current legislation and guidance requires developers to manage surface water runoff from new developments to mitigate flood risk to the site and the surrounding area, and also provide a sustainable means of disposing of runoff from impermeable areas. The following demonstrates compliance with the National Standards for Sustainable Drainage Systems (SSDS) July 2025.

Standard 1: Runoff destination

The surface water runoff should be disposed of according to the following hierarchy:

- priority 1: collected for non-potable use
- priority 2: infiltrated to ground
- priority 3: discharged to an above ground surface water body
- priority 4: discharged to a surface water sewer, or another piped surface water drainage system
- priority 5: discharged to a combined sewer

The site investigation works undertaken for the adjacent site included soakage testing and the results indicate favourable rates of infiltration across the site. As discussed in section 2, due to the reduced ground levels proposed in the application it is anticipated that infiltration results will, as a minimum, be equivalent, but will in all likelihood exceed the tested values. The lowest adjacent infiltration rate has been used for designs:

3.88×10^{-5} m/s (0.1396mm/hr)

The surface water drainage for the development will make use of a combination of SuDS techniques to manage the runoff:

- Bio-infiltration areas to treat runoff and provide biodiversity and amenity benefit
- Permeable paving to control runoff and infiltrate to ground
- Soakaways to infiltrate to ground

Disposal Hierarchy

Priority 1: The runoff comes from paved surfaces and is therefore not suitable for collection and reuse.

Priority 2: Infiltration to ground via permeable paving and soakaways.

Standard 2: Interception

Bio-infiltration and permeable surfacing provide interception and treatment at source. Impermeable areas are captured by a traditional network of channels and below ground pipework, ultimately discharging to soakaway and infiltrating to ground.

The use of bio-infiltration and permeable paving ensures compliance with Standard 2.

Standard 3: Management of extreme rainfall

Rate and volume control for the development will be achieved through storage and infiltration, with soakaways and permeable paving sized for the 1:100yr (1% AEP) + 45% climate change event. No allowance is made for urban creep, as any future extensions would be subject to a new planning application, and a new surface water system would be sized appropriately for said extension.

The landscape design provides small planting areas in the middle of the car park, between the circulation space and the bays. These have been incorporated as bio-infiltration areas and accept lateral flows from portions of the impermeable aisles directly onto the planted surface soil medium, which is an engineered SuDS blend for rain gardens, over a granular storage layer below.

Impermeable asphalt concrete (AC) forms the entrance and circulation routes of the car park, with permeable asphalt used for all parking bays. The central bays accept lateral flows from the western aisle, but in general the levels required to tie into the wider site make it inefficient to drain the aisles to the rest of the bays. T**he permeable paving will offer approximately 71.3m³ of storage.**

Surface level channels and gullies convey the remaining flows from the impermeable areas to a below ground network of pipes before passing through an interceptor and discharging to a geocellular soakaway matrix. **The soakaway offers approximately 50.3m³ of storage.**

The proposed system has been modelled using InfoDrainage software and calculations show the system has sufficient capacity to accept the volume of runoff generated for the simulated events. The bio-infiltration areas by design do not store larger rainfall events and these have been designed with overflows, with exceedance cresting the kerbs and being captured as overland flows in the channels and gullies downstream. Levels and gradients have been set to ensure self-cleansing, and pipes sized to accommodate the hydraulic flows.

Exceedance flows which may occur due to blockage or rainfall events beyond the design criteria have been considered. Although the car park is considered less vulnerable the exceedance flows will still be routed downhill towards the junction with the rest of the service station, which has a separate surface water system. This junction has a number of gullies that convey flows to an infiltration ditch to the east of the site, allowing any exceedance to be routed away from vulnerable areas.

The designed system is compliant with Standard 3.

Standard 4: Water Quality

The "pollution hazard indices for different land use classifications" table has been extracted from CIRIA C753 The SuDS Manual, and applied to the development proposals.

Pollution Hazard Indices:

Land Use	Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydro-carbons
Non-residential car parking with frequent change (eg hospitals, retail)	Medium	0.7	0.6	0.7

The highest pollution hazard level for the proposed development is deemed to be **'medium'**.

The treatment trains for car park run-off are:

- bio-infiltration to proprietary interceptor
- direct to permeable paving

SuDS Mitigation Indices:

Type of SuDS Component	Mitigation Indices		
	TSS	Metals	Hydrocarbons
Bioretention component	0.8	0.8	0.8
SPEL 210C1/ESR	0.8	0.6	0.9
Permeable Pavement	0.7	0.6	0.7

The full calculation of mitigation indices is presented in **Appendix F** and demonstrates compliance with Standard 4.

Standard 5: Amenity

Bio-infiltration rain gardens will form the planted islands in the car park, allowing for visual interest where the SuDS components are integrated into the landscape proposals, showing compliance with Standard 5.

Standard 6: Biodiversity

The rain gardens will be planted with a variety of grasses and plants suitable for the occasional ponding that will be present, and these plants will provide complementary habitat for different insects and pollinators already attracted by the wider landscape proposals, demonstrating compliance with Standard 6.

Standard 7: Construction & Maintenance

The development utilises standard groundwork practices and materials to achieve a cost efficient and timely construction programme. No elements of the proposed design are complex, and are well within the capabilities of experienced contractors to construct safely and with managed risk.

The permeable paving and rain gardens are low impact, surface level installations with few proprietary items beyond geotextiles and pipework, and the pipes, chambers and geocellular crates are constructed using durable PVC-U and polypropylene products, which have a lower carbon impact than concrete.

Maintenance of the drainage system and SuDS features would be carried out in accordance with CIRIA C753 'The SuDS manual' to minimise the residual flood risk of drainage system blockage.

As a private site, maintenance will be the responsibility of the owner to ensure that all drainage systems are fully maintained for the lifetime of the development. The

maintenance requirements have been extracted from The SuDS Manual and are presented in **Appendix G.**

Discussion of the Standards has informed the Drainage Strategy, as described above; a Layout detailing this along with supporting details and calculations can be found in **Appendix H**.

6. Conclusion & Recommendations

AWA were engaged by Bruno Skinner of Blackford Projects to provide a Flood Risk Assessment and Drainage strategy to support a Planning Application for the construction of a car park on greenfield land. Although the site is less than 1.0ha our client has requested the due diligence of a flood risk assessment to ensure risks are correctly understood and mitigated.

Flood mapping shows that the site is located within Flood Zone 1. Under the NPPF the proposed development is classified as 'less vulnerable', and is located within a sequentially appropriate fluvial and tidal Flood Zone and does not require the application of the exception test.

The potential risk of flooding from other sources at the site including surface water, sewers and artificial sources are considered very low. The potential risk of flooding from groundwater is considered low.

The surface water drainage strategy will manage flood risk by storing and infiltrating runoff to greenfield rates. The proposed surface water system is gravity drained, leveraging rain gardens for amenity and biodiversity benefit, and permeable surfaces for direct infiltration, with channels and pipes to deliver run-off to soakaway. The bio-infiltration, permeable paving and interceptor provide suitable treatment to mitigate the pollution hazard in line with the CIRIA C753 The SuDS Manual. The system has been designed to cater for the 1 in 100 year storm event including an additional 45% allowance for climate change.

Pollution risks from the site have been assessed using the simple indexed approach in line with the SuDS Manual, and the proposed design sufficiently mitigates the risks.

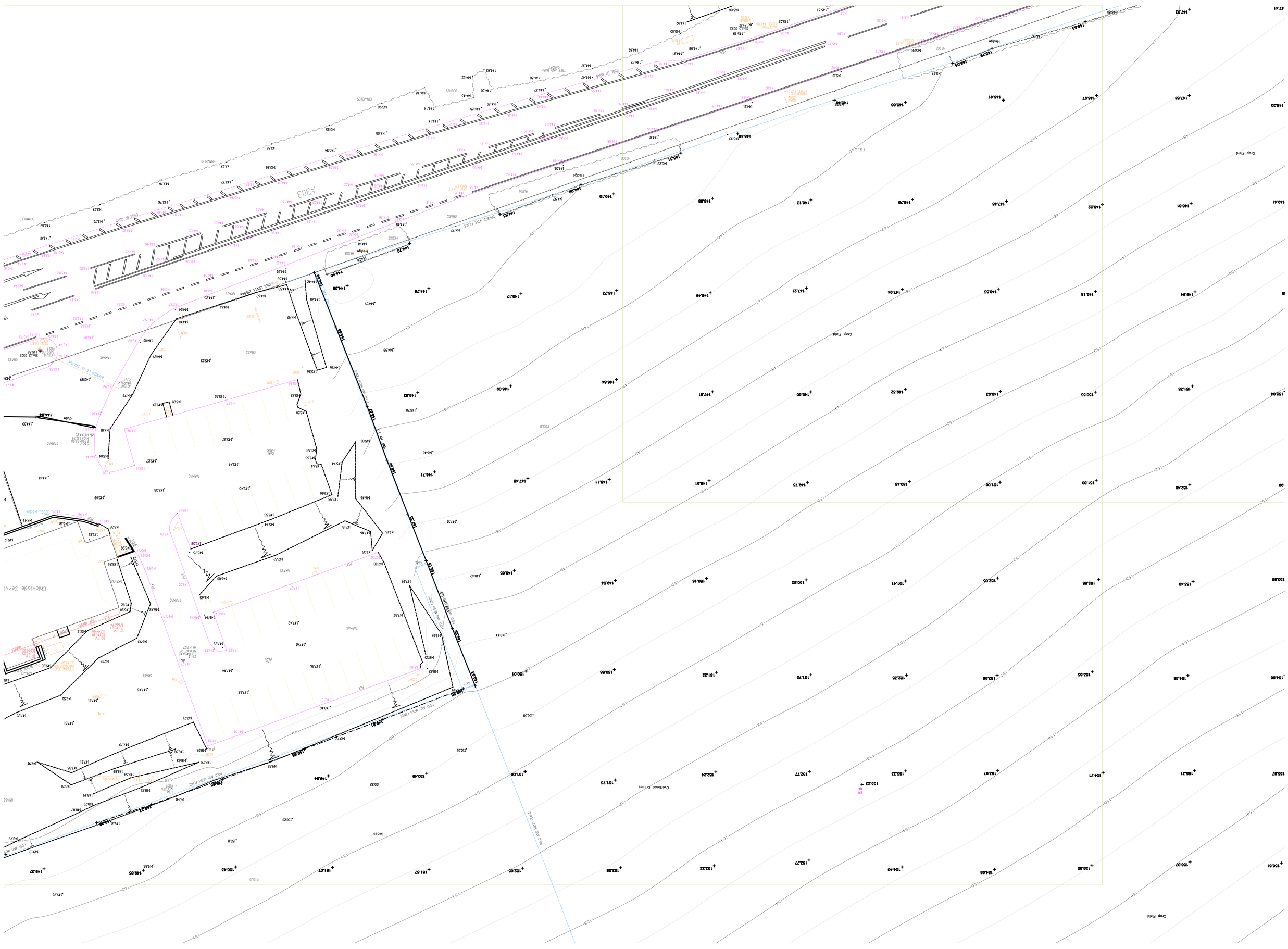
Maintenance of the SuDS will be in line with the SuDS Manual and all private drainage maintenance will be arranged/carried out by the property owner.

In conclusion, this Flood Risk Assessment and Drainage Strategy demonstrates that the proposals are consistent with the aims of the NPPF, planning guidance and the National Standards for Sustainable Drainage Systems.

Authorised by:	**CRAIG BOBBETT**	Position:	**Senior Engineer**
Checked by:	**SCOTT WEBSTER**	Position:	**Technical Director**

Romsey: 01794 524447 Bath: 01225 251498
mail@awaengineers.com
www.awaengineers.com

Appendix A: Topographical Survey


Crop Field
Overhead Cables
Hedge
Drain
Ditch
Gate
Fence
Post and Wire Fence
Post and Mesh Fence
Barbed Wire Fence
Post and Rail Fence
Concrete Post
Wooden Post
Stone Wall
Bushes
Trees
Pylon
Telegraph Pole
Manhole
Inspection Chamber
Soil Pipe
Vent Pipe
Rainwater Pipe
Gully
Kerb
Footpath
Road
Tarmac
Grass
Gravel
Concrete
Brick Paving
Building
Shed
Garage
Porch
Conservatory
Chimney
Roof Light
Solar Panel
Satellite Dish
Air Conditioning Unit
Flood Light
Security Camera
Electricity Meter
Gas Meter
Water Meter
Stop Tap
Boundary Post
Fire Hydrant
Bollard
Traffic Light
Road Sign
Street Light
Bench
Picnic Table
Play Equipment
Sports Pitch
Goal Post
Basketball Hoop
Tennis Court
Swimming Pool
Pond
Stream
River
Canal
Lake
Reservoir
Weir
Sluice
Culvert
Bridge
Tunnel
Underpass
Overpass
Footbridge
Level Crossing
Railway
Railway Track
Railway Platform
Railway Station
Railway Signal
Railway Bridge
Railway Tunnel
Railway Embankment
Railway Cutting
Railway Fence
Railway Gate
Railway Crossing
Railway Sleeper
Railway Ballast
Railway Overhead Line
Railway Pylon
Railway Substation
Railway Depot
Railway Workshop
Railway Yard
Railway Siding
Railway Buffer Stop
Railway Turntable
Railway Water Tower
Railway Coaling Tower
Railway Signal Box
Railway Level Crossing
Railway Footbridge
Railway Bridge
Railway Tunnel
Railway Embankment
Railway Cutting
Railway Fence
Railway Gate
Railway Crossing
Railway Sleeper
Railway Ballast
Railway Overhead Line
Railway Pylon
Railway Substation
Railway Depot
Railway Workshop
Railway Yard
Railway Siding
Railway Buffer Stop
Railway Turntable
Railway Water Tower
Railway Coaling Tower
Railway Signal Box
Railway Level Crossing
Railway Footbridge

Appendix B: Site Investigation Report

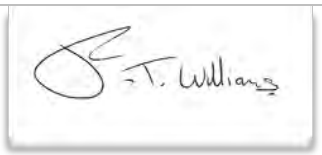
geotechnical and environmental consultants

ground&water

Phase I Desk Study

Chicklade Service Station, Hindon, Salisbury SP3 5ST

On behalf of Instavolt

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Issue	Prepared By	Checked By	Verified By
v.2.01			
	Roger Foord BA (Hons) MSc DIC CGeol FGS CIHM Senior Engineer	Simon Harlow BSc (Hons) FGS Principal Engineer	Francis Williams MGeol (Hons) FGS CEnv CGeol Director

Site Investigations | Environmental Consultants | Geotechnical Engineers

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

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APPENDIX E: GroundSure EnviroInsight and GeoInsight Datasheets 8

EXECUTIVE SUMMARY	
PROPOSED DEVELOPMENT	At the time of reporting, November 2025, the proposed development was understood to comprise the demolition of the existing service station (comprising a petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with soft landscaping, revised vehicular access and formation of a new pedestrian link. The proposed development plan can be seen within Figure 3.
GEOLOGY AND HYDROGEOLOGY	The southern portion of the site was underlain by the Head Deposits (Secondary A Aquifer) and the entire area of the site was underlain by bedrock deposits of the Seaford Chalk Formation (Principal Aquifer). No areas of Made Ground or Reworked Ground were noted within a 250m radius. Examination of the Environment Agency records showed that the site was located within a Total Catchment (Zone III) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater. An Inner Zone (Zone I) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater was located ~3km to the east of the site. Groundwater anticipated to be encountered at depth within the Seaford Chalk Formation. Some amounts of groundwater however may be present at shallower depths within any Made ground and silty/sandy pockets of the Head Deposits. It was considered that groundwater was flowing in an easterly direction in line with local topography.
POTENTIAL SOURCES OF CONTAMINATION	Full details can be seen in sections 6.3 and 6.4 of this report. The Desk Study has revealed the following potential sources of contamination on-site: • Potential for Made Ground (associated with historical construction/demolition on-site) • ACM • Petrol filling station with USTs with pump islands and infrastructure The Desk Study has revealed no potential sources of contamination off-site:
RECOMMENDATIONS / PHASE II OBJECTIVES	A site investigation was recommended across the entire area of the site to confirm the presence of Made Ground. Low risk of ground gas and vapours from Made Ground and anaerobic decomposition of leaked hydrocarbons. Given the age of the former and existing structures on-site, an Asbestos Management Strategy should be put in place so that any potentially asbestos containing materials are identified and removed from site in a suitable manner to prevent cross-contamination.

1. INTRODUCTION

1.1. General

Ground and Water Limited were instructed by Instavolt (the Client) on 26th September 2025 to conduct a Phase I Desk Study on the Chicklade Service Station, Hindon, Salisbury SP3 5ST (the Site). The scope of the investigation was detailed within the fee proposal QU-1868, dated 20th August 2025.

1.2. Aims of the Investigation

This Phase 1 Desk Study was undertaken to advise the client on risk factors pertaining to the site with special reference to former and present day potential contaminative uses and their impact on sensitive receptors, these being human health, controlled waters, buildings, building materials and services.

This Phase I Desk Study comprises a Tier 1 Preliminary Risk Assessment, under Land Contamination Risk Management (LCRM), with the intention of discharging any relevant planning conditions.

1.3. Conditions and Limitations

This report has been prepared based on the terms, conditions and limitations outlined within Appendix A.

Reference has been made to Subadra Remediation Strategy Report referenced IN20485 CL 003, dated July 2022 which summarised the results of the previous desk study and ground investigation (Subadra Consulting Ltd. Environmental Investigation Report Reference IN20485 CL 001, dated December 2020). Ground and Water Limited cannot be held liable for any shortcomings.

The Proposed Development Plan in Figure 2 has also been updated and this report supersedes the Phase II Ground Investigation Report produced by Ground and Water Limited reference GWPR6751/SIR/November 2025 v1.01.

1.4. Technical Glossary

Generic technical terms and their description can be viewed within the glossary provided within Appendix B.

2. SITE SETTING

2.1. Site Location

The site comprised a ~5,200m^2 rectangular-shaped plot of land, orientated in an east to west direction, along the northern side of the A303. The site was located to the west of the village of Chicklade, Wiltshire.

The Easting and Northing for the centre of the site was approximately 90470 34459. A site location plan is given within Figure 1.

2.2. Site Description

A Site Walkover was undertaken on 29th September 2025. A description of the site, as noted during the Site Walkover, is tabulated below. Photographs can be seen in Appendix C.

Site Description Sheet	
Use of site	At the time of the Site Walkover, 29th September 2025, the site was occupied by a closed petrol filling station.
Site topography	The site was sloping gently towards the south. The two car parking areas in the western portion of the site had been terraced into the existing slope.
Area topography	The area topography is a shallow sided valley which the site is located on the northern edge of the valley. The A303 runs along the base of the valley
Structures on-site	The centre of the site was occupied by the buildings of the former filling station, café/bakery and shop. The canopy and former pump islands were located between the building and the southern boundary. A detached former toilet block was located on the eastern boundary with an adjacent partially submerged septic tank. It is understood that up to 6No. Underground fuel tanks were located in the south-eastern corner of the site.
Structures off-site	None in a close proximity.
Use of surrounding ground	Farmland to the north-east and west and the A303 to the immediate south with farmland beyond.
Boundary features	Northern Boundary: ~0.80m post and wire fence. Eastern Boundary: ~0.80m post and wire fence. Southern Boundary: open. Western Boundary: ~0.80m post and wire fence.
Site covering	The majority of the site was covered with tarmac and concrete hardstanding with areas soft landscaping.
Contamination sources on-site	Underground fuel tanks; Pump islands and associated infrastructure. Partially submerged septic tank. ACMs in existing buildings
Contamination sources off-site	None noted
Vegetation on-site	Small hedges along boundaries of the site.
Vegetation off-site	Semi-mature and mature ~2 – 12m high trees were noted on the southern side of the A303.
Services	Numerous services noted.

2.3. Proposed Development

At the time of reporting, November 2025, the proposed development was understood to comprise the demolition of the existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with soft

landscaping, revised vehicular access and formation of a new pedestrian link. The proposed development plan can be seen within Figure 2.

2.4. Geology

The British Geological Survey Solid and Drift Geology Map for the Salisbury area revealed that the site was underlain by superficial Head Deposits and bedrock deposits of the Seaford Chalk Formation. No areas of Made Ground or Reworked Ground were noted within a 250m radius.

There were no BGS borehole records within a 500m radius of the site.

2.5. Hydrogeology and Hydrology

A study of the aquifer maps on the DEFRA website revealed the site was underlain by a **Secondary A Aquifer** comprising the superficial Head Deposits, underlain by a **Principal Aquifer** comprising bedrock deposits of the Seaford Chalk Formation.

Examination of the Environment Agency records showed that the site was located within a Total Catchment (Zone III) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater. An Inner Zone (Zone I) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater was located ~3km to the east of the site.

The nearest surface water feature was an unnamed stream 37m to the south-west and flowing in an easterly direction.

From analysis of hydrogeological and topographical maps the actual groundwater table was anticipated to be encountered at depth within the Seaford Chalk Formation. Some amounts of groundwater however may be present at shallower depths within any Made ground and silty/sandy pockets of the Head Deposits. It was considered that groundwater was flowing in an easterly direction in line with local topography.

Examination of the Environment Agency records showed that the site was located within a Flood Zone 1, i.e. an area with a very low probability of flooding.

The Groundsure Datasheets revealed the site was at no risk of flooding from rivers and the sea. The datasheets also revealed the southern portion of the site was at risk of a high risk of groundwater flooding, while the northern portion had a low risk. The southern portion of the site had a 1 in 30 year period of surface water flooding greater than 1.0m in depth.

2.6. Radon

A review of the freely available UK Health Security Agency radon database, UK Radon, indicated that the site was located within a 1km grid square, where the maximum radon potential of <1% was recorded for above ground structures. Basic radon protection measures are required in areas where more than 3% of houses are at or above the Action Level.

The site was in an area where a risk assessment was not required.

2.7. Internet Search (Site Setting)

2.7.1. Available Unexploded Ordnance (UXO) Map Review

A review of the data available on www.zeticauxo.com/ revealed the site was located within a low-risk area associated with unexploded ordnance (UXO), as indicated by having <15 bombs dropped per 1000 acres. No known Luftwaffe Bombing Target sites were located within a close proximity of the site.

2.7.2. Historical Landfill Tool Review

A review of the data available on www.groundsure.io/ revealed no areas of Historical Landfilling within a close proximity of the site.

3. HISTORICAL REVIEW

3.1. Historical Map Review

The object of this search was to report on the history of the site and its environs from available County Series and Ordnance Survey Maps dating from the mid to late 19th Century to the present day and downloaded from Groundsure. In the following sections dealing with individual maps, only features considered to have a potential contaminative impact on the site and usually within a notional 250 metre radius of the site boundaries are discussed. Any distances quoted for features remote from the site have been scaled from the maps and are only approximate. The north point and approximate extent of the site are indicated on each figure. The historical maps referred to are given within Appendix D. The implications of the map search are discussed later within this report.

Environmental Significance of Data from Historical Maps		
Maps Dated Between	**Site**	**Environs**
1886 – 1856	The site comprised a part of an undeveloped open plot of land, likely agricultural land/fields.	The site environs generally comprised undeveloped agricultural land/fields. An east to west trending road was shown along the southern boundary of the site.
1981 – 2025	The site was shown occupied by the buildings and assumed areas of hardstanding/car parking associated with a petrol filling station. Earthworks showed there had been some terracing into the natural slope to form the car parking area in the north-west corner.	No significant change recorded.

3.2. Historical Aerial Photography Review

The object of this search was to report on the history of the site and its immediate environs from available Aerial Photography dating from the late-20th Century to the present day and downloaded from Google Earth and Groundsure. Any distances quoted for features remote from the site have been scaled from the photography and are only approximate.

At the time of the earliest aerial photography, 1999, the centre of the site was occupied by the main building of the petrol filling station with the canopy and fuel pumps in the southern portion of the site. A further building was located on the eastern boundary and a septic tank, as identified in the site Walkover, in the south-east corner of the site. Two car parking areas were located in the western portion of the site.

No changes were noted on the 2005 aerial photograph.

By the 2013 aerial photograph a rectangular structure was located in the north-east corner of the site and by 2020 a similar structure was shown on the centre of the northern boundary. Both of these structures had been removed by 2023.

4. ENVIRONMENTAL AND GEOLOGICAL INSIGHT

4.1. Groundsure Datasheets

Groundsure Environmental and Geological Datasheets were obtained for the site. Unless the data indicates a significant risk, only information within a 250m buffer zone has been included. The GroundSure Datasheets are also presented in Appendix E and a summary is given below and overleaf(s).

Environmental Insight	
Source	Nearest Distance from Site/Dated/Type
Past Land Use	
Historical Petrol Stations Database	1x on-site record. 1x filling station on-site from 1981
Current Industrial Land Use	
Current or recent petrol stations	1x off-site record within a 250m radius 1x MURCO petrol station 30m east.
Licensed pollutant release (Part A(2)/B)	1x on-site record. Chicklade Filling Station: unloading of petrol into storage at service station – current permit – Type B
Licensed Discharges to controlled waters	5x on-site records. 2x on-site records for Chicklade Service Station: Trade discharges – site drainage: contam surface water via soakaways. 3x on-site records for Eat and Go at Chicklade Service Station: Sewage discharges – final/treated effluent – not water company via soakaways.
Hydrogeology	
Superficial Aquifer	1 x on-site record: 1x Secondary Undifferentiated Aquifer on-site.
Bedrock Aquifer	1x on-site record: 1x Principal Aquifer on-site.
Groundwater Vulnerability	2x on-site records and 1x off-site record within a 250m radius.: 1x on-site record for a Secondary superficial aquifer – high vulnerability, leaching class: high. 1x on-site record for a Principal bedrock aquifer – high vulnerability, leaching class: high. 1x off-site record for a Principal bedrock aquifer 23m south-east – high vulnerability, leaching class: high.
Groundwater Vulnerability – Soluble Rock Risk	16% of grid square covered by maximum risk Significant soluble rocks are likely to be present. Low possibility of localised subsidence or dissolution-related degradation of bedrock occurring naturally, but may be possible in adverse conditions such as high surface or subsurface water flow.
Source Protection Zones	1x on-site record and 1x off-site record within a 250m radius.: 1x on-site record for a Zone 3: Total Catchment. 1x off-site record for a Zone 2: Outer Catchment.
Hydrology	
Water Network (OS MAsterMap)	6x off-site records within a 250m radius. All records are for an inland river not influenced by normal tidal action: 37m, 168m and 179m south-west; 109m, 167m and 179m east
Surface Water Features	4x record within a 250m radius. No details given.
WFD Surface Water Body Catchments	1x on-site record: 1x Fontwell Stream catchment: Avon Hampshire – on-site.
WFD Surface Water Bodies	1x on-site record: 1x Fontwell Stream catchment: Avon Hampshire – on-site.
WFD Groundwater Bodies	1x on-site record: 1x Upper Hampshire Avon on-site.

Visual and Cultural Designations	
Areas of Outstanding Natural Beauty (AONB)	1x on-site record and 1x off-site record within a 250m radius. 1x on-site record for Cranborne Chase and West Wiltshire Downs 1x off-site record for Cranborne Chase and West Wiltshire Downs – 199m south
Agricultural Designations	
Agricultural Land Classification	**2x on-site records:** **1x on-site record for Grade 2,** **1x on-site record for Grade 3.**

Geological Insight	
Source	Nearest Distance from Site/ Type
Superficial Geology (1:10,000 Scale)	1x on-site record: 1x record on-site – Head Deposits (Clay, Silt, Sand and Gravel).
Bedrock Geology (1:10,000 Scale)	1x on-site record: 1x Seaford Chalk Formation (Chalk).
Superficial Geology and Permeability (1:50,000 scale)	1x on-site record: 1x Head. Flow Type: Mixed, Maximum Permeability: High, Minimum Permeability: Very Low.
Bedrock Geology and Permeability (1:50,000 scale)	1x record on-site: 1x Seaford Chalk Formation (Undifferentiated). Flow Type – Fracture, Maximum Permeability – Very High, Minimum Permeability – Very High.
Natural Hazard Findings	
Shrink-Swell Clay	Negligible to Very Low hazard on-site. Negligible hazard 23m south-east.
Running Sands	Negligible to Very Low hazard on-site. Negligible hazard 23m south-east.
Compressible Deposits	Negligible hazard on-site.
Collapsible Deposits	Very Low hazard on-site.
Landslides	Negligible to Very Low hazard on-site. Negligible hazard 23m south-east. Low hazard 41m north-east.
Ground Dissolution of Soluble Rocks	Very Low to Low hazard on-site. Very Low hazard 23m south-east.
Mining, Ground Working and Natural Cavities	
Records of Non-coal Mining	1x on-site record: Class A and related to Chalk. Underground workings are uncommon, although the geology is similar to that worked elsewhere. Potential for difficult ground conditions are unlikely and are at a level where they need not be considered.
Radon	
Is the property in a Radon Affected Area as defined by the Health Protection Agency (HPA) and if so what percentage of homes are above the Action Level?	The property is not in a Radon Affected Area, as less than 1% of properties are above the Action Level.
Is the property in an area where Radon Protection are required for new properties or extensions to existing ones as described in publication BR211 by the Building Research Establishment?	No radon protective measures are necessary.
Estimated Background Soil Chemistry	
Records of BGS estimated background soil chemistry	**8x records on-site and off-site within a 250m radius:** **On-site (x3), 6m north-east, 23m south-east and 28m south:** **Arsenic: 15mg/kg** **Lead 100mg/kg (60mg/kg bioaccessible)** **Cadmium: 1.8mg/kg** **Chromium: 60 – 90mg/kg** **Nickel: 15mg/kg**

	On-site (x3): **Arsenic: 15mg/kg** **Lead 100mg/kg (60mg/kg bioaccessible)** **Cadmium: 1.8mg/kg** **Chromium: 60 – 90mg/kg** **Nickel: 15-30mg/kg**

5. ONLINE REVIEW AND HISTORICAL INVESTIGATIONS

5.1. Online Council Planning Database

A review of the Wiltshire Council Planning Database revealed that four planning applications had been filed for the Chicklade Service Station. Those pertinent to the site development area are tabulated below.

Planning Applications for the site		
Application No./Date	**Proposed**	Decision
PL/2022/06298 30/09/2022	Demolish existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with landscaping, revised vehicular access and formation of a new pedestrian link.	Approve with conditions
PL/2025/06159 21/07/2025	Discharge of conditions 26 Construction Environmental Management Plan (CEMP) & 27 Landscape and Ecology Management Plan (LEMP) of planning application PL/2022/06298 (Demolish existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with landscaping, revised vehicular access and formation of a new pedestrian link).	Approve
PL/2025/06567 06/08/2025	Discharge of condition 20 DCEMP of planning application PL/2022/06298 (Demolish existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with landscaping, revised vehicular access and formation of a new pedestrian link).	Partial discharge of conditions
PL/2025/06825 18/08/2025	Retrospective application for the erection of temporary signage to announce the forthcoming opening of the Chicklade EV hub.	Awaited

5.2. Internet Search

An internet search did not identify any other information pertinent to this report.

5.3. Previous Site Investigations

The results of the previous desk study and ground investigation (Subadra Consulting Ltd. Environmental Investigation Report Reference IN20485 CL 001, dated December 2020) were summarised in the Subadra Remediation Strategy Report referenced IN20485 CL 003, dated July 2022.

The main features of the site history were:

- The site had been a petrol filling station since at least the early 1980s (and most likely since the early 1960s). The below ground petroleum installation is the primary potential source of (hydrocarbon) contamination at the site. The current tanks were reported to have been installed in 1983 and were relined (with interstitial monitoring) in 2013. Plastic fuel lines were installed, also in 2013

The key findings of the intrusive investigation were:

- No Made Ground was encountered.
- Soils described as Head Deposits comprising a brown slightly sandy clay with gravel was encountered overlying the Seaford Chalk Formation comprising a weak weathered putty chalk to a depth of 6.0m bgl.
- Groundwater was not encountered.

The key findings of the geo-environmental assessment were:

- During the intrusive investigation no olfactory or visual signs of contamination were noted in any of the soil samples collected. Very low concentrations of hydrocarbon vapours were recorded in a small number of samples, predominantly from depths <2.00m bgl.
- The results of the chemical analysis carried out on selected soil samples revealed the presence of very low concentrations of xylenes in two samples (BH003 at 0.8 and 1.8m) located at the south-eastern corner of the central forecourt area. Very low concentrations of two PAH compounds were also recorded in a single sample (BH005 at 0.8m), located in the south-east corner of the site.

6. PRELIMINARY RISK ASSESSMENT

6.1. Contaminant Source-Pathway-Receptor Model

In the UK, the assessment of risk from contamination follows the source-pathway-receptor (SPR) approach. For a risk to be present there must be a source of contamination, a receptor or receptors, and a pathway for contaminants to migrate or be absorbed. If one of these three elements are absent, it is considered that there is no risk of harm. If, however, there is a linkage between any given source and any given receptor, then a risk-based approach is used to assess the significance or impact of the pollutant linkage.

This Phase 1 Desk Study has been used to identify potential on-site and off-site sources of contamination, which are summarised in this section of the report. Additional potential sources of contamination identified within the Desk Study have been discounted based on the absence of a realistic SPR linkage (i.e. the distance from the site or the nature or age of any potential contamination sources).

In line with the requirements of BS 21365:2020, *Soil Quality – Conceptual site models for potentially contaminated sites*, the Conceptual Site Model (CSM) can be described in text, tabulated or presented as a figure. A tabulated CSM is provided in Section 6.6 of this report, where each component is discussed in the following sections.

6.2. Potential On-site Sources of Contaminants

This Desk Study revealed that at the time of the earliest historical mapping (1886) the The site comprised a part of an undeveloped open plot of land, likely agricultural land/fields. By the time of the 1981 mapping, the site was shown occupied by the buildings and assumed areas of hardstanding/car parking associated with a petrol filling station. Earthworks showed there had been some terracing into the natural slope to form the car parking area in the north-west corner.

The Phase 1 Desk Study revealed the following on-site sources of contamination:

- The site has been occupied by a filling station with underground fuel storage tanks, pump islands and associated infrastructure since at least 1981.

  Potential sources of contamination associated with a petrol filling station could include: lubricant oils; brake fluids (constitute mainly of polymerised glycols and ethers. Waste fluid is generated during repair work on brake systems); solvents (chlorinated hydrocarbons, carbon tetrachloride, paraffin and proprietary degreasing compounds); paints (lead-based paints, zinc-rich epoxy primers, polyurethanes as decorative finishes); gasoline, diesel, paraffin *(Department of the Environment Industry Profile, Road vehicle fueling, service and repair: garages and filling stations 1996).*

- The site has undergone various phases of construction and as a result various thicknesses of Made Ground resulting from these activities are likely to be encountered.

Contaminants of concern associated with Made Ground deposits include; metals, Petroleum Hydrocarbons (TPHs), Polycyclic aromatic hydrocarbons (PAHs), asbestos, sulphates, volatile organic compounds (VOCs) and ground gases.

- During the Site Walkover a septic tank was noted in close to the eastern boundary of the site. The septic tank was noted to be in good condition with no evidence of spills/leaks.

  Potential contaminants could include ammonia and bacterial compounds such as faecal coliforms and faecal streptococci.

- Given the age of the structures an asbestos survey should be undertaken of each building prior to demolition. An **Asbestos Management Strategy** should be put in place to ensure that any potentially asbestos containing materials are identified and removed from site in a suitable manner to prevent cross-contamination.

6.3. Potential Off-site Sources of Contaminants

The Phase 1 Desk Study revealed the no potential off-site sources of contamination.

6.4. Potential Receptors

At the time of reporting, November 2025, the proposed development was understood to comprise the demolition of the existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with soft landscaping, revised vehicular access and formation of a new pedestrian link. The proposed development plan can be seen within Figure 3.

Based on the proposed development, the potential receptors are presented below and comprise:

Human Health

- End users of the site (Workers/Future site visitors);
- Construction workers during redevelopment;
- Site operatives during maintenance works; and
- Neighbours and members of the public.

Flora and Fauna

- Vegetation within soft landscaped areas.

Building Materials and Services

- Buildings;
- Buried concrete;
- Confined spaces; and
- Underground services (Water Pipes).

Controlled Waters

- Secondary Undifferentiated Aquifer (Head Deposits);
- Principal Aquifer (Seaford Chalk Formation); and
- Stream ~37m to the south.

6.5. Contaminant Absorption Pathways

The potential pathways for contaminant absorption between the identified sources and the identified receptors are as follows:

Human Health:

- Direct ingestion of soil and soil derived dust;
- Dermal contact of soil and soil derived dust;
- Inhalation of dust (indoors and outdoors) with elevated concentration of determinands;
- Ingestion of home-grown produce, and soil attached;
- Direct ingestion of groundwater;
- Inhalation of volatile vapour (indoors and outdoors);
- Inhalation of ground gases.; and
- Explosion.

Flora and Fauna

- Direct uptake of groundwater; and
- Direct uptake of contaminants in the soil.

Building Materials and Services

- Direct contact;
- Explosion.

Controlled Waters

- Vertical and lateral migration in permeable strata horizons;
- Via anthropogenic pathways (infilled ground and service runs);
- Surface water Runoff.

6.6. Tabulated Conceptual Site Model

The tabulated Conceptual Site Model developed as part of this Desk Study is outlined overleaf. For ease of reference and understanding, the risks have been classified within this risk assessment against four possible levels / categories, summarised in the table below.

Risk Categories used in the Tabulated CSM	
Negligible	Regarding this potential SPR linkage, the site is considered suitable for the proposed end-use and there is no plausible risk. Therefore, there is no need to further assess this potential source of contamination.
Low Risk	Regarding this potential SPR linkage, the site is considered suitable for the proposed end-use and there is not considered to be an unacceptable risk to receptors. However, it is considered that further investigation to confirm this is recommended.
Moderate Risk	Regarding this potential SPR linkage, the site may not be suitable for the proposed end-use in its current condition and there may be an unacceptable risk to receptors. Further investigation is required to confirm this.
High Risk	Regarding this potential SPR linkage, the site is probably or certainly not suitable for proposed end-use and there is likely to be an unacceptable risk to receptors. Contaminants probably or certainly present and urgent action required in the short term.

Tabulated Conceptual Site Model – Pollutant Linkage Summary (On-Site Sources)			
Potential Sources	Potential Absorption Pathways	Potential Receptors	Risk Classification
Contaminants introduced on-site by former use of the site as a petrol filling station with underground fuel tanks/oil interceptor. • Asbestos, • PAHs, • TPHs, • VOCs, • Sulphates, and • Metals.	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** • Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Moderate**
Underground fuel tank in south-east corners: • TPHs,	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Moderate**
Septic tank close to eastern boundary: • Ammonia;	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works Neighbours and public	**Low**

• bacterial compounds such as faecal coliforms and faecal streptococci	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** Vegetation within soft landscaped areas	**Low**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Low**
Made Ground from construction activities: • Asbestos, • PAHs, • Sulphates, and • Metals.	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Ingestion of home-grown produce, and soils attached • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** • Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south •	**Low**
Aggressive ground conditions with Made Ground and natural ground, including groundwater: • Sulphates, • PAH/TPH.	• Direct contact with aggressive ground conditions	**Building Materials and Services** • Buried Concrete • Underground services (water pipes)	**Low**
Ground gases generated by generated by the anaerobic decomposition of any leaked hydrocarbons beneath site and Made Ground: • Methane, • Carbon Dioxide, • Hydrogen Sulphide, and • Carbon Monoxide.	• Migration through anthropogenic & natural pathways • Inhalation of Asphyxiating gases • Explosion (methane only)	**Human Health** • End Users (Residents/Future site visitors) • Construction workers during development (especially in confined spaces) • Site operatives during maintenance works in confined spaces • Neighbours and public	**Low**
	• Migration through anthropogenic & natural pathways • Explosion (methane only)	**Building Materials and Services** • Buildings • Confined spaces • Underground services	**Low**

7. RECOMMENDATIONS AND PHASE II OBJECTIVES

This section of the report will present recommendations for the further investigation of each plausible pollutant linkage identified by the Conceptual Site Model.

It is recommended that an intrusive ground investigation is undertaken at the site to evaluate the risk that contaminants of concern within the soils and groundwater may affect end-users. This should determine the underlying ground and groundwater conditions and include an assessment of the level of contamination to enable the quantification of the ground-related risks associated with the proposed redevelopment.

Consideration should be given to the testing of soil samples recovered from exploratory holes for chemical laboratory testing. The testing should be for a broad range of contaminants in accordance with DEFRA / CLEA methodologies and include the contaminants of concern identified within the Conceptual Site Model.

7.1. Soils

It is possible that asbestos and asbestos containing materials will be incorporated within any Made Ground. An asbestos management strategy should be implemented to ensure that any asbestos uncovered during the investigation does not pose a risk to members of the public that use the site.

On the basis of the Phase 1 Site Assessment the following contaminants of concern have been identified and should be included in the chemical analysis suite for the ground investigation:

- Asbestos.
- Semi-metals and heavy metals;
- Poly-cyclic aromatic hydrocarbons (PAHs);
- Speciated TPH including full aliphatic/aromatic split;
- Volatile/semi-volatile organic compounds – BTEX Used as marker compounds;
- Sulphates; and
- Cyanide.

The list above does not imply that these determinands are present on-site or that they are likely to cause contamination issues at the site. The ground investigation will be used to prove the presence or absence of these contaminants. The sampling and testing strategy must be in line with current standards. Given the site has undergone previous development, random sampling should be adopted across the site, with the exception of TPHs which should be taken adjacent to the underground fuel tanks and pump islands. Targeted sampling of proposed soft landscaped areas may be deemed appropriate. Results should be assessed against suitable assessment criteria to be protective of human health as well as vegetation.

Sub-surface concrete may be damaged due to being in contact with aggressive ground conditions. Sampling should be undertaken where the proposed foundations will be in contact with Made Ground and/or natural ground and tested for aggressive ground conditions (sulphates/pH). Classification should then be undertaken of the ground conditions in accordance with Building Research Establishment Special Digest 1, 2005, ‘Concrete in Aggressive Ground’.

7.2. Services

The CSM has identified a low/moderate risk for aggressive ground conditions that may affect water supply pipes as part of the development. Consideration should be given to the targeted sampling (0.75 – 1.50m bfgl) and scheduling for contaminants of concern: TPH, Naphthalene, Phenols, BTEX, VOCs and SVOCs.

7.3. Groundwater

If analytical results show elevated concentrations of contaminants of concern in the soil samples then there might be a requirement to assess the potential risks of leachability of contaminants migrating into the Secondary A Aquifer and the Principal Aquifer groundwater underlying the site. This might mean leachate testing on soils samples is required or groundwater sampling and testing.

7.4. Ground-gas

The CSM has identified low risk from ground gases at the site as a result of limited Made Ground on-site or within the site's environs and the anaerobic degradation of leaked hydrocarbons. Analysis of soil samples should include Total Organic Carbon (TOC) testing in order to enable a pragmatic ground gas risk assessment to be undertaken at the site. This approach will only quantify the risk associated with on-site sources.

7.5. Vapours

The CSM has identified a source / sources and associated contaminants that may pose a risk through the vapour pathway. However, during the previous intrusive investigation no olfactory or visual signs of contamination were noted in any of the soil samples collected. Very low concentrations of hydrocarbon vapours were recorded in a small number of samples tested, predominantly from depths <2.00m bgl. The low risk will need to be reassessed upon removal of the USTs.

7.6. Geotechnical Review

7.6.1. General

The BGS have identified the following natural hazards on-site and within a 50m buffer.

Natural Hazards	
Shrink-Swell Clay	Negligible to Very Low hazard on-site. Negligible hazard 23m south-east.
Running Sands	Negligible to Very Low hazard on-site. Negligible hazard 23m south-east.
Compressible Deposits	Negligible hazard on-site.
Collapsible Deposits	Very Low hazard on-site.
Landslides	Negligible to Very Low hazard on-site. Negligible hazard 23m south-east. Low hazard 41m north-east.
Ground Dissolution of Soluble Rocks	Very Low to Low hazard on-site. Very Low hazard 23m south-east.

Based on the above, it was considered that further investigation in the form of a chalk dissolution risk assessment is carried out for the site.

No pits/quarrying or mining features were identified on-site or within the site's close environs from the historical mapping.

When designing foundations, the potential presence of aggressive ground conditions should be taken into consideration. Further investigation may be required in accordance with the guidance established in BRE Special Digest 1 (SD1) (2005) 'Concrete in aggressive ground'. The BGS do not record any details regarding the potential for aggressive ground conditions within shallow units identified at the site.

It is recommended that as part of the site-specific ground investigation on-site, geotechnical testing is undertaken to determine the underlying ground conditions and to evaluate any geotechnical related risks associated with the proposed redevelopment of the site.

The Groundsure datasheets have indicated that the superficial Head is likely to have a very low to high permeability and may not be suitable for surface water disposal. The underlying chalk was recorded to have a very high permeability and therefore would be suitable for surface water disposal. These will be subject to on-site testing. The water discharge into chalk may worsen/create dissolution features and therefore soakaways should be remote from structures and undertaken in accordance with the current guidance.

7.6.2. Chalk Dissolution Risk Assessment

The presence of dissolution features on-site can prove problematic, as they often provide inconsistency within the underlying geology. Often formed during glacial and periglacial conditions, the depth and width of these features can vary substantially, often providing inconsistency in geological strength data.

If a dissolution feature is suspected, a geotechnical engineer and structural engineer should be contacted for suitable engineering designs, based on further works identifying and classifying the area the feature occupies.

The Groundsure Geoinsight datasheets, presented Appendix C of this report, revealed the following information which may be pertinent to this risk assessment.

- Ground Dissolution of Soluble Rocks – The site was classified as a Very Low to Low risk of ground dissolution of soluble rocks.
    - *A very low risk was defined as "Soluble rocks are present within the ground. Few dissolution features may be present. Potential for difficult ground conditions or localised subsidence are at a level where they need not be considered".*
    - *A Low Risk was described as "Soluble rocks are present within the ground. Some dissolution features may be present. Potential for difficult ground conditions are at a level where they may be considered, localised subsidence need not to be considered except in exceptional circumstances".*
- An on-site Class A Historical Mining area was identified on-site. A Class A area was described as *"Sporadic underground mining of restricted extent may have occurred. Potential for difficult ground conditions are unlikely and localised and are at a level, where they need not be considered"*. However, this is generally a precautionary statement attributed to all areas associated with chalk within the ground profile. Therefore, this does not identify any underground chalk features at the site location.
- No records of Brit Pits, surface ground workings were noted within 500m radius of the site;

- No records of Underground workings, Mining Cavities or JPB Mining Areas were noted within a 500m radius of the site.
- The Edmonds Procedure (Edmonds, C.N., 2001. Predicting natural cavities in chalk. Geological Society, London, Engineering Geology Special Publications, 18(1), pp.29-38.); highlighted no anticipated subsidence hazard classification.

The historical mapping review undertaken in section 3.1 of this report confirmed the findings of the Groundsure datasheets discussed above.

The site was within a location where no lidar was available for review.

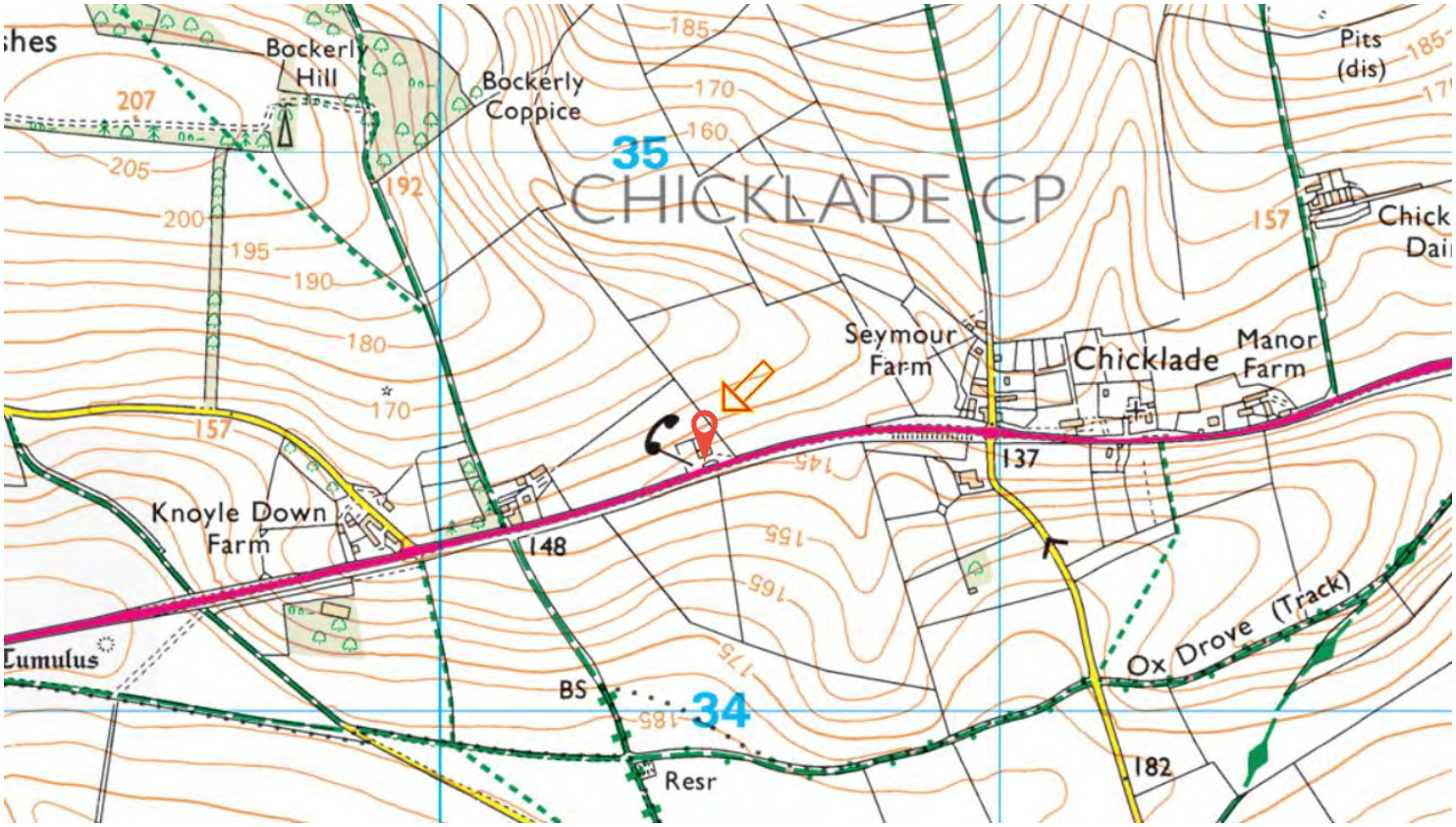
This risk assessment indicates that there is a very low to low risk from the dissolution of soluble rocks at the site.

This chalk dissolution risk assessment should be updated following site investigation works.

FIGURES

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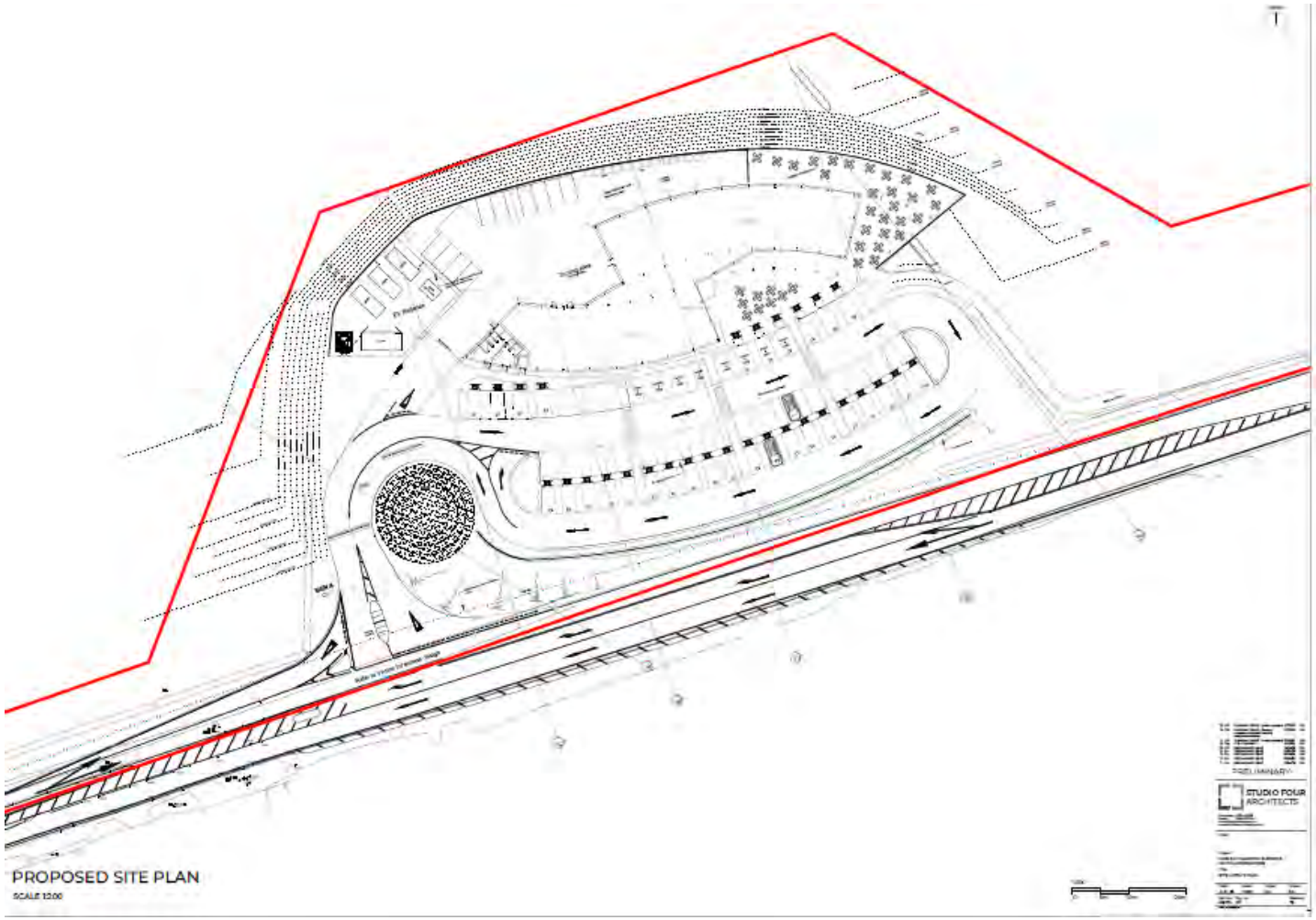


Site Location

Not to Scale

Chicklade Service Station Hindon Salisbury SP3 5ST	
Instavolt	**November 2025**
Figure 1: Site Location Plan	**GWPR6751**

ground&water



Not to Scale

Chicklade Service Station Hindon Salisbury SP3 5ST	
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Figure 2: Site Development Area	**GWPR6751**

APPENDIX A: Conditions and Limitations

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The ground is a product of continuing natural and artificial processes. As a result, the ground will exhibit a variety of characteristics that vary from place to place across a site, and also with time. Whilst a ground investigation will mitigate to a greater or lesser degree against the resulting risk from variation, the risks cannot be eliminated.

The report has been prepared on the basis of information, data and materials which were available at the time of writing. Accordingly, any conclusions, opinions or judgements made in the report should not be regarded as definitive or relied upon to the exclusion of other information, opinions and judgements.

The investigation, interpretations, and recommendations given in this report were prepared for the sole benefit of the client in accordance with their brief; as such these do not necessarily address all aspects of ground behaviour at the site. No liability is accepted for any reliance placed on it by others unless specifically agreed in writing.

Any decisions made by you, or by any organisation, agency or person who has read, received or been provided with information contained in the report ("you" or "the Recipient") are decisions of the Recipient and we will not make, or be deemed to make, any decisions on behalf of any Recipient. We will not be liable for the consequences of any such decisions.

Current regulations and good practice were used in the preparation of this report. An appropriately qualified person must review the recommendations given in this report at the time of preparation of the scheme design to ensure that any recommendations given remain valid in light of changes in regulation and practice, or additional information obtained regarding the site.

Any Recipient must take into account any other factors apart from the Report of which they and their experts and advisers are or should be aware. The information, data, conclusions, opinions and judgements set out in the report may relate to certain contexts and may not be suitable in other contexts. It is your responsibility to ensure that you do not use the information we provide in the wrong context.

This report is based on readily available geological records, the recorded physical investigation, the strata observed in the works, together with the results of completed site and laboratory tests. Whilst skill and care has been taken to interpret these conditions likely between or below investigation points, the possibility of other characteristics not revealed cannot be discounted, for which no liability can be accepted. The impact of our assessment on other aspects of the development required evaluation by other involved parties.

The opinions expressed cannot be absolute due to the limitations of time and resources within the context of the agreed brief and the possibility of unrecorded previous in ground activities. The ground

conditions have been sampled or monitored in recorded locations and tests for some of the more common chemicals generally expected. Other concentrations of types of chemicals may exist.

The conclusions and recommendations relate to the Chicklade Service Station, Hindon, Salisbury SP3 5ST.

Trial hole is a generic term used to describe a method of direct investigation. The term trial pit, borehole or window sampler borehole implies the specific technique used to produce a trial hole.

The depth to roots and/or of desiccation may vary from that found during the investigation. The client is responsible for establishing the depth to roots and/or of desiccation on a plot-by-plot basis prior to the construction of foundations. Where trees are mentioned in the text this means existing trees, recently removed trees (approximately 15 years to full recovery on cohesive soils) and those planned as part of the site landscaping.

Ownership of copyright of all printed material including reports, laboratory test results, trial pit and borehole log sheets, including drillers log sheets, remain with Ground and Water Limited. Licence is for the sole use of the client and may not be assigned, transferred or given to a third party.

Only our client may rely on this report and should this report or any information contained in it be provided to any third party we accept no responsibility to the third party for the contents of this report save to the extent expressly outlined by us in writing in a reliance letter addressed from us to the third party.

Recipients are not permitted to publish this report outside of their organisation without our express written consent.

APPENDIX B: Technical Glossary

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TECHNICAL GLOSSARY

The list of possible definitions within the report may be seen below. Please note that some definitions may not be relevant to this report.

HYDROGEOLOGY:

A **Principal Aquifer** is a layer of rock or drift deposits that have high intergranular and/or fracture permeability - meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale. In most cases, principal aquifers are aquifers previously designated as major aquifer.

Secondary aquifers include a wide range of drift deposits with an equally wide range of water permeability and storage capacities.

Secondary (A) Aquifers consist of deposits with permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as Minor Aquifers.

Secondary (B) Aquifers consist of deposits with predominantly lower permeability layers with may stoke and yield limited amounts of groundwater due to localised features such as fissures, think permeable horizons and weathering. These are generally the water-bearing parts of the former non-aquifers.

Secondary Aquifers (Undifferentiated) are assigned in cases where it has not been possible to attribute either category A or B to a rock type. In most cases, this means that the layer in question has previously been designated as both a minor aquifer and non-aquifer in different locations due to the variable characteristics of the rock type.

Unproductive Strata are rock layers with low permeability that have negligible significance for water supply or river base flow. These were formerly classified as non-aquifers.

FLOOD ZONES:

Environment Agency Flood Zone 2, defined as; land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding; or land having between a 1 in 200 and 1 in 1,000 annual probability of sea flooding.

Environment Agency Flood Zone 3 shows the extent of a river flood with a 1 in 100 (1%0 or greater chance of occurring in any year or a sea flood with a 1 in 200 (0.5%) or greater chance of occurring in any year.

Environment Agency Flood Zone 3 area that benefits from flood defences, defined as; land and property in this flood zone would have a high probability of flooding without the local flood defences. These protect the area against a river flood with a 1% chance of happening each year, or a flood from the sea with a 0.5% chance of happening each year.

GROUNDWATER SOURCE PROTECTION ZONES (SPZS):

Inner zone (SPZ1): This zone is 50 day travel time of pollutant to source with a 50 metres default minimum radius.

Outer zone (SPZ2): This zone is 400 day travel time of pollutant to source. This has a 250 or 500 metres minimum radius around the source depending on the amount of water taken.

Total catchment (SPZ3): This is the area around a supply source within which all the groundwater ends up at the abstraction point. This is the point from where the water is taken. This could extend some distance from the source point.

Zone of special interest (SPZ4): This zone is where local conditions require additional protection.

IN-SITU STRENGTH GEOTECHNICAL TESTING:

Windowless Sample and/or Cable Percussion and/or Rotary Boreholes provide samples of the ground for assessment but they do not give any engineering data. The standard penetration test (SPT) is an in-situ dynamic penetration test designed to provide information on the geotechnical engineering properties of soil. The test uses a thick-walled sample tube, with an outside diameter of 50mm and an inside diameter of 35mm, and a length of around 650mm. This is driven into the ground at the bottom of a borehole by blows from a slide hammer with a weight of 63.5kg falling through a distance of 760mm. The sample tube is driven 150mm into the ground and then the number of blows needed for the tube to penetrate each 75mm up to a depth of 450mm is recorded. The sum of the number of blows is termed the "standard penetration resistance" or the "N-value".

Dynamic Probing involves the driving of a metal cone into the ground via a series of steel rods. These rods are driven from the surface by a hammer system that lifts and drops a 63.5kg (SHDP) hammer onto the top of the rods through a set height, thus ensuring a consistent energy input. The number of hammer blows that are required to drive the cone down by each 100mm increment are recorded. These blow counts then provide a comparative assessment from which correlations have been published, based on dynamic energy, which permits engineering parameters to be generated. (The Dynamic Probe 'Super Heavy' (SHDP) Tests were conducted in accordance with BS 1377; 1990; Part 9, Clause 3.2).

APPENDIX C: Site Photographs

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View of the existing petrol station looking north-east



View of the existing petrol station looking east

Chicklade Service Station Hindon Salisbury SP3 5ST	
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Appendix C – Site Photographs	**GWPR6751**



View of the existing petrol station looking north-east



View of the car park areas looking north along the western boundary

Chicklade Service Station Hindon Salisbury SP3 5ST	
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Appendix C – Site Photographs	**GWPR6751**



View of the forecourt looking west



View of the toilet block and septic tank in eastern portion of the site.

Chicklade Service Station Hindon Salisbury SP3 5ST	
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Appendix C – Site Photographs	**GWPR6751**



View of the toilet block looking north-east



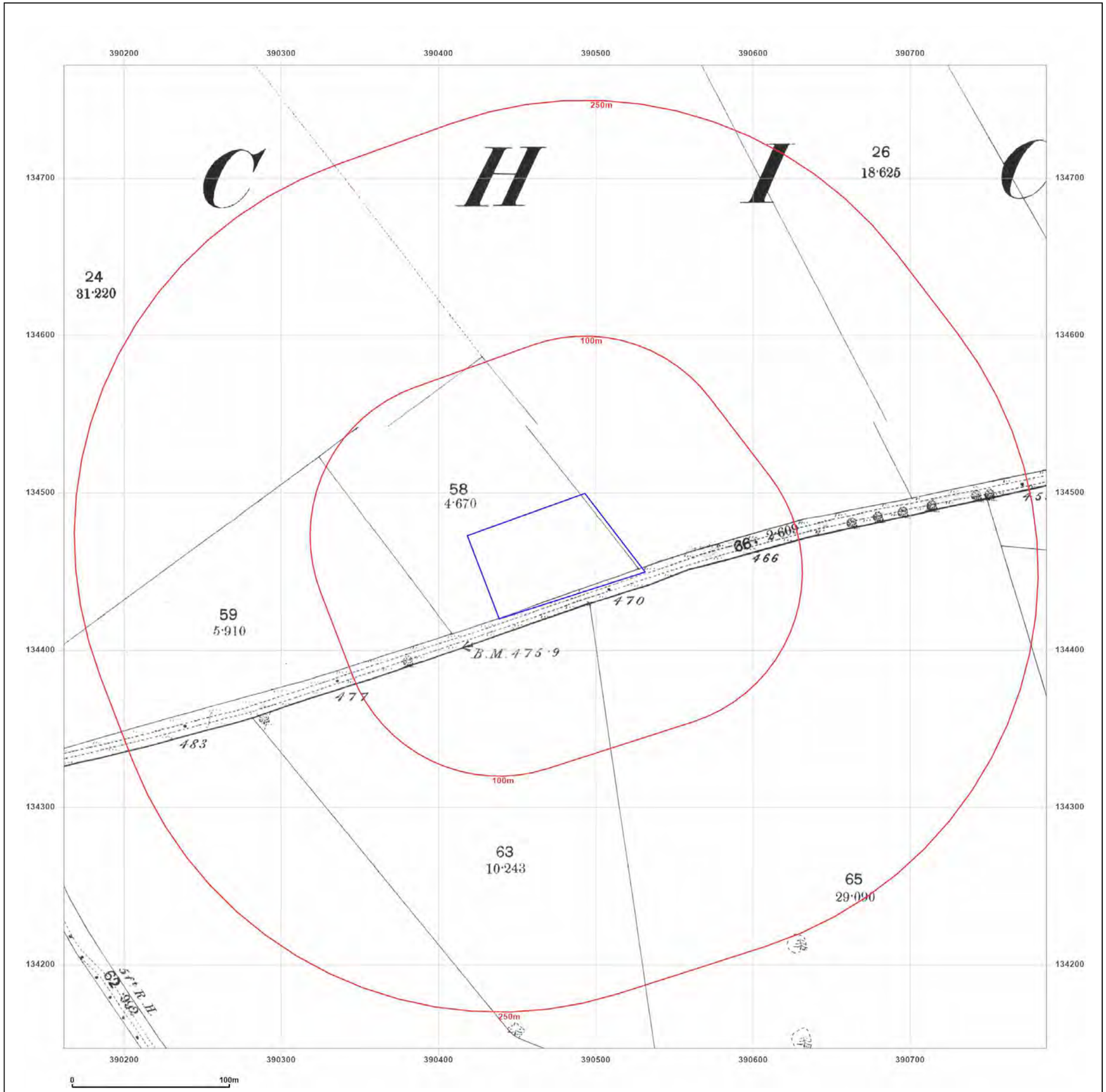
View looking south-west from the eastern boundary

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Appendix C – Site Photographs	**GWPR6751**

APPENDIX D:
Historical Maps

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Groundsure INSIGHTS

Site Details:

CHICKLADE SERVICE STATION,
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SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

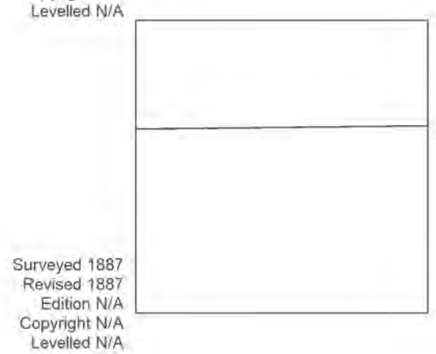
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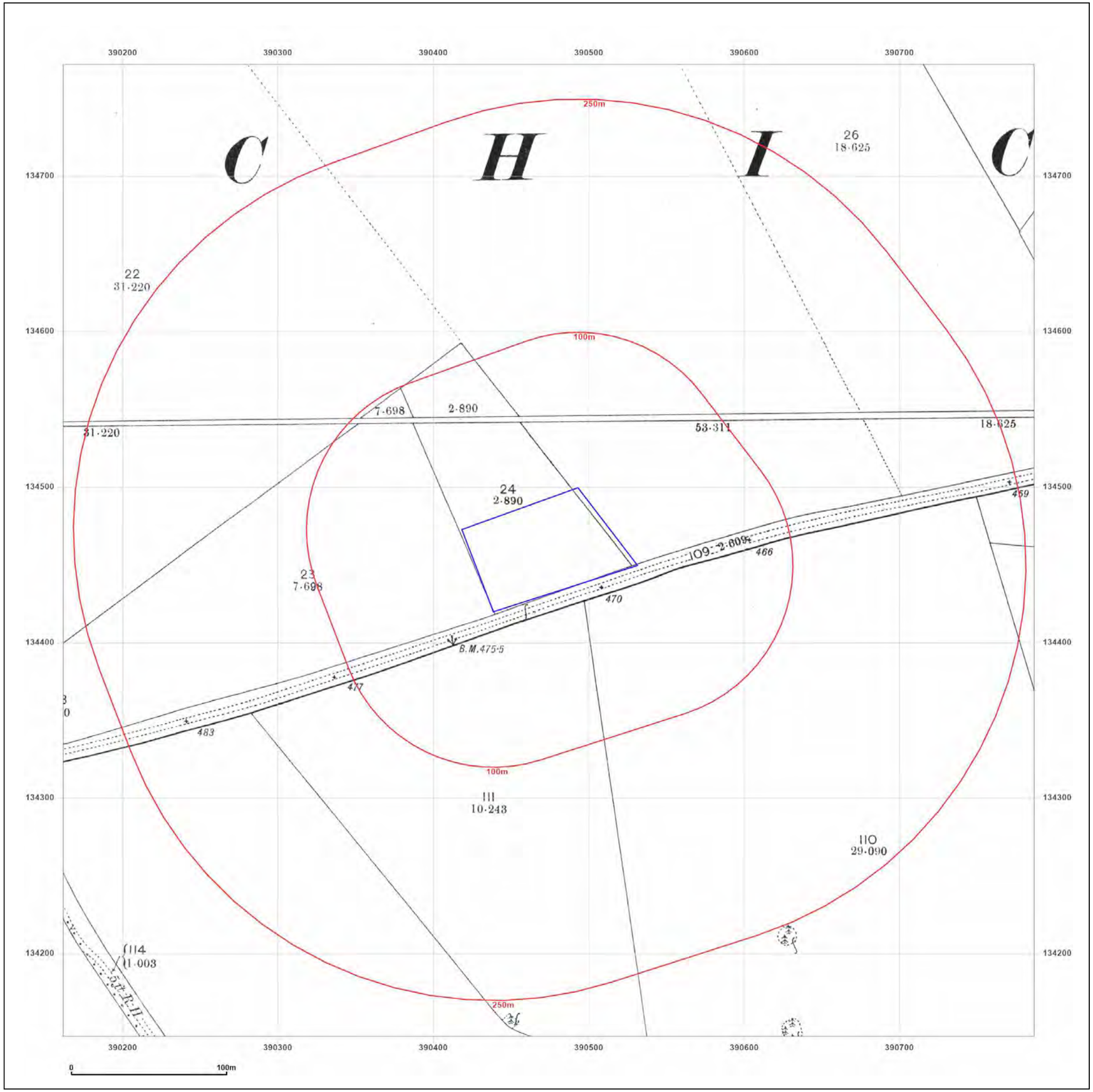
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Report Ref: GS-LMX-ZFN-MB6-9GQ
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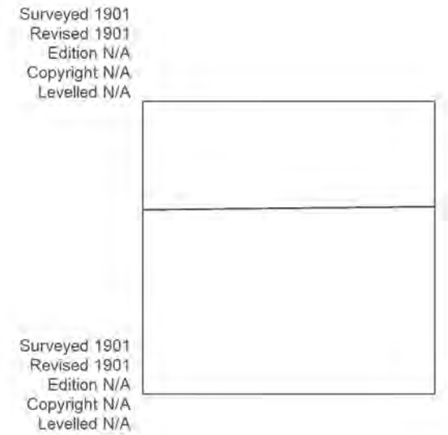
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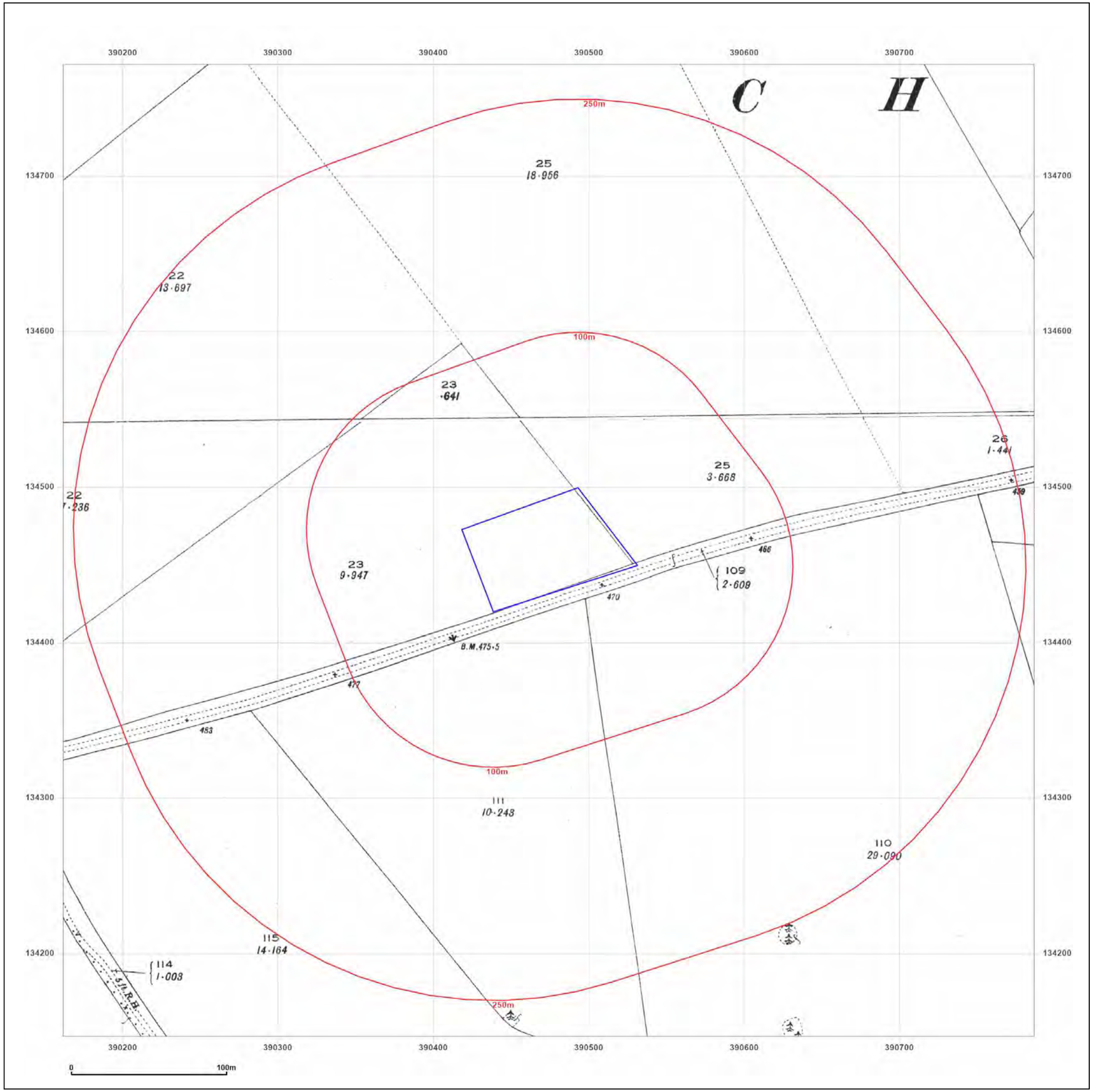
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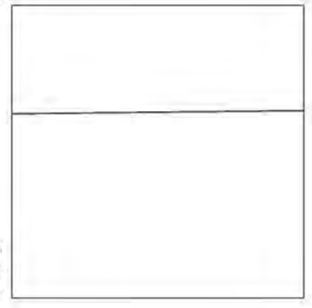
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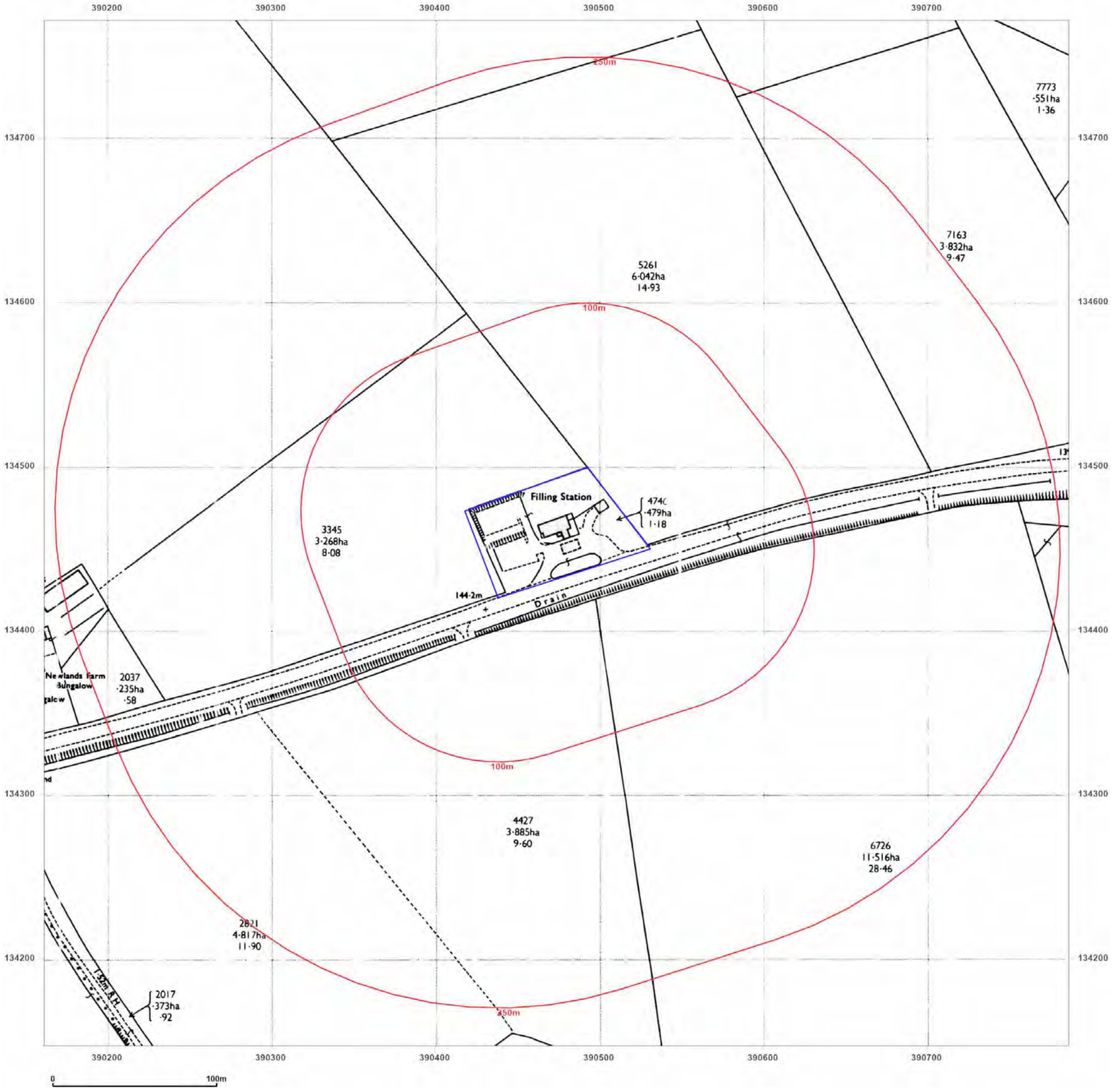
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Report Ref: GS-LMX-ZFN-MB6-9GQ
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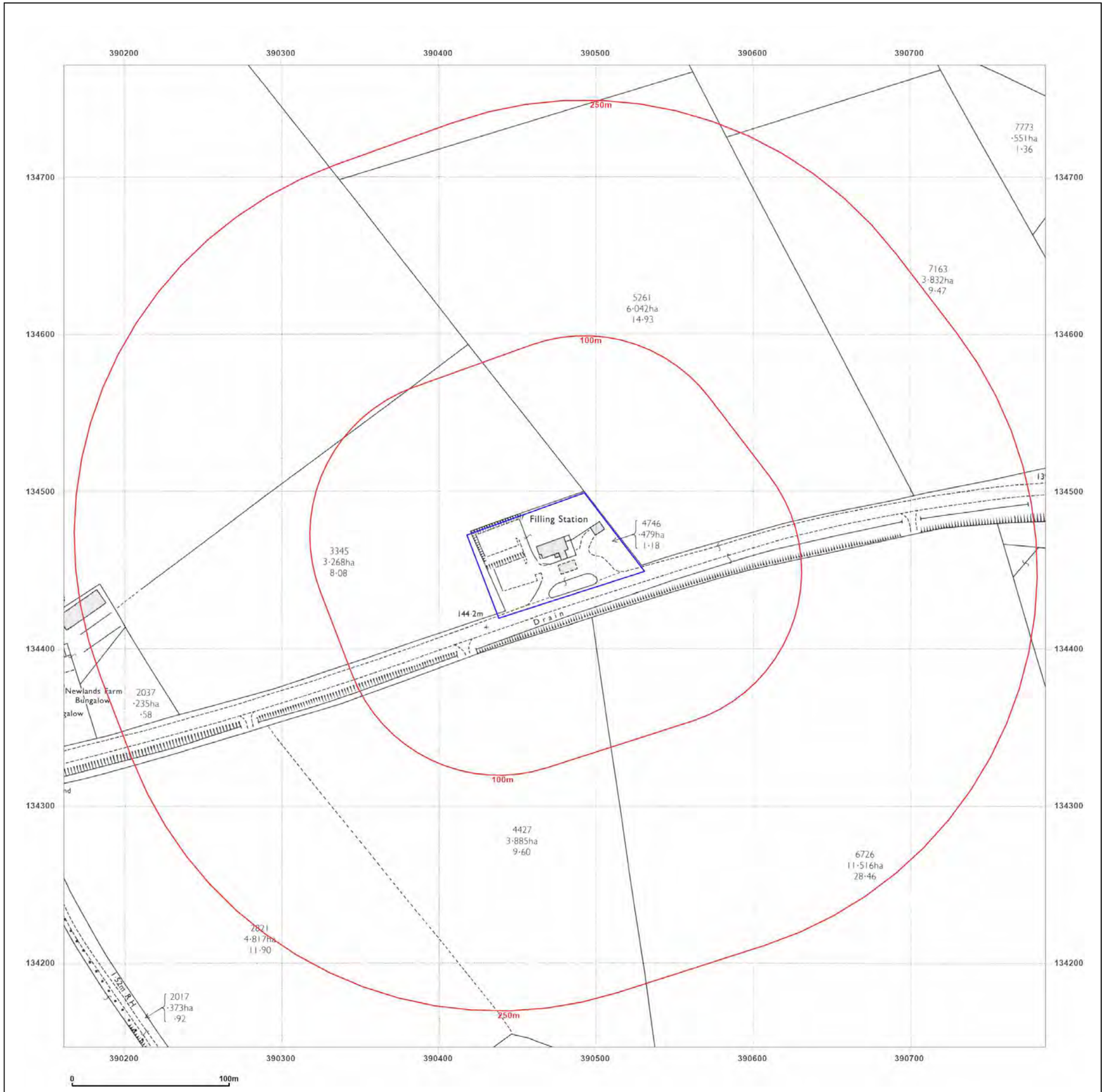
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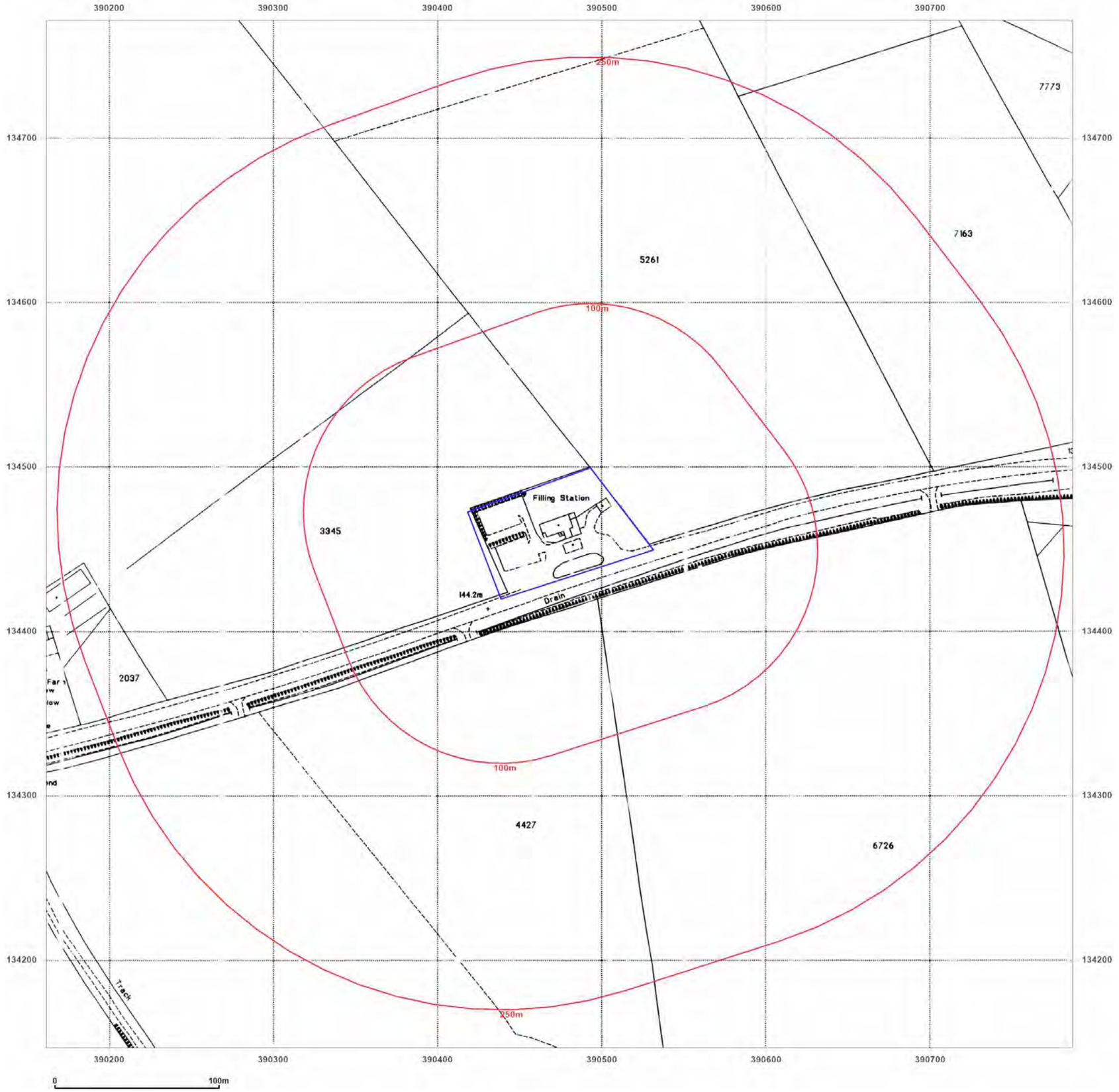
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Report Ref: GS-LMX-ZFN-MB6-9GQ
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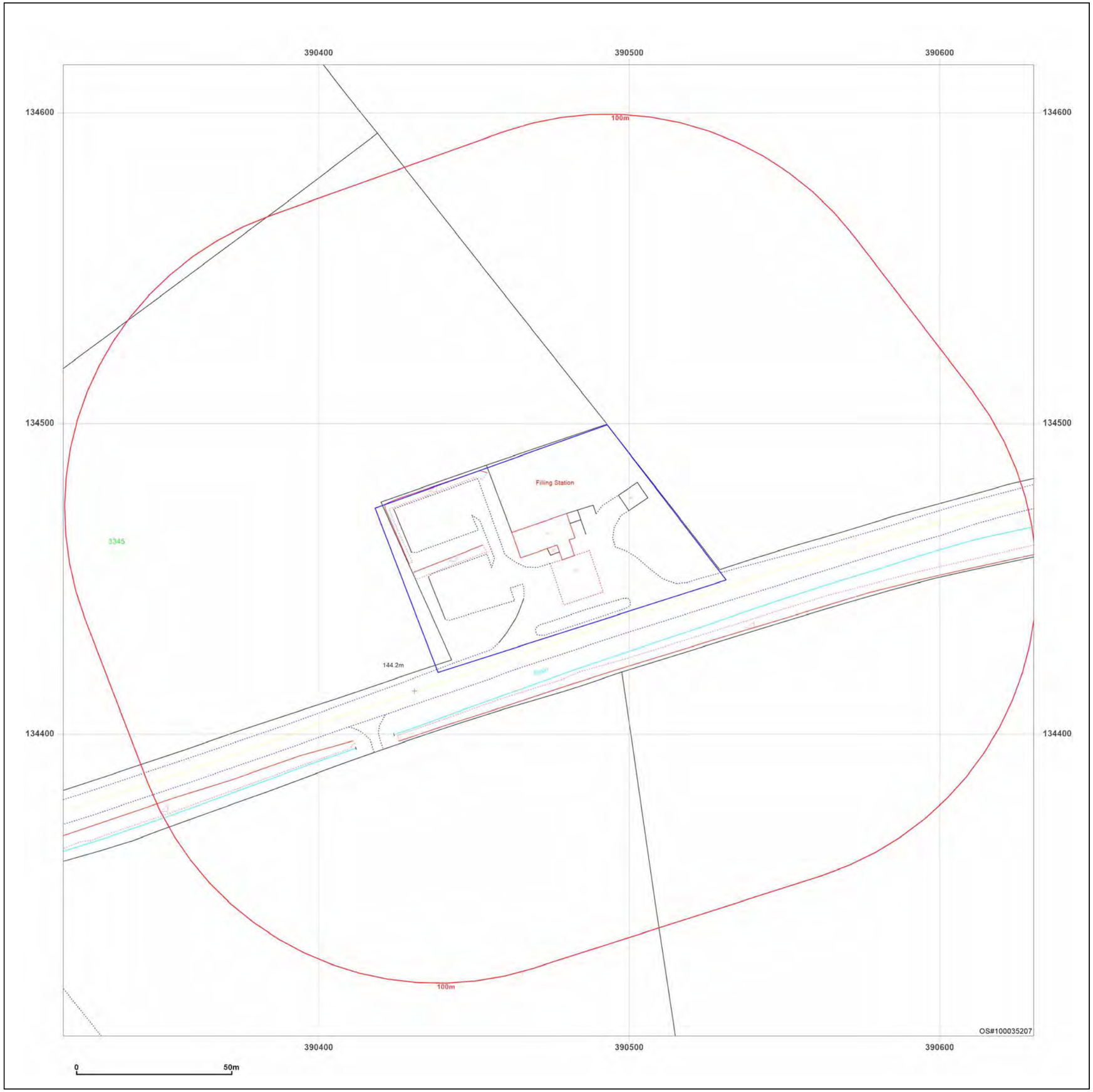
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Site Details:

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A303, CHICKLADE, WILTSHIRE,
SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: LandLine
Map date: **2003**
Scale: 1:1,250
Printed at: 1:1,250

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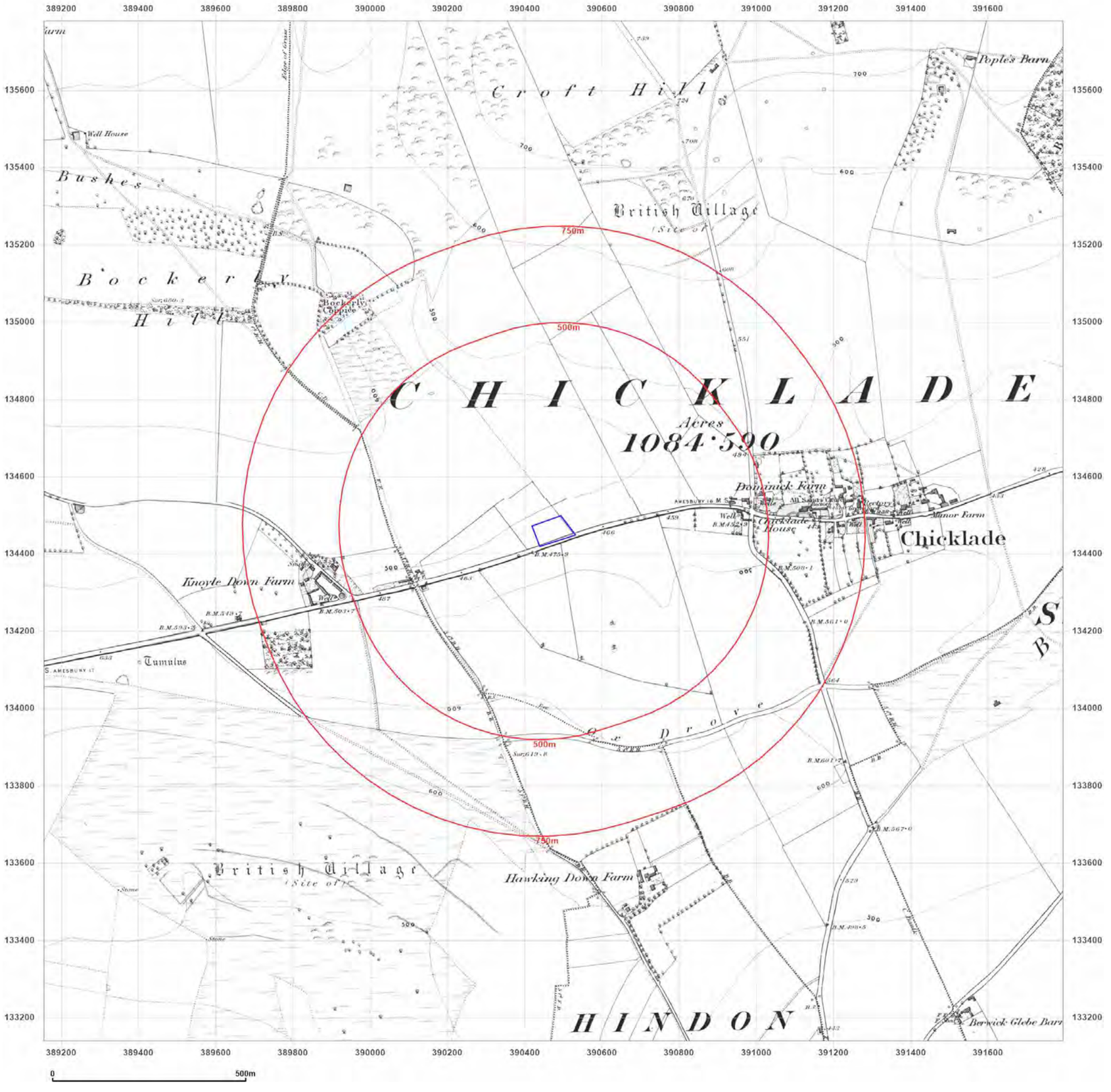
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Site Details:

CHICKLADE SERVICE STATION,
A303, CHICKLADE, WILTSHIRE,
SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: County Series

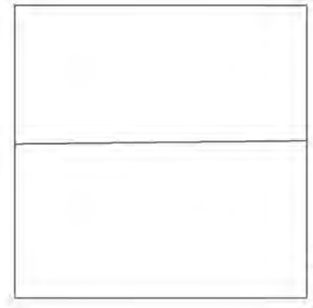
Map date: **1886**

Scale: 1:10,560

Printed at: 1:10,560

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Revised 1886
Edition N/A
Copyright N/A
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Surveyed 1886
Revised 1886
Edition N/A
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Levelled N/A

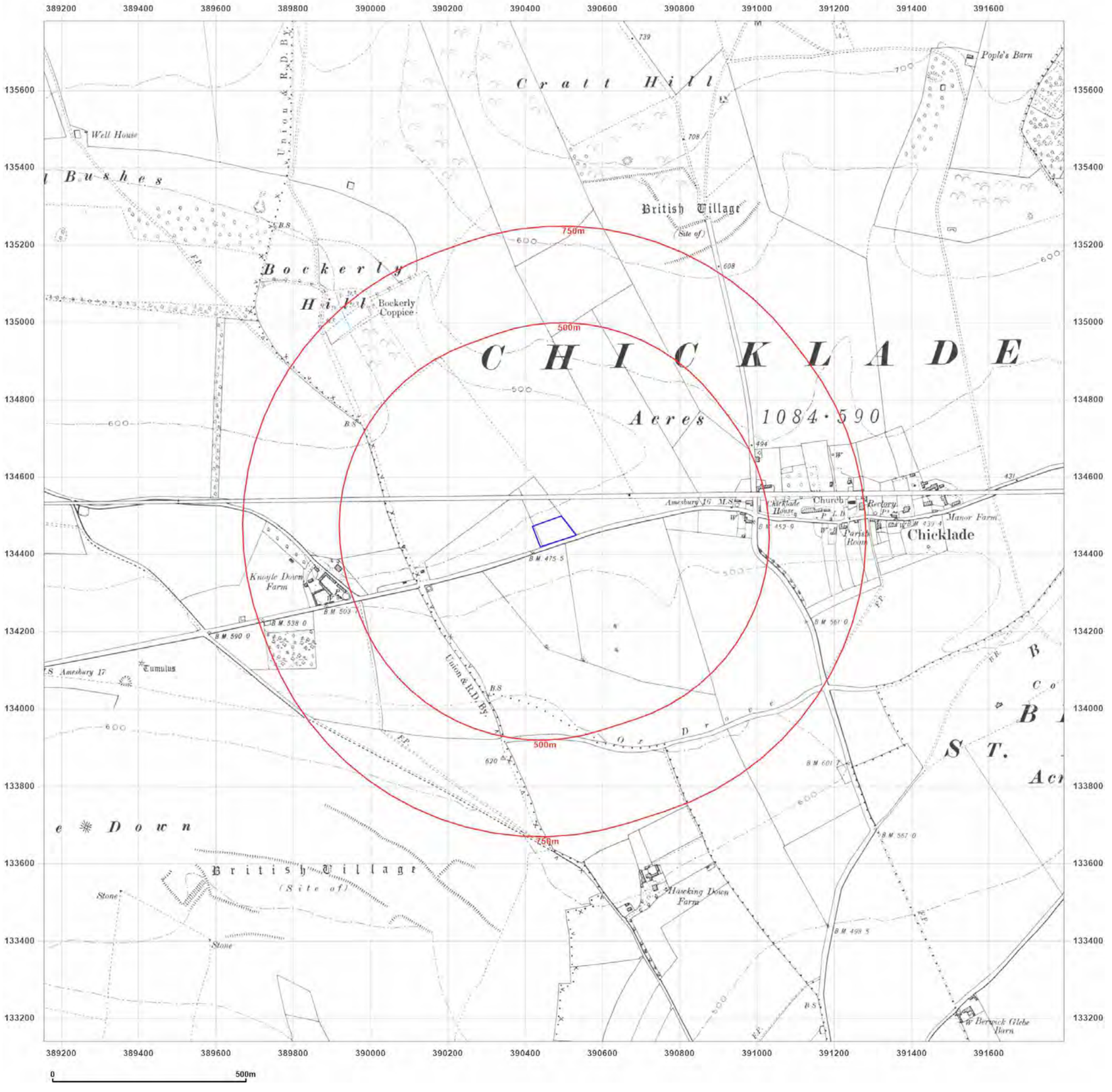
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SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: County Series

Map date: **1899-1900**

Scale: 1:10,560

Printed at: 1:10,560

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Revised 1900
Edition N/A
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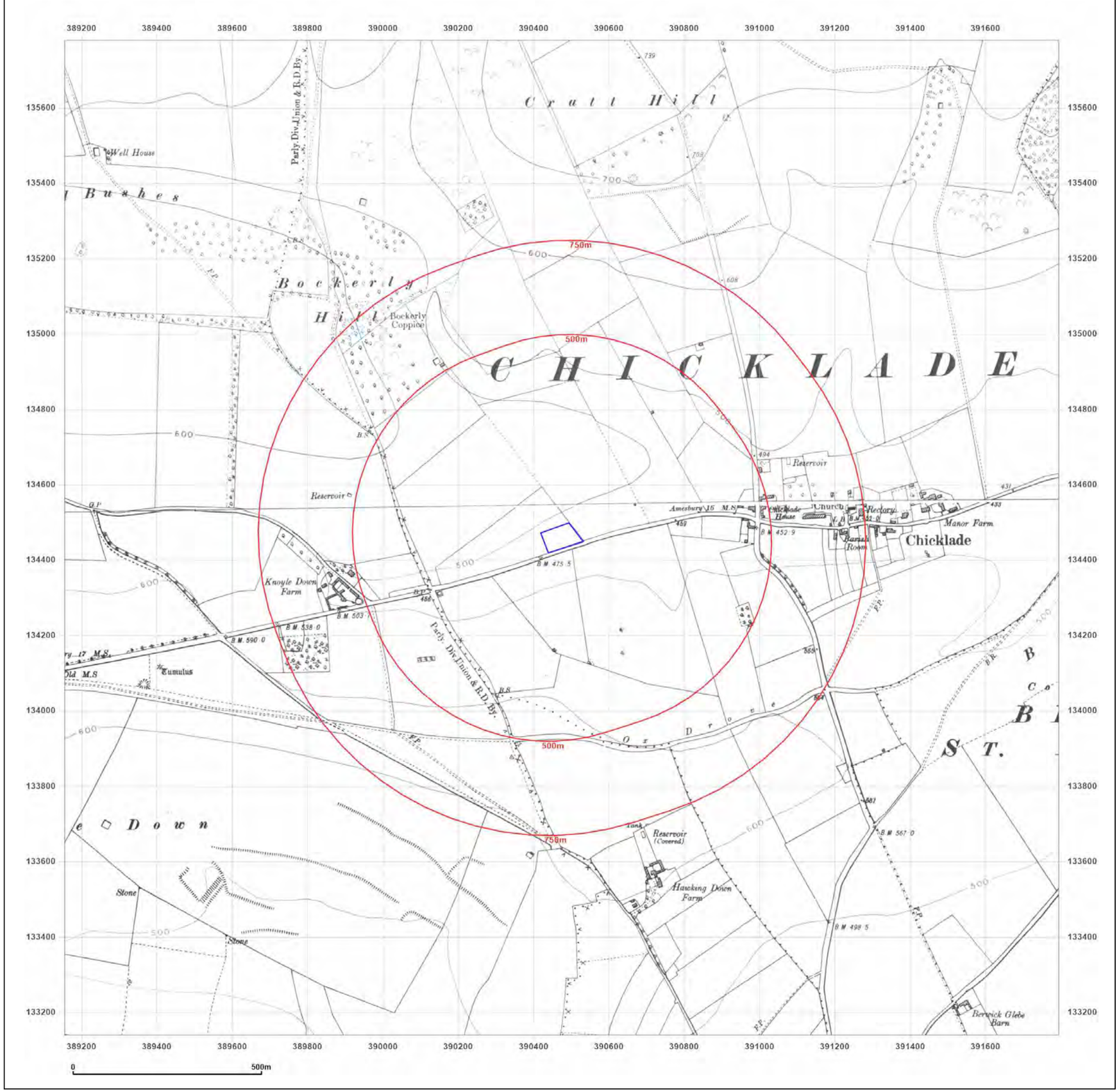
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SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: County Series

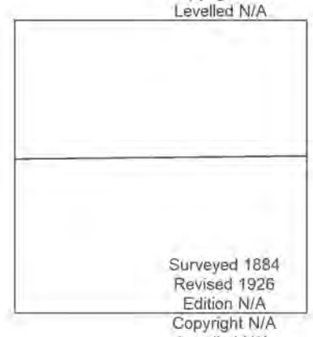
Map date: **1926**

Scale: 1:10,560

Printed at: 1:10,560

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Edition N/A
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Surveyed 1884
Revised 1926
Edition N/A
Copyright N/A
Levelled N/A

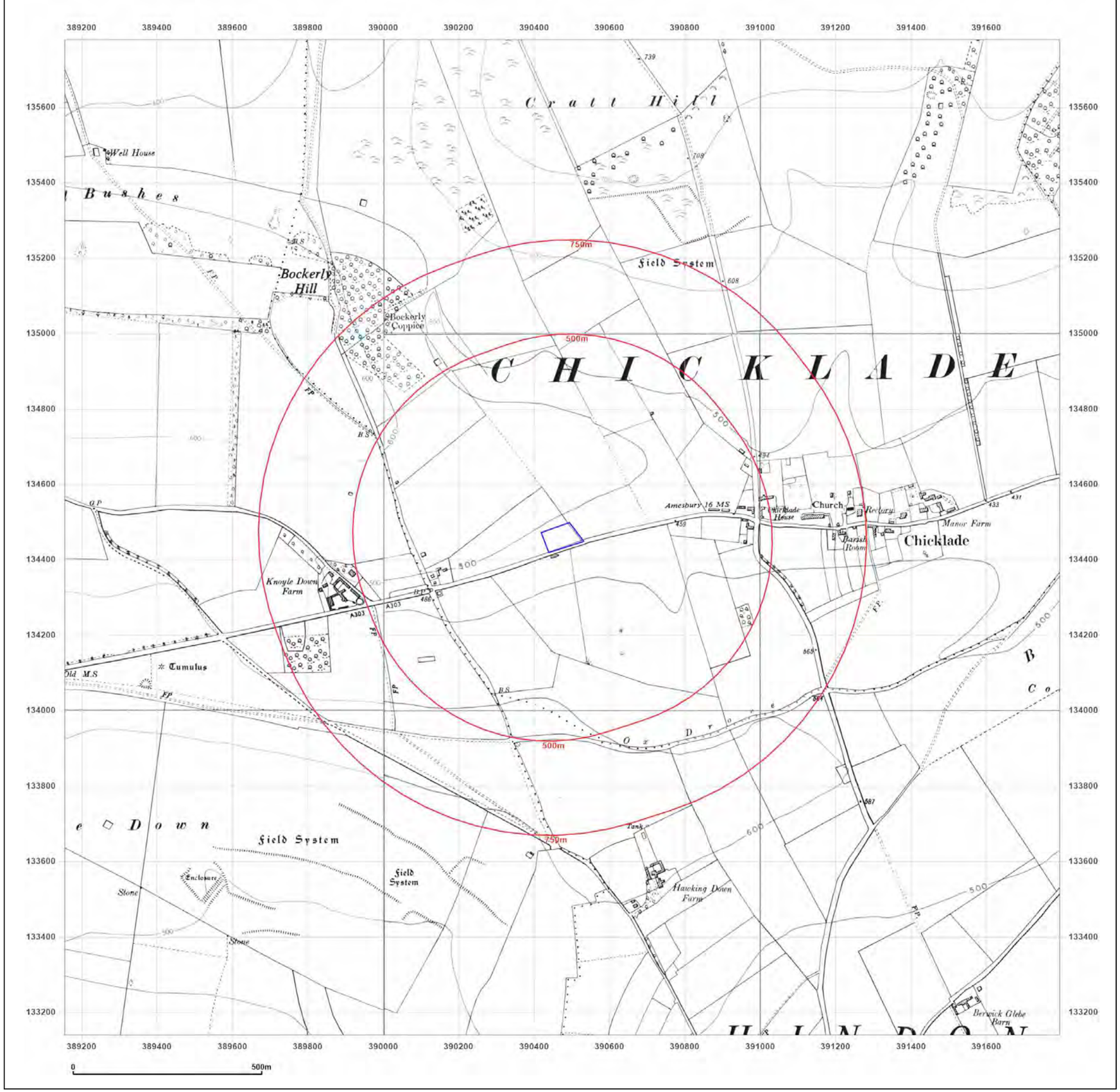
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Site Details:

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SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: Provisional
Map date: **1956**
Scale: 1:10,560
Printed at: 1:10,560

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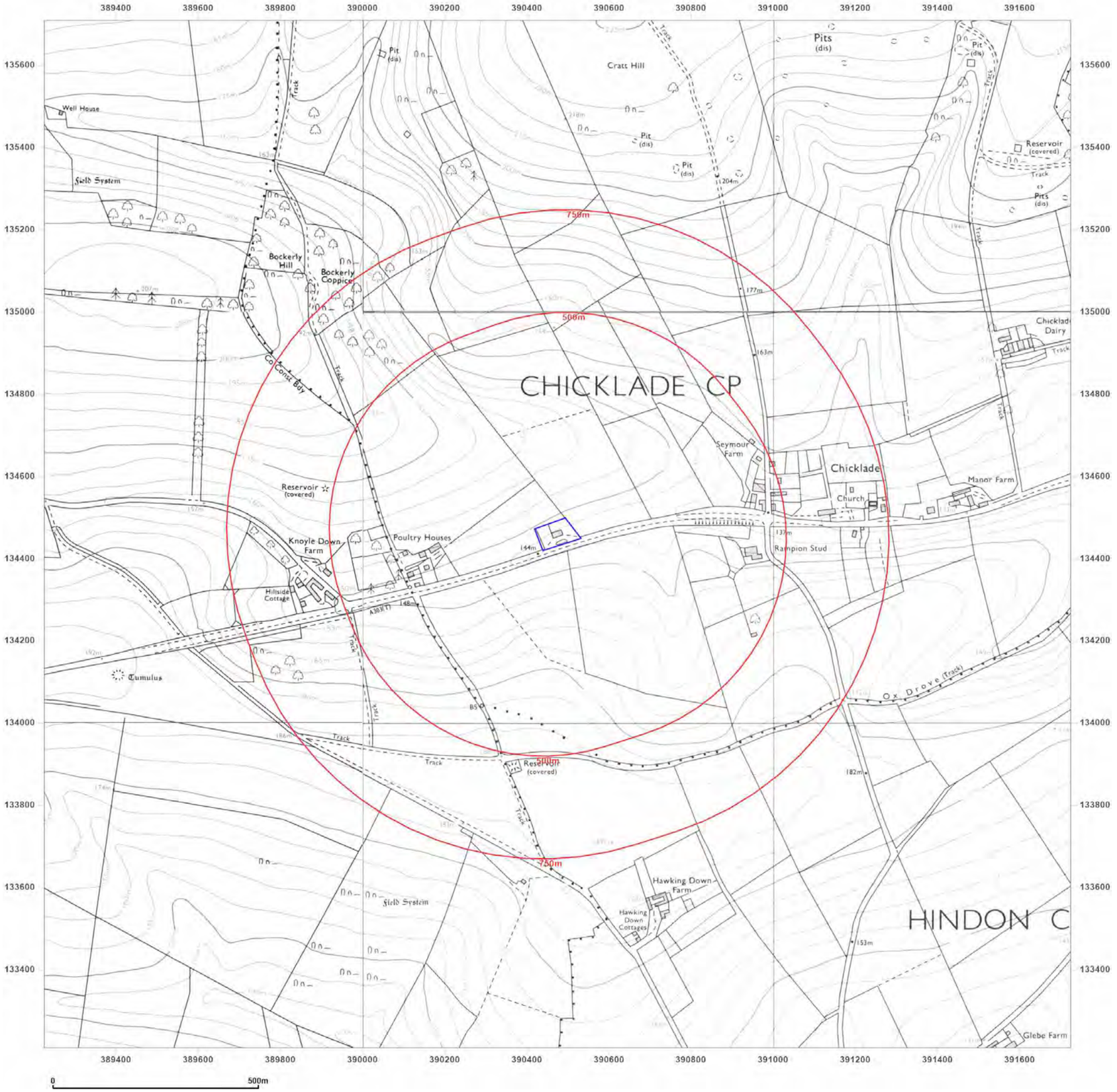
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Site Details:

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A303, CHICKLADE, WILTSHIRE,
SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: National Grid

Map date: **1982**

Scale: 1:10,000

Printed at: 1:10,000

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Revised 1982
Edition N/A
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Revised 1982
Edition N/A
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Edition N/A
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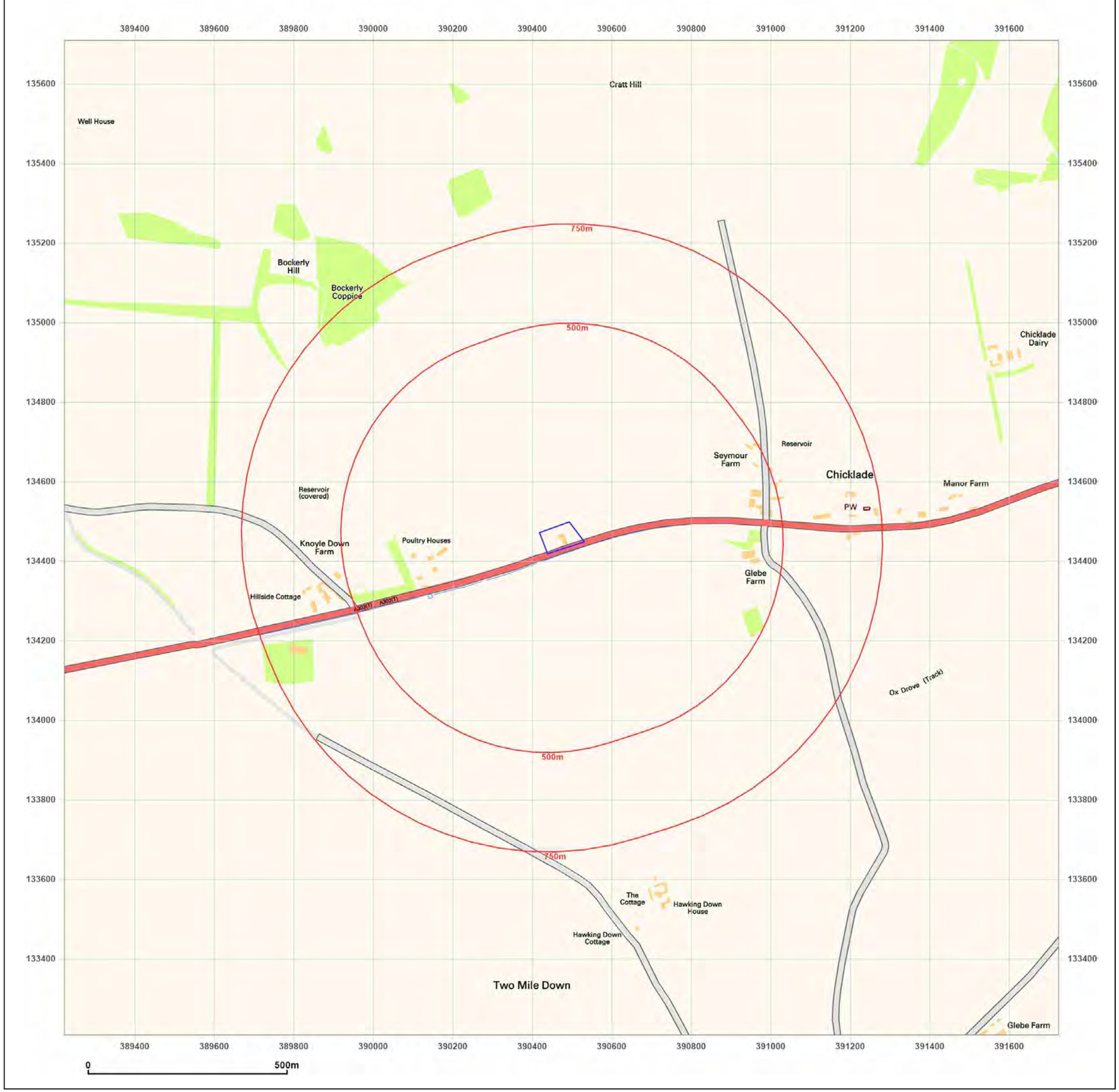
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A303, CHICKLADE, WILTSHIRE,
SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: National Grid

Map date: **2001**

Scale: 1:10,000

Printed at: 1:10,000

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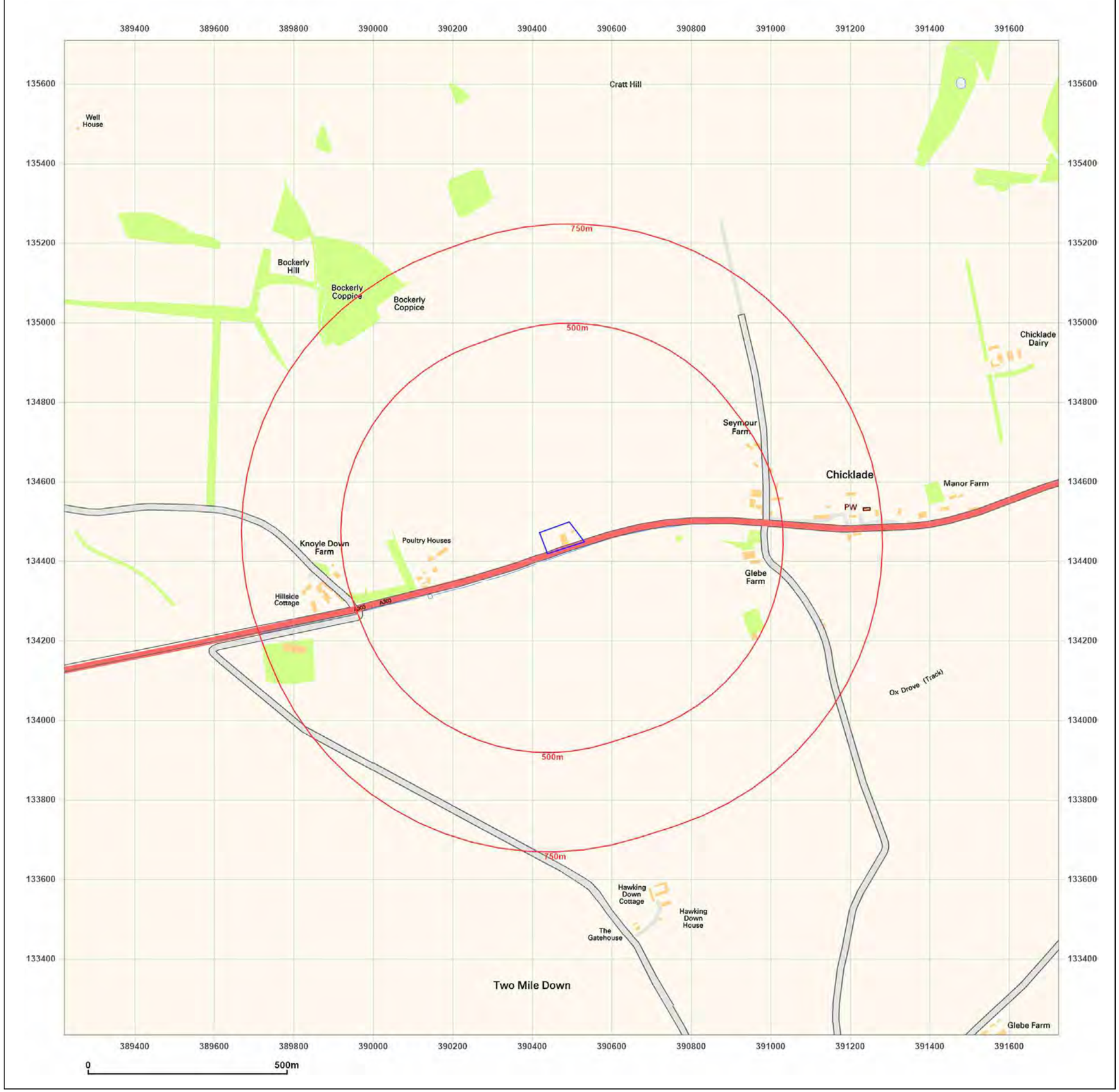
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Groundsure INSIGHTS

Site Details:

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A303, CHICKLADE, WILTSHIRE,
SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: National Grid

Map date: **2010**

Scale: 1:10,000

Printed at: 1:10,000

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Groundsure INSIGHTS

Site Details:

CHICKLADE SERVICE STATION,
A303, CHICKLADE, WILTSHIRE,
SP3 5ST

Client Ref: GWPR6751
Report Ref: GS-LMX-ZFN-MB6-9GQ
Grid Ref: 390474, 134459

Map Name: National Grid
Map date: **2025**
Scale: 1:10,000
Printed at: 1:10,000

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APPENDIX E: GroundSure EnviroInsight and GeoInsight Datasheets

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

Groundsure
LOCATION INTELLIGENCE

Enviro+Geo Insight

CHICKLADE SERVICE STATION, A303, CHICKLADE, WILTSHIRE, SP3 5ST

Order Details

Date: 26/09/2025

Your ref: GWPR6751

Our Ref: GS-E3U-3JW-TYS-ZJR

Site Details

Location: 390470 134459

Area: 0.52 ha

Authority: Wiltshire Council ↗



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Summary of findings p. 2 >

Aerial image p. 9 >

OS MasterMap site plan p.14 >

Insight User Guide ↗

Contact us with any questions at:
info@groundsure.com ↗
01273 257 755

Certified B Corporation

Summary of findings

Page	Section	Past land use >	On site	0-50m	50-250m	250-500m	500-2000m
15	1.1	Historical industrial land uses	0	0	0	0	-
16	1.2	Historical tanks	0	0	0	0	-
16	1.3	Historical energy features	0	0	0	0	-
16 >	1.4 >	Historical petrol stations >	1	0	0	0	-
17 >	1.5 >	Historical garages >	0	0	0	1	-
17	1.6	Historical military land	0	0	0	0	-
Page	Section	Past land use - un-grouped >	On site	0-50m	50-250m	250-500m	500-2000m
18	2.1	Historical industrial land uses	0	0	0	0	-
19	2.2	Historical tanks	0	0	0	0	-
19	2.3	Historical energy features	0	0	0	0	-
19 >	2.4 >	Historical petrol stations >	1	0	0	0	-
19 >	2.5 >	Historical garages >	0	0	0	1	-
Page	Section	Waste and landfill	On site	0-50m	50-250m	250-500m	500-2000m
21	3.1	Active or recent landfill	0	0	0	0	-
21	3.2	Historical landfill (BGS records)	0	0	0	0	-
21	3.3	Historical landfill (LA/mapping records)	0	0	0	0	-
21	3.4	Historical landfill (EA/NRW records)	0	0	0	0	-
21	3.5	Historical waste sites	0	0	0	0	-
22	3.6	Licensed waste sites	0	0	0	0	-
22	3.7	Waste exemptions	0	0	0	0	-
Page	Section	Current industrial land use >	On site	0-50m	50-250m	250-500m	500-2000m
23	4.1	Recent industrial land uses	0	0	0	-	-
23	4.2	National Geographic Database (NGD) - Current or recent tanks	0	0	0	-	-
24 >	4.3 >	Current or recent petrol stations >	0	1	0	0	-
24	4.4	Electricity cables	0	0	0	0	-
24	4.5	Gas pipelines	0	0	0	0	-

24	4.6	Sites determined as Contaminated Land	0	0	0	0	-
24	4.7	Control of Major Accident Hazards (COMAH)	0	0	0	0	-
25	4.8	Regulated explosive sites	0	0	0	0	-
25	4.9	Hazardous substance storage/usage	0	0	0	0	-
25	4.10	Historical licensed industrial activities (IPC)	0	0	0	0	-
25	4.11	Licensed industrial activities (Part A(1))	0	0	0	0	-
25 >	**4.12 >**	**Licensed pollutant release (Part A(2)/B) >**	1	0	0	0	-
26	4.13	Radioactive Substance Authorisations	0	0	0	0	-
26 >	**4.14 >**	**Licensed Discharges to controlled waters >**	5	0	0	0	-
27	4.15	Pollutant release to surface waters (Red List)	0	0	0	0	-
27	4.16	Pollutant release to public sewer	0	0	0	0	-
27	4.17	List 1 Dangerous Substances	0	0	0	0	-
28	4.18	List 2 Dangerous Substances	0	0	0	0	-
28	4.19	Pollution Incidents (EA/NRW)	0	0	0	0	-
28	4.20	Pollution inventory substances	0	0	0	0	-
28	4.21	Pollution inventory waste transfers	0	0	0	0	-
28	4.22	Pollution inventory radioactive waste	0	0	0	0	-

Page	Section	**Hydrogeology >**	On site	0-50m	50-250m	250-500m	500-2000m
29 >	**5.1 >**	**Superficial aquifer >**	Identified (within 500m)				
31 >	**5.2 >**	**Bedrock aquifer >**	Identified (within 500m)				
33 >	**5.3 >**	**Groundwater vulnerability >**	Identified (within 50m)				
34 >	**5.4 >**	**Groundwater vulnerability- soluble rock risk >**	Identified (within 0m)				
34	5.5	Groundwater vulnerability- local information	None (within 0m)				
36 >	**5.6 >**	**Groundwater abstractions >**	0	0	0	0	31
43	5.7	Surface water abstractions	0	0	0	0	0
43 >	**5.8 >**	**Potable abstractions >**	0	0	0	0	11
46 >	**5.9 >**	**Source Protection Zones >**	1	1	0	0	-
46	5.10	Source Protection Zones (confined aquifer)	0	0	0	0	-

Page	Section	**Hydrology >**	On site	0-50m	50-250m	250-500m	500-2000m

47 >	6.1 >	Water Network (OS MasterMap) >	0	1	5	-	-
48 >	6.2 >	Surface water features >	0	1	3	-	-
48 >	6.3 >	WFD Surface water body catchments >	1	-	-	-	-
49 >	6.4 >	WFD Surface water bodies >	0	0	0	-	-
49 >	6.5 >	WFD Groundwater bodies >	1	-	-	-	-
Page	Section	River and coastal flooding	On site	0-50m	50-250m	250-500m	500-2000m
50	7.1	Risk of flooding from rivers and the sea	None (within 50m)				
50	7.2	Historical Flood Events	0	0	0	-	-
50	7.3	Flood Defences	0	0	0	-	-
51	7.4	Areas Benefiting from Flood Defences	0	0	0	-	-
51	7.5	Flood Storage Areas	0	0	0	-	-
52	7.6	Flood Zone 2	None (within 50m)				
52	7.7	Flood Zone 3	None (within 50m)				
Page	Section	Surface water flooding >					
53 >	8.1 >	Surface water flooding >	1 in 30 year, Greater than 1.0m (within 50m)				
Page	Section	Groundwater flooding >					
55 >	9.1 >	Groundwater flooding >	High (within 50m)				
Page	Section	Environmental designations >	On site	0-50m	50-250m	250-500m	500-2000m
56	10.1	Sites of Special Scientific Interest (SSSI)	0	0	0	0	0
57	10.2	Conserved wetland sites (Ramsar sites)	0	0	0	0	0
57	10.3	Special Areas of Conservation (SAC)	0	0	0	0	0
57	10.4	Special Protection Areas (SPA)	0	0	0	0	0
57	10.5	National Nature Reserves (NNR)	0	0	0	0	0
58	10.6	Local Nature Reserves (LNR)	0	0	0	0	0
58 >	10.7 >	Designated Ancient Woodland >	0	0	0	0	9
58	10.8	Biosphere Reserves	0	0	0	0	0
59	10.9	Forest Parks	0	0	0	0	0
59	10.10	Marine Conservation Zones	0	0	0	0	0
59	10.11	Green Belt	0	0	0	0	0

59	10.12	Proposed Ramsar sites	0	0	0	0	0
59	10.13	Possible Special Areas of Conservation (pSAC)	0	0	0	0	0
60	10.14	Potential Special Protection Areas (pSPA)	0	0	0	0	0
60	10.15	Nitrate Sensitive Areas	0	0	0	0	0
60 >	**10.16 >**	**Nitrate Vulnerable Zones >**	1	0	0	0	1
61 >	**10.17 >**	**SSSI Impact Risk Zones >**	1	-	-	-	-
62	10.18	SSSI Units	0	0	0	0	0
Page	Section	**Visual and cultural designations >**	On site	0-50m	50-250m	250-500m	500-2000m
63	11.1	World Heritage Sites	0	0	0	-	-
64 >	**11.2 >**	**Area of Outstanding Natural Beauty >**	1	0	1	-	-
64	11.3	National Parks	0	0	0	-	-
64	11.4	Listed Buildings	0	0	0	-	-
65	11.5	Conservation Areas	0	0	0	-	-
65	11.6	Scheduled Ancient Monuments	0	0	0	-	-
65	11.7	Registered Parks and Gardens	0	0	0	-	-
Page	Section	**Agricultural designations >**	On site	0-50m	50-250m	250-500m	500-2000m
66 >	**12.1 >**	**Agricultural Land Classification >**	Grade 3 (within 250m)				
67	12.2	Open Access Land	0	0	0	-	-
67	12.3	Tree Felling Licences	0	0	0	-	-
67 >	**12.4 >**	**Environmental Stewardship Schemes >**	1	1	0	-	-
68 >	**12.5 >**	**Countryside Stewardship Schemes >**	1	2	0	-	-
Page	Section	Habitat designations	On site	0-50m	50-250m	250-500m	500-2000m
69	13.1	Priority Habitat Inventory	0	0	0	-	-
69	13.2	Habitat Networks	0	0	0	-	-
69	13.3	Open Mosaic Habitat	0	0	0	-	-
69	13.4	Limestone Pavement Orders	0	0	0	-	-
Page	Section	**Geology 1:10,000 scale >**	On site	0-50m	50-250m	250-500m	500-2000m
70 >	**14.1 >**	**10k Availability >**	Identified (within 500m)				
71	14.2	Artificial and made ground (10k)	0	0	0	0	-

72 >	14.3 >	Superficial geology (10k) >	1	0	0	1	-
73	14.4	Landslip (10k)	0	0	0	0	-
74 >	14.5 >	Bedrock geology (10k) >	1	0	0	1	-
75	14.6	Bedrock faults and other linear features (10k)	0	0	0	0	-
Page	Section	Geology 1:50,000 scale >	On site	0-50m	50-250m	250-500m	500-2000m
76 >	15.1 >	50k Availability >	Identified (within 500m)				
77	15.2	Artificial and made ground (50k)	0	0	0	0	-
77	15.3	Artificial ground permeability (50k)	0	0	-	-	-
78 >	15.4 >	Superficial geology (50k) >	1	0	0	0	-
79 >	15.5 >	Superficial permeability (50k) >	Identified (within 50m)				
79	15.6	Landslip (50k)	0	0	0	0	-
79	15.7	Landslip permeability (50k)	None (within 50m)				
80 >	15.8 >	Bedrock geology (50k) >	1	0	0	0	-
81 >	15.9 >	Bedrock permeability (50k) >	Identified (within 50m)				
81	15.10	Bedrock faults and other linear features (50k)	0	0	0	0	-
Page	Section	Boreholes	On site	0-50m	50-250m	250-500m	500-2000m
82	16.1	BGS Boreholes	0	0	0	-	-
Page	Section	Natural ground subsidence >					
83 >	17.1 >	Shrink swell clays >	Very low (within 50m)				
85 >	17.2 >	Running sands >	Very low (within 50m)				
87 >	17.3 >	Compressible deposits >	Negligible (within 50m)				
88 >	17.4 >	Collapsible deposits >	Very low (within 50m)				
89 >	17.5 >	Landslides >	Low (within 50m)				
91 >	17.6 >	Ground dissolution of soluble rocks >	Low (within 50m)				
Page	Section	Mining and ground workings >	On site	0-50m	50-250m	250-500m	500-2000m
93	18.1	BritPits	0	0	0	0	-
94	18.2	Surface ground workings	0	0	0	-	-
94	18.3	Underground workings	0	0	0	0	0
94	18.4	Underground mining extents	0	0	0	0	-

94	18.5	Historical Mineral Planning Areas	0	0	0	0	-
94 >	**18.6 >**	**Non-coal mining >**	1	0	0	1	2
95	18.7	JPB mining areas	None (within 0m)				
95	18.8	The Coal Authority non-coal mining	0	0	0	0	-
96	18.9	Researched mining	0	0	0	0	-
96	18.10	Mining record office plans	0	0	0	0	-
96	18.11	BGS mine plans	0	0	0	0	-
96	18.12	Coal mining	None (within 0m)				
96	18.13	Brine areas	None (within 0m)				
97	18.14	Gypsum areas	None (within 0m)				
97	18.15	Tin mining	None (within 0m)				
97	18.16	Clay mining	None (within 0m)				
Page	Section	Ground cavities and sinkholes	On site	0-50m	50-250m	250-500m	500-2000m
98	19.1	Natural cavities	0	0	0	0	-
98	19.2	Mining cavities	0	0	0	0	0
98	19.3	Reported recent incidents	0	0	0	0	-
98	19.4	Historical incidents	0	0	0	0	-
Page	Section	**Radon >**					
100 >	**20.1 >**	**Radon >**	Less than 1% (within 0m)				
Page	Section	**Soil chemistry >**	On site	0-50m	50-250m	250-500m	500-2000m
102 >	**21.1 >**	**BGS Estimated Background Soil Chemistry >**	4	4	-	-	-
102	21.2	BGS Estimated Urban Soil Chemistry	0	0	-	-	-
103	21.3	BGS Measured Urban Soil Chemistry	0	0	-	-	-
Page	Section	Railway infrastructure and projects	On site	0-50m	50-250m	250-500m	500-2000m
104	22.1	Underground railways (London)	0	0	0	-	-
104	22.2	Underground railways (Non-London)	0	0	0	-	-
104	22.3	Railway tunnels	0	0	0	-	-
104	22.4	Historical railway and tunnel features	0	0	0	-	-
104	22.5	Royal Mail tunnels	0	0	0	-	-

105	22.6	Historical railways	0	0	0	-	-
105	22.7	Railways	0	0	0	-	-
105	22.8	Crossrail 2	0	0	0	0	-
105	22.9	HS2	0	0	0	0	-

Recent aerial photograph



Capture Date: 27/05/2023

Site Area: 0.52ha

Recent site history - 2020 aerial photograph



Capture Date: 23/04/2020

Site Area: 0.52ha

Recent site history - 2013 aerial photograph



Capture Date: 19/07/2013

Site Area: 0.52ha

Recent site history - 2005 aerial photograph



Capture Date: 31/08/2005

Site Area: 0.52ha

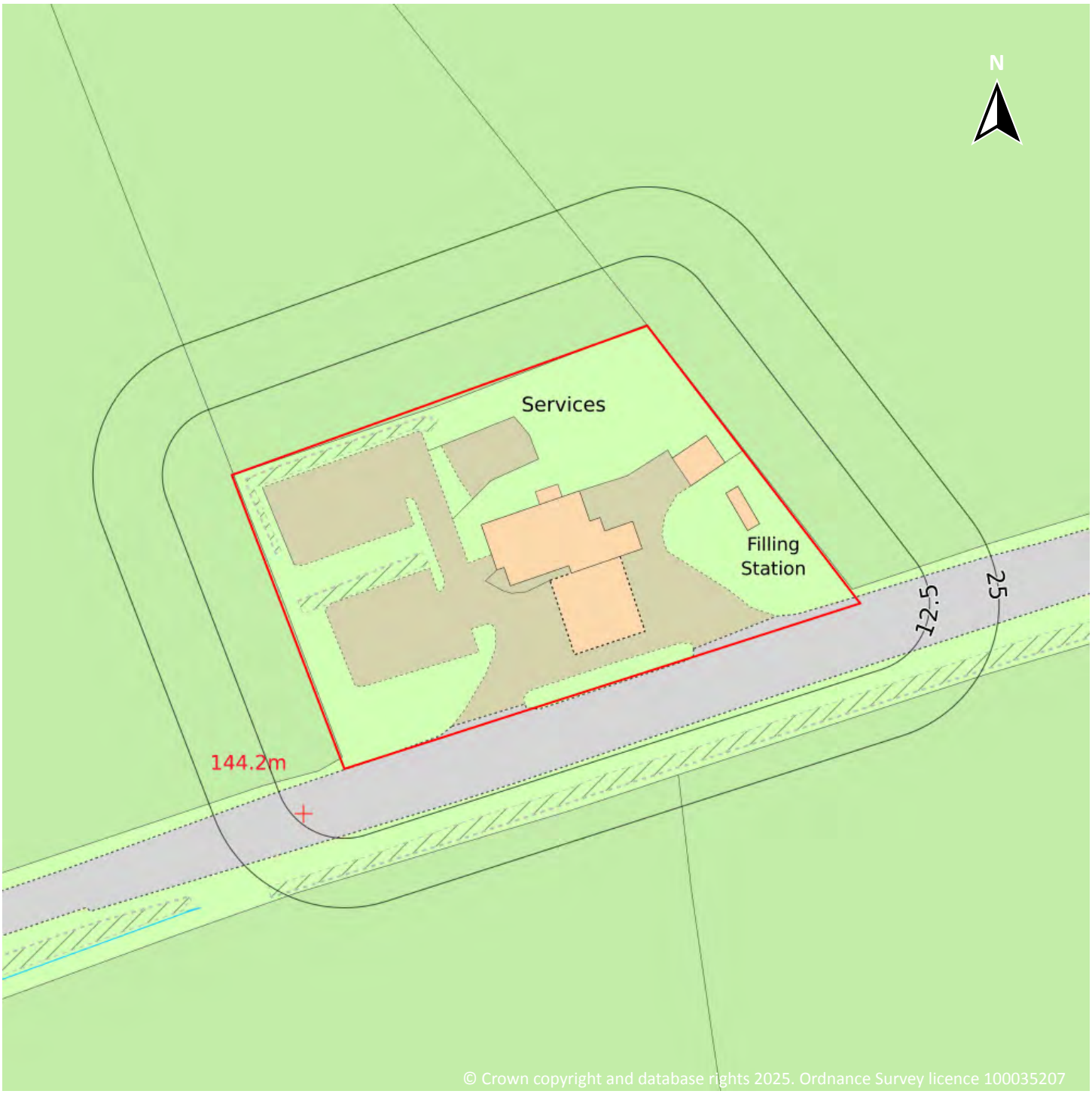
Recent site history - 1999 aerial photograph



Capture Date: 04/09/1999

Site Area: 0.52ha

OS MasterMap site plan



Site Area: 0.52ha

1 Past land use



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Site Outline

Search buffers in metres (m)

Historical petrol stations

Historical garages

1.1 Historical industrial land uses

Records within 500m	0

Potentially contaminative land use features digitised from historical Ordnance Survey mapping at 1:10,000 and 1:10,560 scale, intelligently grouped into contiguous features. To prevent misrepresentation of the size of historical features at any given time, features are only grouped if they have similar geometries within immediately preceding or succeeding map editions. See section 2 for a breakdown of grouping if required. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

This data is sourced from Ordnance Survey / Groundsure.

1.2 Historical tanks

Records within 500m	0

Tank features digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale, intelligently grouped into contiguous features. To prevent misrepresentation of the size of historical features at any given time, features are only grouped if they have similar geometries within immediately preceding or succeeding map editions. See section 2 for a breakdown of grouping if required. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

This data is sourced from Ordnance Survey / Groundsure.

1.3 Historical energy features

Records within 500m	0

Energy features digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale, intelligently grouped into contiguous features. To prevent misrepresentation of the size of historical features at any given time, features are only grouped if they have similar geometries within immediately preceding or succeeding map editions. See section 2 for a breakdown of grouping if required. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

This data is sourced from Ordnance Survey / Groundsure.

1.4 Historical petrol stations

Records within 500m	1

Petrol stations digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale, intelligently grouped into contiguous features. To prevent misrepresentation of the size of historical features at any given time, features are only grouped if they have similar geometries within immediately preceding or succeeding map editions. See section 2 for a breakdown of grouping if required. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

Features are displayed on the Past land use map on **page 15 >**

ID	Location	Land use	Dates present	Group ID
1	**On site**	**Filling Station**	**1981**	**2052**

This data is sourced from Ordnance Survey / Groundsure.

1.5 Historical garages

Records within 500m	1

Garages digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale, intelligently grouped into contiguous features. To prevent misrepresentation of the size of historical features at any given time, features are only grouped if they have similar geometries within immediately preceding or succeeding map editions. See section 2 for a breakdown of grouping if required. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

Features are displayed on the Past land use map on **page 15 >**

ID	Location	Land use	Dates present	Group ID
2	301m W	Garage	1981	35599

This data is sourced from Ordnance Survey / Groundsure.

1.6 Historical military land

Records within 500m	0

Areas of military land digitised from multiple sources including the National Archives, local records, MOD records and verified other sources, intelligently grouped into contiguous features.

This data is sourced from Ordnance Survey / Groundsure / other sources.

2 Past land use - un-grouped



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Site Outline

Search buffers in metres (m)

Historical petrol stations

Historical garages

2.1 Historical industrial land uses

Records within 500m	0

Potentially contaminative land use features digitised from historical Ordnance Survey mapping at 1:10,000 and 10,560 scale. Any records shown are available intelligently grouped in section 1. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

This data is sourced from Ordnance Survey / Groundsure.

2.2 Historical tanks

Records within 500m	0

Tank features digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale. Any records shown are available intelligently grouped in section 1. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

This data is sourced from Ordnance Survey / Groundsure.

2.3 Historical energy features

Records within 500m	0

Energy features digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale. Any records shown are available intelligently grouped in section 1. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

This data is sourced from Ordnance Survey / Groundsure.

2.4 Historical petrol stations

Records within 500m	1

Petrol stations digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale. Any records shown are available intelligently grouped in section 1. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

Features are displayed on the Past land use - un-grouped map on **page 18 >**

ID	Location	Land Use	Date	Group ID
1	**On site**	**Filling Station**	**1981**	**2052**

This data is sourced from Ordnance Survey / Groundsure.

2.5 Historical garages

Records within 500m	1

Garages digitised from historical Ordnance Survey mapping at high-detail 1:1,250 and 1:2,500 scale. Any records shown are available intelligently grouped in section 1. Grouped and the original un-grouped features can be cross-referenced across sections 1 and 2 using the 'Group ID'.

Features are displayed on the Past land use - un-grouped map on **page 18 >**

ID	Location	Land Use	Date	Group ID
2	301m W	Garage	1981	35599

This data is sourced from Ordnance Survey / Groundsure.

3 Waste and landfill

3.1 Active or recent landfill

Records within 500m	0

Active or recently closed landfill sites under Environment Agency/Natural Resources Wales regulation.

This data is sourced from the Environment Agency and Natural Resources Wales.

3.2 Historical landfill (BGS records)

Records within 500m	0

Landfill sites identified on a survey carried out on behalf of the DoE in 1973. These sites may have been closed or operational at this time.

This data is sourced from the British Geological Survey.

3.3 Historical landfill (LA/mapping records)

Records within 500m	0

Landfill sites identified from Local Authority records and high detail historical mapping.

This data is sourced from the Ordnance Survey/Groundsure and Local Authority records.

3.4 Historical landfill (EA/NRW records)

Records within 500m	0

Known historical (closed) landfill sites (e.g. sites where there is no PPC permit or waste management licence currently in force). This includes sites that existed before the waste licensing regime and sites that have been licensed in the past but where a licence has been revoked, ceased to exist or surrendered and a certificate of completion has been issued.

This data is sourced from the Environment Agency and Natural Resources Wales.

3.5 Historical waste sites

Records within 500m	0

Waste site records derived from Local Authority planning records and high detail historical mapping.

This data is sourced from Ordnance Survey/Groundsure and Local Authority records.

3.6 Licensed waste sites

Records within 500m	0

Active or recently closed waste sites under Environment Agency/Natural Resources Wales regulation.

This data is sourced from the Environment Agency and Natural Resources Wales.

3.7 Waste exemptions

Records within 500m	0

Activities involving the storage, treatment, use or disposal of waste that are exempt from needing a permit. Exemptions have specific limits and conditions that must be adhered to.

This data is sourced from the Environment Agency and Natural Resources Wales.

4 Current industrial land use



Site Outline

Search buffers in metres (m)

Current or recent petrol stations

Licensed pollutant release (Part A(2)/B)

Licensed Discharges to controlled waters

4.1 Recent industrial land uses

Records within 250m	0

Current potentially contaminative industrial sites.

This data is sourced from Ordnance Survey.

4.2 National Geographic Database (NGD) - Current or recent tanks

Records within 250m	0

Current or recent tanks identified from the Ordnance Survey NGD.

This data is sourced from Ordnance Survey.

4.3 Current or recent petrol stations

Records within 500m	1

Open, closed, under development and obsolete petrol stations.

Features are displayed on the Current industrial land use map on **page 23 >**

ID	Location	Company	Address	LPG	Status
1	30m E	MURCO	A303, Chicklade, Warminster, Wiltshire, SP3 5ST	No	Closed

This data is sourced from Experian.

4.4 Electricity cables

Records within 500m	0

High voltage underground electricity transmission cables.

This data is sourced from National Grid.

4.5 Gas pipelines

Records within 500m	0

High pressure underground gas transmission pipelines.

This data is sourced from National Grid.

4.6 Sites determined as Contaminated Land

Records within 500m	0

Contaminated Land Register of sites designated under Part 2a of the Environmental Protection Act 1990.

This data is sourced from Local Authority records.

4.7 Control of Major Accident Hazards (COMAH)

Records within 500m	0

Control of Major Accident Hazards (COMAH) sites. This data includes upper and lower tier sites, and includes a historical archive of COMAH sites and Notification of Installations Handling Hazardous Substances (NIHHS) records.

This data is sourced from the Health and Safety Executive.

4.8 Regulated explosive sites

Records within 500m	0

Sites registered and licensed by the Health and Safety Executive under the Manufacture and Storage of Explosives Regulations 2005 (MSER). The last update to this data was in April 2011.

This data is sourced from the Health and Safety Executive.

4.9 Hazardous substance storage/usage

Records within 500m	0

Consents granted for a site to hold certain quantities of hazardous substances at or above defined limits in accordance with the Planning (Hazardous Substances) Regulations 2015.

This data is sourced from Local Authority records.

4.10 Historical licensed industrial activities (IPC)

Records within 500m	0

Integrated Pollution Control (IPC) records of substance releases to air, land and water. This data represents a historical archive as the IPC regime has been superseded.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.11 Licensed industrial activities (Part A(1))

Records within 500m	0

Records of Part A(1) installations regulated under the Environmental Permitting (England and Wales) Regulations 2016 for the release of substances to the environment.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.12 Licensed pollutant release (Part A(2)/B)

Records within 500m	1

Records of Part A(2) and Part B installations regulated under the Environmental Permitting (England and Wales) Regulations 2016 for the release of substances to the environment.

Features are displayed on the Current industrial land use map on **page 23 >**

ID	Location	Address	Details	
A	**On site**	**Chicklade Filling Station, Chicklade, Salisbury, SP3 5ST**	**Process: Unloading of Petrol into Storage at Service Stations Status: Current Permit Permit Type: Part B**	**Enforcement: No Enforcements Notified Date of enforcement: No Enforcements Notified Comment: No Enforcements Notified**

This data is sourced from Local Authority records.

4.13 Radioactive Substance Authorisations

Records within 500m	**0**

Records of the storage, use, accumulation and disposal of radioactive substances regulated under the Radioactive Substances Act 1993.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.14 Licensed Discharges to controlled waters

Records within 500m	**5**

Discharges of treated or untreated effluent to controlled waters under the Water Resources Act 1991.

Features are displayed on the Current industrial land use map on **page 23 >**

ID	Location	Address	Details	
A	**On site**	**CHICKLADE SERVICE STATION, CHICKLADE, WILTSHIRE, SP3 5ST**	**Effluent Type: TRADE DISCHARGES - SITE DRAINAGE (CONTAM SURFACE WATER, NOT WASTE SIT Permit Number: 401563 Permit Version: 2 Receiving Water: GROUNDWATER VIA SOAKAWAY**	**Status: REVOKED - UNSPECIFIED Issue date: 17/12/2012 Effective Date: 17/12/2012 Revocation Date: 06/09/2019**
A	**On site**	**EAT AND GO AT CHICKLADE SERVICE STN, A303, HINDON, SALISBURY, WILTSHIRE, SP3 5ST**	**Effluent Type: SEWAGE DISCHARGES - FINAL/TREATED EFFLUENT - NOT WATER COMPANY Permit Number: 040653 Permit Version: 2 Receiving Water: SOAKAWAY**	**Status: REVOKED - UNSPECIFIED Issue date: 17/12/2012 Effective Date: 17/12/2012 Revocation Date: 17/03/2016**
A	**On site**	**EAT AND GO AT CHICKLADE SERVICE STN, A303, HINDON, SALISBURY, WILTSHIRE, SP3 5ST**	**Effluent Type: SEWAGE DISCHARGES - FINAL/TREATED EFFLUENT - NOT WATER COMPANY Permit Number: 040653 Permit Version: 1 Receiving Water: SOAKAWAY**	**Status: NEW CONSENT, BY APPLICATION (WRA 91, SECTION 88) Issue date: 31/03/1988 Effective Date: 01/04/1988 Revocation Date: 16/12/2012**

ID	Location	Address	Details	
A	**On site**	**CHICKLADE SERVICE STATION, CHICKLADE, WILTSHIRE, SP3 5ST**	**Effluent Type: TRADE DISCHARGES - SITE DRAINAGE (CONTAM SURFACE WATER, NOT WASTE SIT Permit Number: 401563 Permit Version: 1 Receiving Water: GROUNDWATER VIA SOAKAWAY**	**Status: NEW CONSENT, BY APPLICATION (WRA 91, SECTION 88) Issue date: 17/11/2003 Effective Date: 17/11/2003 Revocation Date: 16/12/2012**
A	**On site**	**EAT AND GO AT CHICKLADE SERVICE STN, A303, HINDON, SALISBURY, WILTSHIRE, SP3 5ST**	**Effluent Type: SEWAGE DISCHARGES - FINAL/TREATED EFFLUENT - NOT WATER COMPANY Permit Number: EPRHB3290EB Permit Version: 1 Receiving Water: GROUNDWATER VIA INFILTRATION**	**Status: NEW ISSUED UNDER EPR 2010 Issue date: 20/12/2017 Effective Date: 19/12/2017 Revocation Date: -**

This data is sourced from the Environment Agency and Natural Resources Wales.

4.15 Pollutant release to surface waters (Red List)

Records within 500m	**0**

Discharges of specified substances under the Environmental Protection (Prescribed Processes and Substances) Regulations 1991.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.16 Pollutant release to public sewer

Records within 500m	**0**

Discharges of Special Category Effluents to the public sewer.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.17 List 1 Dangerous Substances

Records within 500m	**0**

Discharges of substances identified on List I of European Directive E 2006/11/EC, and regulated under the Environmental Damage (Prevention and Remediation) Regulations 2015.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.18 List 2 Dangerous Substances

Records within 500m	0

Discharges of substances identified on List II of European Directive E 2006/11/EC, and regulated under the Environmental Damage (Prevention and Remediation) Regulations 2015.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.19 Pollution Incidents (EA/NRW)

Records within 500m	0

Records of substantiated pollution incidents. Since 2006 this data has only included category 1 (major) and 2 (significant) pollution incidents.

This data is sourced from the Environment Agency and Natural Resources Wales.

4.20 Pollution inventory substances

Records within 500m	0

The pollution inventory (substances) includes reporting on annual emissions of certain regulated substances to air, controlled waters and land. A reporting threshold for each substance is also included. Where emissions fall below the reporting threshold, no value will be given. The data is given for the most recent complete year available.

This data is sourced from the Environment Agency and the Scottish Environment Protection Agency.

4.21 Pollution inventory waste transfers

Records within 500m	0

The pollution inventory (waste transfers) includes reporting on annual transfers and recovery/disposal of controlled wastes from a site. A reporting threshold for each waste type is also included. Where releases fall below the reporting threshold, no value will be given. The data is given for the most recent complete year available.

This data is sourced from the Environment Agency and the Scottish Environment Protection Agency.

4.22 Pollution inventory radioactive waste

Records within 500m	0

The pollution inventory (radioactive wastes) includes reporting on annual releases of radioactive substances from a site, including the means of release. Where releases fall below the reporting threshold, no value will be given. The data is given for the most recent complete year available.

This data is sourced from the Environment Agency and the Scottish Environment Protection Agency.

5 Hydrogeology - Superficial aquifer



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Site Outline

Search buffers in metres (m)

- Principal
- Secondary A
- Secondary B
- Secondary Undifferentiated
- Unproductive
- Unknown

5.1 Superficial aquifer

Records within 500m	2

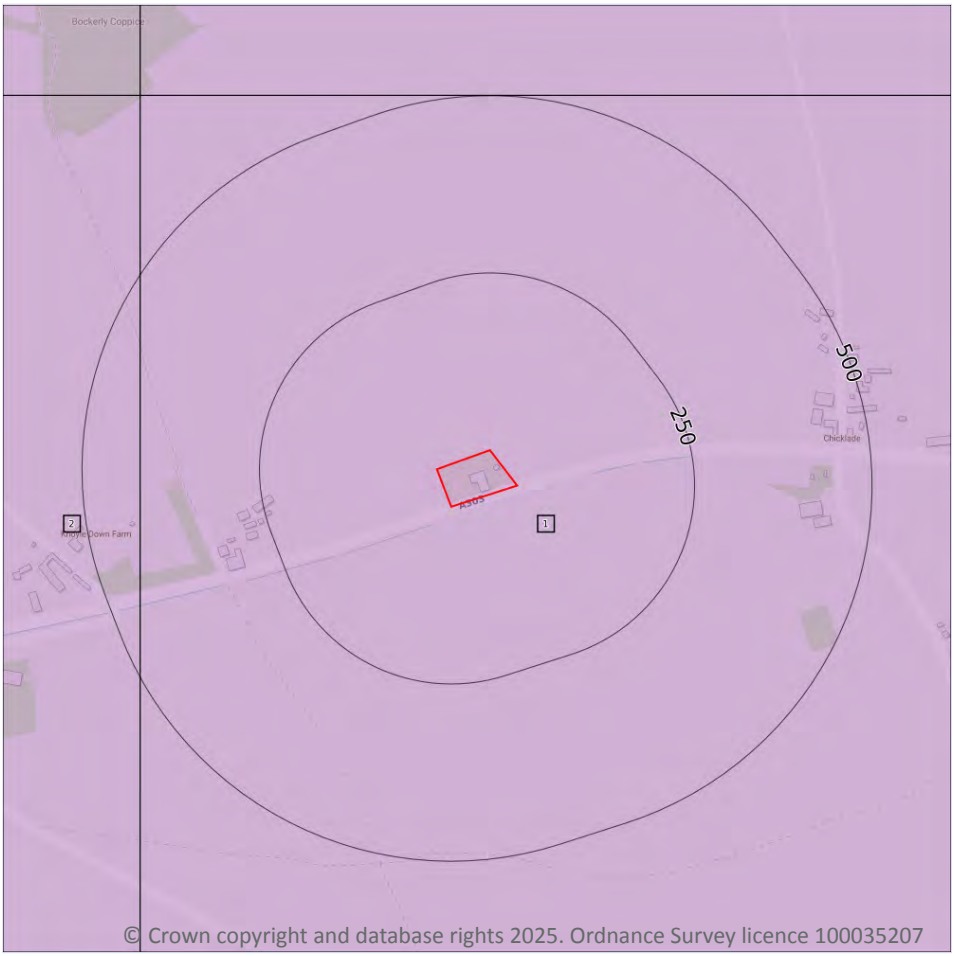
Aquifer status of groundwater held within superficial geology.

Features are displayed on the Hydrogeology map on **page 29 >**

ID	Location	Designation	Description
1	**On site**	**Secondary Undifferentiated**	**Assigned where it is not possible to attribute either category A or B to a rock type. In general these layers have previously been designated as both minor and non-aquifer in different locations due to the variable characteristics of the rock type**
2	440m W	Secondary Undifferentiated	Assigned where it is not possible to attribute either category A or B to a rock type. In general these layers have previously been designated as both minor and non-aquifer in different locations due to the variable characteristics of the rock type

This data is sourced from the British Geological Survey, the Environment Agency and Natural Resources Wales.

Bedrock aquifer



Site Outline

Search buffers in metres (m)

- Principal
- Secondary A
- Secondary B
- Secondary Undifferentiated
- Unproductive

5.2 Bedrock aquifer

Records within 500m	2

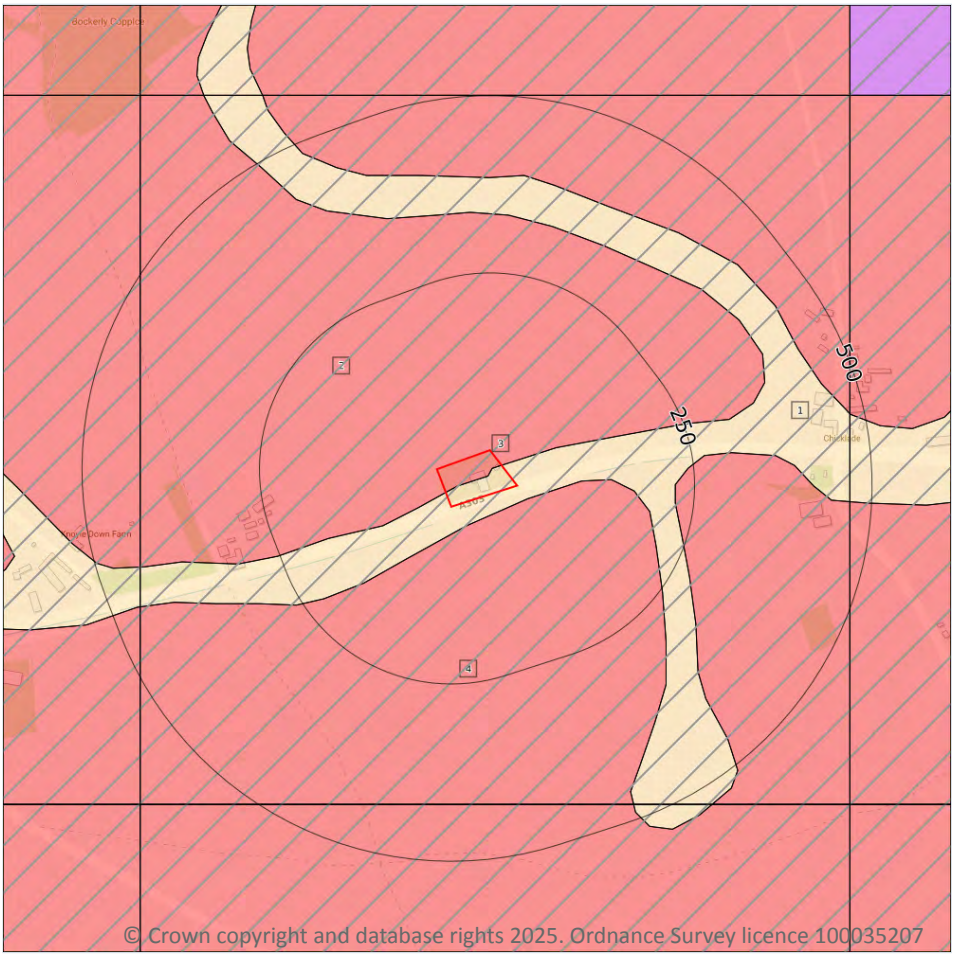
Aquifer status of groundwater held within bedrock geology.

Features are displayed on the Bedrock aquifer map on **page 31 >**

ID	Location	Designation	Description
1	**On site**	**Principal**	**Geology of high intergranular and/or fracture permeability, usually providing a high level of water storage and may support water supply/river base flow on a strategic scale. Generally principal aquifers were previously major aquifers**
2	418m W	Principal	Geology of high intergranular and/or fracture permeability, usually providing a high level of water storage and may support water supply/river base flow on a strategic scale. Generally principal aquifers were previously major aquifers

This data is sourced from the British Geological Survey, the Environment Agency and Natural Resources Wales.

Groundwater vulnerability



Site Outline

Search buffers in metres (m)

Superficial vulnerability

- Principal superficial aquifer, high vulnerability
- Secondary superficial aquifer, high vulnerability
- Principal superficial aquifer, medium vulnerability
- Secondary superficial aquifer, medium vulnerability
- Principal superficial aquifer, low vulnerability
- Secondary superficial aquifer, low vulnerability

Bedrock vulnerability

- Principal bedrock aquifer, high vulnerability
- Secondary bedrock aquifer, high vulnerability
- Principal bedrock aquifer, medium vulnerability
- Secondary bedrock aquifer, medium vulnerability
- Principal bedrock aquifer, low vulnerability
- Secondary bedrock aquifer, low vulnerability

Other information

- Unproductive aquifer
- Soluble rock risk
- Local information

5.3 Groundwater vulnerability

Records within 50m	3

An assessment of the vulnerability of groundwater to a pollutant discharged at ground level based on the hydrological, geological, hydrogeological and soil properties within a one kilometre square grid. Groundwater vulnerability is described as High, Medium or Low as follows:

- High - Areas able to easily transmit pollution to groundwater. They are likely to be characterised by high leaching soils and the absence of low permeability superficial deposits.
- Medium - Intermediate between high and low vulnerability.
- Low - Areas that provide the greatest protection from pollution. They are likely to be characterised by low leaching soils and/or the presence of superficial deposits characterised by a low permeability.

Features are displayed on the Groundwater vulnerability map on **page 33 >**

ID	Location	Summary	Soil / surface	Superficial geology	Bedrock geology
1	**On site**	**Summary Classification: Secondary superficial aquifer - High Vulnerability Combined classification: Productive Bedrock Aquifer, Productive Superficial Aquifer**	**Leaching class: High Infiltration value: >70% Dilution value: >550mm/year**	**Vulnerability: High Aquifer type: Secondary Thickness: <3m Patchiness value: <90% Recharge potential: No Data**	**Vulnerability: High Aquifer type: Principal Flow mechanism: Well connected fractures**
2	**On site**	**Summary Classification: Principal bedrock aquifer - High Vulnerability Combined classification: Productive Bedrock Aquifer, No Superficial Aquifer**	**Leaching class: High Infiltration value: >70% Dilution value: >550mm/year**	**Vulnerability: - Aquifer type: - Thickness: <3m Patchiness value: <90% Recharge potential: No Data**	**Vulnerability: High Aquifer type: Principal Flow mechanism: Well connected fractures**
4	23m SE	Summary Classification: Principal bedrock aquifer - High Vulnerability Combined classification: Productive Bedrock Aquifer, No Superficial Aquifer	Leaching class: High Infiltration value: >70% Dilution value: >550mm/year	Vulnerability: - Aquifer type: - Thickness: <3m Patchiness value: <90% Recharge potential: No Data	Vulnerability: High Aquifer type: Principal Flow mechanism: Well connected fractures

This data is sourced from the British Geological Survey, the Environment Agency and Natural Resources Wales.

5.4 Groundwater vulnerability- soluble rock risk

Records on site	**1**

This dataset identifies areas where solution features that enable rapid movement of a pollutant may be present within a 1km grid square.

ID	Maximum soluble risk category	Percentage of grid square covered by maximum risk
3	**Significant soluble rocks are likely to be present. Low possibility of localised subsidence or dissolution-related degradation of bedrock occurring naturally, but may be possible in adverse conditions such as high surface or subsurface water flow.**	**16.0%**

This data is sourced from the British Geological Survey and the Environment Agency.

5.5 Groundwater vulnerability- local information

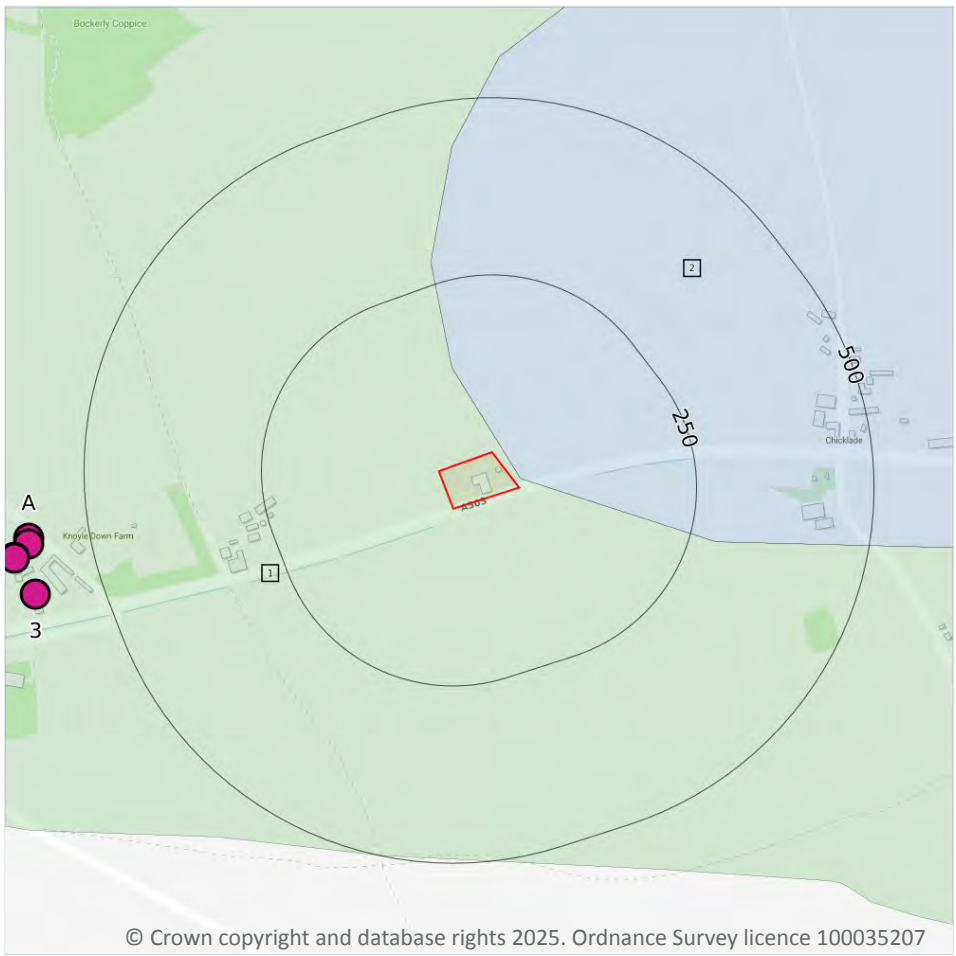
Records on site	**0**

This dataset identifies areas where additional local information affecting vulnerability is held by the Environment Agency. Further information can be obtained by contacting the Environment Agency local Area groundwater team through the Environment Agency National Customer Call Centre on 03798 506 506 or by

email on enquiries@environment-agency.gov.uk ↗.

This data is sourced from the British Geological Survey and the Environment Agency.

Abstractions and Source Protection Zones



Site Outline

Search buffers in metres (m)

Source Protection Zone 1
Inner catchment

Source Protection Zone 2
Outer catchment

Source Protection Zone 3
Total catchment

Source Protection Zone 4
Zone of Special Interest

Source Protection Zone 1c
Inner catchment - confined aquifer

Source Protection Zone 2c
Outer catchment - confined aquifer

Source Protection Zone 3c
Total catchment - confined aquifer

Drinking water abstraction licences

Drinking water abstraction licences
Polygon features

Drinking water abstraction licences
Linear features

Groundwater abstraction licence (point)

Groundwater abstraction licence (area)

Groundwater abstraction licence (linear)

Surface Water Abstractions (point)

Surface Water Abstractions (area)

Surface Water Abstractions (linear)

5.6 Groundwater abstractions

Records within 2000m	31

Licensed groundwater abstractions for sites extracting more than 20 cubic metres of water a day and includes active and historical records. The data may be for a single abstraction point, between two points (line data) or a larger area.

Features are displayed on the Abstractions and Source Protection Zones map on **page 36 >**

ID	Location	Details	
A	586m W	Status: Active Licence No: 13/43/022/G/048 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: KNOYLE DOWN FARM BOREHOLE POINT 'B' Data Type: Point Name: Hindon Farms Ltd Easting: 389840 Northing: 134378	Annual Volume (m^3): 8296 Max Daily Volume (m^3): 22.73 Original Application No: NPS/WR/028774 Original Start Date: 01/07/1966 Expiry Date: - Issue No: 103 Version Start Date: 29/05/2018 Version End Date: -
A	587m W	Status: Historical Licence No: 13/43/022/G/048 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: KNOYLE DOWN FARM BOREHOLE POINT 'B' Data Type: Point Name: Hindon Farms Ltd Easting: 389840 Northing: 134370	Annual Volume (m^3): 8296 Max Daily Volume (m^3): 22.73 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: - Issue No: 101 Version Start Date: 01/04/2008 Version End Date: -
3	594m W	Status: Historical Licence No: 13/43/022/G/048 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: KNOYLE DOWN FARM WELL #1 Data Type: Point Name: Hindon Farms Ltd Easting: 389850 Northing: 134300	Annual Volume (m^3): 8296 Max Daily Volume (m^3): 22.73 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: - Issue No: 100 Version Start Date: 02/09/1996 Version End Date: -
A	610m W	Status: Active Licence No: 13/43/022/G/048 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: KNOYLE DOWN FARM WELL POINT 'A' Data Type: Point Name: Hindon Farms Ltd Easting: 389820 Northing: 134351	Annual Volume (m^3): 8296 Max Daily Volume (m^3): 22.73 Original Application No: NPS/WR/028774 Original Start Date: 01/07/1966 Expiry Date: - Issue No: 103 Version Start Date: 29/05/2018 Version End Date: -
A	611m W	Status: Historical Licence No: 13/43/022/G/048 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: KNOYLE DOWN FARM WELL POINT 'A' Data Type: Point Name: Hindon Farms Ltd Easting: 389820 Northing: 134350	Annual Volume (m^3): 8296 Max Daily Volume (m^3): 22.73 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: - Issue No: 101 Version Start Date: 01/04/2008 Version End Date: -

ID	Location	Details	
-	956m S	Status: Historical Licence No: 13/43/022/G/047 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: HINDON WELL #1 Data Type: Point Name: Morrison Easting: 390700 Northing: 133500	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: - Issue No: 100 Version Start Date: 01/07/1966 Version End Date: -
-	956m S	Status: Historical Licence No: 13/43/022/G/047 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: HINDON WELL #1 Data Type: Point Name: Morrison Easting: 390700 Northing: 133500	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: - Issue No: 101 Version Start Date: 12/03/2001 Version End Date: -
-	1531m NW	Status: Historical Licence No: 13/43/022/G/024 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: "PERTWOOD MANOR FARM, PERTWOOD BOREHOLE #1" Data Type: Point Name: Giles Easting: 389200 Northing: 135400	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/06/1966 Expiry Date: - Issue No: 100 Version Start Date: 29/09/1987 Version End Date: -
-	1531m NW	Status: Historical Licence No: 13/43/022/G/024 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: PERTWOOD MANOR FARM, PERTWOOD BOREHOLE #1 Data Type: Point Name: Giles Easting: 389200 Northing: 135400	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/06/1966 Expiry Date: - Issue No: 100 Version Start Date: 29/09/1987 Version End Date: -
-	1852m E	Status: Historical Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Penning Power & Water Limited Easting: 392300 Northing: 133900	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -

ID	Location	Details	
-	1852m E	Status: Historical Licence No: SW/043/0022/004 Details: General Use Relating To Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Penning Power & Water Limited Easting: 392300 Northing: 133900	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1852m E	Status: Historical Licence No: 13/43/022/G/050 Details: General Use Relating To Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392300 Northing: 133900	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1852m E	Status: Historical Licence No: 13/43/022/G/050 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392300 Northing: 133900	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1852m E	Status: Active Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Penning Power & Water Limited Easting: 392300 Northing: 133900	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -
-	1852m E	Status: Active Licence No: SW/043/0022/004 Details: General Use Relating To Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Penning Power & Water Limited Easting: 392300 Northing: 133900	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -

ID	Location	Details	
-	1960m NE	Status: Historical Licence No: SW/043/0022/004 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1960m NE	Status: Historical Licence No: SW/043/0022/004 Details: General Use Relating To Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1960m NE	Status: Historical Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1960m NE	Status: Historical Licence No: SW/043/0022/004 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: BERWICK ST LEONARD WELL/BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2006 Issue No: 102 Version Start Date: 31/07/2001 Version End Date: -

ID	Location	Details	
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: General use relating to Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: BERWICK ST LEONARD WELL/BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2006 Issue No: 102 Version Start Date: 31/07/2001 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK ST LEONARD WELL/BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2006 Issue No: 102 Version Start Date: 31/07/2001 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: BERWICK ST LEONARD WELL/BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2006 Issue No: 102 Version Start Date: 31/07/2001 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Water Bottling Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -

ID	Location	Details	
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: General Use Relating To Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1960m NE	Status: Active Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -
-	1960m NE	Status: Active Licence No: SW/043/0022/004 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -
-	1960m NE	Status: Active Licence No: SW/043/0022/004 Details: General Farming & Domestic Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -

ID	Location	Details	
-	1960m NE	Status: Active Licence No: SW/043/0022/004 Details: General Use Relating To Secondary Category (Low Loss) Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -

This data is sourced from the Environment Agency and Natural Resources Wales.

5.7 Surface water abstractions

Records within 2000m	**0**

Licensed surface water abstractions for sites extracting more than 20 cubic metres of water a day and includes active and historical records. The data may be for a single abstraction point, a stretch of watercourse or a larger area.

This data is sourced from the Environment Agency and Natural Resources Wales.

5.8 Potable abstractions

Records within 2000m	**11**

Licensed potable water abstractions for sites extracting more than 20 cubic metres of water a day and includes active and historical records. The data may be for a single abstraction point, a stretch of watercourse or a larger area.

Features are displayed on the Abstractions and Source Protection Zones map on **page 36 >**

ID	Location	Details	
-	1852m E	Status: Historical Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Penning Power & Water Limited Easting: 392300 Northing: 133900	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -

ID	Location	Details	
-	1852m E	Status: Historical Licence No: 13/43/022/G/050 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392300 Northing: 133900	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1852m E	Status: Active Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK HILL DOWN BOREHOLE Data Type: Point Name: Penning Power & Water Limited Easting: 392300 Northing: 133900	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -
-	1960m NE	Status: Historical Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1960m NE	Status: Historical Licence No: SW/043/0022/004 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: 31/03/2025 Issue No: 2 Version Start Date: 22/06/2018 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Water Bottling Direct Source: Ground Water - Fresh Point: BERWICK ST LEONARD WELL/BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2006 Issue No: 102 Version Start Date: 31/07/2001 Version End Date: -

ID	Location	Details	
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: BERWICK ST LEONARD WELL/BOREHOLE Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): - Max Daily Volume (m^3): - Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2006 Issue No: 102 Version Start Date: 31/07/2001 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Water Bottling Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1960m NE	Status: Historical Licence No: 13/43/022/G/050 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Fonthill Estate / Penning Water Ltd Easting: 392240 Northing: 135410	Annual Volume (m^3): 37986 Max Daily Volume (m^3): 240 Original Application No: - Original Start Date: 01/07/1966 Expiry Date: 31/03/2013 Issue No: 105 Version Start Date: 03/11/2006 Version End Date: -
-	1960m NE	Status: Active Licence No: SW/043/0022/004 Details: Water Bottling Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -
-	1960m NE	Status: Active Licence No: SW/043/0022/004 Details: Drinking, Cooking, Sanitary, Washing, (Small Garden) - Household Direct Source: Ground Water - Fresh Point: PENNINGS BOREHOLE, BERWICK ST LEONARD Data Type: Point Name: Penning Power & Water Limited Easting: 392240 Northing: 135410	Annual Volume (m^3): 68986 Max Daily Volume (m^3): 240 Original Application No: NPS/WR/025471 Original Start Date: 01/04/2013 Expiry Date: - Issue No: 2 Version Start Date: 01/04/2025 Version End Date: -

This data is sourced from the Environment Agency and Natural Resources Wales.

5.9 Source Protection Zones

Records within 500m	2

Source Protection Zones define the sensitivity of an area around a potable abstraction site to contamination.

Features are displayed on the Abstractions and Source Protection Zones map on **page 36 >**

ID	Location	Type	Description
1	**On site**	**3**	**Total catchment**
2	10m NE	2	Outer catchment

This data is sourced from the Environment Agency and Natural Resources Wales.

5.10 Source Protection Zones (confined aquifer)

Records within 500m	0

Source Protection Zones in the confined aquifer define the sensitivity around a deep groundwater abstraction to contamination. A confined aquifer would normally be protected from contamination by overlying geology and is only considered a sensitive resource if deep excavation/drilling is taking place.

This data is sourced from the Environment Agency and Natural Resources Wales.

6 Hydrology



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Site Outline

Search buffers in metres (m)

Water Network (OS MasterMap)

Surface water features (wider than 5m)

Surface water features (narrower than 5m)

WFD River, canal and surface water transfer water bodies

WFD Lake water bodies

WFD Transitional and coastal water bodies

WFD Surface water body catchments boundaries

WFD Groundwater body boundaries

6.1 Water Network (OS MasterMap)

Records within 250m	6

Detailed water network of Great Britain showing the flow and precise central course of every river, stream, lake and canal.

Features are displayed on the Hydrology map on **page 47 >**

ID	Location	Type of water feature	Ground level	Permanence	Name
B	37m SW	Inland river not influenced by normal tidal action.	On ground surface	Watercourse contains water year round (in normal circumstances)	-

ID	Location	Type of water feature	Ground level	Permanence	Name
C	109m E	Inland river not influenced by normal tidal action.	On ground surface	Watercourse contains water year round (in normal circumstances)	-
C	167m E	Inland river not influenced by normal tidal action.	Underground	Watercourse contains water year round (in normal circumstances)	-
1	168m SW	Inland river not influenced by normal tidal action.	Underground	Watercourse contains water year round (in normal circumstances)	-
C	179m E	Inland river not influenced by normal tidal action.	On ground surface	Watercourse contains water year round (in normal circumstances)	-
D	179m SW	Inland river not influenced by normal tidal action.	On ground surface	Watercourse contains water year round (in normal circumstances)	-

This data is sourced from the Ordnance Survey.

6.2 Surface water features

Records within 250m	**4**

Covering rivers, streams and lakes (some overlap with OS MasterMap Water Network data in previous section) but additionally covers smaller features such as ponds. Rivers and streams narrower than 5m are represented as a single line. Lakes, ponds and rivers or streams wider than 5m are represented as polygons.

Features are displayed on the Hydrology map on **page 47 >**

This data is sourced from the Ordnance Survey.

6.3 WFD Surface water body catchments

Records on site	**1**

The Water Framework Directive is an EU-led framework for the protection of inland surface waters, estuaries, coastal waters and groundwater through river basin-level management planning. In terms of surface water, these basins are broken down into smaller units known as management, operational and water body catchments.

Features are displayed on the Hydrology map on **page 47 >**

ID	Location	Type	Water body catchment	Water body ID	Operational catchment	Management catchment
A	**On site**	**River**	**Fonthill Stream**	**GB108043022500**	**Avon Hampshire**	**Avon Hampshire**

This data is sourced from the Environment Agency and Natural Resources Wales.

6.4 WFD Surface water bodies

Records identified	1

Surface water bodies under the Directive may be rivers, lakes, estuary or coastal. To achieve the purpose of the Directive, environmental objectives have been set and are reported on for each water body. The progress towards delivery of the objectives is then reported on by the relevant competent authorities at the end of each six-year cycle. The river water body directly associated with the catchment listed in the previous section is detailed below, along with any lake, canal, coastal or artificial water body within 250m of the site. Click on the water body ID in the table to visit the EA Catchment Explorer to find out more about each water body listed.

Features are displayed on the Hydrology map on **page 47 >**

ID	Location	Type	Name	Water body ID	Overall rating	Chemical rating	Ecological rating	Year
-	2196m SE	River	Fonthill Stream	GB108043022500 ↗	Moderate	Fail	Moderate	2019

This data is sourced from the Environment Agency and Natural Resources Wales.

6.5 WFD Groundwater bodies

Records on site	1

Groundwater bodies are also covered by the Directive and the same regime of objectives and reporting detailed in the previous section is in place. Click on the water body ID in the table to visit the EA Catchment Explorer to find out more about each groundwater body listed.

Features are displayed on the Hydrology map on **page 47 >**

ID	Location	Name	Water body ID	Overall rating	Chemical rating	Quantitative	Year
A	**On site**	**Upper Hampshire Avon**	**GB40801G806900 ↗**	**Poor**	**Poor**	**Good**	**2019**

This data is sourced from the Environment Agency and Natural Resources Wales.

7 River and coastal flooding

7.1 Risk of flooding from rivers and the sea

Records within 50m	0

The chance of flooding from rivers and/or the sea in any given year, based on cells of 50m within the Risk of Flooding from Rivers and Sea (RoFRaS)/Flood Risk Assessment Wales (FRAW) models. Each cell is allocated one of four flood risk categories, taking into account flood defences and their condition. The risk categories for RoFRaS for rivers and the sea and FRAW for rivers are; Very low (less than 1 in 1000 chance in any given year), Low (less than 1 in 100 but greater than or equal to 1 in 1000 chance), Medium (less than 1 in 30 but greater than or equal to 1 in 100 chance) or High (greater than or equal to 1 in 30 chance). The risk categories for FRAW for the sea are; Very low (less than 1 in 1000 chance in any given year), Low (less than 1 in 200 but greater than or equal to 1 in 1000 chance), Medium (less than 1 in 30 but greater than or equal to 1 in 200 chance) or High (greater than or equal to 1 in 30 chance).

This data is sourced from the Environment Agency and Natural Resources Wales.

7.2 Historical Flood Events

Records within 250m	0

Records of historic flooding from rivers, the sea, groundwater and surface water. Records began in 1946 when predecessor bodies started collecting detailed information about flooding incidents, although limited details may be included on flooding incidents prior to this date. Takes into account the presence of defences, structures, and other infrastructure where they existed at the time of flooding, and includes flood extents that may have been affected by overtopping, breaches or blockages.

This data is sourced from the Environment Agency and Natural Resources Wales.

7.3 Flood Defences

Records within 250m	0

Records of flood defences owned, managed or inspected by the Environment Agency and Natural Resources Wales. Flood defences can be structures, buildings or parts of buildings. Typically these are earth banks, stone and concrete walls, or sheet-piling that is used to prevent or control the extent of flooding.

This data is sourced from the Environment Agency and Natural Resources Wales.

7.4 Areas Benefiting from Flood Defences

Records within 250m	0

Areas that would benefit from the presence of flood defences in a 1 in 100 (1%) chance of flooding each year from rivers or 1 in 200 (0.5%) chance of flooding each year from the sea.

This data is sourced from the Environment Agency and Natural Resources Wales.

7.5 Flood Storage Areas

Records within 250m	0

Areas that act as a balancing reservoir, storage basin or balancing pond to attenuate an incoming flood peak to a flow level that can be accepted by the downstream channel or to delay the timing of a flood peak so that its volume is discharged over a longer period.

This data is sourced from the Environment Agency and Natural Resources Wales.

River and coastal flooding - Flood Zones

7.6 Flood Zone 2

Records within 50m	0

Areas of land at risk of flooding, when the presence of flood defences are ignored. Covering land between Flood Zone 3 (see next section) and the extent of the flooding from rivers or the sea with a 1 in 1000 (0.1%) chance of flooding each year.

This data is sourced from the Environment Agency and Natural Resources Wales.

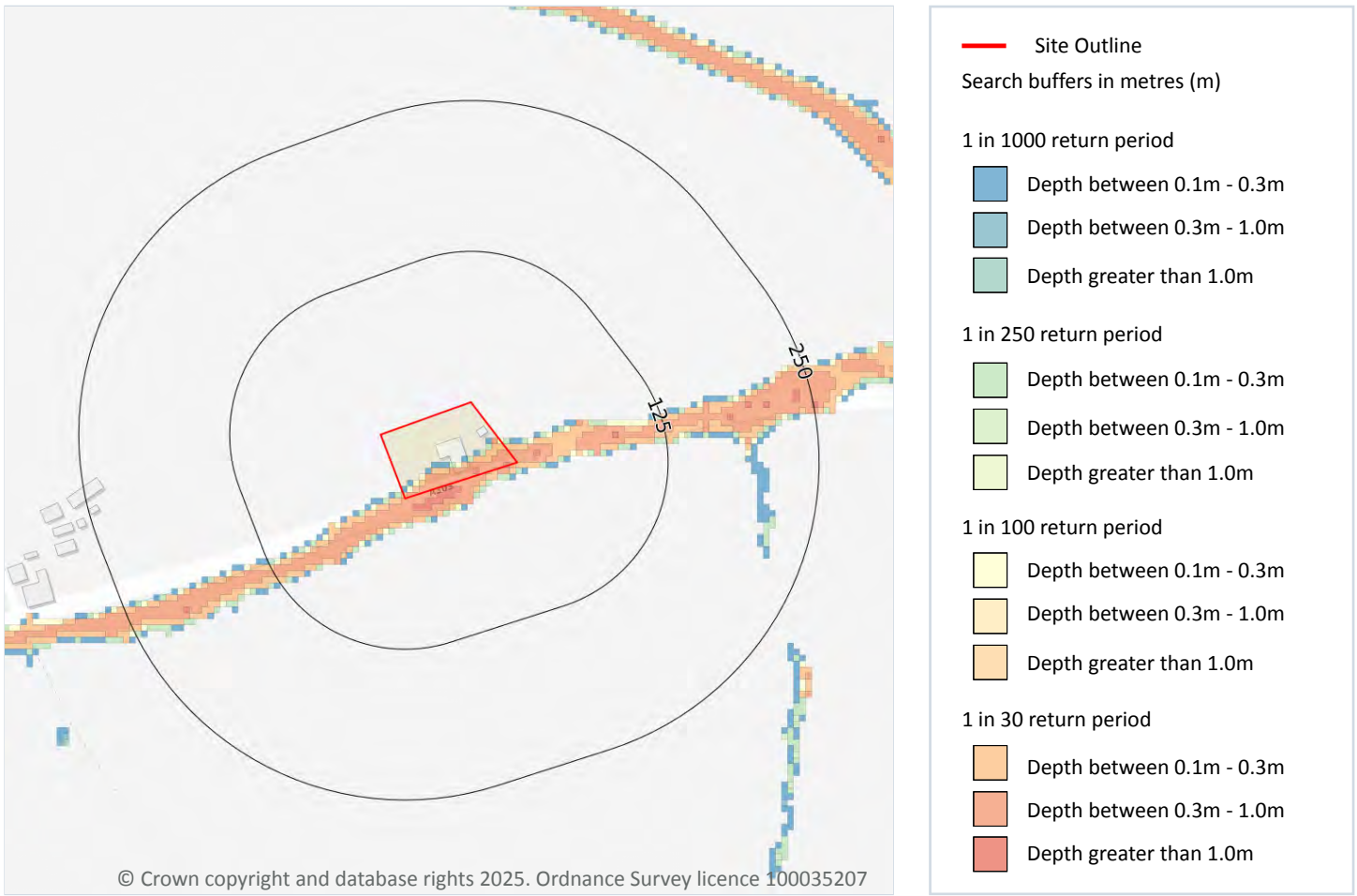
7.7 Flood Zone 3

Records within 50m	0

Areas of land at risk of flooding, when the presence of flood defences are ignored. Covering land with a 1 in 100 (1%) or greater chance of flooding each year from rivers or a 1 in 200 (0.5%) or greater chance of flooding each year from the sea.

This data is sourced from the Environment Agency and Natural Resources Wales.

8 Surface water flooding



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8.1 Surface water flooding

Highest risk on site	**1 in 30 year, Greater than 1.0m**
Highest risk within 50m	**1 in 30 year, Greater than 1.0m**

Ambiental Risk Analytics surface water (pluvial) FloodMap identifies areas likely to flood as a result of extreme rainfall events, i.e. land naturally vulnerable to surface water ponding or flooding. This data set was produced by simulating 1 in 30 year, 1 in 100 year, 1 in 250 year and 1 in 1,000 year rainfall events. Modern urban drainage systems are typically built to cope with rainfall events between 1 in 20 and 1 in 30 years, though some older ones may flood in a 1 in 5 year rainfall event.

Features are displayed on the Surface water flooding map on **page 53 >**

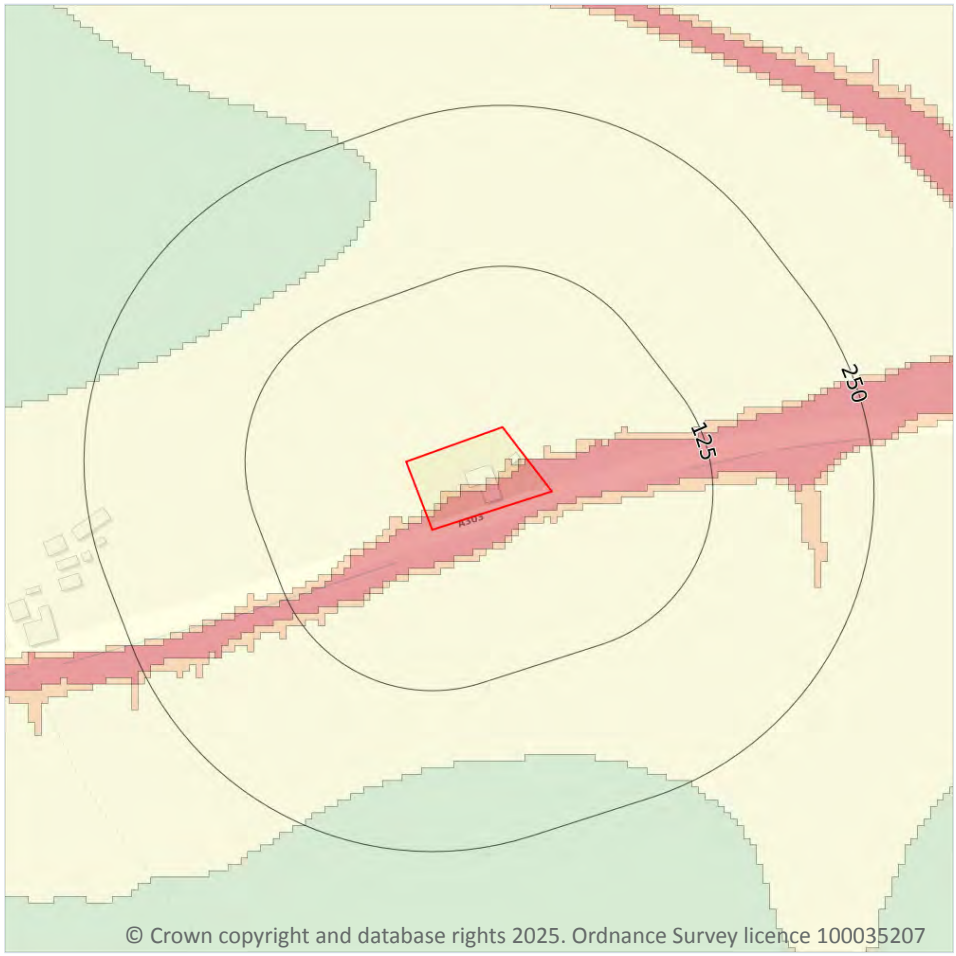
The data shown on the map and in the table above shows the highest likelihood of flood events happening at the site. Lower likelihood events may have greater flood depths and hence a greater potential impact on a site.

The table below shows the maximum flood depths for a range of return periods for the site.

Return period	Maximum modelled depth
1 in 1000 year	Greater than 1.0m
1 in 250 year	Greater than 1.0m
1 in 100 year	Greater than 1.0m
1 in 30 year	Greater than 1.0m

This data is sourced from Ambiental Risk Analytics.

9 Groundwater flooding



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Site Outline

Search buffers in metres (m)

High

Moderate - High

Moderate

Low

Negligible

9.1 Groundwater flooding

Highest risk on site	**High**
Highest risk within 50m	**High**

Groundwater flooding is caused by unusually high groundwater levels. It occurs when the water table rises above the ground surface or within underground structures such as basements or cellars. Groundwater flooding tends to exhibit a longer duration than surface water flooding, possibly lasting for weeks or months, and as a result it can cause significant damage to property. This risk assessment is based on a 1 in 100 year return period and a 5m Digital Terrain Model (DTM).

Features are displayed on the Groundwater flooding map on **page 55 >**

This data is sourced from Ambiental Risk Analytics.

10 Environmental designations



Site Outline

Search buffers in metres (m)

Designated Ancient Woodland

10.1 Sites of Special Scientific Interest (SSSI)

Records within 2000m	0

Sites providing statutory protection for the best examples of UK flora, fauna, or geological or physiographical features. Originally notified under the National Parks and Access to the Countryside Act 1949, SSSIs were re-notified under the Wildlife and Countryside Act 1981. Improved provisions for the protection and management of SSSIs were introduced by the Countryside and Rights of Way Act 2000 (in England and Wales) and (in Scotland) by the Nature Conservation (Scotland) Act 2004 and the Wildlife and Natural Environment (Scotland) Act 2010.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.2 Conserved wetland sites (Ramsar sites)

Records within 2000m	0

Ramsar sites are designated under the Convention on Wetlands of International Importance, agreed in Ramsar, Iran, in 1971. They cover all aspects of wetland conservation and wise use, recognizing wetlands as ecosystems that are extremely important for biodiversity conservation in general and for the well-being of human communities. These sites cover a broad definition of wetland; marsh, fen, peatland or water, whether natural or artificial, permanent or temporary, with water that is static or flowing, fresh, brackish or salt, and even some marine areas.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.3 Special Areas of Conservation (SAC)

Records within 2000m	0

Areas which have been identified as best representing the range and variety within the European Union of habitats and (non-bird) species listed on Annexes I and II to the Directive. SACs are designated under the EC Habitats Directive.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.4 Special Protection Areas (SPA)

Records within 2000m	0

Sites classified by the UK Government under the EC Birds Directive, SPAs are areas of the most important habitat for rare (listed on Annex I to the Directive) and migratory birds within the European Union.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.5 National Nature Reserves (NNR)

Records within 2000m	0

Sites containing examples of some of the most important natural and semi-natural terrestrial and coastal ecosystems in Great Britain. They are managed to conserve their habitats, provide special opportunities for scientific study or to provide public recreation compatible with natural heritage interests.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.6 Local Nature Reserves (LNR)

Records within 2000m	0

Sites managed for nature conservation, and to provide opportunities for research and education, or simply enjoying and having contact with nature. They are declared by local authorities under the National Parks and Access to the Countryside Act 1949 after consultation with the relevant statutory nature conservation agency.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.7 Designated Ancient Woodland

Records within 2000m	9

Ancient woodlands are classified as areas which have been wooded continuously since at least 1600 AD. This includes semi-natural woodland and plantations on ancient woodland sites. 'Wooded continuously' does not mean there is or has previously been continuous tree cover across the whole site, and not all trees within the woodland have to be old.

Features are displayed on the Environmental designations map on **page 56 >**

ID	Location	Name	Woodland Type
-	1384m N	Unknown	Ancient & Semi-Natural Woodland
-	1389m N	Unknown	Ancient Replanted Woodland
-	1451m N	Unknown	Ancient Replanted Woodland
4	1537m NE	Unknown	Ancient Replanted Woodland
-	1555m NE	Unknown	Ancient & Semi-Natural Woodland
-	1613m N	Unknown	Ancient & Semi-Natural Woodland
-	1690m NE	Unknown	Ancient Replanted Woodland
-	1700m NE	Unknown	Ancient Replanted Woodland
-	1909m NE	Unknown	Ancient Replanted Woodland

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.8 Biosphere Reserves

Records within 2000m	0

Biosphere Reserves are internationally recognised by UNESCO as sites of excellence to balance conservation and socioeconomic development between nature and people. They are recognised under the Man and the Biosphere (MAB) Programme with the aim of promoting sustainable development founded on the work of the local community.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.9 Forest Parks

Records within 2000m	0

These are areas managed by the Forestry Commission designated on the basis of recreational, conservation or scenic interest.

This data is sourced from the Forestry Commission.

10.10 Marine Conservation Zones

Records within 2000m	0

A type of marine nature reserve in UK waters established under the Marine and Coastal Access Act (2009). They are designated with the aim to protect nationally important, rare or threatened habitats and species.

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

10.11 Green Belt

Records within 2000m	0

Areas designated to prevent urban sprawl by keeping land permanently open.

This data is sourced from the Ministry of Housing, Communities and Local Government.

10.12 Proposed Ramsar sites

Records within 2000m	0

Ramsar sites are areas listed as a Wetland of International Importance under the Convention on Wetlands of International Importance especially as Waterfowl Habitat (the Ramsar Convention) 1971. The sites here supplied have a status of 'Proposed' having been identified for potential adoption under the framework.

This data is sourced from Natural England.

10.13 Possible Special Areas of Conservation (pSAC)

Records within 2000m	0

Special Areas of Conservation are areas which have been identified as best representing the range and variety within the European Union of habitats and (non-bird) species listed on Annexes I and II to the Directive. SACs are designated under the EC Habitats Directive. Those sites supplied here are those with a status of 'Possible' having been identified for potential adoption under the framework.

This data is sourced from Natural England and Natural Resources Wales.

10.14 Potential Special Protection Areas (pSPA)

Records within 2000m	0

Special Protection Areas (SPAs) are areas designated (or 'classified') under the European Union Wild Birds Directive for the protection of nationally and internationally important populations of wild birds. Those sites supplied here are those with a status of 'Potential' having been identified for potential adoption under the framework.

This data is sourced from Natural England.

10.15 Nitrate Sensitive Areas

Records within 2000m	0

Areas where nitrate concentrations in drinking water sources exceeded or was at risk of exceeding the limit of 50 mg/l set by the 1980 EC Drinking Water Directive. Voluntary agricultural measures as a means of reducing the levels of nitrate were introduced by DEFRA as MAFF, with payments being made to farmers who complied. The scheme was started as a pilot in 1990 in ten areas, later implemented within 32 areas. The scheme was closed to further new entrants in 1998, although existing agreements continued for their full term. All Nitrate Sensitive Areas fell within the areas designated as Nitrate Vulnerable Zones (NVZs) in 1996 under the EC Nitrate Directive (91/676/EEC).

This data is sourced from Natural England.

10.16 Nitrate Vulnerable Zones

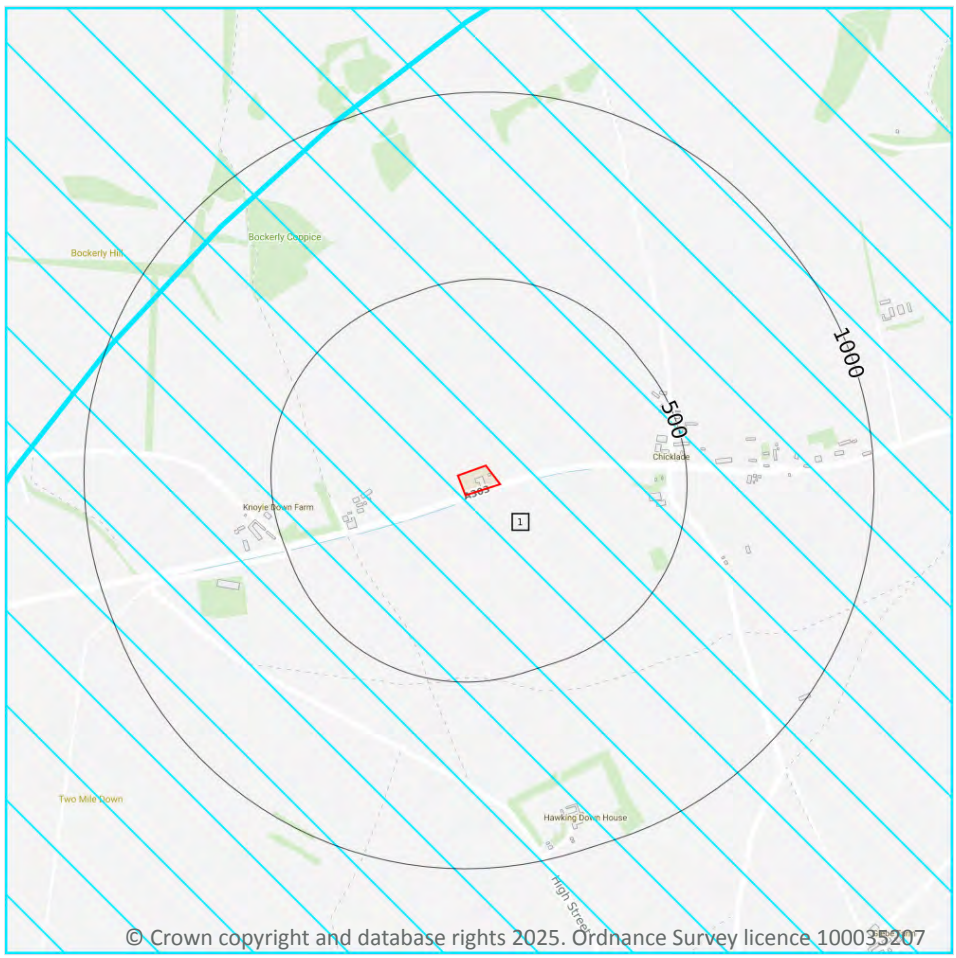
Records within 2000m	2

Areas at risk from agricultural nitrate pollution designated under the EC Nitrate Directive (91/676/EEC). These are areas of land that drain into waters polluted by nitrates. Farmers operating within these areas have to follow mandatory rules to tackle nitrate loss from agriculture.

Location	Name	Type	NVZ ID	Status
On site	**South Wessex**	**Groundwater**	**151**	**Existing**
1482m N	South Wessex	Groundwater	151	Existing

This data is sourced from Natural England and Natural Resources Wales.

SSSI Impact Zones and Units



Site Outline

Search buffers in metres (m)

SSSI Impact Risk Zones

SSSI Units

- Not recorded
- Favourable
- Unfavourable - Recovering
- Unfavourable - No change
- Unfavourable - Declining
- Partially destroyed
- Destroyed

10.17 SSSI Impact Risk Zones

Records on site	1

Developed to allow rapid initial assessment of the potential risks to SSSIs posed by development proposals. They define zones around each SSSI which reflect the particular sensitivities of the features for which it is notified and indicate the types of development proposal which could potentially have adverse impacts.

Features are displayed on the SSSI Impact Zones and Units map on **page 61 >**

ID	Location	Type of developments requiring consultation
1	**On site**	**https://irz.geodata.org.uk/IRZ/step2.html?irzcode=4300000430050&notes=11103&location=393499,135388%20(IRZ%20polygon%20centre)**

This data is sourced from Natural England.

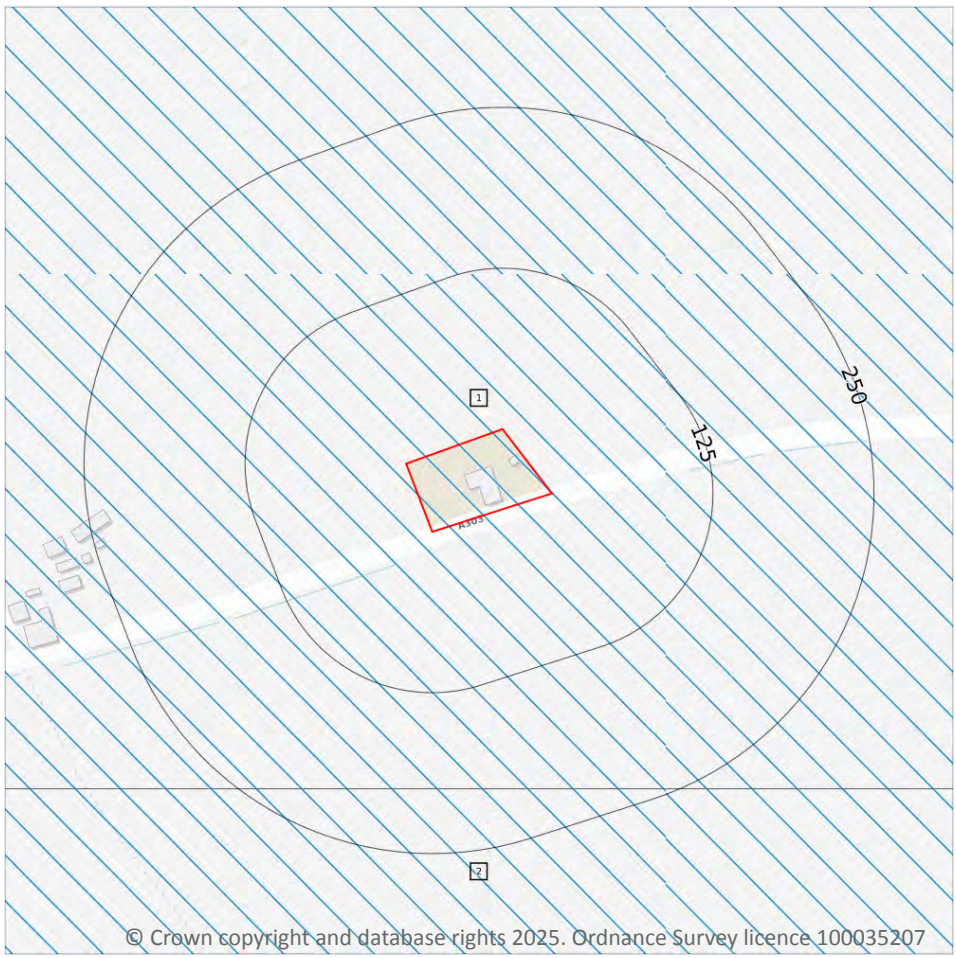
10.18 SSSI Units

Records within 2000m	0

Divisions of SSSIs used to record management and condition details. Units are the smallest areas for which Natural England gives a condition assessment, however, the size of units varies greatly depending on the types of management and the conservation interest.

This data is sourced from Natural England and Natural Resources Wales.

11 Visual and cultural designations



Site Outline

Search buffers in metres (m)

Listed buildings

Conservation areas

Conservation areas - no data

National Parks

Areas of Outstanding Natural Beauty

Registered parks and gardens

Scheduled Monuments

World Heritage Sites

11.1 World Heritage Sites

Records within 250m	0

Sites designated for their globally important cultural or natural interest requiring appropriate management and protection measures. World Heritage Sites are designated to meet the UK's commitments under the World Heritage Convention.

This data is sourced from Historic England, Cadw and Historic Environment Scotland.

11.2 Area of Outstanding Natural Beauty

Records within 250m	2

Areas of Outstanding Natural Beauty (AONB) are conservation areas, chosen because they represent 18% of the finest countryside. Each AONB has been designated for special attention because of the quality of their flora, fauna, historical and cultural associations, and/or scenic views. The National Parks and Access to the Countryside Act of 1949 created AONBs and the Countryside and Rights of Way Act, 2000 added further regulation and protection. There are likely to be restrictions to some developments within these areas.

Features are displayed on the Visual and cultural designations map on **page 63 >**

ID	Location	NAME	Data Source
1	**On site**	**Cranborne Chase & West Wiltshire Downs**	**Natural England**
2	199m S	Cranborne Chase & West Wiltshire Downs	Natural England

This data is sourced from Natural England, Natural Resources Wales and Scottish Natural Heritage.

11.3 National Parks

Records within 250m	0

In England and Wales, the purpose of National Parks is to conserve and enhance landscapes within the countryside whilst promoting public enjoyment of them and having regard for the social and economic well-being of those living within them. In Scotland National Parks have the additional purpose of promoting the sustainable use of the natural resources of the area and the sustainable social and economic development of its communities. The National Parks and Access to the Countryside Act 1949 established the National Park designation in England and Wales, and The National Parks (Scotland) Act 2000 in Scotland.

This data is sourced from Natural England, Natural Resources Wales and the Scottish Government.

11.4 Listed Buildings

Records within 250m	0

Buildings listed for their special architectural or historical interest. Building control in the form of 'listed building consent' is required in order to make any changes to that building which might affect its special interest. Listed buildings are graded to indicate their relative importance, however building controls apply to all buildings equally, irrespective of their grade, and apply to the interior and exterior of the building in its entirety, together with any curtilage structures.

This data is sourced from Historic England, Cadw and Historic Environment Scotland.

11.5 Conservation Areas

Records within 250m	0

Local planning authorities are obliged to designate as conservation areas any parts of their own area that are of special architectural or historic interest, the character and appearance of which it is desirable to preserve or enhance. Designation of a conservation area gives broader protection than the listing of individual buildings. All the features within the area, listed or otherwise, are recognised as part of its character. Conservation area designation is the means of recognising the importance of all factors and of ensuring that planning decisions address the quality of the landscape in its broadest sense.

This data is sourced from Historic England, Cadw and Historic Environment Scotland.

11.6 Scheduled Ancient Monuments

Records within 250m	0

A scheduled monument is an historic building or site that is included in the Schedule of Monuments kept by the Secretary of State for Digital, Culture, Media and Sport. The regime is set out in the Ancient Monuments and Archaeological Areas Act 1979. The Schedule of Monuments has c.20,000 entries and includes sites such as Roman remains, burial mounds, castles, bridges, earthworks, the remains of deserted villages and industrial sites. Monuments are not graded, but all are, by definition, considered to be of national importance.

This data is sourced from Historic England, Cadw and Historic Environment Scotland.

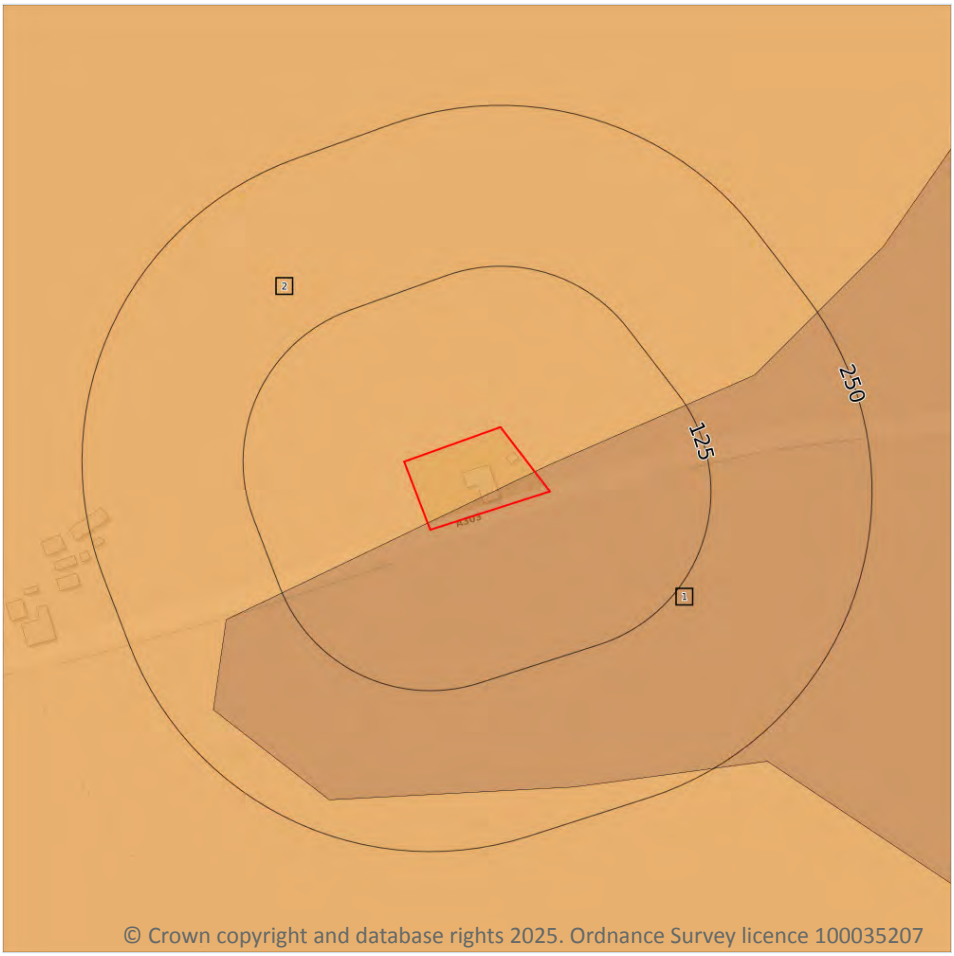
11.7 Registered Parks and Gardens

Records within 250m	0

Parks and gardens assessed to be of particular interest and of special historic interest. The emphasis being on 'designed' landscapes, rather than on planting or botanical importance. Registration is a 'material consideration' in the planning process, meaning that planning authorities must consider the impact of any proposed development on the special character of the landscape.

This data is sourced from Historic England, Cadw and Historic Environment Scotland.

12 Agricultural designations



Site Outline

Search buffers in metres (m)

- Grade 1 - excellent quality
- Grade 2 - very good quality
- Grade 3 - good to moderate quality
- Grade 3a - good quality
- Grade 3b - moderate quality
- Grade 4 - poor quality
- Grade 5 - very poor quality
- Non-agricultural land
- Urban land
- Exclusion land
- Tree felling licences
- Open Access land

12.1 Agricultural Land Classification

Records within 250m	2

Classification of the quality of agricultural land taking into consideration multiple factors including climate, physical geography and soil properties. It should be noted that the categories for the grading of agricultural land are not consistent across England, Wales and Scotland.

Features are displayed on the Agricultural designations map on **page 66 >**

ID	Location	Classification	Description
1	**On site**	**Grade 2**	**Very good quality agricultural land. Land with minor limitations which affect crop yield, cultivations or harvesting. A wide range of agricultural and horticultural crops can usually be grown but on some land in the grade there may be reduced flexibility due to difficulties with the production of the more demanding crops such as winter harvested vegetables and arable root crops. The level of yield is generally high but may be lower or more variable than Grade 1.**
2	**On site**	**Grade 3**	**Good to moderate quality agricultural land. Land with moderate limitations which affect the choice of crops, timing and type of cultivation, harvesting or the level of yield. Where more demanding crops are grown yields are generally lower or more variable than on land in Grades 1 and 2.**

This data is sourced from Natural England.

12.2 Open Access Land

Records within 250m	**0**

The Countryside and Rights of Way Act 2000 (CROW Act) gives a public right of access to land without having to use paths. Access land includes mountains, moors, heaths and downs that are privately owned. It also includes common land registered with the local council and some land around the England Coast Path. Generally permitted activities on access land are walking, running, watching wildlife and climbing.

This data is sourced from Natural England and Natural Resources Wales.

12.3 Tree Felling Licences

Records within 250m	**0**

Felling Licence Application (FLA) areas approved by Forestry Commission England. Anyone wishing to fell trees must ensure that a licence or permission under a grant scheme has been issued by the Forestry Commission before any felling is carried out or that one of the exceptions apply.

This data is sourced from the Forestry Commission.

12.4 Environmental Stewardship Schemes

Records within 250m	**2**

Environmental Stewardship covers a range of schemes that provide financial incentives to farmers, foresters and land managers to look after and improve the environment. The schemes identified may be historical schemes that have now expired, or may still be active.

Location	Reference	Scheme	Start Date	End date
On site	**AG00289929**	**Entry Level plus Higher Level Stewardship**	**01/10/2009**	**30/09/2022**

Location	Reference	Scheme	Start Date	End date
18m S	AG00289929	Entry Level plus Higher Level Stewardship	01/10/2009	30/09/2022

This data is sourced from Natural England.

12.5 Countryside Stewardship Schemes

Records within 250m	**3**

Countryside Stewardship covers a range of schemes that provide financial incentives to farmers, foresters and land managers to look after and improve the environment. Main objectives are to improve the farmed environment for wildlife and to reduce diffuse water pollution.

Location	Reference	Scheme	Start Date	End Date
On site	**1266537**	**Countryside Stewardship (Middle Tier)**	**01/01/2022**	**31/12/2026**
On site	**1637514**	**Countryside Stewardship (Higher Tier)**	**01/10/2023**	**30/09/2026**
19m S	1266537	Countryside Stewardship (Middle Tier)	01/01/2022	31/12/2026

This data is sourced from Natural England.

13 Habitat designations

13.1 Priority Habitat Inventory

Records within 250m	0

Habitats of principal importance as named under Natural Environment and Rural Communities Act (2006) Section 41.

This data is sourced from Natural England.

13.2 Habitat Networks

Records within 250m	0

Habitat networks for 18 priority habitat networks (based primarily, but not exclusively, on the priority habitat inventory) and areas suitable for the expansion of networks through restoration and habitat creation.

This data is sourced from Natural England.

13.3 Open Mosaic Habitat

Records within 250m	0

Sites verified as Open Mosaic Habitat. Mosaic habitats are brownfield sites that are identified under the UK Biodiversity Action Plan as a priority habitat due to the habitat variation within a single site, supporting an array of invertebrates.

This data is sourced from Natural England.

13.4 Limestone Pavement Orders

Records within 250m	0

Limestone pavements are outcrops of limestone where the surface has been worn away by natural means over millennia. These rocks have the appearance of paving blocks, hence their name. Not only do they have geological interest, they also provide valuable habitats for wildlife. These habitats are threatened due to their removal for use in gardens and water features. Many limestone pavements have been designated as SSSIs which affords them some protection. In addition, Section 34 of the Wildlife and Countryside Act 1981 gave them additional protection via the creation of Limestone Pavement Orders, which made it a criminal offence to remove any part of the outcrop. The associated Limestone Pavement Priority Habitat is part of the UK Biodiversity Action Plan priority habitat in England.

This data is sourced from Natural England.

14 Geology 1:10,000 scale - Availability



Site Outline

Search buffers in metres (m)

Full coverage

Partial coverage

No coverage

14.1 10k Availability

Records within 500m **2**

An indication on the coverage of 1:10,000 scale geology data for the site, the most detailed dataset provided by the British Geological Survey. Either 'Full', 'Partial' or 'No coverage' for each geological theme.

Features are displayed on the Geology 1:10,000 scale - Availability map on **page 70 >**

ID	Location	Artificial	Superficial	Bedrock	Mass movement	Sheet No.
1	**On site**	**Full**	**Full**	**Full**	**No coverage**	**ST93SW**
2	418m W	Full	Full	Full	Full	ST83SE

This data is sourced from the British Geological Survey.

Geology 1:10,000 scale - Artificial and made ground

14.2 Artificial and made ground (10k)

Records within 500m	0

Details of made, worked, infilled, disturbed and landscaped ground at 1:10,000 scale. Artificial ground can be associated with potentially contaminated material, unpredictable engineering conditions and instability.

This data is sourced from the British Geological Survey.

Geology 1:10,000 scale - Superficial



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Site Outline

Search buffers in metres (m)

Landslip (10k)

Superficial geology (10k)
Please see table for more details.

14.3 Superficial geology (10k)

Records within 500m **2**

Superficial geological deposits at 1:10,000 scale. Also known as 'drift', these are the youngest geological deposits, formed during the Quaternary. They rest on older deposits or rocks referred to as bedrock.

Features are displayed on the Geology 1:10,000 scale - Superficial map on **page 72 >**

ID	Location	LEX Code	Description	Rock description
1	**On site**	**HEAD-XCZSV**	**Head - Clay, Silt, Sand And Gravel**	**Clay, Silt, Sand And Gravel**
2	443m W	HEAD-XCZSV	Head - Clay, Silt, Sand And Gravel	Clay, Silt, Sand And Gravel

This data is sourced from the British Geological Survey.

14.4 Landslip (10k)

Records within 500m	0

Mass movement deposits on BGS geological maps at 1:10,000 scale. Primarily superficial deposits that have moved down slope under gravity to form landslips. These affect bedrock, other superficial deposits and artificial ground.

This data is sourced from the British Geological Survey.

Geology 1:10,000 scale - Bedrock



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Site Outline

Search buffers in metres (m)

Bedrock faults and other linear features (10k)

Bedrock geology (10k)
Please see table for more details.

14.5 Bedrock geology (10k)

Records within 500m **2**

Bedrock geology at 1:10,000 scale. The main mass of rocks forming the Earth and present everywhere, whether exposed at the surface in outcrops or concealed beneath superficial deposits or water.

Features are displayed on the Geology 1:10,000 scale - Bedrock map on **page 74 >**

ID	Location	LEX Code	Description	Rock age
1	**On site**	**SECK-CHLK**	**Seaford Chalk Formation - Chalk**	**Santonian Age - Coniacian Age**
2	418m W	SECK-CHLK	Seaford Chalk Formation - Chalk	Santonian Age - Coniacian Age

This data is sourced from the British Geological Survey.

14.6 Bedrock faults and other linear features (10k)

Records within 500m	0

Linear features at the ground or bedrock surface at 1:10,000 scale of six main types; rock, fault, fold axis, mineral vein, alteration area or landform. Features are either observed or inferred, and relate primarily to bedrock.

This data is sourced from the British Geological Survey.

15 Geology 1:50,000 scale - Availability



Site Outline

Search buffers in metres (m)

Geological map tile

15.1 50k Availability

Records within 500m	1

An indication on the coverage of 1:50,000 scale geology data for the site. Either 'Full' or 'No coverage' for each geological theme.

Features are displayed on the Geology 1:50,000 scale - Availability map on **page 76 >**

ID	Location	Artificial	Superficial	Bedrock	Mass movement	Sheet No.
1	**On site**	**Full**	**Full**	**Full**	**Full**	**EW297_wincanton_v4**

This data is sourced from the British Geological Survey.

Geology 1:50,000 scale - Artificial and made ground

15.2 Artificial and made ground (50k)

Records within 500m	0

Details of made, worked, infilled, disturbed and landscaped ground at 1:50,000 scale. Artificial ground can be associated with potentially contaminated material, unpredictable engineering conditions and instability.

This data is sourced from the British Geological Survey.

15.3 Artificial ground permeability (50k)

Records within 50m	0

A qualitative classification of estimated rates of vertical movement of water from the ground surface through the unsaturated zone of any artificial deposits (the zone between the land surface and the water table).

This data is sourced from the British Geological Survey.

Geology 1:50,000 scale - Superficial



Site Outline

Search buffers in metres (m)

Landslip (50k)

Superficial geology (50k)
Please see table for more details.

15.4 Superficial geology (50k)

Records within 500m **1**

Superficial geological deposits at 1:50,000 scale. Also known as 'drift', these are the youngest geological deposits, formed during the Quaternary. They rest on older deposits or rocks referred to as bedrock.

Features are displayed on the Geology 1:50,000 scale - Superficial map on **page 78 >**

ID	Location	LEX Code	Description	Rock description
1	**On site**	**HEAD-XCZSV**	**HEAD**	**CLAY, SILT, SAND AND GRAVEL**

This data is sourced from the British Geological Survey.

15.5 Superficial permeability (50k)

Records within 50m	1

A qualitative classification of estimated rates of vertical movement of water from the ground surface through the unsaturated zone of any superficial deposits (the zone between the land surface and the water table).

Location	Flow type	Maximum permeability	Minimum permeability
On site	**Mixed**	**High**	**Very Low**

This data is sourced from the British Geological Survey.

15.6 Landslip (50k)

Records within 500m	0

Mass movement deposits on BGS geological maps at 1:50,000 scale. Primarily superficial deposits that have moved down slope under gravity to form landslips. These affect bedrock, other superficial deposits and artificial ground.

This data is sourced from the British Geological Survey.

15.7 Landslip permeability (50k)

Records within 50m	0

A qualitative classification of estimated rates of vertical movement of water from the ground surface through the unsaturated zone of any landslip deposits (the zone between the land surface and the water table).

This data is sourced from the British Geological Survey.

Geology 1:50,000 scale - Bedrock



Site Outline

Search buffers in metres (m)

Bedrock faults and other linear features (50k)

Bedrock geology (50k)
Please see table for more details.

15.8 Bedrock geology (50k)

Records within 500m **1**

Bedrock geology at 1:50,000 scale. The main mass of rocks forming the Earth and present everywhere, whether exposed at the surface in outcrops or concealed beneath superficial deposits or water.

Features are displayed on the Geology 1:50,000 scale - Bedrock map on **page 80 >**

ID	Location	LEX Code	Description	Rock age
1	**On site**	**SECK-CHLK**	**SEAFORD CHALK FORMATION - CHALK**	**CONIACIAN**

This data is sourced from the British Geological Survey.

15.9 Bedrock permeability (50k)

Records within 50m	1

A qualitative classification of estimated rates of vertical movement of water from the ground surface through the unsaturated zone of bedrock (the zone between the land surface and the water table).

Location	Flow type	Maximum permeability	Minimum permeability
On site	**Fracture**	**Very High**	**Very High**

This data is sourced from the British Geological Survey.

15.10 Bedrock faults and other linear features (50k)

Records within 500m	0

Linear features at the ground or bedrock surface at 1:50,000 scale of six main types; rock, fault, fold axis, mineral vein, alteration area or landform. Features are either observed or inferred, and relate primarily to bedrock.

This data is sourced from the British Geological Survey.

16 Boreholes

16.1 BGS Boreholes

Records within 250m	0

The Single Onshore Boreholes Index (SOBI); an index of over one million records of boreholes, shafts and wells from all forms of drilling and site investigation work held by the British Geological Survey. Covering onshore and nearshore boreholes dating back to at least 1790 and ranging from one to several thousand metres deep.

This data is sourced from the British Geological Survey.

17 Natural ground subsidence - Shrink swell clays



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17.1 Shrink swell clays

Records within 50m	3

The potential hazard presented by soils that absorb water when wet (making them swell), and lose water as they dry (making them shrink). This shrink-swell behaviour is controlled by the type and amount of clay in the soil, and by seasonal changes in the soil moisture content (related to rainfall and local drainage).

Features are displayed on the Natural ground subsidence - Shrink swell clays map on **page 83 >**

Location	Hazard rating	Details
On site	**Negligible**	**Ground conditions predominantly non-plastic.**
On site	**Very low**	**Ground conditions predominantly low plasticity.**
23m SE	Negligible	Ground conditions predominantly non-plastic.

This data is sourced from the British Geological Survey.

Natural ground subsidence - Running sands



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17.2 Running sands

Records within 50m	3

The potential hazard presented by rocks that can contain loosely-packed sandy layers that can become fluidised by water flowing through them. Such sands can 'run', removing support from overlying buildings and causing potential damage.

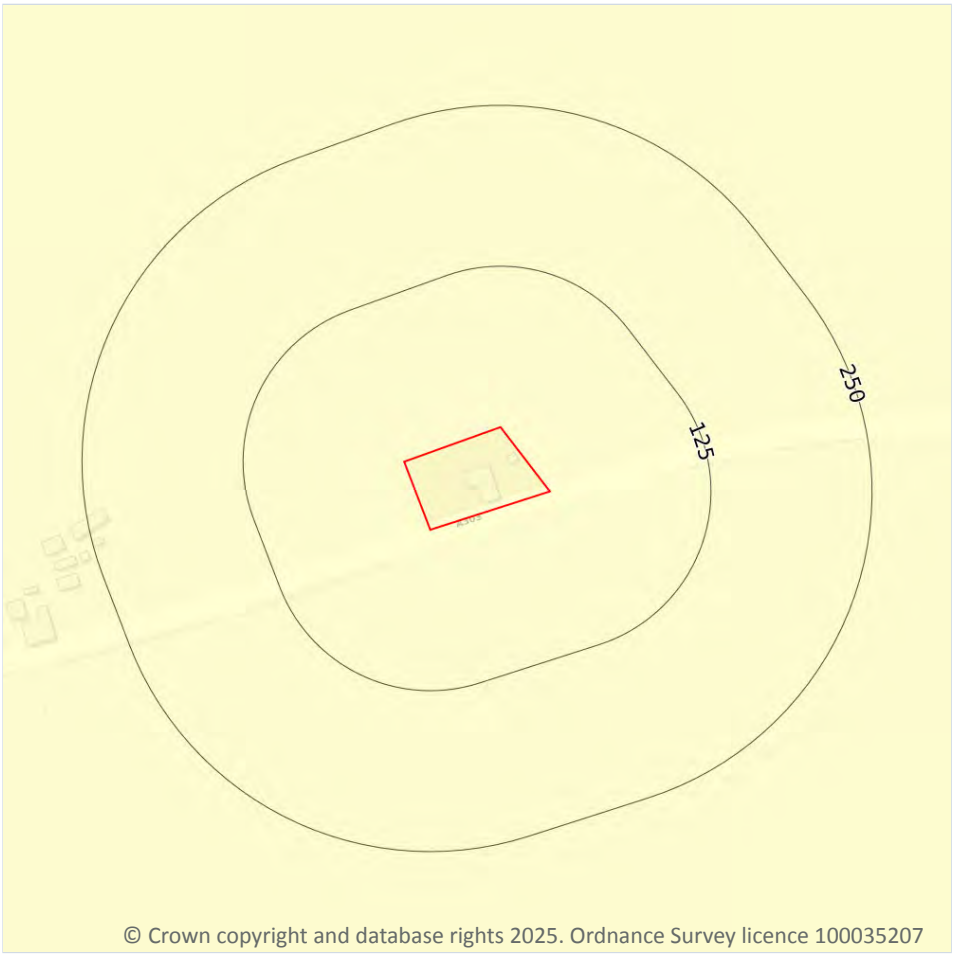
Features are displayed on the Natural ground subsidence - Running sands map on **page 85 >**

Location	Hazard rating	Details
On site	**Negligible**	**Running sand conditions are not thought to occur whatever the position of the water table. No identified constraints on lands use due to running conditions.**

Location	Hazard rating	Details
On site	**Very low**	**Running sand conditions are unlikely. No identified constraints on land use due to running conditions unless water table rises rapidly.**
23m SE	Negligible	Running sand conditions are not thought to occur whatever the position of the water table. No identified constraints on lands use due to running conditions.

This data is sourced from the British Geological Survey.

Natural ground subsidence - Compressible deposits



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Site Outline

Search buffers in metres (m)

- No data
- Negligible
- Very low
- Low
- Moderate
- High

17.3 Compressible deposits

Records within 50m	1

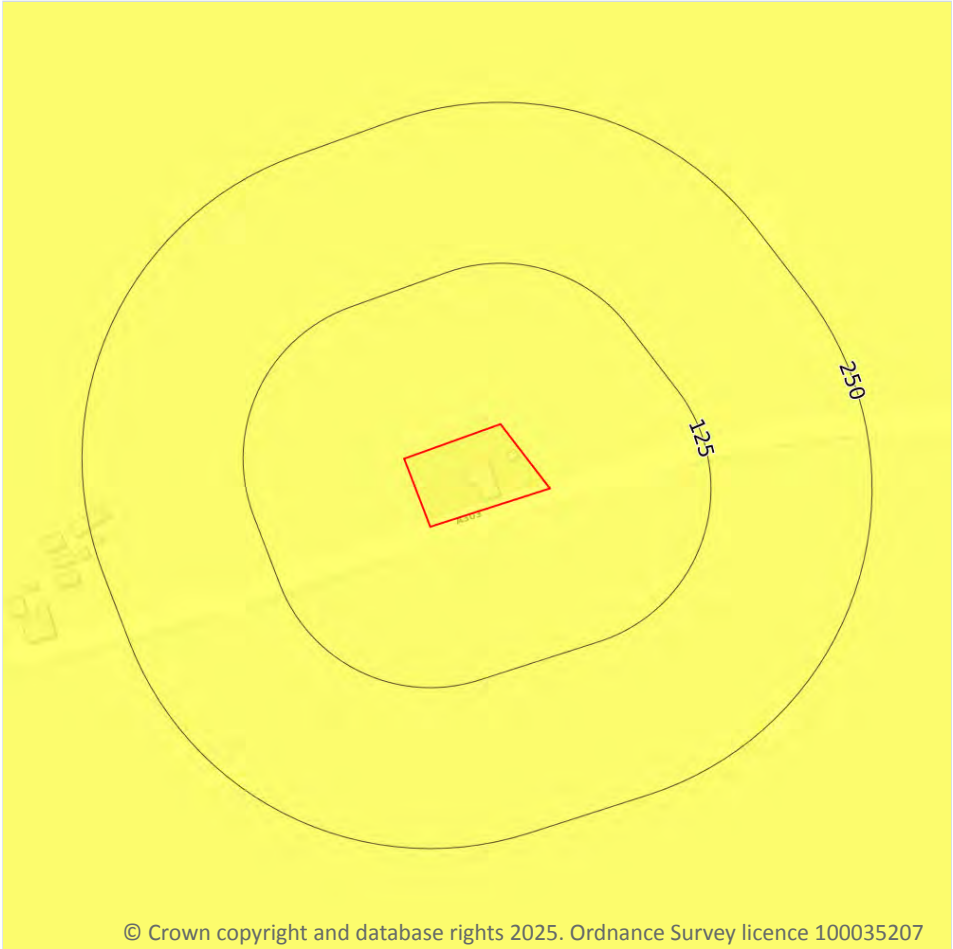
The potential hazard presented by types of ground that may contain layers of very soft materials like clay or peat and may compress if loaded by overlying structures, or if the groundwater level changes, potentially resulting in depression of the ground and disturbance of foundations.

Features are displayed on the Natural ground subsidence - Compressible deposits map on **page 87 >**

Location	Hazard rating	Details
On site	**Negligible**	**Compressible strata are not thought to occur.**

This data is sourced from the British Geological Survey.

Natural ground subsidence - Collapsible deposits



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Site Outline

Search buffers in metres (m)

- No data
- Negligible
- Very low
- Low
- Moderate
- High

17.4 Collapsible deposits

Records within 50m	1

The potential hazard presented by natural deposits that could collapse when a load (such as a building) is placed on them or they become saturated with water.

Features are displayed on the Natural ground subsidence - Collapsible deposits map on **page 88 >**

Location	Hazard rating	Details
On site	**Very low**	**Deposits with potential to collapse when loaded and saturated are unlikely to be present.**

This data is sourced from the British Geological Survey.

Natural ground subsidence - Landslides



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Site Outline

Search buffers in metres (m)

- No data
- Negligible
- Very low
- Low
- Moderate
- High

17.5 Landslides

Records within 50m	4

The potential for landsliding (slope instability) to be a hazard assessed using 1:50,000 scale digital maps of superficial and bedrock deposits, combined with information from the BGS National Landslide Database and scientific and engineering reports.

Features are displayed on the Natural ground subsidence - Landslides map on **page 89 >**

Location	Hazard rating	Details
On site	**Negligible**	**Slope instability problems are not thought to occur but consideration to potential problems of adjacent areas impacting on the site should always be considered.**

Location	Hazard rating	Details
On site	**Very low**	**Slope instability problems are not likely to occur but consideration to potential problems of adjacent areas impacting on the site should always be considered.**
23m SE	Negligible	Slope instability problems are not thought to occur but consideration to potential problems of adjacent areas impacting on the site should always be considered.
41m NE	Low	Slope instability problems may be present or anticipated. Site investigation should consider specifically the slope stability of the site.

This data is sourced from the British Geological Survey.

Natural ground subsidence - Ground dissolution of soluble rocks



17.6 Ground dissolution of soluble rocks

Records within 50m	3

The potential hazard presented by ground dissolution, which occurs when water passing through soluble rocks produces underground cavities and cave systems. These cavities reduce support to the ground above and can cause localised collapse of the overlying rocks and deposits.

Features are displayed on the Natural ground subsidence - Ground dissolution of soluble rocks map on **page 91 >**

Location	Hazard rating	Details
On site	**Very low**	**Soluble rocks are present within the ground. Few dissolution features are likely to be present. Potential for difficult ground conditions or localised subsidence are at a level where they need not be considered.**

Location	Hazard rating	Details
On site	**Low**	**Soluble rocks are present within the ground. Some dissolution features may be present. Potential for difficult ground conditions are at a level where they may be considered, localised subsidence need not be considered except in exceptional circumstances.**
23m SE	Very low	Soluble rocks are present within the ground. Few dissolution features are likely to be present. Potential for difficult ground conditions or localised subsidence are at a level where they need not be considered.

This data is sourced from the British Geological Survey.

18 Mining and ground workings



Site Outline

Search buffers in metres (m)

BritPits

Surface ground workings

Underground workings

Underground mining extents

Historical mineral planning areas

TCA non-coal mining

Non Coal Mining

Sporadic underground mining of restricted extent possible

Localised small scale underground mining possible

Small scale mining possible

Underground mining known or likely within or in close proximity

Underground mining known within or in very close proximity

18.1 BritPits

Records within 500m	0

BritPits (an abbreviation of British Pits) is a database maintained by the British Geological Survey of currently active and closed surface and underground mineral workings. Details of major mineral handling sites, such as wharfs and rail depots are also held in the database.

This data is sourced from the British Geological Survey.

18.2 Surface ground workings

Records within 250m	0

Historical land uses identified from Ordnance Survey mapping that involved ground excavation at the surface. These features may or may not have been subsequently backfilled.

This is data is sourced from Ordnance Survey/Groundsure.

18.3 Underground workings

Records within 1000m	0

Historical land uses identified from Ordnance Survey mapping that indicate the presence of underground workings e.g. mine shafts.

This is data is sourced from Ordnance Survey/Groundsure.

18.4 Underground mining extents

Records within 500m	0

This data identifies underground mine workings that could present a potential risk, including adits and seam workings. These features have been identified from BGS Geological mapping and mine plans sourced from the BGS and various collections and sources.

This data is sourced from Groundsure.

18.5 Historical Mineral Planning Areas

Records within 500m	0

Boundaries of mineral planning permissions for England and Wales. This data was collated between the 1940s (and retrospectively to the 1930s) and the mid 1980s. The data includes permitted, withdrawn and refused permissions.

This data is sourced from the British Geological Survey.

18.6 Non-coal mining

Records within 1000m	4

The potential for historical non-coal mining to have affected an area. The assessment is drawn from expert knowledge and literature in addition to the digital geological map of Britain. Mineral commodities may be divided into seven general categories - vein minerals, chalk, oil shale, building stone, bedded ores, evaporites and 'other' commodities (including ball clay, jet, black marble, graphite and chert).

Features are displayed on the Mining and ground workings map on **page 93 >**

ID	Location	Name	Commodity	Class	Likelihood
1	**On site**	**Not available**	**Chalk**	**A**	**Underground mine workings are uncommon, although the geology is similar to that worked elsewhere. Potential for difficult ground conditions are unlikely and are at a level where they need not be considered.**
2	418m W	Not available	Chalk	A	Underground mine workings are uncommon, although the geology is similar to that worked elsewhere. Potential for difficult ground conditions are unlikely and are at a level where they need not be considered.
3	500m N	Not available	Chalk	A	Underground mine workings are uncommon, although the geology is similar to that worked elsewhere. Potential for difficult ground conditions are unlikely and are at a level where they need not be considered.
6	673m NW	Not available	Chalk	A	Underground mine workings are uncommon, although the geology is similar to that worked elsewhere. Potential for difficult ground conditions are unlikely and are at a level where they need not be considered.

This data is sourced from the British Geological Survey.

18.7 JPB mining areas

Records on site	**0**

Areas which could be affected by former coal and other mining. This data includes some mine plans unavailable to the Coal Authority.

This data is sourced from Johnson Poole and Bloomer.

18.8 The Coal Authority non-coal mining

Records within 500m	**0**

This data provides an indication of the potential zone of influence of recorded underground non-coal mining workings. Any and all analysis and interpretation of Coal Authority Data in this report is made by Groundsure, and is in no way supported, endorsed or authorised by the Coal Authority. The use of the data is restricted to the terms and provisions contained in this report. Data reproduced in this report may be the copyright of the Coal Authority and permission should be sought from Groundsure prior to any re-use.

This data is sourced from The Coal Authority.

18.9 Researched mining

Records within 500m	0

This data indicates areas of potential mining identified from alternative or archival sources, including; BGS Geological paper maps, Lidar data, aerial photographs (from World War II onwards), archaeological data services, websites, Tithe maps, and various text/plans from collected books and reports. Some of this data is approximate and Groundsure have interpreted the resultant risk area and, where possible, specific areas of risk have been captured.

This data is sourced from Groundsure.

18.10 Mining record office plans

Records within 500m	0

This dataset is representative of Mining Record Office and/or plan extents held by Groundsure and should be considered approximate. Where possible, plans have been located and any specific areas of risk they depict have been captured.

This data is sourced from Groundsure.

18.11 BGS mine plans

Records within 500m	0

This dataset is representative of BGS mine plans held by Groundsure and should be considered approximate. Where possible, plans have been located and any specific areas of risk they depict have been captured.

This data is sourced from Groundsure.

18.12 Coal mining

Records on site	0

Areas which could be affected by past, current or future coal mining.

This data is sourced from the Coal Authority.

18.13 Brine areas

Records on site	0

The Cheshire Brine Compensation District indicates areas that may be affected by salt and brine extraction in Cheshire and where compensation would be available where damage from this mining has occurred. Damage from salt and brine mining can still occur outside this district, but no compensation will be available.

This data is sourced from the Cheshire Brine Subsidence Compensation Board.

18.14 Gypsum areas

Records on site	0

Generalised areas that may be affected by gypsum extraction.

This data is sourced from British Gypsum.

18.15 Tin mining

Records on site	0

Generalised areas that may be affected by historical tin mining.

This data is sourced from Groundsure.

18.16 Clay mining

Records on site	0

Generalised areas that may be affected by kaolin and ball clay extraction.

This data is sourced from the Kaolin and Ball Clay Association (UK).

19 Ground cavities and sinkholes

19.1 Natural cavities

Records within 500m	0

Industry recognised national database of natural cavities. Sinkholes and caves are formed by the dissolution of soluble rock, such as chalk and limestone, gulls and fissures by cambering. Ground instability can result from movement of loose material contained within these cavities, often triggered by water.

This data is sourced from Stantec UK Ltd.

19.2 Mining cavities

Records within 1000m	0

Industry recognised national database of mining cavities. Degraded mines may result in hazardous subsidence (crown holes). Climatic conditions and water escape can also trigger subsidence over mine entrances and workings.

This data is sourced from Stantec UK Ltd.

19.3 Reported recent incidents

Records within 500m	0

This data identifies sinkhole information gathered from media reports and Groundsure's own records. This data goes back to 2014 and includes relative accuracy ratings for each event and links to the original data sources. The data is updated on a regular basis and should not be considered a comprehensive catalogue of all sinkhole events. The absence of data in this database does not mean a sinkhole definitely has not occurred during this time.

This data is sourced from Groundsure.

19.4 Historical incidents

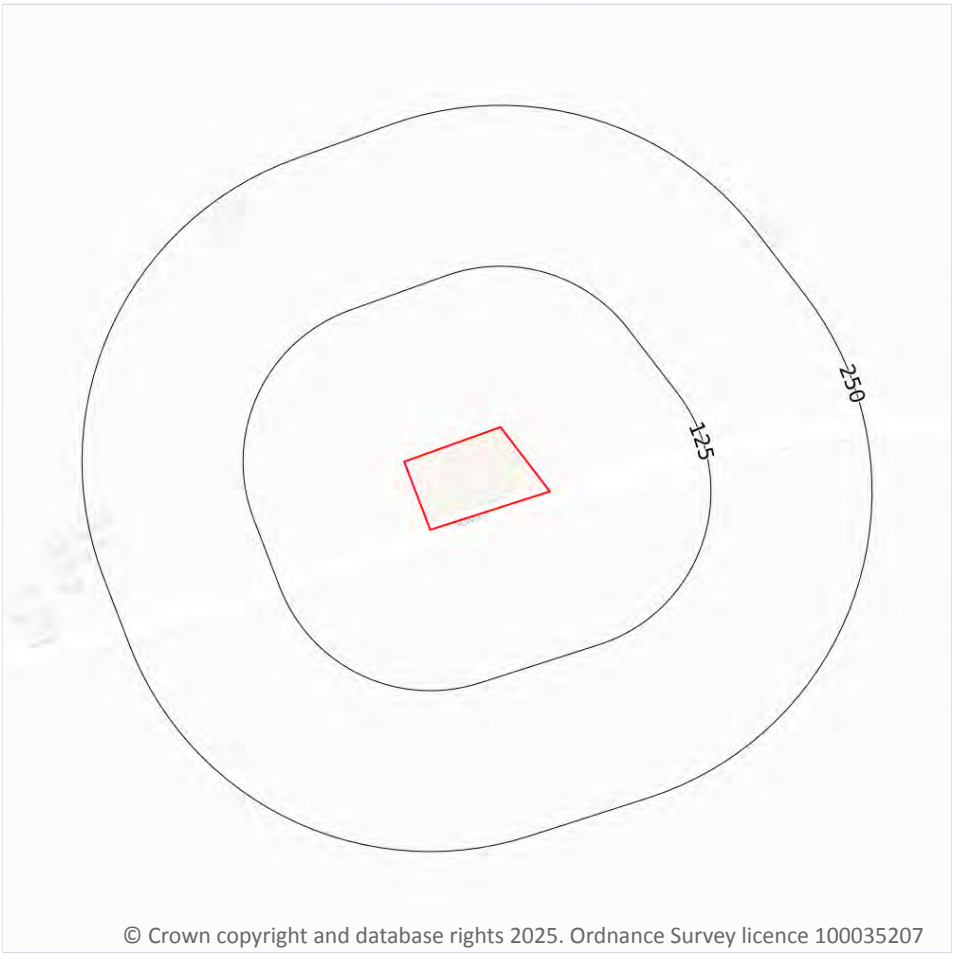
Records within 500m	0

This dataset comprises an extract of 1:10,560, 1:10,000, 1:2,500 and 1:1,250 scale historical Ordnance Survey maps held by Groundsure, dating back to the 1840s. It shows shakeholes, deneholes and other 'holes' as noted on these maps. Dene holes are medieval chalk extraction pits, usually comprising a narrow shaft with a number of chambers at the base of the shaft. Shakeholes are an alternative name for suffusion sinkholes, most commonly found in the limestone landscapes of North Yorkshire but also extensively noted around the Brecon Beacons National Park.

Not all 'holes' noted on Ordnance Survey mapping will necessarily be present within this dataset.

This data is sourced from Groundsure.

20 Radon



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Site Outline

Search buffers in metres (m)

- Greater than 30%
- Between 10% and 30%
- Between 5% and 10%
- Between 3% and 5%
- Between 1% and 3%
- Less than 1%

20.1 Radon

Records on site	1

The Radon Potential data classifies areas based on their likelihood of a property having a radon level at or above the Action Level in Great Britain. The dataset is intended for use at 1:50,000 scale and was derived from both geological assessments and indoor radon measurements (more than 560,000 records). A minimum 50m buffer should be considered when searching the maps, as the smallest detectable feature at this scale is 50m. The findings of this section should supersede any estimations derived from the Indicative Atlas of Radon in Great Britain (1:100,000 scale).

Features are displayed on the Radon map on **page 100 >**

Location	Estimated properties affected	Radon Protection Measures required
On site	**Less than 1%**	**None**

This data is sourced from the British Geological Survey and UK Health Security Agency.

21 Soil chemistry

21.1 BGS Estimated Background Soil Chemistry

Records within 50m	8

The estimated values provide the likely background concentration of the potentially harmful elements Arsenic, Cadmium, Chromium, Lead and Nickel in topsoil. The values are estimated primarily from rural topsoil data collected at a sample density of approximately 1 per 2 km^2. In areas where rural soil samples are not available, estimation is based on stream sediment data collected from small streams at a sampling density of 1 per 2.5 km^2; this is the case for most of Scotland, Wales and southern England. The stream sediment data are converted to soil-equivalent concentrations prior to the estimation.

Location	Arsenic	Bioaccessible Arsenic	Lead	Bioaccessible Lead	Cadmium	Chromium	Nickel
On site	**15 mg/kg**	**No data**	**100 mg/kg**	**60 mg/kg**	**1.8 mg/kg**	**60 - 90 mg/kg**	**15 mg/kg**
On site	**15 mg/kg**	**No data**	**100 mg/kg**	**60 mg/kg**	**1.8 mg/kg**	**60 - 90 mg/kg**	**15 mg/kg**
On site	**15 mg/kg**	**No data**	**100 mg/kg**	**60 mg/kg**	**1.8 mg/kg**	**60 - 90 mg/kg**	**15 - 30 mg/kg**
On site	**15 mg/kg**	**No data**	**100 mg/kg**	**60 mg/kg**	**1.8 mg/kg**	**60 - 90 mg/kg**	**15 - 30 mg/kg**
On site	**15 mg/kg**	**No data**	**100 mg/kg**	**60 mg/kg**	**1.8 mg/kg**	**60 - 90 mg/kg**	**15 mg/kg**
6m NE	15 mg/kg	No data	100 mg/kg	60 mg/kg	1.8 mg/kg	60 - 90 mg/kg	15 mg/kg
23m SE	15 mg/kg	No data	100 mg/kg	60 mg/kg	1.8 mg/kg	60 - 90 mg/kg	15 mg/kg
28m S	15 mg/kg	No data	100 mg/kg	60 mg/kg	1.8 mg/kg	60 - 90 mg/kg	15 mg/kg

This data is sourced from the British Geological Survey.

21.2 BGS Estimated Urban Soil Chemistry

Records within 50m	0

Estimated topsoil chemistry of Arsenic, Cadmium, Chromium, Copper, Nickel, Lead, Tin and Zinc and bioaccessible Arsenic and Lead in 23 urban centres across Great Britain. These estimates are derived from interpolation of the measured urban topsoil data referred to above and provide information across each city between the measured sample locations (4 per km^2).

This data is sourced from the British Geological Survey.

21.3 BGS Measured Urban Soil Chemistry

Records within 50m	0

The locations and measured total concentrations (mg/kg) of Arsenic, Cadmium, Chromium, Copper, Nickel, Lead, Tin and Zinc in urban topsoil samples from 23 urban centres across Great Britain. These are collected at a sample density of 4 per km^2.

This data is sourced from the British Geological Survey.

22 Railway infrastructure and projects

22.1 Underground railways (London)

Records within 250m	0

Details of all active London Underground lines, including approximate tunnel roof depth and operational hours.

This data is sourced from publicly available information by Groundsure.

22.2 Underground railways (Non-London)

Records within 250m	0

Details of the Merseyrail system, the Tyne and Wear Metro and the Glasgow Subway. Not all parts of all systems are located underground. The data contains location information only and does not include a depth assessment.

This data is sourced from publicly available information by Groundsure.

22.3 Railway tunnels

Records within 250m	0

Railway tunnels taken from contemporary Ordnance Survey mapping.

This data is sourced from the Ordnance Survey.

22.4 Historical railway and tunnel features

Records within 250m	0

Railways and tunnels digitised from historical Ordnance Survey mapping as scales of 1:1,250, 1:2,500, 1:10,000 and 1:10,560.

This data is sourced from Ordnance Survey/Groundsure.

22.5 Royal Mail tunnels

Records within 250m	0

The Post Office Railway, otherwise known as the Mail Rail, is an underground railway running through Central London from Paddington Head District Sorting Office to Whitechapel Eastern Head Sorting Office. The line is 10.5km long. The data includes details of the full extent of the tunnels, the depth of the tunnel, and the depth to track level.

This data is sourced from Groundsure/the Postal Museum.

22.6 Historical railways

Records within 250m	0

Former railway lines, including dismantled lines, abandoned lines, disused lines, historic railways and razed lines.

This data is sourced from OpenStreetMap.

22.7 Railways

Records within 250m	0

Currently existing railway lines, including standard railways, narrow gauge, funicular, trams and light railways.

This data is sourced from Ordnance Survey and OpenStreetMap.

22.8 Crossrail 2

Records within 500m	0

Crossrail 2 is a proposed railway linking the national rail networks in Surrey and Hertfordshire via an underground tunnel through London.

This data is sourced from publicly available information by Groundsure.

22.9 HS2

Records within 500m	0

HS2 is a proposed high speed rail network running from London to Manchester and Leeds via Birmingham. Main civils construction on Phase 1 (London to Birmingham) of the project began in 2019, and it is currently anticipated that this phase will be fully operational by 2026. Construction on Phase 2a (Birmingham to Crewe) is anticipated to commence in 2021, with the service fully operational by 2027. Construction on Phase 2b (Crewe to Manchester and Birmingham to Leeds) is scheduled to begin in 2023 and be operational by 2033.

This data is sourced from HS2 ltd.

Data providers

Groundsure works with respected data providers to bring you the most relevant and accurate information. To find out who they are and their areas of expertise see https://www.groundsure.com/sources-reference ↗.

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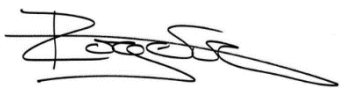
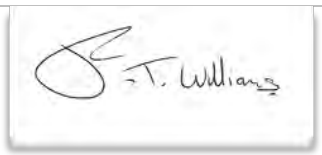
geotechnical and environmental consultants

ground&water

Phase II Site Investigation Report

Chicklade Service Station, Hindon, Salisbury SP3 5ST

On behalf of Instavolt

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v.2.01			
	Roger Foord BA (Hons) MSc DIC CGeol FGS CIHM Senior Engineer	Simon Harlow BSc (Hons) FGS Principal Engineer	Francis Williams MGeol (Hons) FGS CEnv CGeol Director

Site Investigations | Environmental Consultants | Geotechnical Engineers

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

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EXECUTIVE SUMMARY	
PROPOSED DEVELOPMENT	At the time of reporting, November 2025, the proposed development was understood to comprise the demolition of the existing service station (comprising a petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with landscaping, revised vehicular access and formation of a new pedestrian link. The proposed development plan can be seen within Figure 3.
GEOLOGY AND HYDROGEOLOGY	The southern portion of the site was underlain by the Head Deposits (Secondary A Aquifer) and the entire area of the site was underlain by bedrock deposits of the Seaford Chalk Formation (Principal Aquifer). No areas of Made Ground or Reworked Ground were noted within a 250m radius. Examination of the Environment Agency records showed that the site was located within a Total Catchment (Zone III) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater. An Inner Zone (Zone I) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater was located ~3km to the east of the site. Groundwater anticipated to be encountered at depth within the Seaford Chalk Formation. Some amounts of groundwater however may be present at shallower depths within any Made ground and silty/sandy pockets of the Head Deposits. It was considered that groundwater was flowing in an easterly direction in line with local topography.
VOLUME CHANGE POTENTIAL	The Head Deposits were shown to have a medium to high volume change potential in accordance with both BRE240 and NHBC Standards Chapter 4.2.
CHALK DRY DENSITY	The geotechnical laboratory testing indicated the chalk clasts within the boreholes to be of medium to high density, with a dry density of between 1.55 – 1.71mg/m^3. The Seaford Chalk Formation was anticipated to have no volume change potential in accordance with CIRIA C574.
FOUNDATION DESIGN	Foundations constructed on the Head Deposits can be designed based on a general bearing capacity of **80kN/m^2** for foundations within the Head Deposits. Foundations constructed on the Seaford Chalk Formation can be designed based on a general bearing capacity of **125kN/m^2** for foundations within the chalk. If the formation level proves to be competent Dc chalk, a higher bearing capacity reaching 225kN/m^2 might be achievable.
GROUND BEARING SLABS	Ground bearing slabs should be able to be adopted for the proposed structure. Where possible the any remaining Made Ground should be removed from below the slab and the formation level should be proof rolled and any soft spots removed and infilled with a suitably compacted granular material. Site levels should be brought up to underside of proposed slab level using with suitable granular soil (Type I or Type II) rolled in thin layers. The slabs must be reinforced and cast in small bays with dowels cast at the interface between each of the bays to reduce the risk of differential settlement affecting the slab.
SUB-SURFACE CONCRETE	According to BRE Special Digest 1, 2005, 'Concrete in Aggressive Ground' a Sulphate Design Class of DS-**1** could be used for sub-surface concrete in contact with the soils beneath the site. Table C1 of the Digest indicated an ACEC

	(Aggressive Chemical Environment for Concrete) classification of AC-**1**.
CONTAMINATION AND REMEDIATION	Due to the nature of the contamination encountered at the site and the proposed end use, it is not considered that there is an unacceptable risk to future human health receptors and no remediation is required as part of any site development at the present time. Given the historical land-use of the site, the presence of tanks and other underground infrastructure of the re-fuelling station, gross contamination may be encountered during the removal of these features. A discovery strategy must be in place. Should contamination be encountered during and following removal of the USTs and the septic tank or should the proposed development change then these conclusions would need to be re-evaluated following additional testing. Elevated concentrations of zinc were noted in one sample of Made Ground tested above the phytotoxicity threshold triggers value. Consequently, specialist planting may be required and the advice of a suitably experienced horticulturalist should be obtained. No asbestos has been identified during the testing, however, given the age of the structures on-site, an **Asbestos Management Strategy** should be put in place so that any potentially asbestos containing materials are identified and removed from site in a suitable manner to prevent cross-contamination. An Asbestos Survey should be undertaken pre-demolition of any buildings/structures. **This is subject to review following removal of the USTs.**
GROUND GAS	Based on soil analysis, the site falls into the CS1 classification.

1. INTRODUCTION

1.1. General

Ground and Water Limited were instructed by Instavolt on the 26th September 2025 to conduct a Phase II Ground Investigation on the Chicklade Service Station, Hindon, Salisbury SP3 5ST. The scope of the investigation was detailed within the fee proposal QU-1868, dated 20th August 2025.

1.2. Aims of the Investigation

The aim of this report was to supply the client and their designers with information regarding the ground conditions underlying the site to assist them in preparing an appropriate scheme for development.

The investigation was to be undertaken to provide parameters for the design of foundations by means of in-situ and laboratory geotechnical testing undertaken on soil samples recovered from trial holes.

A contamination assessment was to be undertaken following chemical laboratory testing on soil samples recovered from the site to enable recommendations for the safe redevelopment of the site and the protection of site workers, end-users and the public from any potential contamination identified. The assessment comprises a Tier 2 Generic Quantitative Risk Assessment, under Land Contamination Risk Management (LCRM), with the intention of discharging relevant planning conditions.

This Site Investigation Report must be read in conjunction with the Desk Study Report referenced GWPR6751/DS/November 2025.

Reference has been made to Subadra Remediation Strategy Report referenced IN20485 CL 003, dated July 2022 which summarised the results of the previous desk study and ground investigation (Subadra Consulting Ltd. Environmental Investigation Report Reference IN20485 CL 001, dated December 2020). Ground and Water Limited cannot be held liable for any shortcomings.

The techniques adopted for the investigation were chosen considering the requirements of the client, anticipated ground conditions, and bearing in mind the nature of the site, limitations to site access and other logistical limitations.

1.3. Conditions and Limitations

This report has been prepared based on the terms, conditions and limitations outlined within Appendix A.

Reference has been made to Subadra Remediation Strategy Report referenced IN20485 CL 003, dated July 2022 which summarised the results of the previous desk study and ground investigation (Subadra Consulting Ltd. Environmental Investigation Report Reference IN20485 CL 001, dated December 2020). Ground and Water Limited cannot be held liable for any shortcomings.

Following comments made by the Consulting Engineers on bearing capacity of the Head Deposits and the potential adoption of a ground bearing slab, this report has been updated and supersedes the Phase II Ground Investigation Report produced by Ground and Water Limited reference GWPR6751/SIR/November 2025 v1.01. The Proposed Development Plan in Figure 2 has also been replaced.

1.4. Technical Glossary

Generic technical terms and their description can be viewed within the glossary provided within Appendix B.

2. SITE SETTING

2.1. Site Location

The site comprised a ~5,200m^2 rectangular-shaped plot of land, orientated in an east to west direction, along the northern side of the A303. The site was located to the west of the village of Chicklade, Wiltshire.

The Easting/Northing for the centre of the site was approximately 090470, 034459. A site location plan is given within Figure 1.

2.2. Site Description

A Site Walkover was undertaken on the 29th September 2025. A description of the site, as noted during the Site Walkover, is tabulated below.

Site Description Sheet	
Use of site	At the time of the Site Walkover, 29th September 2025, the site was occupied by a closed petrol filling station.
Site topography	The site was sloping gently towards the south. The two car parking areas in the western portion of the site had been terraced into the existing slope.
Area topography	The area topography is a shallow sided valley where the site is located on the northern edge of the valley. The A303 runs along the base of the valley.
Structures on-site	The centre of the site was occupied by the buildings of the former filling station, café/bakery and shop. The canopy and former pump islands were located between the building and the southern boundary. A detached former toilet block was located on the eastern boundary with an adjacent partially submerged septic tank. It is understood that up to 6No. Underground fuel tanks were located in the south-eastern corner of the site.
Structures off-site	None in a close proximity.
Use of surrounding ground	Farmland to the north-east and west and the A303 to the immediate south with farmland beyond.
Boundary features	Northern Boundary: ~0.80m post and wire fence. Eastern Boundary: ~0.80m post and wire fence. Southern Boundary: open. Western Boundary: ~0.80m post and wire fence.
Site covering	The majority of the site was covered with tarmac and concrete hardstanding with areas soft landscaping.
Contamination sources on-site	Underground fuel tanks; Pump islands and associated infrastructure. Partially submerged septic tank. ACMs in existing buildings
Contamination sources off-site	None noted
Vegetation on-site	Small hedges along boundaries of the site.
Vegetation off-site	Semi-mature and mature ~2 – 12m high trees were noted on the southern side of the A303.
Services	Numerous services noted.

2.3. Proposed Development

At the time of reporting, November 2025, the proposed development was understood to comprise the demolition of the existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with soft

landscaping, revised vehicular access and formation of a new pedestrian link. The proposed development plan can be seen within Figure 2.

2.4. Geology

The British Geological Survey Solid and Drift Geology Map for the Salisbury area revealed that the site was underlain by superficial Head Deposits and bedrock deposits of the Seaford Chalk Formation. No areas of Made Ground or Reworked Ground were noted within a 250m radius.

There were no BGS borehole records within a 500m radius of the site.

2.5. Hydrogeology and Hydrology

A study of the aquifer maps on the DEFRA website revealed the site was underlain by a **Secondary A Aquifer** comprising the superficial Head Deposits, underlain by a **Principal Aquifer** comprising bedrock deposits of the Seaford Chalk Formation.

Examination of the Environment Agency records showed that the site was located within a Total Catchment (Zone III) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater. An Inner Zone (Zone I) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater was located ~3km to the east of the site.

The nearest surface water feature was an unnamed stream 37m to the south-west and flowing in an easterly direction.

From analysis of hydrogeological and topographical maps the actual groundwater table was anticipated to be encountered at depth within the Seaford Chalk Formation. Some amounts of groundwater however may be present at shallower depths within any Made Ground and silty/sandy pockets of the Head Deposits. It was considered that groundwater was flowing in an easterly direction in line with local topography.

Examination of the Environment Agency records showed that the site was located within a Flood Zone 1, i.e. an area with a very low probability of flooding.

The Groundsure Datasheets revealed the site was at no risk of flooding from rivers and the sea. The datasheets also revealed the southern portion of the site was at risk of a high risk of groundwater flooding, while the northern portion had a low risk. The southern portion of the site had a 1 in 30 year period of surface water flooding greater than 1.0m in depth.

2.6. Radon

A review of the freely available UK Health Security Agency radon database, UK Radon, indicated that the site was located within a 1km grid square, where the maximum radon potential of <1% was recorded for above ground structures. Basic radon protection measures are required in areas where more than 3% of houses are at or above the Action Level.

The site was in an area where a risk assessment was not required.

2.7. Internet Search (Site Setting)

2.7.1. Available Unexploded Ordnance (UXO) Map Review

A review of the data available on www.zeticauxo.com/ revealed the site was located within a low-risk area associated with unexploded ordnance (UXO), as indicated by having <15 bombs dropped per 1000 acres. No known Luftwaffe Bombing Target sites were located within a close proximity of the site.

2.7.2. Historical Landfill Tool Review

A review of the data available on www.groundsure.io/ revealed no areas of Historical Landfilling within a close proximity of the site.

2.8. Previous Ground Investigation

The results of the previous desk study and ground investigation (Subadra Consulting Ltd. Environmental Investigation Report Reference IN20485 CL 001, dated December 2020) were summarised in the Subadra Remediation Strategy Report referenced IN20485 CL 003, dated July 2022.

The main features of the site history were:

- The site had been a petrol filling station since at least the early 1980s (and most likely since the early 1960s). The below ground petroleum installation is the primary potential source of (hydrocarbon) contamination at the site. The current tanks were reported to have been installed in 1983 and were relined (with interstitial monitoring) in 2013. Plastic fuel lines were installed, also in 2013.

The key findings of the intrusive investigation were:

- No Made Ground was encountered.
- Soils described as Head Deposits comprising a brown slightly sandy clay with gravel was encountered overlying the Seaford Chalk Formation comprising a weak weathered putty chalk to a depth of 6.0m bgl.
- Groundwater was not encountered.

The key findings of the geo-environmental assessment were:

- During the intrusive investigation no olfactory or visual signs of contamination were noted in any of the soil samples collected. Very low concentrations of hydrocarbon vapours were recorded in a small number of samples, predominantly from depths <2.0m.
- The results of the chemical analysis carried out on selected soil samples revealed the presence of very low concentrations of xylenes in two samples (BH003 at 0.8 and 1.8m) located at the south-eastern corner of the central forecourt area. Very low concentrations of two PAH compounds were also recorded in a single sample (BH005 at 0.8m), located in the south-east corner of the site.

2.9. Preliminary Risk Assessment

The tabulated Conceptual Site Model developed as part of the Ground and Water Ltd Desk Study (GWPR6751/DS/November 2025 is outlined overleaf. For ease of reference and understanding, the

risks have been classified within this risk assessment against four possible levels / categories, summarised in the table below. Any risk deemed to be negligible within the Preliminary Risk Assessment have been excluded from this report.

Risk Categories used in the Tabulated CSM	
Low Risk	Regarding this potential SPR linkage, the site is considered suitable for the proposed end-use and there is not considered to be an unacceptable risk to receptors. However, it is considered that further investigation to confirm this is recommended.
Moderate Risk	Regarding this potential SPR linkage, the site may not be suitable for the proposed end-use in its current condition and there may be an unacceptable risk to receptors. Further investigation is required to confirm this.
High Risk	Regarding this potential SPR linkage, the site is probably or certainly not suitable for proposed end-use and there is likely to be an unacceptable risk to receptors. Contaminants probably or certainly present and urgent action required in the short term.

Tabulated Conceptual Site Model – Pollutant Linkage Summary (On-Site Sources)			
Potential Sources	Potential Absorption Pathways	Potential Receptors	Risk Classification
Contaminants introduced on-site by former use of the site as a petrol filling station with underground fuel tanks/oil interceptor. • Asbestos, • PAHs, • TPHs, • VOCs, • Sulphates, and • Metals.	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** • Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Moderate**
Underground fuel tank in south-east corners: • TPHs,	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Moderate**
Septic tank close to eastern boundary:	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Low**

• Ammonia; • bacterial compounds such as faecal coliforms and faecal streptococci	• Inhalation of volatile vapour (indoors and outdoors)		
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** Vegetation within soft landscaped areas	**Low**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Low**
Made Ground from construction activities: • Asbestos, • PAHs, • Sulphates, and • Metals.	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** • Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south •	**Low**
Aggressive ground conditions with Made Ground and natural ground, including groundwater: • Sulphates, • PAH/TPH.	• Direct contact with aggressive ground conditions	**Building Materials and Services** • Buried Concrete • Underground services (water pipes)	**Low**
Ground gases generated by generated by the anaerobic decomposition of any leaked hydrocarbons beneath site and Made Ground: • Methane, • Carbon Dioxide, • Hydrogen Sulphide, and	• Migration through anthropogenic & natural pathways • Inhalation of Asphyxiating gases • Explosion (methane only)	**Human Health** • End Users (Residents/Future site visitors) • Construction workers during development (especially in confined spaces) • Site operatives during maintenance works in confined spaces • Neighbours and public	**Low**
	• Migration through anthropogenic & natural pathways • Explosion (methane only)	**Building Materials and Services** • Buildings • Confined spaces	**Low**

• Carbon Monoxide.		• Underground services	

3. SITE WORKS

3.1. Scope of Works

Site works were undertaken in two phases.

Phase I was undertaken between 29th September and 1st October 2025 and comprised the drilling of 3No. Cable Percussion Boreholes (BH01 – BH03) to a depth of between 8.20 - 10.00m bgl. BH01 was terminated at a depth of 8.20m bgl due to the density of the strata/possible flint obstruction.

Standard Penetration Tests were carried out at 1.00m intervals to a depth of 5m bgl and at 1.50m intervals for the remaining depth of the boreholes. A combined ground-gas/groundwater monitoring standpipe, with an internal diameter of 50mm, was installed in each of the boreholes. The installation details can be viewed within the table below.

Combined Ground-gas and Groundwater Monitoring Well Construction					
Trial Hole	Depth of Installation (m bgl)	Thickness of slotted piping with gravel filter pack (m)	Depth of plain piping with bentonite seal (m bgl)	Response Zone (m bgl)	Piping internal diameter (mm)
BH01	8.20	7.20	1.00	1.00 – 8.20	50
BH02	10.00	9.00	1.00	1.00 – 10.00	50
BH03	10.00	9.00	1.00	1.00 – 10.00	50

Additionally, 4No. Trial Pits (TP01 – TP04) were machine excavated to a depth of 1.00m bgl to undertake infiltration tests in accordance with BRE365. Due to mechanical issues and resulting time constraints, it was not possible to excavate the proposed TP05 – TP11.

Phase II was undertaken on 22nd October 2025 when 4No. Trial Pits (TP05 – TP08) were machine excavated to a depth of 1.00m bgl to undertake infiltration tests in accordance with BRE365. Three further trial pits (TP09 – TP11) were excavated to a depth of 3.00m bgl.

Additionally, in-situ California Bearing Ratio (CBR) testing by Dynamic Cone Penetrometer (DCP) was undertaken at 3No. locations (CBR01 – CBR03) to determine the CBR values of the soils underlying the site to shallow depth in accordance with BS1377 Part 9: 2016: 4.3. The concrete hardstanding and underlying Made Ground was broken out to a depth of 0.50m bgl where the tests were started from and a sample was taken for contamination analyses at each location. The results can be seen within Appendix C. The location of the trial hole locations can be seen within Figures 3 and 4.

Prior to commencing the ground investigation, a walkover survey was carried out to identify the presence of underground services and drainage. Where underground services/drainage were suspected and/or positively identified, the exploratory position was relocated away from these areas.

As a further precautionary measure, the borehole was hand excavated to 1.20m below the local ground level (bgl) and scanned with a Cable Avoidance Tool (CAT scanner) and signal generator (Genny) to minimise the risk to services.

Upon completion of the drilling works, the trial holes were backfilled and made good, in relation to the surrounding area.

3.2. Sampling Procedures

Small disturbed samples and environmental samples were recovered from the trial holes at the depths shown on the trial hole records. Soil samples were generally retrieved from each change of strata and/or at specific areas of concern. Samples were also taken at approximately 0.5m intervals during broad homogenous soil horizons.

A selection of samples were despatched to a suitably accredited laboratory for both geotechnical and chemical testing purposes.

4. ENCOUNTERED GROUND CONDITIONS

4.1. Soil Conditions

The trial holes were logged by a Ground and Water Limited representative, generally in accordance with BS EN 14688 'Geotechnical Investigation and Testing – Identification and Classification of Soil'. The chalk samples were logged in accordance with CIRIA 574.

The ground conditions encountered within the trial holes constructed on the site did generally conform to that anticipated from examination of the geology map. A capping of Topsoil/Made Ground was noted to overlie the superficial Head Deposits and the bedrock Seaford Chalk Formation.

The succession of conditions and description of soils encountered in the trial holes in descending order is tabulated below.

Summary of Strata Encountered			
Strata	**Top Depth (m bgl)**	**Base Depth (m bgl)**	**Thickness (m)**
MADE GROUND (BH01 – BH03): Tarmac.	GL	0.10	0.10
MADE GROUND (TP09 – TP10): Concrete	GL	0.10	0.10
MADE GROUND (BH01 – BH03, TP09 – TP10): Red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%).	0.10	0.50	0.40
MADE GROUND (TP09 and TP11): Grey gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk.	0.50 – 0.60	0.80 – 0.90	0.30
TOPSOIL (TP01 – TP08, TP11): Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk.	GL	0.25 – 0.60	0.25 – 0.60
HEAD DEPOSITS (TP01 – TP04, TP06): Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk.	0.25 – 0.50	0.90 – 1.70	0.60 – 1.20
HEAD DEPOSITS (TP05 – TP08): Light brown slightly gravelly sandy gravelly CLAY. Sand is fine. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk.	0.30 – 0.90	>1.00	>0.10 – 0.70
HEAD DEPOSITS (TP09 – TP11): Orangish brown slightly clayey gravelly fine SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint.	0.50 – 0.90	1.40 – 2.70	1.65 – 1.80
SEAFORD CHALK FORMATION (TP09 – TP10): Structureless CHALK composed of white and pale brown gravelly sandy SILT. The matrix was pale brown and soft. The gravel was weak, white and sub-angular. (CIRIA 574: Grade Dm).	1.40 – 2.70	>3.00	>0.30 – >1.60
SEAFORD CHALK FORMATION (BH01 – BH03): Recovered as a structureless CHALK composed of off white to white slightly sandy medium and coarse GRAVEL with many sub-angular cobbles. Gravel and cobbles were medium dense. Matrix is uncompact and off white. (CIRIA C574: Grade Dc).	1.20 – 1.70	>8.20 – >10.00	>7.00 – >8.80

For details of the composition of the soils encountered at particular points, reference must be made to the individual trial hole logs within Appendix C of this report. A trial hole location plan can also be viewed within Figures 3 and 4.

4.2. Roots Encountered

Fresh roots were proven to 0.30m bgl within trial pits TP01 – TP07 and TP08 to 0.40m bgl in TP07 and to 0.60m bgl in TP11. No roots were noted in TP09 – TP10 and the boreholes (BH01 – BH03).

It must be noted that the chance of determining actual depth of root penetration through a narrow diameter borehole is low. Roots may be found to greater depths at other locations on the site, particularly close to trees and/or trees that have been removed both within the site and its close environs.

4.3. Groundwater Conditions

Groundwater was not encountered in the boreholes or the trial pits. It should be noted that groundwater strikes may be obscured by the drilling/excavation process.

Changes in groundwater level occur for a number of reasons including seasonal effects and variations in drainage. The investigation was undertaken in September – October 2025 when groundwater levels are likely to be rising their annual minimum (lowest elevation). Exact groundwater levels may only be determined through long term measurements from monitoring wells installed on-site.

4.4. Obstructions

BH01 was terminated at a depth of 8.20m bgl due to the density of the strata/possible flint obstruction.

No other artificial or natural sub-surface obstructions were noted during drilling/excavation of the trial holes.

5. IN-SITU AND LABORATORY TESTING

5.1. In-situ Strength Testing

Standard Penetration Tests (SPTs) were undertaken as part of the site investigation. The results of the SPT's have not been amended to consider hammer efficiency, rod lengths and overburden pressure in accordance with Eurocode 7. The test results are presented on the borehole logs within Appendix D. An interpretation of the in-situ geotechnical testing results is given in the table below.

Interpretation of In-situ Geotechnical Testing Results				
Strata	**SPT "N" Blow Counts**	**Equivalent Undrained Shear Strength (Cu) (kPa) (Stroud (1974))**	**Soil Type**	**Trial Hole/s**
			Cohesive (Cu)	
Head Deposits (Cohesive)	13 – 14	65 – 70	Medium	BH01/0.50 – 1.70m bgl BH02/0.50 – 1.70m bgl
Seaford Chalk Formation	20 – >50 22 – 48 11 – 40	-	-	BH01/1.70 – >8.20m bgl BH02/1.70 – >10.00m bgl BH03/1.20 – >10.00m bgl

It must be noted that field measurements of undrained shear strength (Cu) are dependent on a number of variables including disturbance of sample, method of investigation and also the size of specimen or test zone.

5.2. Geotechnical Laboratory Testing

A programme of geotechnical laboratory testing, scheduled by Ground and Water Limited and carried out by an accredited geotechnical testing laboratory was undertaken on samples recovered. Details of the specific tests used in each case are given below.

Standard Methodology for Laboratory Geotechnical Testing		
Test	**Standard**	**Number of Tests**
Atterberg Limit Tests	BS1377:2016:Part 2:Clauses 3.2, 4.3 & 5	3
Determination of Saturation Moisture Content of Chalk	BS1377:2016:Part 2:Clause 3.3	6
Water Soluble Sulphate and pH Test	BS1377:2018:Part 3:Clause 5	1
BRE Special Digest 1 Tests	BRE Special Digest 1 "Concrete in Aggressive Ground (BRE, 2005).	2

5.2.1. Atterberg Limit Testing

A précis of Atterberg limit testing undertaken can be seen tabulated below. The test results are presented within Appendix D.

Atterberg Limit Tests Results Summary							
Stratum	**Moisture Content (%)**	**Passing 425 µm sieve (%)**	**Modified PI (%)**	**Soil Class**	**Consistency Index (Ic)**	**Volume Change Potential**	
						BRE	**NHBC**
Head Deposits	23.3 – 33.5	66 – 74	16.5 – 22.4	CI - CH	Very Stiff	Low to Medium	Low to Medium
• **NP – Non-plastic** • **BRE Volume Change Potential refers to BRE Digest 240 (based on Atterberg results)** • **Soil Classification based on British Soil Classification System.** • **Consistency Index (Ic) based on BS EN ISO 14688-2:2018.**							

5.2.2. Moisture Deficit Assessment

The results of the Atterberg Limit tests were analysed to determine the Liquidity Index of the samples, to give an indication as to whether the samples recovered showed a moisture deficit as well as assessing their degree of consolidation. Liquid Limit analyses was undertaken to assess whether there were any potentially significant moisture deficits within the samples tested.

No potential moisture deficits were noted within the three samples tested.

5.2.3. Determination of Saturation Moisture Content of Chalk

The results of saturation moisture content testing undertaken are tabulated below. The results can be seen in Appendix E.

Summary of Saturation Moisture Content Test Results					
Stratum	**Moisture Content (%)**	**Saturation Moisture Content (%)**	**Bulk Density (Mg/m^3)**	**Dry Density (Mg/m^3)**	**CIRIA Dry Density Classification**
Seaford Chalk Formation	22 – 27	21 – 27	1.97 – 2.08	1.55 – 1.71	Medium to High Density
Density in accordance with CIRIA 574 Chapter 3.0, Table 3.7					

6. ENGINEERING CONSIDERATIONS

6.1.1. Chalk Dissolution Risk Assessment

The presence of dissolution features on-site can prove problematic, as they often provide inconsistency within the underlying geology. Often formed during glacial and periglacial conditions, the depth and width of these features can vary substantially, often providing inconsistency in geological strength data.

If a dissolution feature is suspected, a geotechnical engineer and structural engineer should be contacted for suitable engineering designs, based on further works identifying and classifying the area the feature occupies.

The Groundsure Geoinsight datasheets, presented Appendix C of this report, revealed the following information which may be pertinent to this risk assessment.

- Ground Dissolution of Soluble Rocks – The site was classified as a Very Low to Low risk of ground dissolution of soluble rocks.
    - *A very low risk was defined as "Soluble rocks are present within the ground. Few dissolution features may be present. Potential for difficult ground conditions or localised subsidence are at a level where they need not be considered".*
    - *A Low Risk was described as "Soluble rocks are present within the ground. Some dissolution features may be present. Potential for difficult ground conditions are at a level where they may be considered, localised subsidence need not to be considered except in exceptional circumstances".*
- An on-site Class A Historical Mining area was identified on-site. A Class A area was described as *"Sporadic underground mining of restricted extent may have occurred. Potential for difficult ground conditions are unlikely and localised and are at a level, where they need not be considered"*. However, this is generally a precautionary statement attributed to all areas associated with chalk within the ground profile. Therefore, this does not identify any underground chalk features at the site location.
- No records of Brit Pits, surface ground workings were noted within 500m radius of the site;
- No records of Underground workings, Mining Cavities or JPB Mining Areas were noted within a 500m radius of the site.
- The Edmonds Procedure (Edmonds, C.N., 2001. Predicting natural cavities in chalk. Geological Society, London, Engineering Geology Special Publications, 18(1), pp.29-38.); highlighted no anticipated subsidence hazard classification.

This risk assessment indicates that there is a very low to low risk from the dissolution of soluble rocks at the site.

The intrusive investigation encountered Topsoil or Made Ground to a depth of between 0.60 – 0.90m bgl the Holywell Nodular Chalk Formation for the remaining depth of the trial holes at a maximum depth of 10.00m bgl in BH02 and BH03. As a result of sample disturbance during the drilling/excavation process the chalk was generally classified as Grade Dc, in accordance with CIRIA 574. The geotechnical laboratory testing indicated the chalk clasts within the boreholes to be of

medium to high density, with a dry density of between 1.55 – 1.71mg/m^3. The Seaford Chalk Formation was anticipated to have no volume change potential in accordance with CIRIA C574.

The SPT "N" blow counts displayed within Section 5.1.2 the Seaford Chalk Formation had an SPT blow count of between 11 and >50 in the boreholes **which are considered moderate with regards to geotechnical assessment.**

No groundwater was encountered during the intrusive investigation.

6.2. Soil Characteristics and Foundation Considerations

A summary of the soil characteristics following the intrusive site investigation and laboratory testing and the relevant foundation considerations has been provided below. The following information from the ground investigation was considered pertinent to the design of foundations.

- Foundations should be taken through any Made Ground/Topsoil/Head Deposits and either into, or onto a suitable underlying Seaford Chalk Formation.
- For the chalk bedrock, no volume change potential can be considered.
- Given the presence of cohesive soils at shallow depth, special foundation precautions may be required to prevent possible future shrinkage/swelling of the soils affecting the integrity of the faces of foundations (underfloor void diameter/compressible material/void formers etc.).
- The loads of proposed foundations should not exceed the allowable bearing capacity of the soils they are founding upon.
- Foundations must not be placed within fresh root penetrated and/or desiccated soils with volume change potential. It is recommended that foundations are taken at least 300mm into non-fresh root penetrated strata if the soils have volume change potential, or into soils of no volume change potential.
- The influence of trees on or surrounding the site will need to be taken into account in final design (NHBC Standards Chapter 4. 2) (tree rings).
- Any water ingress must be prevented from entering foundation trenches and excavations must be kept dry and either concreted or blinded as soon after excavation as possible. If water were allowed to accumulate within the excavation for even a short period of time, an increase in heave occur. The shear strength will also be reduced, resulting in lower bearing capacities, resulting in increased settlements. Instability issues may arise within the foundation trenches, in case of perched water being present.

6.3. Shallow Foundations

Based on the previous section, it was considered that strip foundations could be adopted for the proposed redevelopment with foundations taken through any Topsoil, disturbed ground, below any roots if encountered and into or onto the Head Deposits and/or the underlying Seaford Chalk Formation.

Foundations constructed on the Head Deposits can be designed based on a general bearing capacity of **80kN/m^2** for foundations within the Head Deposits.

Foundations constructed on the Seaford Chalk Formation can be designed based on a general bearing capacity of **125kN/m²** for foundations within the chalk. If the formation level proves to be competent Dc chalk, a higher bearing capacity reaching 225kN/m^2 might be achievable.

This is based on the trial holes logs, in-situ testing, reference to CIRIA 574, a 5m long by 1m wide foundation and a maximum settlement of 25mm. Differential settlement may occur where foundations cross between the more clast dominated chalk to more structured chalk and this should be taken into account in final design.

Additional Comments
A number of comments and recommendations has been provided below.

- It is prudent to avoid having a foundation spanning across materials of different consistency, like clay and gravel, in order to avoid potential differential settlements. If unavoidable, foundations must be reinforced to allow for such potential differential movement. It should be noted that differential settlement may also occur between strip and isolated pad foundations.
- Foundation excavations must be carefully bottomed out and any loose soil or soft spots removed prior to the foundation concrete or blinding being placed. Failure to ensure that foundation excavations are suitably bottomed out could result in additional settlements.
- Inspection of foundation excavations, prior to concreting, must be made by a competent and suitably qualified person to check for soft spots and to check for the presence of roots.
- Foundations must not be cast over foundations of former structures and/or other hard spots.
- Isolated Pad Foundations must be at least 1.5 times the width of the widest pad apart to keep to the anticipated settlements.
- Final designs for the foundations should be carried out by a suitably qualified Engineer based on the findings of this investigation and with reference to the anticipated loadings, serviceability requirements for the structure and the developments proximity to former, present, and proposed trees. Where trees are mentioned in the text this means existing trees, recently removed trees (approximately 15 years to full recovery on cohesive soils) and those planned as part of the site landscaping.

6.4. Piled Foundation

Based on the results of the intrusive investigation, a piled foundation scheme was unlikely to be required.

6.5. Ground Bearing Slabs

During the site investigation, Made Ground was encountered to a maximum depth of 0.90m bgl, overlying gravelly clay and slightly clayey gravelly sand of the Head Deposits to a depth of between 1.20 – 2.70m bgl overlying chalk to the base of the boreholes at a maximum depth of 10.00m bgl. No groundwater was encountered.

We understand that site levels are to be reduced by approximately 1.40m in the area of the proposed structure.

Based upon this, ground bearing slabs should be able to be adopted for the proposed structure. Where possible any remaining Made Ground should be removed from below the slab and the formation level should be proof rolled and any soft spots removed and infilled with a suitably compacted granular material. Site levels should be brought up to underside of proposed slab level using with suitable granular soil (Type I or Type II) rolled in thin layers. The slabs must be reinforced and cast in small bays with dowels cast at the interface between each of the bays to reduce the risk of differential settlement affecting the slab.

6.6. Excavations and Stability

Shallow excavations in the Topsoil/Made Ground and Head Deposits are likely to be marginally stable at best. Long, deep excavations, through these strata and into the underlying Seaford Chalk Formation are likely to become unstable.

For excavations in the chalk, in the **short term only** an angle of 75° can be adopted for both the unstructured and the structured chalk provided that appropriate risk assessments and method statements (RAMS) allow for some spalling to occur.

A definitive duration cannot be given to the length of time to which "Short Term" applies. No definition is given in CIRIA574 but it is likely the term refers to the construction stage. The fact that the slopes have been cut for a short period of time is unlikely to adversely affect the performance of the slopes during the construction phase.

Please find below a copy of Fig 6.2 form CIRIA C574.

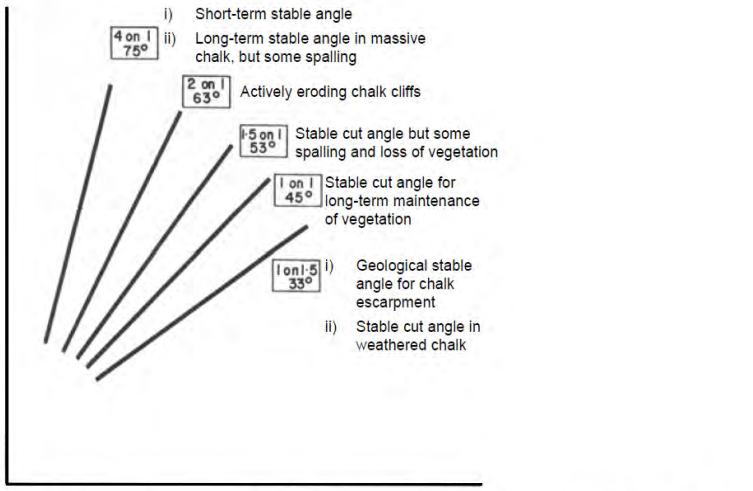


Figure 6.2 *Performance of chalk slopes in southern England (after Williams, 1990)*

As you will see 75° angle in the long term is for massive chalks with some spalling.

During the construction phase, regular inspections of the overall stability of the slope should be undertaken to maintain the safe working conditions below the cut slopes and this should be included within the RAMS.

Unsupported earth faces formed during excavation may be liable to collapse without warning and suitable safety precautions should therefore be taken to ensure that such earth faces are adequately supported before excavations are entered by personnel.

Retaining walls within the unstructured matrix and clast supported chalk should be designed based on an effective cohesion of 0kpa and an angle of friction of 33° (CIRIA 574).

Retaining wall design will need to be in accordance with Section 6 of CIRIA 574.

6.7. Sub-Surface Concrete Design

Concrete to be placed in contact with soil or groundwater must be designed in accordance with the recommendations of Building Research Establishment Special Digest 1, 2005, *'Concrete in Aggressive Ground'* considering the pH of the soils. For the classification given below, the "mobile" and "brownfield" land use case was adopted given the geology encountered and the residential use of the site. This assessment was based on the results of laboratory testing summarised within Section 5, specifically relating to sulphates, pH and other water soluble chemicals.

Made Ground

The water-soluble sulphates in the Made Ground were noted to be 20.2 - 109mg/l, with a pH of 8.2 – 8.5, from chemical laboratory testing.

Head Deposits

According to Box C6 of BRE Special Digest 1, 2005, 'Concrete in Aggressive Ground', the Head Deposits did not within a list of UK geological formations known to contain pyrite. Consequently, it was not required to consider the levels of total potential sulphate in the classification process.

The water-soluble sulphates in the Head Deposits were noted to be 43.5mg/l, with a pH of 8.0, from geotechnical laboratory testing.

According to BRE Special Digest 1, 2005, 'Concrete in Aggressive Ground' a Sulphate Design Class of DS-1 should be used for sub-surface concrete in contact with the Head Deposits. Table C1 of the Digest indicated an ACEC (Aggressive Chemical Environment for Concrete) classification of AC-1.

Seaford Chalk Formation

According to Box C6 of BRE Special Digest 1, 2005, 'Concrete in Aggressive Ground', the Seaford Chalk Formation did not within a list of UK geological formations known to contain pyrite. Consequently, it was not required to consider the levels of total potential sulphate in the classification process.

The water-soluble sulphates in the Seaford Chalk Formation were noted to be 53.7 - 301mg/l, with a pH of 78.2 – 8.5, from geotechnical laboratory testing.

According to BRE Special Digest 1, 2005, 'Concrete in Aggressive Ground' a Sulphate Design Class of DS-1 should be used for sub-surface concrete in contact with the Seaford Chalk Formation. Table C1 of the Digest indicated an ACEC (Aggressive Chemical Environment for Concrete) classification of AC-1.

6.8. Roads and Hardstanding Design

This section provides information relating to the construction of roads and hardstanding, for each strata encountered.

6.8.1. Anticipated CBR Values and Frost Susceptibility

Topsoil

Roads and hardstanding should bypass Topsoil given the likely low strength, high compressibility and general variability.

Made Ground

Where roads and hardstanding are to be founded on Made Ground, a CBR value of 1% should be adopted, as a result of the variable nature of Made Ground in terms of consistency, strength and compressibility.

Head Deposits

Roads and hardstanding constructed on the cohesive Head Deposits should be designed based on a CBR value of 4.0%. This is based on reference to Table C1 of TRL1132 and average construction conditions. Based on wet conditions a CBR of 2.5% should be adopted, with dry conditions a CBR of 4.5% being applicable.

These clay-based soils (poorly drained) with plasticity indexes generally greater 20% are likely to be non-frost susceptible. Due to their granular content however, they should be treated as frost susceptible in design.

Seaford Chalk Formation

In accordance with CIRIA 574, Part 4.7 *The Action of Frost on Chalk*, all chalk deposits can be seen to be frost susceptible. The magnitude of frost heave increases linearly with the saturation moisture content of the chalk aggregate. The weak chalk of south-east England is particularly prone to frost heave in the climatic conditions of the area. Therefore a minimum construction thickness of 450mm should be maintained.

All formation levels for roads and hardstanding should be proof rolled to highlight any soft areas which should then be dug out and replaced with selected granular material compacted in thin layers to a suitable specification.

6.8.2. In-Situ California Bearing Ratio Tests by Dynamic Cone Penetrometer

In-situ California Bearing Ratio (CBR) testing by dynamic cone penetrometer (DCP) was undertaken to determine the CBR values of the soils underlying the site to shallow depth in accordance with BS1377 Part 9: 2016: 4.3 and a summary of the results is tabulated below. The full test results are presented in Appendix C.

In-situ CBR Test Results Summary					
Test Location	**Layer 1 (Head Depositsl)**	**Layer 2 (Head Deposits)**	**Layer 3 (Head Deposits)**	**Layer 4 (Head Deposits)**	**Layer 5 (Head Deposits)**

	Thickness (mm)	In-situ CBR (%)	Thickness (mm)	In-situ CBR (%)	Thickness (mm)	In-situ CBR (%)	Thickness (mm)	In-situ CBR (%)	Thickness (mm)	In-situ CBR (%)
DCP01	375mm	22%	430mm	8%	-	-	-	-	-	-
DCP02	40mm	6%	125mm	25%	5mm	239%	505mm	12%	80mm	16%
DCP03	110mm	16%	50mm	32%	260mm	21%	90mm	36%	335mm	16%

6.9. Surface Water Disposal

Soakaway testing in accordance with BRE365 was undertaken within TP01 – TP08 within the Head Deposits and the results are tabulated below.

Soakage Testing Results						
Trial Hole	**Test Number**	**Depth (m bgl)**	**Start Depth (m bgl)**	**Finish Depth (m bgl)**	**Time Taken (mins)**	**Infiltration Rate (m/sec)**
TP01	1	1.00	0.60	1.00	60	4.22×10^{-5}
	2	1.00	0.18	0.88	94	3.76×10^{-5}
	3	1.00	0.40	0.93	90	4.14×10^{-5}
TP02	1	1.00	0.54	1.00	20	1.85×10^{-4}
	2	1.00	0.45	1.00	69	8.42×10^{-5}
	3	1.00	0.39	0.97	60	7.18×10^{-5}
TP03	1	1.00	0.10	1.00	50	9.90×10^{-5}
	2	1.00	0.39	0.91	70	6.02×10^{-5}
	3	1.00	0.45	0.93	90	4.36×10^{-5}
TP04	1	1.00	0.53	1.00	30	2.02×10^{-4}
	2	1.00	0.30	0.90	70	5.94×10^{-5}
	3	1.00	0.37	0.95	95	3.88×10^{-5}
TP05	1*	1.00	0.17	0.62	240	7.52×10^{-6}
	2	-	-	-	-	
	3	-	-	-	-	-
TP06	1*	1.00	0.17	0.61	240	7.06×10^{-6}
	2	-	-	-	-	-
	3	-	-	-	-	-
TP07	1*	1.00	0.14	0.69	240	9.75×10^{-6}
	2	-	-	-	-	
	3	-	-	-	-	-
TP08	1	1.00	0.20	1.00	39	7.56×10^{-5}
	2	1.00	0.26	1.00	53	5.87×10^{-5}
	3	1.00	0.24	0.81	85	2.38×10^{-5}
***Given the relatively slow infiltration rate encountered, only one test could be undertaken and it did not reach 75% mark.**						

Any soakaways should be located sufficiently away from buildings and infrastructure, in order to prevent undermining of foundations.

The submission of a Sustainable Urban Drainage Scheme (SUDS) is likely to be required for this site due to the proposed development increasing the amounts of hardstanding.

Consultation with the Environment Agency must be sought regarding any use that may have an impact on groundwater resources, abstractions and surface water features/watercourses.

It should be noted that water discharge into the chalk may worsen / create dissolution features. Therefore, soakaways should be remote from structures and undertaken in accordance with available guidance.

A number of published information sources make reference to an appropriate distance at which a soakaway should be sited in relation to any building foundations in the vicinity:

- The British Standard BS8301 :1985 "Code of practice for building drainage" (1985), suggests that: "A soakaway is not desirable nearer to a building than about 5 metres nor in such a position that the ground below foundations is likely to be adversely affected."
- The Building Research Establishment Digest 365 "Soakaway design" (1991), suggests that "Soakaways should not normally be constructed closer than 5m to building foundations. In chalk or other soil and fill material subject to modification or instability, the advice of a specialist geotechnologist should be sought as to the advisability and siting of a soakaway".

In addition, the distance a soakaway is located from any foundations can be informed by both the soakaway type and chalk density present at the site.

- The Kent County Council Soakaway Design Guide (July 2000) states that "Highway soakaways shall be situated the following minimum distances from dwellings (measured from the outside edge of the soakaway): (1) 5 metres for conventional perforated soakaway; (2) 10 metres for deep bored soakaway. In addition, adjacent soakaways of a similar type shall also be spaced a minimum of 10 metres apart.
- CIRIA 574 states that soakaways should be sited at least 5-10m away from any structure, depending on the chalk density. Where the chalk present is of low or unknown density, soakaways should be sited at least 10m from any foundations, and where the chalk is of medium density or higher, any foundations should be sited at least 5m from any foundations.

7. CONTAMINATION RISK ASSESSMENT

7.1. Results of the Preliminary Risk Assessment

The tabulated Conceptual Site Model (CSM) developed by the Ground and Water Desk Study (ref. GWPR6751/DS/November 2025) is reproduced in section 2.8. For the complete information please see the Desk Study report.

No additional source of contamination, pathway or receptor was identified between the desk study and the ground investigation and as such, it was not necessary to modify the CSM.

In the UK, the assessment of risk from contamination follows the source-pathway-receptor (SPR) approach. For a risk to be present there must be a source of contamination, a receptor or receptors, and a pathway for contaminants to migrate or be absorbed. If there is a linkage between any given source and receptor, then a quantitative risk assessment is used to assess the significance or impact of the pollutant linkage.

7.2. Sampling Locations

The synopsis for the trial hole locations and their final depth can be seen tabulated below. A trial hole location plan is provided within Figures 3 and 4. The table also indicates whether any locations targeted a source of contamination as well as the proposed end-use. The proposed development can be seen in Figure 2.

Trial Hole Location Synopsis		
Trial Hole/Final Depth	**Sampling Strategy**	**Proposed End-Use**
BH01/8.20m	Random	Hardstanding
BH02/10.00m	Random	Footprint of Proposed Building
BH03/10.00m	Random	Footprint of Proposed Building
TP01/1.00m	Random	Soft Landscaping
TP02/1.00m	Random	Soft Landscaping
TP03/1.00m	Random	Soft Landscaping
TP04/1.00m	Random	Soft Landscaping
TP05/1.00m	Random	Soft Landscaping
TP06/1.00m	Random	Soft Landscaping
TP07/1.00m	Random	Soft Landscaping
TP08/1.00m	Random	Soft Landscaping
TP09/3.00m	Random	Hardstanding
TP10/3.00m	Random	Hardstanding
TP11/3.00m	Random	Hardstanding
CBR01/0.50m	Random	Hardstanding
CBR02/0.50m	Random	Hardstanding
CBR03/0.50m	Random	Hardstanding

The investigation area totals approximately 5,200m^2 and with seventeen sampling locations, given an unknown hotspot shape, the sampling density means that a hotspot with an area of approximately 460m^2 and a radius of approximately 12.1m would be encountered (CLR 4).

7.3. Soil Risk Assessment

Sampling depths were chosen to reflect the receptor of concern (e.g. human health, controlled waters and vegetation). The samples were scheduled under a random strategy. The receptors relevant to the sampling depths are tabulated below.

Summary of Receptors per Sample Depth	
Depth	**Receptors**
Shallow Samples (< 0.75m bgl)	• End Users (Residents/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public • Vegetation within soft landscaped areas (shallow rooted) • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Buried Concrete • Receiving landfill

Chemical laboratory testing, scheduled by Ground and Water Limited, and carried out by i2 Analytical Limited, was undertaken on six samples of Made Ground (TP09/0.50m, TP10/0.20m, TP11/0.50m, CBR01/0.50m, CBR02/0.50m and CBR03/0.50m bgl). The samples were subjected to a combination of analysis that comprised:

- Asbestos screen;
- Semi-metals and heavy metals incl. Arsenic, Cadmium, Chromium (incl. Hexavalent Chromium), Copper, Lead, Mercury, Nickel, Selenium, Vanadium, Zinc;
- Polycyclic Aromatic Hydrocarbons (PAH's) incl. Naphthalene, Acenaphthylene, Acenaphthene, Fluorene, Phenanthrene, Anthracene, Fluoranthene, Pyrene, Benzo(a)anthracene, Chrysene, Benzo(b)fluoranthene, Benzo(k)fluoranthene, Benzo(a)pyrene, Indeno(1,2,3-cd)pyrene, Dibenz(a,h)anthracene, Benzo(ghi)perylene;
- Speciated TPH including full aliphatic/aromatic split; and
- BTEX compounds (Benzene, Toluene, Ethylbenzene, Xylene) and MTBE – used as marker compounds for Volatile Organic Compounds (VOCs).

A breakdown of the number of tests undertaken for each determinand is discussed in further sections. The chemical laboratory results are presented in Appendix E.

7.3.1. Soil Assessment Criteria and Results

At the time of reporting, November 2025, the proposed development was understood to comprise the demolition of the existing service station (comprising petrol filling station, shop, cafe, outbuilding and associated infrastructure) and erection of a new service station (comprising electrical vehicle charging points, shop, restaurant / cafe, toilets and associated infrastructure) together with soft landscaping, revised vehicular access and formation of a new pedestrian link. The proposed development plan can be seen within Figure 2.

Based on the proposed development, the results of the chemical laboratory testing were compared to the Generic Assessment Criteria (GAC) for a ***'Commercial'*** land-use scenario, as this was considered the most appropriate land-use scenario.

Where a contaminant of concern's GAC varies according to the Soil's Organic Matter (SOM), the SOM recorded for each uncontaminated soil type was used to derive the appropriate GAC for each soil type. A SOM value of 1%, the most conservative value, was utilised.

A summary of the analytical results and the comparison to the determinands adopted screening value are presented in the table below and overleaf.

Soil Contamination Test Results and Screening Values					
Determinand	**Number of Samples**	**Minimum Concentration (mg/kg)**	**Maximum Concentration (mg/kg)**	**Adopted Screening Value (mg/kg)**	**Was the adopted screening value exceeded for relevant land-use scenario?**
INORGANICS					
Asbestos	6	ND	ND	ND	No
Cyanide (total)	6	< 1.0	< 1.0	1400	No
Phenols	6	< 1.0	< 1.0	280	No
METALS					
Arsenic	6	8.8	14	640	No
Boron	6	0.3	15	240,000	No
Cadmium	6	0.8	1.3	190	No
Chromium (III)	6	11	18	8,600	No
Chromium (Hexavalent)	6	< 1.8	< 1.8	33	No
Copper	6	6.5	19	6,800	No
Lead	6	50	82	2,300	No
Mercury	6	< 0.3	< 0.3	58	No
Nickel	6	8.6	17	980	No
Selenium	6	< 1.0	1.1	12,000	No
Vanadium	6	14	20	9,000	No
Zinc	6	160	310	730,000	No
TOTAL PETROLEUM HYDROCARBONS (TPHs)					
Aliphatic >C5 - C6	3	< 0.01	< 0.01	3,700	No
Aliphatic >C6 - C8	3	< 0.01	< 0.05	7,800	No
Aliphatic >C8 - C10	3	< 0.01	< 0.01	2,00	No
Aliphatic >C10 - C12	3	1.4	6.0	9,700	No
Aliphatic >C12 - C16	3	3.8	20	59,000	No
Aliphatic >C16 - C21	3	< 8.0	41	1,600,000	No
Aliphatic >C21 - C35	3	23	130	1,600,00	No
Aliphatic >C35 – C40	3	< 10	44	1,600,000	No
Aromatic >C5 - C7	3	< 0.01	< 0.01	26,000	No
Aromatic >C7 - C8	3	< 0.01	< 0.01	56,00	No
Aromatic >C8 - C10	3	< 0.02	< 0.02	3,500	No
Aromatic >C10 - C12	3	< 1.0	< 1.0	16,000	No
Aromatic >C12 - C16	3	3.6	18	36,000	No
Aromatic >C16 - C21	3	54	240	28,000	No
Aromatic >C21 - C35	3	190	670	28,000	No
Aromatic >C35 – C40	3	24	140	28,000	
Benzene	3	< 0.005	< 0.005	27	No
Toluene	3	< 0.005	< 0.005	5,600	No
Ethylbenzene	3	< 0.005	< 0.005	5,700	No
m,p-Xylene	3	< 0.008	< 0.008	6,600	No
o-Xylene	3	< 0.005	< 0.005	6,600	No

POLYCYCLIC-AROMATIC HYDROCARBONS					
Acenapthene	6	< 0.05	1.70	84,000	No
Acenapthylene	6	< 0.05	0.48	83,000	No
Anthracene	6	< 0.05	4.1	520,000	No
Benzo(a)anthracene	6	0.10	19	170	No
Benzo(a)pyrene	6	0.11	31	35	No
Benzo(b)fluoranthene	6	0.16	43	44	No
Benzo(ghi)perylene	6	0.06	27	3,900	No
Benzo(k)fluoranthene	6	< 0.05	13	1,200	No
Chrysene	6	0.09	19	350	No
Dibenzo(a,h)anthracene	6	< 0.05	5.1	3.50	**Yes TP10/0.20m bgl**
Fluoranthene	6	0.16	31	23,000	No
Fluorene	6	< 0.05	1.1	63,000	No
Indeno(1,2,3-cd)pyrene	6	< 0.05	24	500	No
Naphthalene	6	< 0.05	< 0.05	1,600	No
Phenanthrene	6	< 0.05	11	22,000	No
Pyrene	6	0.16	29	54,000	No
MARKER FOR VOLATILE ORGANIC COMPOUND					
Methyl tert-butyl ether (MTBE)	3	< 0.05	< 0.05	7,900	No
The derivation of the Soil Assessment Criteria used within this report can be seen within Appendix E. The soil concentrations were typically compared to the LQM/CIEH Suitable for Use Levels (S4UL) (2014), based upon the assumption of 1% soil organic matter. A screening value for Lead is based on the Category 4 Screening Levels (C4SLs). Where no S4UL or C4SL is available for Cyanide, a SoBRA acute GAC is assigned. Where no S4UL or C4SL is available for MTBE, a CL:AIRE 2010 GAC is assigned. Where exceedances are noted, the trial hole and depth of sample is recorded.					

The proposed development includes areas of soft-landscaping, where shallow rooted vegetation may be exposed to aggressive ground conditions. The previous table compares the results of the laboratory analysis to GAC that is protective of Human Health and therefore different GAC is required for vegetation.

Soil Contamination Test Results and Screening Value					
Determinand	**Number of Samples**	**Minimum Concentration (mg/kg)**	**Maximum Concentration (mg/kg)**	**Adopted Screening Value (mg/kg)**	**Was the adopted screening value exceeded for relevant land-use scenario?**
Copper	6	6.5	19	200	No
Nickel	6	8.6	17	110	No
Zinc	6	160	310	300	**Yes TP11/0.50m**
The derivation of the Soil Assessment Criteria used within this report can be seen within Appendix E. The soil concentrations were compared to BS3882:2015 *Specification for Topsoil* and based on differing pH. For the purpose of conservatism, the pH of the soils have been assumed to be below 6 within this table. Where exceedances are noted, the trial hole and depth of sample is recorded.					

An elevated concentration of zinc was noted marginally over the threshold in the sample of Made Ground within TP11 tested above the phytotoxicity threshold trigger value. Consequently, specialist planting may be required in this area and the advice of a suitably experienced horticulturalist should be obtained.

7.3.2. Soil Assessment

The comparison between the laboratory analysis and the assessment criteria indicates that there is an unacceptable risk to future receptors caused by an exceedance of Dibenzo(a,h)anthracene in the sample from TP10/0.20m bgl (5.1mg/kg). **The location of TP10 falls under a proposed area of hardstanding and therefore does not present an unacceptable risk to future receptors. However, construction workers should wear appropriate PPE**

The remaining determinands were identified at below the adopted screening value and are not considered to represent an unacceptable risk to future receptors. Where an unacceptable risk to future receptors remains, mitigation is required.

Due to the nature of the contamination encountered at the site and the proposed end use, it is not considered that there is an unacceptable risk to future human health receptors and no remediation is required as part of any site development at the present time. Given the historical land-use of the site, the presence of tanks and other underground infrastructure of the re-fuelling station, gross contamination may be encountered during the removal of these features. A discovery strategy must be in place. Should contamination be encountered during and following removal of the USTs and the septic tank or should the proposed development change then these conclusions would need to be re-evaluated following additional testing.

Asbestos

No asbestos has been identified during the testing, however, given the age of the structures on-site, an **Asbestos Management Strategy** should be put in place so that any potentially asbestos containing materials are identified and removed from site in a suitable manner to prevent cross-contamination. An Asbestos Survey should be undertaken pre-demolition of any buildings/structures.

7.3.3. Water Supply Pipe Risk Assessment

As part of the proposed development it is expected that new water supply pipes will be installed at the site. Water supply pipes have the potential to be exposed to contaminants in the ground that can lead to unacceptable concentrations within the drinking water. If a risk is identified, specialist pipes that are protective of the drinking water are required.

A pipe selection risk assessment should be submitted to the water supply company as part of the development based on the supplier's assessment criteria. An indication of whether there is likely to be a risk can be seen tabulated below, where the PE pipe thresholds are based on *UKWIR Contaminated Land Assessment Guidance, 2014*.

Pipe Selection Risk Assessment			
Determinand	**PE Pipe Threshold (mg/kg)**	**Laboratory Analysis Range (mg/kg)**	**Was the Threshold Exceeded?**
Total BTEX and MTBE	0.1	LOD	No
Total VOCs	0.5	Markers at BTEX	No
Total SVOCs	2	Markers at BTEX	No
Phenols	2	LOD	No
TPH EC5 – EC10 (aliphatics and aromatics)	2	LOD	No

TPH EC10 – EC16 (aliphatics and aromatics)	10	9.0 – 40	**Yes TP11/0.50m CBR02/0.50m**
TPH EC16 – EC40 (aliphatics and aromatics)	500	291 – 1,256	**Yes TP11/0.50m CBR02/0.50m**
Naphthalene	5	LOD	No

Due to elevated levels of the hydrocarbons (TPH EC10 – EC16 and EC16 – EC40 (aliphatics and aromatics)) identified in chemical laboratory testing above the threshold for PE Pipe, barrier or metal pipes must be used. Additional testing and sampling of the soils at the exact depth and location of pipes should be undertaken to confirm.

7.4. Groundwater Risk Assessment

In the UK, the water environment is protected under a number of regulatory regimes, mainly governed by the Environment Agency. The Environment Agency is consulted when there may be a risk to Controlled Waters, or where a pollution incident has occurred historically. The Water Framework Directive is implemented in various guises covering groundwater, surface water and drinking water supply.

A study of the aquifer maps on the DEFRA website revealed the site was underlain by a **Secondary A Aquifer** comprising the superficial Head Deposits, underlain by a **Principal Aquifer** comprising bedrock deposits of the Seaford Chalk Formation.

Examination of the Environment Agency records showed that the site was located within a Total Catchment (Zone III) Groundwater Source Protection Zone (SPZ) as classified in the Policy and Practice for the Protection of Groundwater.

The nearest surface water feature was an unnamed stream 37m to the south-west and flowing in a easterly direction.

From analysis of hydrogeological and topographical maps the actual groundwater table was anticipated to be encountered at depth within the Seaford Chalk Formation. Some amounts of groundwater however may be present at shallower depths within any Made ground and silty/sandy pockets of the Head Deposits. It was considered that groundwater was flowing in an easterly direction in line with local topography.

During the site investigation, Made Ground was encountered to a maximum depth of 0.90m bgl, overlying gravelly clay and slightly clayey gravelly sand of the Head Deposits to a depth of between 1.20 – 2.70m bgl overlying chalk to the base of the boreholes at a maximum depth of 10.00m bgl. No groundwater was encountered.

Low levels of hydrocarbons were detected in the shallow Made Ground that were below the threshold for the human health receptor for the proposed development. No groundwater was encountered in the deep boreholes that were drilled to a depth of 10m bgl. The presence of cohesive Head Deposits will likely mitigate any migration from the shallow soils into the underlying groundwater that will be encountered below 10m bgl. Based on the information to date, it is unlikely that the groundwater

would rise and come into contact with the impacted soils, therefore reducing the pathway to the underlying deep aquifer. The presence of hardstanding across the majority of the site with help to reduce any eluate/leachate production.

Should contamination be encountered during and following removal of the existing concrete slab, USTs and the septic tank then further risk assessment should be undertaken to confirm any impact and remedial measures that may be required to protect the underlying Principal Aquifer within the Seaford Chalk Formation.

7.5. Ground Gas Risk Assessment

A review of Phase I Desk Study did not identify any source of ground gas on-site or within influencing distance of the site that warranted investigation through ground gas monitoring. The only source of ground gases was anticipated to be limited Made Ground and therefore classified as 'Low risk', where the pragmatic approach to ground gas risk assessment presented within the CL:AIRE research bulletin RB17 and included within Annex D of BS8485:2015+A1:2019 can be considered.

7.5.1. Pragmatic Approach

The alternative approach to ground gas risk assessment considers the uncertainty from the measurement of ground gas concentrations and flow rates in monitoring wells for low-risk sites and removes the need for monitoring. Low risk sites typically comprise:

- Natural soils with high carbonate content (e.g. chalk);
- Natural soils that can produce gases (e.g. alluvium, peat etc.);
- Made Ground between 1.00m to 5.00m, with low organic content; and
- Areas of flooded mine workings, unless a building is to be within 20m of shaft or adit.

The method considers the gas generation potential of the soils from data that is not subject to external influences, for example atmospheric pressure can change the recorded flow and concentrations of ground gas during spot monitoring, or a rising groundwater level can displace ground gases in pore spaces. Determination of the characteristic situation is based on the total organic carbon (TOC) of the soils, coupled with any organic-rich material identified within the borehole logs, such as wood gravels. TOC is easily analysed by the laboratory and can also be calculated by the equation outlined below.

Total Organic Carbon (TOC) = 0.58 x Soil Organic Matter (SOM) = 1.33 x Degradable Organic Carbon (DOC)

As part of the ground investigation works, six samples of Made Ground were submitted to the laboratory for analysis, which identified TOC to vary between 1.1% and 1.9% in the Made Ground. The trial hole logs identified the gravels within the Made Ground to comprise brick (60%) and concrete (40%). Made Ground was identified to a maximum depth of 0.90m bgl.

7.5.2. Data Quality

A review was made of the quality of the available data for the site, which can be viewed in the table below. CIRIA Report C665 (2007), BS 8485:2015+A1:2019 and BS 8576:2013 stress the need for risk assessments to be based on good quality data and give guidance as to best practice in this respect.

Review of Data Quality			
Data Type	**Current Understanding**	**UK Practice**	**Recommendation**

Geological and hydro-geological conditions	The CSM identified the site to be located on limited Made Ground (<0.90m), overlying both permeable and impermeable strata of the Head Deposits over permeable Seaford Chalk Formation No groundwater has been encountered.	CIRA C665 & C659, Wilson and Card (1999) and BS 8485 recommend that geology and hydrogeology be fully understood.	The geological and hydrogeological model is understood to a standard that will not comprise the findings of the risk assessment. **No further action required.**
Soil Laboratory Analysis	A dataset of soil analysis from 6No. samples of Made Ground have identified a range of TOC concentrations between 1.1% and 1.9%.	CL:AIRE RB17 outlines an approach to ground gas risk assessment away from the influences that directly affect ground gas monitoring. It should typically be used on sites with limited sources identified in the CSM and Made Ground with average thickness <3m and maximum thickness <5m. TOC should be < 4/6%.	The TOC concentrations are below the limit of 6% in order to use the soil derived approach for the risk assessment. **No further action required.**

7.5.3. Risk Assessment

The intrusive investigation has revealed a maximum thickness of Made Ground of 0.90m overlying Head Deposits comprising cohesive and granular soils which in turn rest upon the Seaford Chalk Formation to the base of the boreholes. Site has undergone a previous phase of construction and limited degradable material has been identified. The introduction of oxygen to the Made Ground through redevelopment may lead to an initial increase of ground gas generation as degradable material within the soils present on-site may act as source of ground gas. The soils at the site are considered to be of variable permeability (low to high) in nature and pathways for ground gas migration will be present.

Using the soil derived method, it is considered that a TOC value of <1% within soil is unlikely to pose an unacceptable risk from ground gases and the site can be characterised as Characteristic Situation CS1. Any value between 3% and 6% warrants classification as a Characteristic Situation CS3. Any value over 4% requires investigation through ground gas monitoring. Any site with Made Ground <1.00m bgl will have a limited gas generation potential from the organic carbon present and consideration should be given to this thickness within the risk assessment.

An alternative approach for ground gas risk assessment (CL:AIRE RB17) can be considered utilising measured concentrations from soil parameters that are independent of influencing factors such as atmospheric pressure and groundwater level. Total Organic Carbon (TOC) was tested on 8No. samples with the analytical results ranging between 1.1 and 1.9%. Furthermore, it is considered that a thickness of <0.80m would not generate sufficient ground gas to pose an unacceptable risk.

Based on the soil derived method outlined and the maximum TOC concentration encountered, the soils from site would warrant classification as **Characteristic Situation CS1**. It should be noted that this only represents the assessment from an on-site source.

7.6. Vapour Risk Assessment

The CSM identified sources of contamination in the form of USTs that may pose an unacceptable risk to receptors through vapours. During both the previous and current investigation no olfactory or visual signs of contamination were noted in any of the soil samples collected. However, very low concentrations of hydrocarbon vapours were recorded in a small number of samples, predominantly from depths <2.00m bgl during the previous investigation.

During the removal of tanks should any olfactory or visual signs of contamination be noted then samples should be taken and tested for contaminants that may be present in the vapour phase (Mercury, Napthalene, TPHs and Volatile Organic Compounds (VOCs)) which would enable a vapour risk assessment to be undertaken. PID readings should also be taken during logging of the samples.

7.7. Updated Conceptual Site Model

Following completion of the Phase 2 Site Investigation, the CSM, inclusive of associated risk, within Section 2.8 of this report was re-evaluated.

For ease of reference and understanding, the risks have been re-assigned within this risk assessment against three possible levels / categories, summarised in the table below.

Risk Categories used in the Tabulated CSM	
Low Risk	Regarding this potential SPR linkage, the site is considered suitable for the proposed end-use and there is not considered to be an unacceptable risk to receptors.
Moderate Risk	Regarding this potential SPR linkage, the site is suitable for the proposed end-use however remediation and/or further investigation is required.
High Risk	Regarding this potential SPR linkage, the site is not suitable for proposed end-use and there is an unacceptable risk to receptors. Urgent action required in the short term.

Tabulated Conceptual Site Model – Pollutant Linkage Summary (On-Site Sources)			
Potential Sources	Potential Absorption Pathways	Potential Receptors	Risk Classification
Contaminants introduced on-site by former use of the site as a petrol filling station with underground fuel tanks/oil interceptor. • Asbestos, • PAHs, • TPHs, • VOCs, • Sulphates, and • Metals. Elevated concentrations of Dibenzo(a,h)anthracene were identified in the sample from TP10/0.20m bgl (5.1mg/kg). The location of TP10 falls under a proposed area of hardstanding and therefore does not present an unacceptable risk to future receptors and no remediation with respect to end users is required. Construction workers should wear appropriate PPE. Elevated concentrations of zinc were noted in one sample of Made Ground tested above the phytotoxicity threshold trigger value. Consequently, specialist planting may be required and the	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Low**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** • Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Low subject to findings after removal of USTs and associated infrastructure**

advice of a suitably experienced horticulturalist should be obtained. No other elevated levels noted during investigation.			
Underground fuel tank in south-east corners: • TPHs, The presence of tanks and other underground infrastructure of the re-fuelling station, gross contamination may be encountered during the removal of these features. A discovery strategy must be in place.	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works Neighbours and public	**Moderate**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** Vegetation within soft landscaped areas	**Moderate**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Moderate**
Septic tank close to eastern boundary: • Ammonia; • bacterial compounds such as faecal coliforms and faecal streptococci The void created following tank removal should be investigated.	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater • Inhalation of volatile vapour (indoors and outdoors)	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works • Neighbours and public	**Low**
	• Direct uptake of groundwater • Direct uptake of determinands in the soil	**Flora and Fauna** Vegetation within soft landscaped areas	**Low**
	• Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	**Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south	**Low**
Made Ground from construction activities: • Asbestos,	• Direct ingestion of soil and soil derived dust • Dermal contact of soil and soil derived dust • Inhalation of dust (indoors and outdoors) with elevated concentration of determinands • Direct ingestion of groundwater	**Human Health** • End Users (Site workers/Future site visitors) • Construction workers during development • Site operatives during maintenance works	**Low**

• PAHs, • Sulphates, and • Metals. Elevated concentrations of Dibenzo(a,h)anthracene were identified in the sample from TP10/0.20m bgl (5.1mg/kg). The location of TP10 falls under a proposed area of hardstanding and therefore does not present an unacceptable risk to future receptors and no remediation with respect to end users is required. Construction workers should wear appropriate PPE Elevated concentrations of zinc were noted in one sample of Made Ground tested above the phytotoxicity threshold trigger value. Consequently, specialist planting may be required and the advice of a suitably experienced horticulturalist should be obtained. No other elevated levels noted during investigation.	• Inhalation of volatile vapour (indoors and outdoors) • Direct uptake of groundwater • Direct uptake of determinands in the soil • Anthropogenic (man-made) pathways • Vertical and lateral migration in permeable strata • Surface water runoff	• Neighbours and public **Flora and Fauna** • Vegetation within soft landscaped areas **Controlled Waters** • Secondary A Aquifer (Head Deposits) • Principal Aquifer (Seaford Chalk Formation) • Stream 37m to the south •	**Moderate** **Low**
Aggressive ground conditions with Made Ground and natural ground, including groundwater:	• Direct contact with aggressive ground conditions	**Building Materials and Services** • Buried Concrete • Underground services (water pipes)	**Low (concrete)** **Moderate (water pipes)**

• Sulphates, • PAH/TPH. The sulphates fell into Class DS-1 and AC-1 Due to elevated levels of the hydrocarbons (TPH EC10 – EC16 and EC16 – EC40 (aliphatics and aromatics)) identified in chemical laboratory testing above the threshold for PE Pipe, barrier or metal pipes must be used. Additional testing and sampling of the soils at the exact depth and location of pipes should be undertaken to confirm.			
Ground gases generated by generated by the anaerobic decomposition of any leaked hydrocarbons beneath site and Made Ground: • Methane, • Carbon Dioxide, • Hydrogen Sulphide, and • Carbon Monoxide Based on soil analysis, the site falls into the CS1 classification.	• Migration through anthropogenic & natural pathways • Inhalation of Asphyxiating gases • Explosion (methane only)	**Human Health** • End Users (Residents/Future site visitors) • Construction workers during development (especially in confined spaces) • Site operatives during maintenance works in confined spaces • Neighbours and public	**Low**
	• Migration through anthropogenic & natural pathways • Explosion (methane only)	**Building Materials and Services** • Buildings • Confined spaces • Underground services	**Low**

8. CONTAMINATION RECOMMENDATIONS AND REMEDIATION STRATEGY

Based on the results of the intrusive works, chemical analyses and risk assessments undertaken, no contamination requiring remediation has been identified. Elevated concentrations of Dibenzo(a,h)anthracene were identified in the sample from TP10/0.20m bgl (5.1mg/kg). **The location of TP10 falls under a proposed area of hardstanding and therefore does not present an unacceptable risk to future receptors and no remediation with respect to end users is required** The presence of cohesive Head Deposits underlying the location of TP10 will likely mitigate any migration from the shallow soils into the underlying groundwater that will be encountered below 10m bgl.

Elevated concentrations of zinc were noted in one sample of Made Ground tested above the phytotoxicity threshold trigger value. Consequently, specialist planting may be required and the advice of a suitably experienced horticulturalist should be obtained.

Given the age of the structures on-site, an **Asbestos Management Strategy** should be put in place so that any potentially asbestos containing materials are identified and removed from site in a suitable manner to prevent cross-contamination. An Asbestos Survey should be undertaken pre-demolition of any buildings/structures.

Prior to any construction on-site, a degree of site preparation is required. This should include the decommissioning of any boreholes, demolition of existing structures, and the removal of below ground infrastructure (tanks, oil-water interceptor, pipes etc.).

Given the historical land-use of the site, the presence of tanks and other underground infrastructure of the re-fuelling station, gross contamination may be encountered during the removal of these features. A discovery strategy must be in place. Should contamination be encountered during and following removal of the USTs and the septic tank or should the proposed development change then these conclusions would need to be re-evaluated following additional testing.

8.1.1. Removal of Infrastructure and Gross Contamination

During the works, the contractor must remove all below ground tanks, chambers and pipework under the supervision of a Ground and Water Limited engineer. This process should include the off-site removal of any potentially contaminated liquids, sludges or oil product, as well as any visual or olfactorily contaminated soils that surround the infrastructure. This process is to remove any gross contamination and its source to ensure groundwater conditions do not deteriorate over time.

The metalwork and soils shall be removed and stockpiled on an impermeable liner with raised edges and covered at all times. This contamination stockpile destined for removal from site shall be stored away from imported soils to avoid cross-contamination. Materials to be removed off-site must be classified by carrying out Waste Acceptance Produce (WAP) testing. A registered contractor must undertake the removal of waste. Consignment notes for the removal of waste must be obtained and kept for inclusion within the final Validation Report.

Following removal, the void shall be inspected and samples must be taken from the base and walls of the excavation and tested for TPHs and VOCs/SVOCs to enable validation that the no associated contamination remains. Should these samples indicate unacceptable concentrations, a Petrofix

solution should be sprayed into the excavations in order to further reduce the contamination risk of the groundwater.

The PetroFix method removes hydrocarbons from the dissolved phase by adsorbing it onto activated carbon particles and then stimulates biodegradation by adding electron acceptors. This method will take around 2 weeks to remediate the site. The design and quantity of the solution should be agreed with a specialist remediation contractor.

The method adopted allows for the targeted remediation of the known location of contaminants, prior to the backfilling of the voids with clean inert validated soils.

The void shall be re-inspected with samples must be taken from the base and walls of the excavation and tested for TPHs and VOCs/SVOCs to enable validation that the no associated contamination remains.

All stages must be photographed for inclusion within the validation report.

8.2. Groundwater

Low levels of hydrocarbons were detected in the shallow Made Ground that were below the threshold for the human health receptor for the proposed development. No groundwater was encountered in the deep boreholes that were drilled to a depth of 10m bgl. The presence of cohesive Head Deposits will likely mitigate any migration from the shallow soils into the underlying groundwater that will be encountered below 10m bgl.

Should contamination be encountered during and following removal of the existing concrete slab, USTs and the septic tank then it will be possible to assess the risk to the underlying Principal Aquifer within the Seaford Chalk Formation.

8.3. Ground Gas

Based on risk assessment undertaken, the site warrants classification as **Characteristic Situation CS1**, where no precautions to prevent the ingress of ground-gas are needed.

8.4. Vapours

With regards to the risk from vapours, BTEX-MTBE analysis did not show elevated levels. Should impacted soils be discovered during the removal of the USTs and associated infrastructure, then additional sampling and testing should be undertaken. PID readings should also be taken. A vapour risk assessment should then be undertaken.

8.5. Radon

The site was in an area where a radon risk assessment and radon remedial measures were not required.

8.6. Discovery Strategy

There may be areas of contamination that have not been identified during the course of the intrusive investigation. Such occurrences may be discovered during the demolition and construction phases for the redevelopment of the site.

Groundworkers should be instructed to report to the Site Manager any evidence for such contamination; this may comprise visual indicators, such as fibrous materials within the soil, discolouration, or odours and emission. Upon discovery advice must be taken from a suitably qualified person before proceeding, such that appropriate remedial measures and health and safety protection may be applied.

Should a new source of contamination be suspected or identified then the Local Authority will need to be informed.

8.7. Imported Material

It is not envisaged that soils will be brought onto site as part of the development works, however, any soil which is to be imported onto the site must undergo chemical analysis to prove that it is suitable for the purpose for which it is intended.

The soil must be fit for purpose and must either be supplied with traceable chemical laboratory test certificates or be tested, prior to placing (ideally) and after placing, to ensure that the future receptors (human health and vegetation) cannot come into contact with compounds that could be detrimental to their health. The compounds that are to be tested for are those given in the Generic Assessment Criteria, which can be viewed in Appendix E of this report.

8.8. Waste Disposal

The excavation of foundations and other soils is likely to produce waste which will require classification and then recycling or removal from site.

Under the Landfill (England and Wales) Regulations 2002 (as amended), prior to disposal, all waste must be classified as either: Inert; Non-hazardous; or Hazardous.

The Environment Agency's Hazardous Waste Technical Guidance (WM3) document outlines the methodology for classifying wastes. Once classified the waste can be removed to the appropriately licensed facilities, with some waste requiring pre-treatments prior to disposal.

Following the investigation, 6No. samples of Made Ground (TP09/0.50m, TP10/0.20m, TP11/0.50m, CBR01/0.50m, CBR02/0.50m and CBR03/0.50m bgl) were submitted to the analytical laboratory to undergo a suite of testing for contamination testing, as discussed in the previous sections. Sampling depths were chosen to reflect the receptor of concern, human health, and typically comprised a surface or near surface sample and periodically to 1.00m bgl. Any horizon where olfactory or visual evidence of contamination was present was also sampled.

Based on a risk phase analysis of the chemical laboratory test results, in accordance with EC Hazardous Waste Directive and undertaken by Ground and Water Limited, 1No sample of Made Ground (TP11/0.50m bgl) encountered on-site were HAZARDOUS 1No sample of Made Ground (TP11/0.50m bgl) encountered on-site was HAZARDOUS due to the carcinogenic and mutagenic effects of the levels of TPHs (C6 – C40) within the sample. The remaining 5No samples of Made Ground encountered on-site were NON-HAZARDOUS. The results of the assessment are given within Appendix H.

It is important to note that whilst we consider our in-house assessment tool to be an accurate interpretation of the requirements of WM3, therefore producing an initial classification in accordance

with the guidance, this method classifies soils as either non-hazardous or hazardous and landfill operators have their own assessment tools and can often come to different conclusions. As a result, some landfill operators could refuse to take apparently suitable waste. It is recommended that the receiving landfill views the results of this assessment and the chemical laboratory results to determine their own classification.

Where contaminated soils are to be removed from the vicinity of TP10 and TP11, they should be placed on an impermeable membrane (visqueen or similar) to ensure that no cross-contamination of soils occurs.

WAC testing should be undertaken on any materials that are to be taken off site.

8.9. Duty of Care

Groundworkers must maintain a good standard of personal hygiene including the wearing of overalls, boots, gloves and eye protectors and the use of dust masks during periods of dry weather.

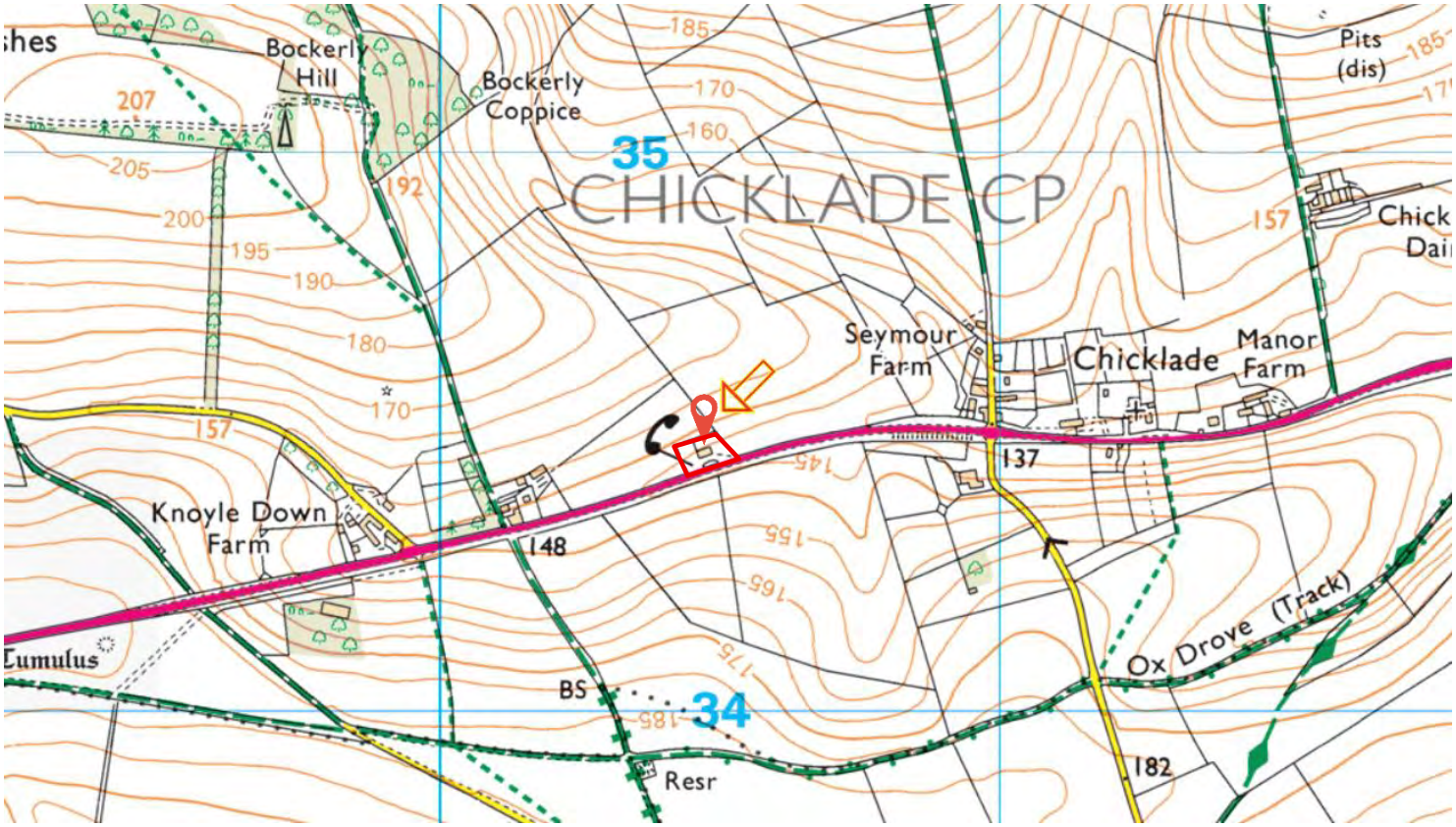
To prevent exposure to airborne dust by both the general public and construction personnel the site should be kept damp during dry weather and at other times when dust would be generated as a result of construction activities.

The site should be securely fenced at all times to prevent unauthorised access. Washing facilities should be provided and eating restricted to mess huts.

FIGURES

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

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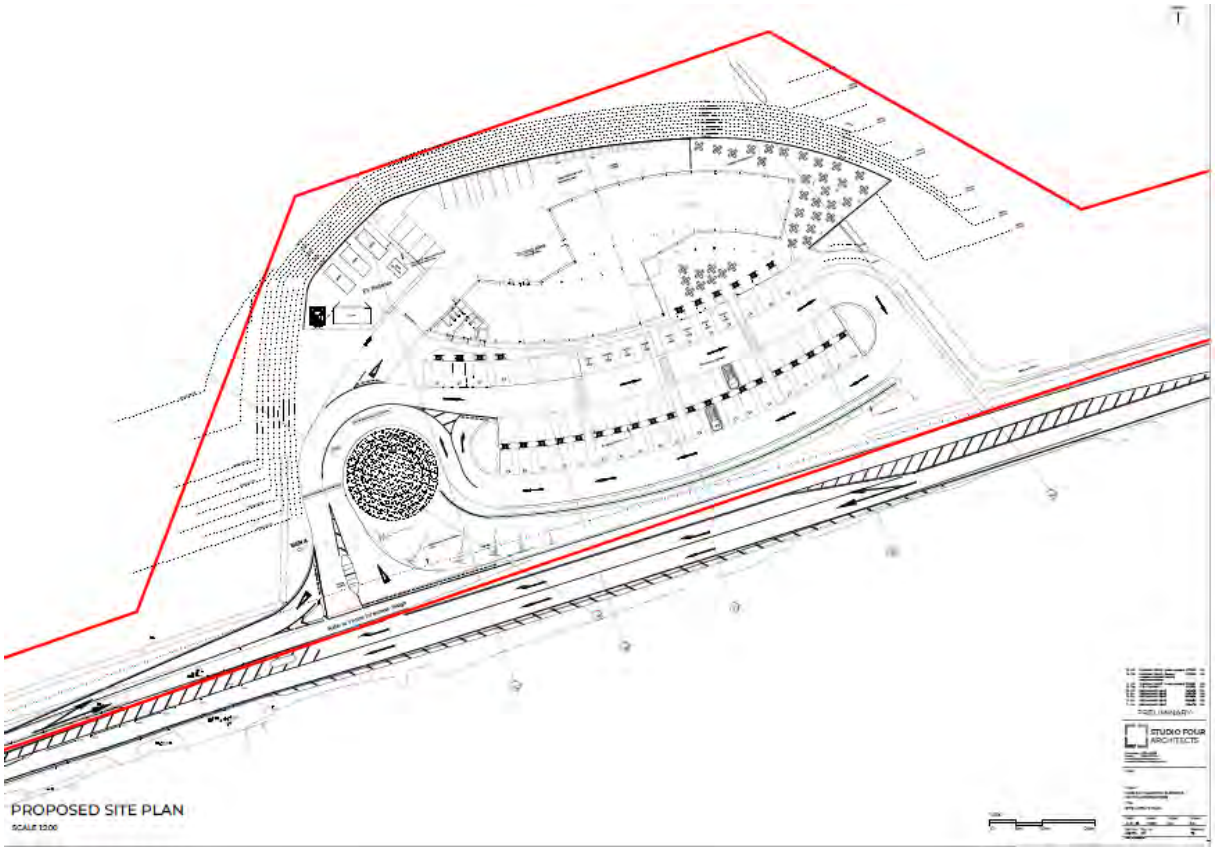


Site Location

Not to Scale

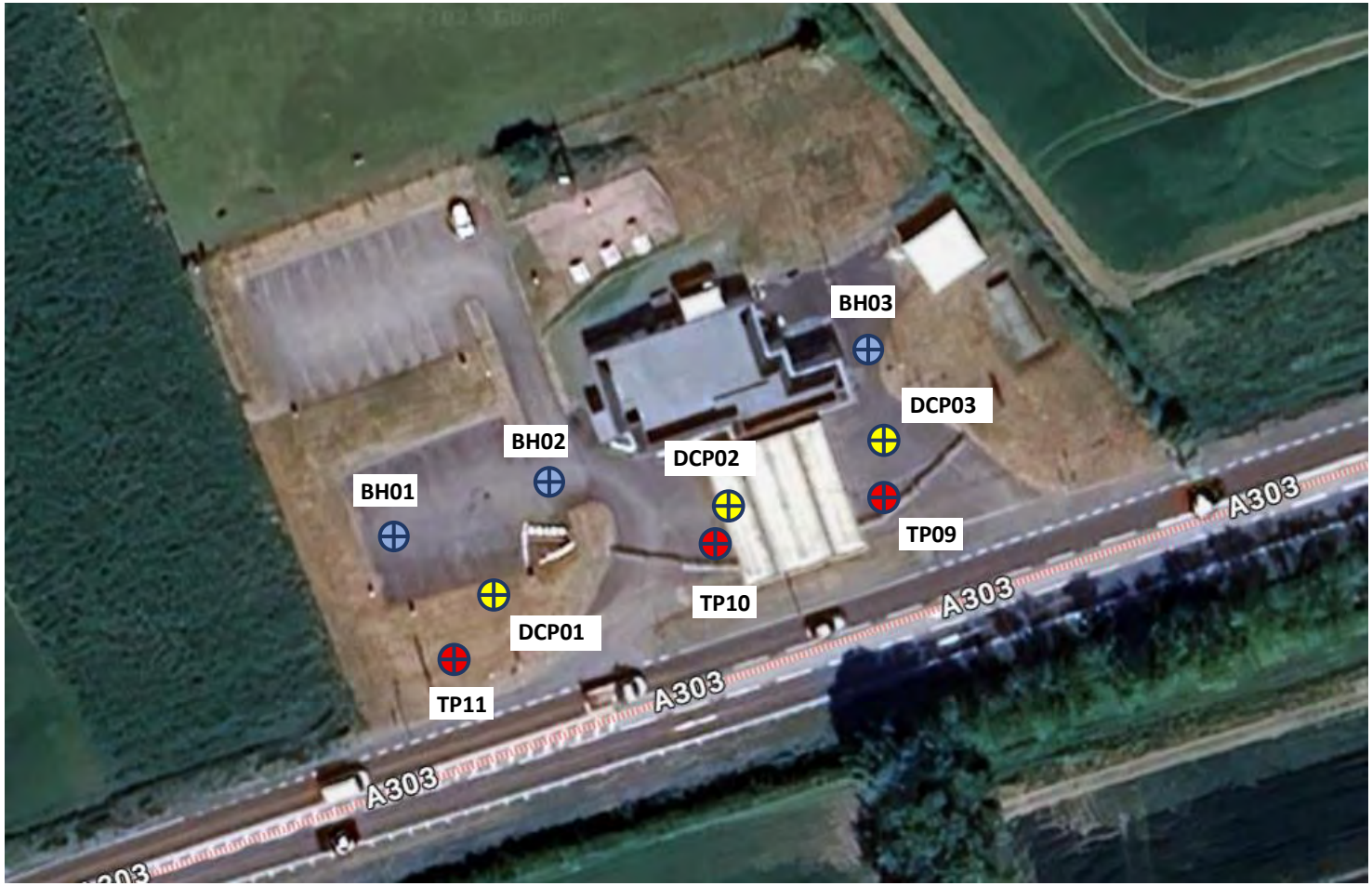
Chicklade Service Station Hindon Salisbury SP3 5ST	
Instavolt	**November 2025**
Figure 1: Site Location Plan	**GWPR6751**

ground&water



Not to Scale

Chicklade Service Station Hindon Salisbury SP3 5ST	
Instavolt	**November 2025**
Figure 2: Proposed Development Plan	**GWPR6751**



Cable Percussion Borehole

Trial Pit

DCPs

Not to Scale

Chicklade Service Station Hindon Salisbury SP3 5ST	
Instavolt	**November 2025**
Figure 3: Trial Hole Location Plan – boreholes, trial pits and DCPs	**GWPR6751**



Not to Scale

Chicklade Service Station Hindon Salisbury SP3 5ST	
Instavolt	November 2025
Figure 4: Trial Hole Location Plan soakage tests	GWPR6751

APPENDIX A: Conditions and Limitations

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The ground is a product of continuing natural and artificial processes. As a result, the ground will exhibit a variety of characteristics that vary from place to place across a site, and also with time. Whilst a ground investigation will mitigate to a greater or lesser degree against the resulting risk from variation, the risks cannot be eliminated.

The report has been prepared on the basis of information, data and materials which were available at the time of writing. Accordingly, any conclusions, opinions or judgements made in the report should not be regarded as definitive or relied upon to the exclusion of other information, opinions and judgements.

The investigation, interpretations, and recommendations given in this report were prepared for the sole benefit of the client in accordance with their brief; as such these do not necessarily address all aspects of ground behaviour at the site. No liability is accepted for any reliance placed on it by others unless specifically agreed in writing.

Any decisions made by you, or by any organisation, agency or person who has read, received or been provided with information contained in the report ("you" or "the Recipient") are decisions of the Recipient and we will not make, or be deemed to make, any decisions on behalf of any Recipient. We will not be liable for the consequences of any such decisions.

Current regulations and good practice were used in the preparation of this report. An appropriately qualified person must review the recommendations given in this report at the time of preparation of the scheme design to ensure that any recommendations given remain valid in light of changes in regulation and practice, or additional information obtained regarding the site.

Any Recipient must take into account any other factors apart from the Report of which they and their experts and advisers are or should be aware. The information, data, conclusions, opinions and judgements set out in the report may relate to certain contexts and may not be suitable in other contexts. It is your responsibility to ensure that you do not use the information we provide in the wrong context.

This report is based on readily available geological records, the recorded physical investigation, the strata observed in the works, together with the results of completed site and laboratory tests. Whilst skill and care has been taken to interpret these conditions likely between or below investigation points, the possibility of other characteristics not revealed cannot be discounted, for which no liability can be accepted. The impact of our assessment on other aspects of the development required evaluation by other involved parties.

The opinions expressed cannot be absolute due to the limitations of time and resources within the context of the agreed brief and the possibility of unrecorded previous in ground activities. The ground conditions have been sampled or monitored in recorded locations and tests for some of the more common chemicals generally expected. Other concentrations of types of chemicals may exist.

The conclusions and recommendations relate to Chicklade Service Station, Hindon, Salisbury SP3 5ST

Trial hole is a generic term used to describe a method of direct investigation. The term trial pit, borehole or window sampler borehole implies the specific technique used to produce a trial hole.

The depth to roots and/or of desiccation may vary from that found during the investigation. The client is responsible for establishing the depth to roots and/or of desiccation on a plot-by-plot basis prior to the construction of foundations. Where trees are mentioned in the text this means existing trees, recently removed trees (approximately 15 years to full recovery on cohesive soils) and those planned as part of the site landscaping.

Ownership of copyright of all printed material including reports, laboratory test results, trial pit and borehole log sheets, including drillers log sheets, remain with Ground and Water Limited. Licence is for the sole use of the client and may not be assigned, transferred or given to a third party.

Only our client may rely on this report and should this report or any information contained in it be provided to any third party we accept no responsibility to the third party for the contents of this report save to the extent expressly outlined by us in writing in a reliance letter addressed from us to the third party.

Recipients are not permitted to publish this report outside of their organisation without our express written consent.

APPENDIX B: Technical Glossary

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

TECHNICAL GLOSSARY

The list of possible definitions within the report may be seen below. Please note that some definitions may not be relevant to this report.

HYDROGEOLOGY:

A **Principal Aquifer** is a layer of rock or drift deposits that have high intergranular and/or fracture permeability - meaning they usually provide a high level of water storage. They may support water supply and/or river base flow on a strategic scale. In most cases, principal aquifers are aquifers previously designated as major aquifer.

Secondary aquifers include a wide range of drift deposits with an equally wide range of water permeability and storage capacities.

Secondary (A) Aquifers consist of deposits with permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers. These are generally aquifers formerly classified as Minor Aquifers.

Secondary (B) Aquifers consist of deposits with predominantly lower permeability layers with may stoke and yield limited amounts of groundwater due to localised features such as fissures, think permeable horizons and weathering. These are generally the water-bearing parts of the former non-aquifers.

Secondary Aquifers (Undifferentiated) are assigned in cases where it has not been possible to attribute either category A or B to a rock type. In most cases, this means that the layer in question has previously been designated as both a minor aquifer and non-aquifer in different locations due to the variable characteristics of the rock type.

Unproductive Strata are rock layers with low permeability that have negligible significance for water supply or river base flow. These were formerly classified as non-aquifers.

FLOOD ZONES:

Environment Agency Flood Zone 2, defined as; land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding; or land having between a 1 in 200 and 1 in 1,000 annual probability of sea flooding.

Environment Agency Flood Zone 3 shows the extent of a river flood with a 1 in 100 (1%0 or greater chance of occurring in any year or a sea flood with a 1 in 200 (0.5%) or greater chance of occurring in any year.

Environment Agency Flood Zone 3 area that benefits from flood defences, defined as; land and property in this flood zone would have a high probability of flooding without the local flood defences. These protect the area against a river flood with a 1% chance of happening each year, or a flood from the sea with a 0.5% chance of happening each year.

GROUNDWATER SOURCE PROTECTION ZONES (SPZS):

Inner zone (SPZ1): This zone is 50 day travel time of pollutant to source with a 50 metres default minimum radius.

Outer zone (SPZ2): This zone is 400 day travel time of pollutant to source. This has a 250 or 500 metres minimum radius around the source depending on the amount of water taken.

Total catchment (SPZ3): This is the area around a supply source within which all the groundwater ends up at the abstraction point. This is the point from where the water is taken. This could extend some distance from the source point.

Zone of special interest (SPZ4): This zone is where local conditions require additional protection.

IN-SITU STRENGTH GEOTECHNICAL TESTING:

Windowless Sample and/or Cable Percussion and/or Rotary Boreholes provide samples of the ground for assessment but they do not give any engineering data. The standard penetration test (SPT) is an in-situ dynamic penetration test designed to provide information on the geotechnical engineering properties of soil. The test uses a thick-walled sample tube, with an outside diameter of 50mm and an inside diameter of 35mm, and a length of around 650mm. This is driven into the ground at the bottom of a borehole by blows from a slide hammer with a weight of 63.5kg falling through a distance of 760mm. The sample tube is driven 150mm into the ground and then the number of blows needed for the tube to penetrate each 75mm up to a depth of 450mm is recorded. The sum of the number of blows is termed the "standard penetration resistance" or the "N-value".

Dynamic Probing involves the driving of a metal cone into the ground via a series of steel rods. These rods are driven from the surface by a hammer system that lifts and drops a 63.5kg (SHDP) hammer onto the top of the rods through a set height, thus ensuring a consistent energy input. The number of hammer blows that are required to drive the cone down by each 100mm increment are recorded. These blow counts then provide a comparative assessment from which correlations have been published, based on dynamic energy, which permits engineering parameters to be generated. (The Dynamic Probe 'Super Heavy' (SHDP) Tests were conducted in accordance with BS 1377; 1990; Part 9, Clause 3.2).

APPENDIX C: Trial Hole Logs and In-situ DCPs

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

Percussion Drilling Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 29/09/2025 - 30/09/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Drilling Equipment:

Borehole Number	Hole Type	Level	Logged By	Scale	Page Number
BH01	CP			1:50	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.10			Tarmac. (MADE GROUND).
		0.20	D					Red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%). (MADE GROUND).
		0.50	D		0.50			Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D					
		1.20 - 1.70	B					
		1.20	SPT	N=14 (3,2/4,4,3,3)				
					1.70			Recovered as a structureless CHALK composed of off white to white slightly sandy medium and coarse GRAVEL with many sub-angular cobbles. Gravel and cobbles were medium dense. Matrix is uncompact and off white. (CIRIA C574: Grade Dc). (SEAFORD CHALK FORMATION)
		2.00	SPT	N=20 (2,3/4,4,6,6)				
		2.50	B					
		3.00	SPT	N=27 (3,4/6,6,7,8)				
		3.60	D					
		4.00	SPT	N=28 (4,5/5,6,8,9)				
		4.50	D					
		5.00	SPT	N=28 (5,5/5,6,7,10)				
		5.50	D					
		6.00	SPT	N=50 (6,7/50 for 295mm)				
		7.00	D					
		7.50	SPT	N=45 (6,6/8,11,14,12)				
		8.20	SPT	N=50 (12,13/50 for 245mm)	8.20			End of Borehole at 8.200m

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation
8.20	150	1.70	150								

Remarks

Groundwater was not encountered. No roots were noted.

Percussion Drilling Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 01/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Drilling Equipment:

Borehole Number	Hole Type	Level	Logged By	Scale	Page Number
BH02	CP			1:50	Sheet 1 of 2

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.10			Tarmac. (MADE GROUND).
		0.50	D		0.50			Red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%). (MADE GROUND).
		1.00	D					Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.20 - 1.70	B					
		1.20	SPT	N=13 (3,2/2,3,4,4)				
					1.70			
		2.00	SPT	N=22 (3,4/5,5,6,6)				Recovered as a structureless CHALK composed of off white to white slightly sandy medium and coarse GRAVEL with many sub-angular cobbles. Gravel and cobbles were medium dense. Matrix is uncompact and off white. (CIRIA C574: Grade Dc). (SEAFORD CHALK FORMATION)
		2.50	D					
		3.00	SPT	N=25 (4,4/6,5,5,9)				
		3.60	D					
		4.00	SPT	N=28 (3,4/5,7,7,9)				
		4.50	D					
		5.00	SPT	N=34 (5,5/5,6,9,14)				
		5.50	D					
		6.00	SPT	N=44 (5,7/8,12,12,12)				
		7.00	D					
		7.50	SPT	N=36 (5,6/6,9,10,11)				
		8.50	D					
		9.00	SPT	N=48 (7,7/9,12,13,14)				
		9.50	D					
					10.00			

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation
10.00	150	1.20	150								

Remarks

Groundwater was not encountered. No roots were noted.

Percussion Drilling Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 01/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Drilling Equipment:

Borehole Number	Hole Type	Level	Logged By	Scale	Page Number
BH02	CP			1:50	Sheet 2 of 2

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		10.00	SPT	N=48 (6,8/10,11,12,15)				End of Borehole at 10.000m

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation
10.00	150	1.20	150								

Remarks

Groundwater was not encountered. No roots were noted.

Percussion Drilling Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 01/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Drilling Equipment:

Borehole Number	Hole Type	Level	Logged By	Scale	Page Number
BH03	CP			1:50	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing Depth (m)	Type	Results	Depth (m)	Level (m)	Legend	Stratum Description
		0.20	D		0.10			Tarmac. (MADE GROUND).
		0.50	D		0.50			Red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%). (MADE GROUND).
		1.00	D					Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.20 - 1.70	B		1.20			
		1.20	SPT	N=11 (1,2/3,2,3,3)				Recovered as a structureless CHALK composed of off white to white slightly sandy medium and coarse GRAVEL with many sub-angular cobbles. Gravel and cobbles were medium dense. Matrix is uncompact and off white. (CIRIA C574: Grade Dc). (SEAFORD CHALK FORMATION)
		2.00	SPT	N=13 (1,2/2,3,4,4)				
		2.50 - 3.00	B					
		3.00	SPT	N=13 (2,3/3,3,3,4)				
		3.50	D					
		4.00	SPT	N=19 (2,3/3,4,6,6)				
		4.50	D					
		5.00	SPT	N=27 (4,5/5,7,7,8)				
		5.50	D					
		6.00	SPT	N=28 (4,5/6,6,8,8)				
		7.00	D					
		7.50	SPT	N=42 (5,6/8,10,11,13)				
		8.50	D					
		9.00	SPT	N=38 (6,8/8,9,10,11)				
		9.50	D					
		10.00	SPT	N=40 (5,8/9,9,11,11)	10.00			End of Borehole at 10.000m

Hole Diameter		Casing Diameter		Chiselling				Inclination and Orientation			
Depth Base	Diameter	Depth Base	Diameter	Depth Top	Depth Base	Duration	Tool	Depth Top	Depth Base	Inclination	Orientation
10.00	150	1.20	150								

Remarks

Groundwater was not encountered. No roots were noted.

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 30/09/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP01	TP		AP	1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.25			Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D					Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

1
2
3
4
5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks
2.65	0.62						

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 30/09/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP02	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.30			Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D					Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

1
2
3
4
5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 30/09/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP03	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D					Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
					0.35			
		0.50	D					Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 30/09/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP04	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.25			Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D					Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

1
2
3
4
5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP05	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.30			Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D					Light brown slightly gravelly sandy gravelly CLAY. Sand is fine. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

1
2
3
4
5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP06	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.30			Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D		0.90			Brown gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			Light brown slightly gravelly sandy gravelly CLAY. Sand is fine. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
								End of Borehole at 1.000m

1
2
3
4
5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP07	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D					Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D		0.60			Light brown slightly gravelly sandy gravelly CLAY. Sand is fine. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

1 2 3 4 5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.40m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP08	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D		0.30			Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D					Light brown slightly gravelly sandy gravelly CLAY. Sand is fine. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (HEAD DEPOSITS).
		1.00	D		1.00			End of Borehole at 1.000m

1
2
3
4
5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.30m bgl

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP09	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.10			Concrete (MADE GROUND).
		0.20	D					Red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%). (MADE GROUND).
		0.50	D		0.50			Grey gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (MADE GROUND).
					0.80			Orangish brown slightly clayey gravelly fine SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint. (HEAD DEPOSITS).
		1.00	D					
		1.50	D					
		2.00	D					
		2.50	D		2.45			Structureless CHALK composed of white and pale brown gravelly sandy SILT. The matrix was pale brown and soft. The gravel was weak, white and sub-angular. (CIRIA 574: Grade Dm). (SEAFORD CHALK FORMATION).
		3.00	D		3.00			End of Borehole at 3.000m

1 2 3 4 5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. No roots were noted.

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP10	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
					0.10			Concrete (MADE GROUND).
		0.20	D					Red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%). (MADE GROUND).
		0.50	D		0.50			Orangish brown slightly clayey gravelly fine SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint. (HEAD DEPOSITS).
		1.00	D					
		1.50	D		1.40			Structureless CHALK composed of white and pale brown gravelly sandy SILT. The matrix was pale brown and soft. The gravel was weak, white and sub-angular. (CIRIA 574: Grade Dm). (SEAFORD CHALK FORMATION).
		2.00	D					
		2.50	D					
		3.00	D		3.00			End of Borehole at 3.000m

1 2 3 4 5

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

Remarks

Groundwater was not encountered. N roots were noted..

Trial Pit Log

Project Name: Chicklade Service Station	Client: Instavolt	Date: 22/10/2025
Location: Hindon, Salisbury SP3 5ST	Contractor: CJ Associates	
Project No. : GWPR6751	Crew Name:	Equipment:

Location Number	Location Type	Level	Logged By	Scale	Page Number
TP11	TP			1:25	Sheet 1 of 1

Well	Water Strikes	Sample and In Situ Testing			Depth (m)	Level (m)	Legend	Stratum Description
		Depth (m)	Type	Results				
		0.20	D					Brown slightly sandy gravelly CLAY. The sand was fine to medium grained and the gravel was fine, sub-angular flint and chalk (TOPSOIL).
		0.50	D					
					0.60			Grey gravelly CLAY. Gravel is fine to coarse, sub-angular to sub-rounded flint and chalk. (MADE GROUND).
					0.90			Orangish brown slightly clayey gravelly fine SAND. Gravel is fine to coarse, sub-angular to sub-rounded flint. (HEAD DEPOSITS).
		1.00	D					
		1.50	D					
		2.00	D					
		2.50	D					
					2.70			Structureless CHALK composed of white and pale brown gravelly sandy SILT. The matrix was pale brown and soft. The gravel was weak, white and sub-angular. (CIRIA 574: Grade Dm). (SEAFORD CHALK FORMATION).
		3.00	D		3.00			End of Borehole at 3.000m

Dimensions		Trench Support and Comment			Pumping Data		
Pit Length	Pit Width	Pit Stability	Shoring Used	Remarks	Date	Rate	Remarks

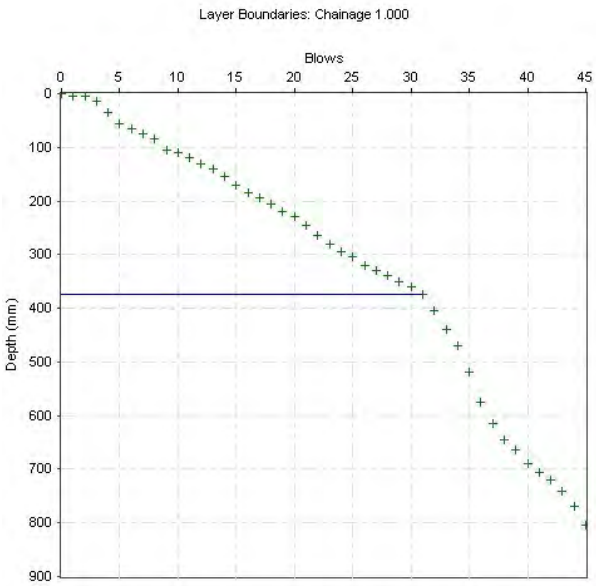
Remarks

Groundwater was not encountered. Roots were noted. to a depth of 0.60m bgl

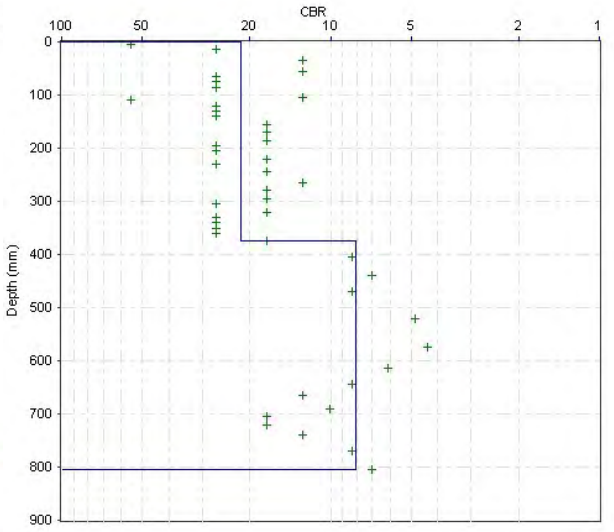
DCP Layer Strength Analysis Report

Project Name: GWPR6751 DCP1

Chainage (km):	1.000	Surface Type:	Unpaved
Direction:		Thickness (mm):	0
Location/Offset:	Lay-by / other/1.00m	Base Type:	
Cone Angle:	60 degrees	Thickness (mm):	
Zero Error (mm):	135	Surface Moisture:	Unknown
Test Date:	22/10/2025	Moisture adjustment factor:	Not adjusted



Layer Boundaries Chart



CBR Chart

Layer Properties

No.	Penetration Rate (mm/blow)	CBR (%)	Thickness (mm)	Depth to layer bottom (mm)
1	12.10	22	375	375
2	30.71	8	430	805

CBR Relationship:

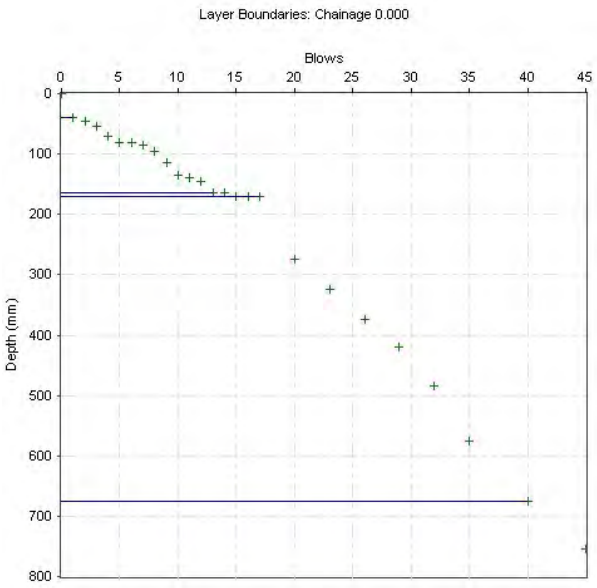
TRL equation: $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$

Report produced by .....

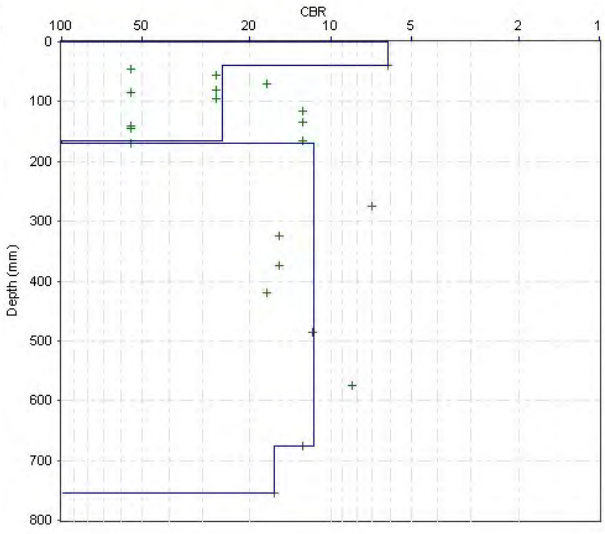
DCP Layer Strength Analysis Report

Project Name: GWPR6751 DCP2

Chainage (km):	0.000	Surface Type:	Unpaved
Direction:		Thickness (mm):	0
Location/Offset:	Carriageway	Base Type:	
Cone Angle:	60 degrees	Thickness (mm):	
Zero Error (mm):	175	Surface Moisture:	Unknown
Test Date:	01/10/2025	Moisture adjustment factor:	Not adjusted



Layer Boundaries Chart



CBR Chart

Layer Properties

No.	Penetration Rate (mm/blow)	CBR (%)	Thickness (mm)	Depth to layer bottom (mm)
1	40.00	6	40	40
2	10.42	25	125	165
3	1.25	239	5	170
4	21.96	12	505	675
5	16.00	16	80	755

CBR Relationship:

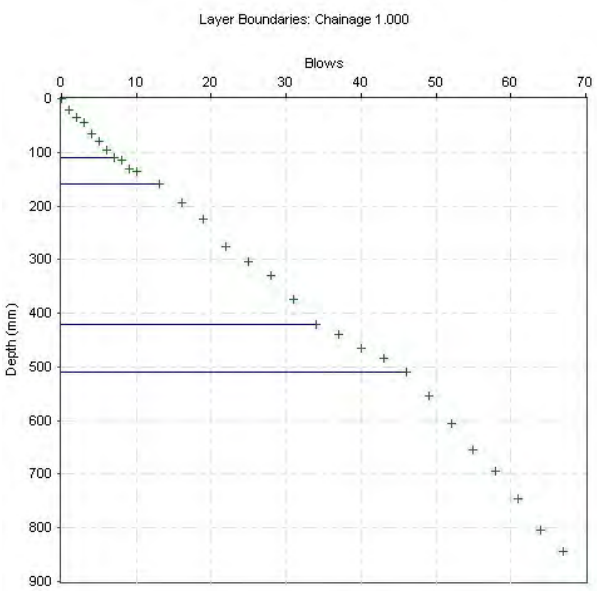
TRL equation: $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$

Report produced by .....

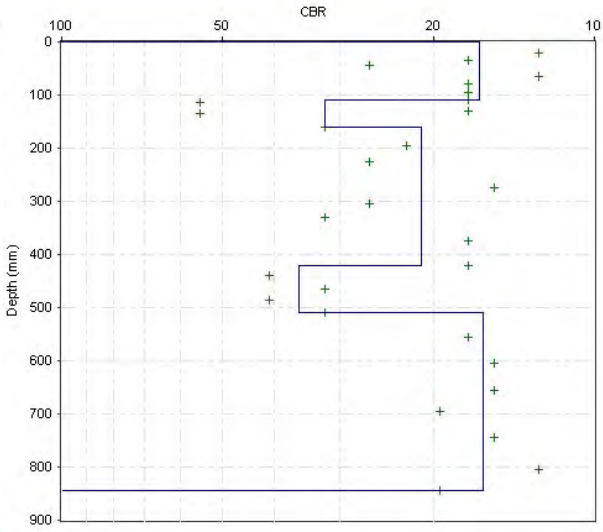
DCP Layer Strength Analysis Report

Project Name: GWPR6751 DCP3

Chainage (km):	1.000	Surface Type:	Unpaved
Direction:		Thickness (mm):	0
Location/Offset:	Lay-by / other/1.00m	Base Type:	
Cone Angle:	60 degrees	Thickness (mm):	
Zero Error (mm):	105	Surface Moisture:	Unknown
Test Date:	22/10/2025	Moisture adjustment factor:	Not adjusted



Layer Boundaries Chart



CBR Chart

Layer Properties

No.	Penetration Rate (mm/blow)	CBR (%)	Thickness (mm)	Depth to layer bottom (mm)
1	15.71	16	110	110
2	8.33	32	50	160
3	12.38	21	260	420
4	7.50	36	90	510
5	15.95	16	335	845

CBR Relationship:

TRL equation: $\log_{10}(CBR) = 2.48 - 1.057 \times \log_{10}(Strength)$

Report produced by .....

APPENDIX D: Geotechnical Laboratory Testing

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

Laboratory Report

GSTL
GEOTECHNICAL SITE & TESTING LABORATORIES
A PHENNA GROUP COMPANY

UKAS TESTING 2788

Contract Number: 81755

Client Ref: **GWPR6751**
Client PO: **GWPR6751**

Date Received: **17-10-2025**
Date Completed: **31-10-2025**
Report Date: **31-10-2025**

Client: **Ground and Water Limited**

This report has been checked and approved by:

Contract Title: **Chicklade Service Station, Hindon, Salisbury, SP3 5ST**
For the attention of: **Roger Foord**

Shaun Jones
Laboratory manager

Description	Qty
Determination of water content BS EN ISO 17892-1:2014 - * UKAS	3
1 point Liquid & Plastic Limit.. BS EN ISO 17892-12 - * UKAS	3
Saturated Moisture Content of chalk (SMC) BS 1377:1990 - Part 2 : 3.3 - * UKAS	6

Notes: **Observations and Interpretations are outside the UKAS Accreditation**
*** - denotes test included in laboratory scope of accreditation**
- denotes test carried out by approved contractor
@ - denotes non accredited tests

This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This test report/certificate shall not be reproduced except in full, without the approval of GEO Site & Testing Services Ltd. Any opinions or interpretations stated - within this report/certificate are excluded from the laboratories UKAS accreditation.

Approved Signatories:
Brendan Evans (Business Support Co-ordinator) - Darren Bourne (Quality Senior Technician) - Luke Williams (Senior Laboratory Technician)
Paul Evans (Director) - Richard John (Quality/Technical Manager) - Shaun Jones (Laboratory manager)
Shaun Thomas (Site Manager) - Wayne Honey (HR & HSE Manager)

GEO Site & Testing Services Ltd
Unit 3-4 Heol Aur, Dafen Ind Est, Llanelli, Carmarthenshire SA14 8QN
Tel: 01554 784 040 Fax: 01554 784 040 info@gstl.co.uk https://gstl.co.uk

GSTL GEOTECHNICAL SITE & TESTING LABORATORIES	**WATER CONTENT, LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX** **BS 1377-2:2022 Clause 5.3** **BS EN ISO 17892-12:2018+A2:2022 1 Point Liquid Limit** **BS EN ISO 17892-1:2014+A1:2022 Determination of Water Content**	
Contract Number	81755	
Project Name	Chicklade Service Station, Hindon, Salisbury, SP3 5ST	
Date Tested	31/10/2025	
	DESCRIPTIONS	

Sample/Hole Reference	Sample Number	Sample Type	Depth (m)			Descriptions
BH1		D	1.00	-		Brown fine to coarse gravelly silty CLAY
BH2		D	1.00	-		Brown fine to coarse gravelly silty CLAY
BH3		D	1.00	-		Brown fine to coarse gravelly silty CLAY
				-		
				-		
				-		
				-		
				-		
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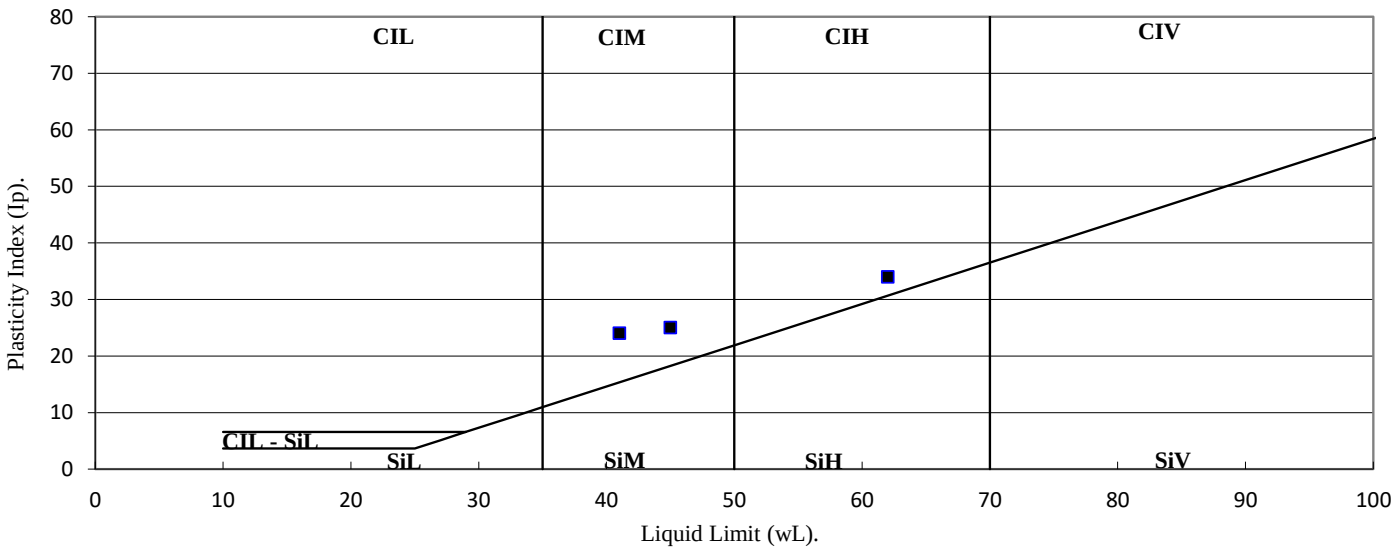
Operator
Aaron. H

GSTL GEOTECHNICAL SITE & TESTING LABORATORIES	**WATER CONTENT, LIQUID LIMIT, PLASTIC LIMIT AND PLASTICITY INDEX** **BS 1377-2:2022 Clause 5.3** **BS EN ISO 17892-12:2018+A2:2022 1 Point Liquid Limit** **BS EN ISO 17892-1:2014+A1:2022 Determination of Water Content**	
Contract Number	81755	
Project Name	Chicklade Service Station, Hindon, Salisbury, SP3 5ST	
Date Tested	31/10/2025	
Test Comments	80g/30° Fall cone used	

Sample/Hole Reference	Sample Number	Sample Type	Depth (m)			Water Content %	Liquid Limit %	Plastic Limit %	Plasticity index %	Passing 0.425mm %	Factor Applied	Remarks
BH1		D	1.00	-		33.5	62	28	34	66	1.034	CH High Plasticity
BH2		D	1.00	-		24.1	45	20	25	66	1.030	CI Intermediate Plasticity
BH3		D	1.00	-		23.3	41	17	24	74	1.018	CI Intermediate Plasticity
				-								
				-								
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SYMBOLS : NP = Non Plastic

PLASTICITY CHART
BS EN ISO 14688-2:2018 Clause 4.4



***For sample descriptions please see sample descriptions sheet**

Operator
Aaron. H

GSTL
GEOTECHNICAL SITE & TESTING LABORATORIES

Saturation Moisture Content of Chalk
BS 1377 : Part 2 : 1990 Section 3.3

Contract Number	81755
Project Name	Chicklade Service Station, Hindon, Salisbury, SP3 5ST
Date Tested	30/10/2025

Hole Reference	Sample Number	Sample Type	Depth (m)			Moisture Content	Bulk Density	Dry Density	Saturated Moisture Content	Remarks
BH1		D	2.50	-		22	2.00	1.64	24	NON-STANDARD
BH1		D	7.00	-		23	2.04	1.65	23	NON-STANDARD
BH2		D	2.50	-		25	2.03	1.62	25	NON-STANDARD
BH2		D	8.50	-		23	2.03	1.65	23	NON-STANDARD
BH3		D	3.50	-		27	1.97	1.55	27	NON-STANDARD
BH3		D	9.50	-		22	2.08	1.71	21	NON-STANDARD
				-						
				-						
				-						
				-						
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Key	Reported As	Clause
Moisture Content	%	3.2
Bulk Density	Mg/m^3	7.2
Dry Density	Mg/m^3	7.2
SMC	%	3.3.6

If there is a NON-STANDARD in the remarks next to the reported SMC result this means there was no lump of chalk of sufficient volume (300mL-500mL) present in the samples received to meet the testing requirements of the stated standard.

Operator
Julian Jones

APPENDIX E: Chemical Laboratory Testing

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

Ground and Water Ltd
2 The Long Barn
Norton Farm
Selbourne Road
Alton
Hampshire
GU34 3NB

t:

e: roger.foord@groundandwater.co.uk

i2 Analytical Ltd.
7 Woodshots Meadow,
Croxley Green
Business Park,
Watford,
Herts,
WD18 8YS

t: 01923 225404
f: 01923 237404
e: info-i2analytical@normecgroup.com

Analytical Report Number : 25-057486

Project / Site name:	Chickland Service Station Hindon Salisbury	**Samples received on:**	23/10/2025
Your job number:	GWPR6751	**Samples instructed on/ Analysis started on:**	23/10/2025
Your order number:	GWPR6751	**Analysis completed by:**	31/10/2025
Report Issue Number:	1	**Report issued on:**	31/10/2025
Samples Analysed:	6 soil samples		

Signed:

Anna Goc
PL Head of Reporting Team
For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41-711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

soils	- 4 weeks from reporting
leachates	- 2 weeks from reporting
waters	- 2 weeks from reporting
asbestos	- 6 months from reporting
air	- once the analysis is complete

Excel copies of reports are only valid when accompanied by this PDF certificate.

Retention period for records and reports is minimum 6 years from the date of issue of the final report.
Some records may be kept for longer according to other legal/best practice requirements.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement.
Application of uncertainty of measurement would provide a range within which the true result lies.
An estimate of measurement uncertainty can be provided on request.

This certificate should not be reproduced, except in full, without the express permission of the laboratory.
The results included within the report are representative of the samples submitted for analysis.

Analytical Report Number: 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury
Your Order No: GWPR6751

Lab Sample Number				725824	725825	725826	725827	725828
Sample Reference				TP09	TP10	TP11	CBR01	CBR02
Sample Number				None Supplied	None Supplied	None Supplied	None Supplied	None Supplied
Water Matrix				N/A	N/A	N/A	N/A	N/A
Depth (m)				0.50	0.20	0.50	0.50	0.50
Date Sampled				22/10/2025	22/10/2025	22/10/2025	22/10/2025	22/10/2025
Time Taken				None Supplied	None Supplied	None Supplied	None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Test Limit of detection	Test Accreditation Status					
Stone Content	%	0.1	NONE	64.6	81.6	64	49.7	76.2
Moisture Content	%	0.01	NONE	9.3	4.3	9	3.8	6
Total mass of sample received	kg	0.1	NONE	1.5	2	1.3	1.6	1.2
Asbestos								
Asbestos in Soil Detected/Not Detected	Type	N/A	ISO 17025	Not-detected	Not-detected	Not-detected	Not-detected	Not-detected
Asbestos Analyst ID	N/A	N/A	N/A	KWB	KWB	KWB	KWB	KWB
Analysis completed	N/A	N/A	N/A	29/10/2025	29/10/2025	29/10/2025	29/10/2025	29/10/2025
General Inorganics								
pH (L099)	pH Units	N/A	MCERTS	8.4	8.4	8.2	8.5	8.2
Total Cyanide	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Water Soluble Sulphate as SO_4 16hr extraction (2:1)	mg/kg	2.5	MCERTS	120	73	40	140	100
Water Soluble SO_4 16hr extraction (2:1)	mg/l	1.25	MCERTS	60.8	36.5	20.2	68	51.7
Organic Matter (automated)	%	0.1	MCERTS	2.1	2.5	4.8	2	3.3
Total Organic Carbon (TOC) - Automated	%	0.1	MCERTS	1.2	1.4	2.8	1.1	1.9
Total Phenols								
Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0	< 1.0	< 1.0	< 1.0	< 1.0
Speciated PAHs								
Naphthalene	mg/kg	0.05	MCERTS	< 0.05	< 0.05	< 0.05	< 0.05	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	0.33	0.48	0.32	0.31	0.3
Acenaphthene	mg/kg	0.05	MCERTS	0.87	0.69	1.7	0.74	0.77
Fluorene	mg/kg	0.05	MCERTS	0.53	0.55	1.1	0.55	0.72
Phenanthrene	mg/kg	0.05	MCERTS	6.5	4	11	3	4.5
Anthracene	mg/kg	0.05	MCERTS	2.6	2	4.1	1.4	1.9
Fluoranthene	mg/kg	0.05	MCERTS	31	23	23	14	13
Pyrene	mg/kg	0.05	MCERTS	29	21	23	13	12
Benzo(a)anthracene	mg/kg	0.05	MCERTS	19	14	11	7.7	7.7
Chrysene	mg/kg	0.05	MCERTS	19	14	11	9.6	8.2
Benzo(b)fluoranthene	mg/kg	0.05	ISO 17025	32	43	14	25	24
Benzo(k)fluoranthene	mg/kg	0.05	ISO 17025	13	11	6.3	8.1	8.4
Benzo(a)pyrene	mg/kg	0.05	MCERTS	23	31	13	19	19
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	12	24	6.6	13	13
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	3.2	5.1	0.75	3.1	2.8
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	12	27	8	15	14
Total PAH								
Speciated Total EPA-16 PAHs	mg/kg	0.8	ISO 17025	204	219	134	132	131

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The results included within the report are representative of the samples submitted for analysis.

Analytical Report Number: 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury
Your Order No: GWPR6751

Lab Sample Number				725824	725825	725826	725827	725828
Sample Reference				TP09	TP10	TP11	CBR01	CBR02
Sample Number				None Supplied	None Supplied	None Supplied	None Supplied	None Supplied
Water Matrix				N/A	N/A	N/A	N/A	N/A
Depth (m)				0.50	0.20	0.50	0.50	0.50
Date Sampled				22/10/2025	22/10/2025	22/10/2025	22/10/2025	22/10/2025
Time Taken				None Supplied	None Supplied	None Supplied	None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Test Limit of detection	Test Accreditation Status					
Heavy Metals / Metalloids								
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	8.8	13	14	12	10
Boron (water soluble)	mg/kg	0.2	MCERTS	0.8	0.6	1.1	0.3	1.3
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	1	1.3	1.1	1.1	0.8
Chromium (hexavalent)	mg/kg	1.8	MCERTS	< 1.8	< 1.8	< 1.8	< 1.8	< 1.8
Chromium (aqua regia extractable)	mg/kg	1	MCERTS	11	14	16	18	15
Copper (aqua regia extractable)	mg/kg	1	MCERTS	7.9	9.3	19	11	6.5
Lead (aqua regia extractable)	mg/kg	1	MCERTS	70	82	53	50	60
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	< 0.3	< 0.3	< 0.3	< 0.3	< 0.3
Nickel (aqua regia extractable)	mg/kg	1	MCERTS	8.6	13	11	17	14
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	< 1.0	1.1	< 1.0	1	1
Vanadium (aqua regia extractable)	mg/kg	1	MCERTS	14	16	20	17	19
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	160	250	310	210	170
Petroleum Hydrocarbons								
TPHCWG - Aliphatic >EC5 - EC6 $_{HS\_1D\_AL}$	mg/kg	0.01	MCERTS	< 0.010	-	< 0.010	-	< 0.010
TPHCWG - Aliphatic >EC6 - EC8 $_{HS\_1D\_AL}$	mg/kg	0.01	MCERTS	< 0.010	-	< 0.010	-	< 0.010
TPHCWG - Aliphatic >EC8 - EC10 $_{HS\_1D\_AL}$	mg/kg	0.01	MCERTS	< 0.010	-	< 0.010	-	< 0.010
TPHCWG - Aliphatic >EC10 - EC12 $_{EH\_CU\_1D\_AL}$	mg/kg	1	MCERTS	1.6	-	6	-	1.4
TPHCWG - Aliphatic >EC12 - EC16 $_{EH\_CU\_1D\_AL}$	mg/kg	2	MCERTS	3.8	-	16	-	20
TPHCWG - Aliphatic >EC16 - EC21 $_{EH\_CU\_1D\_AL}$	mg/kg	8	MCERTS	< 8.0	-	32	-	41
TPHCWG - Aliphatic >EC21 - EC35 $_{EH\_CU\_1D\_AL}$	mg/kg	8	MCERTS	23	-	130	-	74
TPHCWG - Aliphatic >EC35 - EC40 $_{EH\_CU\_1D\_AL}$	mg/kg	10	NONE	< 10	-	44	-	23
TPHCWG - Aliphatic >EC5 - EC35 $_{EH\_CU+HS\_1D\_AL}$	mg/kg	10	NONE	29	-	180	-	140
TPHCWG - Aliphatic >EC5 - EC40 $_{EH\_CU+HS\_1D\_AL}$	mg/kg	10	NONE	29	-	230	-	160
TPHCWG - Aromatic >EC5 - EC7 $_{HS\_1D\_AR}$	mg/kg	0.01	MCERTS	< 0.010	-	< 0.010	-	< 0.010
TPHCWG - Aromatic >EC7 - EC8 $_{HS\_1D\_AR}$	mg/kg	0.01	MCERTS	< 0.010	-	< 0.010	-	< 0.010
TPHCWG - Aromatic >EC8 - EC10 $_{HS\_1D\_AR}$	mg/kg	0.02	MCERTS	< 0.020	-	< 0.020	-	< 0.020
TPHCWG - Aromatic >EC10 - EC12 $_{EH\_CU\_1D\_AR}$	mg/kg	1	MCERTS	< 1.0	-	< 1.0	-	< 1.0
TPHCWG - Aromatic >EC12 - EC16 $_{EH\_CU\_1D\_AR}$	mg/kg	2	MCERTS	3.6	-	18	-	6.2
TPHCWG - Aromatic >EC16 - EC21 $_{EH\_CU\_1D\_AR}$	mg/kg	10	MCERTS	54	-	240	-	59
TPHCWG - Aromatic >EC21 - EC35 $_{EH\_CU\_1D\_AR}$	mg/kg	10	MCERTS	190	-	670	-	350
TPHCWG - Aromatic >EC35 - EC40 $_{EH\_CU\_1D\_AR}$	mg/kg	10	NONE	24	-	140	-	60
TPHCWG - Aromatic >EC5 - EC35 $_{EH\_CU+HS\_1D\_AR}$	mg/kg	10	NONE	250	-	930	-	420
TPHCWG - Aromatic >EC5 - EC40 $_{EH\_CU+HS\_1D\_AR}$	mg/kg	10	NONE	280	-	1100	-	480
TPH Total >EC5 - EC40 $_{EH\_CU+HS\_1D\_TOTAL}$	mg/kg	10	NONE	300	-	1300	-	630
VOCs								
MTBE (Methyl Tertiary Butyl Ether)	µg/kg	5	MCERTS	< 5.0	-	< 5.0	-	< 5.0
Benzene	µg/kg	5	MCERTS	< 5.0	-	< 5.0	-	< 5.0
Toluene	µg/kg	5	MCERTS	< 5.0	-	< 5.0	-	< 5.0
Ethylbenzene	µg/kg	5	MCERTS	< 5.0	-	< 5.0	-	< 5.0
p & m-Xylene	µg/kg	8	MCERTS	< 8.0	-	< 8.0	-	< 8.0
o-Xylene	µg/kg	5	MCERTS	< 5.0	-	< 5.0	-	< 5.0

U/S = Unsuitable Sample I/S = Insufficient Sample ND = Not detected

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The results included within the report are representative of the samples submitted for analysis.

Analytical Report Number: 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury
Your Order No: GWPR6751

Lab Sample Number				725829
Sample Reference				CBR03
Sample Number				None Supplied
Water Matrix				N/A
Depth (m)				0.50
Date Sampled				22/10/2025
Time Taken				None Supplied
Analytical Parameter (Soil Analysis)	Units	Test Limit of detection	Test Accreditation Status	
Stone Content	%	0.1	NONE	57.3
Moisture Content	%	0.01	NONE	3.1
Total mass of sample received	kg	0.1	NONE	1.7

Asbestos

Asbestos in Soil Detected/Not Detected	Type	N/A	ISO 17025	Not-detected
Asbestos Analyst ID	N/A	N/A	N/A	KWB
Analysis completed	N/A	N/A	N/A	29/10/2025

General Inorganics

pH (L099)	pH Units	N/A	MCERTS	8.2
Total Cyanide	mg/kg	1	MCERTS	< 1.0
Water Soluble Sulphate as SO_4 16hr extraction (2:1)	mg/kg	2.5	MCERTS	220
Water Soluble SO_4 16hr extraction (2:1)	mg/l	1.25	MCERTS	109
Organic Matter (automated)	%	0.1	MCERTS	2.3
Total Organic Carbon (TOC) - Automated	%	0.1	MCERTS	1.3

Total Phenols

Total Phenols (monohydric)	mg/kg	1	MCERTS	< 1.0

Speciated PAHs

Naphthalene	mg/kg	0.05	MCERTS	< 0.05
Acenaphthylene	mg/kg	0.05	MCERTS	< 0.05
Acenaphthene	mg/kg	0.05	MCERTS	< 0.05
Fluorene	mg/kg	0.05	MCERTS	< 0.05
Phenanthrene	mg/kg	0.05	MCERTS	< 0.05
Anthracene	mg/kg	0.05	MCERTS	< 0.05
Fluoranthene	mg/kg	0.05	MCERTS	0.16
Pyrene	mg/kg	0.05	MCERTS	0.16
Benzo(a)anthracene	mg/kg	0.05	MCERTS	0.1
Chrysene	mg/kg	0.05	MCERTS	0.09
Benzo(b)fluoranthene	mg/kg	0.05	ISO 17025	0.16
Benzo(k)fluoranthene	mg/kg	0.05	ISO 17025	< 0.05
Benzo(a)pyrene	mg/kg	0.05	MCERTS	0.11
Indeno(1,2,3-cd)pyrene	mg/kg	0.05	MCERTS	< 0.05
Dibenz(a,h)anthracene	mg/kg	0.05	MCERTS	< 0.05
Benzo(ghi)perylene	mg/kg	0.05	MCERTS	0.06

Total PAH

Speciated Total EPA-16 PAHs	mg/kg	0.8	ISO 17025	0.84

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Analytical Report Number: 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury
Your Order No: GWPR6751

Lab Sample Number				725829
Sample Reference				CBR03
Sample Number				None Supplied
Water Matrix				N/A
Depth (m)				0.50
Date Sampled				22/10/2025
Time Taken				None Supplied
Analytical Parameter (Soil Analysis)	Units	Test Limit of detection	Test Accreditation Status	
Heavy Metals / Metalloids				
Arsenic (aqua regia extractable)	mg/kg	1	MCERTS	12
Boron (water soluble)	mg/kg	0.2	MCERTS	15
Cadmium (aqua regia extractable)	mg/kg	0.2	MCERTS	1
Chromium (hexavalent)	mg/kg	1.8	MCERTS	< 1.8
Chromium (aqua regia extractable)	mg/kg	1	MCERTS	16
Copper (aqua regia extractable)	mg/kg	1	MCERTS	9.1
Lead (aqua regia extractable)	mg/kg	1	MCERTS	71
Mercury (aqua regia extractable)	mg/kg	0.3	MCERTS	< 0.3
Nickel (aqua regia extractable)	mg/kg	1	MCERTS	13
Selenium (aqua regia extractable)	mg/kg	1	MCERTS	< 1.0
Vanadium (aqua regia extractable)	mg/kg	1	MCERTS	17
Zinc (aqua regia extractable)	mg/kg	1	MCERTS	160
Petroleum Hydrocarbons				
TPHCWG - Aliphatic >EC5 - EC6 $_{HS\_1D\_AL}$	mg/kg	0.01	MCERTS	-
TPHCWG - Aliphatic >EC6 - EC8 $_{HS\_1D\_AL}$	mg/kg	0.01	MCERTS	-
TPHCWG - Aliphatic >EC8 - EC10 $_{HS\_1D\_AL}$	mg/kg	0.01	MCERTS	-
TPHCWG - Aliphatic >EC10 - EC12 $_{EH\_CU\_1D\_AL}$	mg/kg	1	MCERTS	-
TPHCWG - Aliphatic >EC12 - EC16 $_{EH\_CU\_1D\_AL}$	mg/kg	2	MCERTS	-
TPHCWG - Aliphatic >EC16 - EC21 $_{EH\_CU\_1D\_AL}$	mg/kg	8	MCERTS	-
TPHCWG - Aliphatic >EC21 - EC35 $_{EH\_CU\_1D\_AL}$	mg/kg	8	MCERTS	-
TPHCWG - Aliphatic >EC35 - EC40 $_{EH\_CU\_1D\_AL}$	mg/kg	10	NONE	-
TPHCWG - Aliphatic >EC5 - EC35 $_{EH\_CU+HS\_1D\_AL}$	mg/kg	10	NONE	-
TPHCWG - Aliphatic >EC5 - EC40 $_{EH\_CU+HS\_1D\_AL}$	mg/kg	10	NONE	-
TPHCWG - Aromatic >EC5 - EC7 $_{HS\_1D\_AR}$	mg/kg	0.01	MCERTS	-
TPHCWG - Aromatic >EC7 - EC8 $_{HS\_1D\_AR}$	mg/kg	0.01	MCERTS	-
TPHCWG - Aromatic >EC8 - EC10 $_{HS\_1D\_AR}$	mg/kg	0.02	MCERTS	-
TPHCWG - Aromatic >EC10 - EC12 $_{EH\_CU\_1D\_AR}$	mg/kg	1	MCERTS	-
TPHCWG - Aromatic >EC12 - EC16 $_{EH\_CU\_1D\_AR}$	mg/kg	2	MCERTS	-
TPHCWG - Aromatic >EC16 - EC21 $_{EH\_CU\_1D\_AR}$	mg/kg	10	MCERTS	-
TPHCWG - Aromatic >EC21 - EC35 $_{EH\_CU\_1D\_AR}$	mg/kg	10	MCERTS	-
TPHCWG - Aromatic >EC35 - EC40 $_{EH\_CU\_1D\_AR}$	mg/kg	10	NONE	-
TPHCWG - Aromatic >EC5 - EC35 $_{EH\_CU+HS\_1D\_AR}$	mg/kg	10	NONE	-
TPHCWG - Aromatic >EC5 - EC40 $_{EH\_CU+HS\_1D\_AR}$	mg/kg	10	NONE	-
TPH Total >EC5 - EC40 $_{EH\_CU+HS\_1D\_TOTAL}$	mg/kg	10	NONE	-
VOCs				
MTBE (Methyl Tertiary Butyl Ether)	µg/kg	5	MCERTS	-
Benzene	µg/kg	5	MCERTS	-
Toluene	µg/kg	5	MCERTS	-
Ethylbenzene	µg/kg	5	MCERTS	-
p & m-Xylene	µg/kg	8	MCERTS	-
o-Xylene	µg/kg	5	MCERTS	-

U/S = Unsuitable Sample I/S = Insufficient Sample ND = Not detected

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The results included within the report are representative of the samples submitted for analysis.

Analytical Report Number : 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury

* These descriptions are only intended to act as a cross check if sample identities are questioned. The major constituent of the sample is intended to act with respect to MCERTS validation. The laboratory is accredited for sand, clay and loam (MCERTS) soil types. Data for unaccredited types of solid should be interpreted with care.

Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
725824	TP09	None Supplied	0.5	Brown clay and sand with gravel and stones
725825	TP10	None Supplied	0.2	Brown clay and sand with gravel and stones
725826	TP11	None Supplied	0.5	Brown loam and sand with vegetation and stones
725827	CBR01	None Supplied	0.5	Brown gravelly sand with stones
725828	CBR02	None Supplied	0.5	Brown clay and sand with gravel and stones
725829	CBR03	None Supplied	0.5	Brown rubble with stones [g]

Analytical Report Number : 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury

Water matrix abbreviations:
Surface Water (SW) Potable Water (PW) Ground Water (GW) Process Waters Heating/Cooling (PrW) DI Process Water (DI PrW)
Final Sewage Effluent (FSE) Landfill Leachate (LL)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Asbestos identification in Soil	Asbestos Identification with the use of polarised light microscopy in conjunction with dispersion staining techniques	In-house method based on HSG 248, 2021	A001B	D	ISO 17025
Organic matter (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate (Walkley Black Method)	In-house method	L009B	D	MCERTS
Total organic carbon (Automated) in soil	Determination of organic matter in soil by oxidising with potassium dichromate followed by titration with iron (II) sulphate (Walkley Black Method)	In-house method	L009B	D	MCERTS
Moisture Content	Moisture content, determined gravimetrically (up to 30°C)	In-house method	L019B	W	NONE
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight	In-house method based on British Standard Methods and MCERTS requirements.	L019B	D	NONE
Metals in soil by ICP-OES	Determination of metals in soil by aqua-regia digestion followed by ICP-OES	In-house method based on MEWAM 2006 Methods for the Determination of Metals in Soil	L038B	D	MCERTS
Boron, water soluble, in soil	Determination of water soluble boron in soil by hot water extract followed by ICP-OES	In-house method based on Second Site Properties version 3	L038B	D	MCERTS
Sulphate, water soluble, in soil (16hr extraction)	Sulphate, water soluble, in soil (16hr extraction)	In-house method	L038B	D	MCERTS
Speciated PAHs and/or Semi-volatile organic compounds in soil	Determination of semi-volatile organic compounds (including PAH) in soil by extraction in dichloromethane and hexane followed by GC-MS	In-house method based on USEPA 8270	L064B	D	MCERTS
BTEX and/or Volatile organic compounds in soil	Determination of volatile organic compounds in soil by headspace GC-MS	In-house method based on USEPA 8260	L073B	W	MCERTS
Total petroleum hydrocarbons with carbon banding by GC-FID in soil	Determination of total petroleum hydrocarbons in soil by GC-FID with carbon banding aliphatic and aromatic	In-house method	L076B	D	MCERTS
Total petroleum hydrocarbons with carbon banding by GC-FID/GC-MS HS in soil (Summed Bands)	Determination of total petroleum hydrocarbons in soil by GC-FID/GC-MS HS with carbon banding aliphatic and aromatic (Summed Bands).	Calculation	L076B/L088-PL	D/W	NONE
Hexavalent chromium in soil	Determination of hexavalent chromium in soil by extraction in NaOH and addition of 1,5 diphenylcarbazide followed by colorimetry	In-house method	L080-PL	W	MCERTS
Monohydric phenols in soil	Determination of phenols in soil by extraction with sodium hydroxide followed by distillation followed by colorimetry	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton	L080-PL	W	MCERTS
Total cyanide in soil	Determination of total cyanide by distillation followed by colorimetry	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton	L080-PL	W	MCERTS
Total petroleum hydrocarbons with carbon banding by HS-GC/MS in soil	Determination of total petroleum hydrocarbons in soil by HS-GC/MS with carbon banding aliphatic and aromatic	In-house method	L088-PL	W	MCERTS

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Analytical Report Number : 25-057486
Project / Site name: Chickland Service Station Hindon Salisbury

Water matrix abbreviations:
Surface Water (SW) Potable Water (PW) Ground Water (GW) Process Waters Heating/Cooling (PrW) DI Process Water (DI PrW)
Final Sewage Effluent (FSE) Landfill Leachate (LL)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement	In-house method	L099-PL	D	MCERTS
Soil Descriptions	Textural classification	In-house method	L019B	W	NONE

For method numbers ending in 'UK' or 'A' analysis have been carried out in our laboratory in the United Kingdom (Watford).
For method numbers ending in 'F' analysis have been carried out in our laboratory in the United Kingdom (East Kilbride).
For method numbers ending in 'PL' or 'B' analysis have been carried out in our laboratory in Poland.
Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.
Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

Quality control parameter failure associated with individual result applies to calculated sum of individuals.
The result for sum should be interpreted with caution

*g - Unaccredited sample matrix.

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Ground and Water Ltd
2 The Long Barn
Norton Farm
Selbourne Road
Alton
Hampshire
GU34 3NB

t:

e: roger.foord@groundandwater.co.uk

i2 Analytical Ltd.
7 Woodshots Meadow,
Croxley Green
Business Park,
Watford,
Herts,
WD18 8YS

t: 01923 225404
f: 01923 237404
e: info-i2analytical@normecgroup.com

Analytical Report Number : 25-055739

Project / Site name:	Chicklade Service Station Hindon Salisbury SP3 5ST	**Samples received on:**	14/10/2025
Your job number:	GWPR6751	**Samples instructed on/ Analysis started on:**	14/10/2025
Your order number:	GWPR6751	**Analysis completed by:**	23/10/2025
Report Issue Number:	1	**Report issued on:**	23/10/2025
Samples Analysed:	3 soil samples		

Joanna Biolik

Signed:

Joanna Biolik
Senior Reporting Specialist
For & on behalf of i2 Analytical Ltd.

Standard Geotechnical, Asbestos and Chemical Testing Laboratory located at: ul. Pionierów 39, 41-711 Ruda Śląska, Poland.

Accredited tests are defined within the report, opinions and interpretations expressed herein are outside the scope of accreditation.

Standard sample disposal times, unless otherwise agreed with the laboratory, are :

soils	- 4 weeks from reporting
leachates	- 2 weeks from reporting
waters	- 2 weeks from reporting
asbestos	- 6 months from reporting
air	- once the analysis is complete

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Retention period for records and reports is minimum 6 years from the date of issue of the final report.
Some records may be kept for longer according to other legal/best practice requirements.

Any assessments of compliance with specifications are based on actual analytical results with no contribution from uncertainty of measurement.
Application of uncertainty of measurement would provide a range within which the true result lies.
An estimate of measurement uncertainty can be provided on request.

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Analytical Report Number: 25-055739
Project / Site name: Chicklade Service Station Hindon Salisbury SP3 5ST
Your Order No: GWPR6751

Lab Sample Number				716569	716570	716571
Sample Reference				BH1	BH2	BH3
Sample Number				None Supplied	None Supplied	None Supplied
Water Matrix				N/A	N/A	N/A
Depth (m)				4.50	5.50	1.20-1.70
Date Sampled				29/09/2025	30/09/2025	01/10/2025
Time Taken				None Supplied	None Supplied	None Supplied
Analytical Parameter (Soil Analysis)	Units	Test Limit of detection	Test Accreditation Status			
Stone Content	%	0.1	NONE	< 0.1	< 0.1	< 0.1
Moisture Content	%	0.01	NONE	17	19	26
Total mass of sample received	kg	0.1	NONE	2	2	2

General Inorganics

pH (L099)	pH Units	N/A	MCERTS	8.5	8.2	8
Total Sulphate as SO_4	%	0.005	MCERTS	-	-	0.107
Water Soluble Sulphate as SO_4 16hr extraction (2:1)	mg/kg	2.5	MCERTS	110	600	87
Water Soluble SO_4 16hr extraction (2:1 Leachate Equivalent)	mg/l	1.25	MCERTS	53.7	301	43.5
Water Soluble Chloride (2:1) (leachate equivalent)	mg/l	0.5	MCERTS	-	-	43
Total Sulphur	mg/kg	50	MCERTS	-	-	400
Total Sulphur	%	0.005	MCERTS	-	-	0.04
Ammoniacal Nitrogen as NH_4^+	mg/kg	0.5	MCERTS	-	-	< 0.5
Ammonium as NH_4^+ (10:1 leachate equivalent)	mg/l	0.05	MCERTS	-	-	< 0.05
Water Soluble Nitrate (2:1) as NO_3	mg/kg	2	NONE	-	-	33
Water Soluble Nitrate (2:1) as NO_3 (leachate equivalent)	mg/l	5	NONE	-	-	16

Heavy Metals / Metalloids

Magnesium (leachate equivalent)	mg/l	2.5	NONE	-	-	2.9
Magnesium (water soluble)	mg/kg	5	NONE	-	-	5.8

U/S = Unsuitable Sample I/S = Insufficient Sample ND = Not detected

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Analytical Report Number : 25-055739
Project / Site name: Chicklade Service Station Hindon Salisbury SP3 5ST

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Stone content of a sample is calculated as the % weight of the stones not passing a 10 mm sieve. Results are not corrected for stone content.

Lab Sample Number	Sample Reference	Sample Number	Depth (m)	Sample Description *
716569	BH1	None Supplied	4.5	Beige clay with chalk
716570	BH2	None Supplied	5.5	Beige clay with chalk
716571	BH3	None Supplied	1.20-1.70	Brown clay

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Analytical Report Number : 25-055739
Project / Site name: Chicklade Service Station Hindon Salisbury SP3 5ST

Water matrix abbreviations:
Surface Water (SW) Potable Water (PW) Ground Water (GW) Process Waters Heating/Cooling (PrW) DI Process Water (DI PrW)
Final Sewage Effluent (FSE) Landfill Leachate (LL)

Analytical Test Name	Analytical Method Description	Analytical Method Reference	Method number	Wet / Dry Analysis	Accreditation Status
Moisture Content	Moisture content, determined gravimetrically (up to 30°C)	In-house method	L019B	W	NONE
Stones content of soil	Standard preparation for all samples unless otherwise detailed. Gravimetric determination of stone > 10 mm as % dry weight	In-house method based on British Standard Methods and MCERTS requirements.	L019B	D	NONE
Sulphate, water soluble, in soil (16hr extraction)	Sulphate, water soluble, in soil (16hr extraction)	In-house method	L038B	D	MCERTS
pH in soil (automated)	Determination of pH in soil by addition of water followed by automated electrometric measurement	In-house method	L099-PL	D	MCERTS
Soil Descriptions	Textural classification	In-house method	L019B	W	NONE
Magnesium, water soluble, in soil	Determination of water soluble magnesium by extraction with water followed by ICP-OES	In-house method based on TRL 447	L038B	D	NONE
Total sulphate (as SO4 in soil)	Determination of total sulphate in soil by extraction with 10% HCl followed by ICP-OES	In-house method	L038B	D	MCERTS
Total Sulphur in soil	Determination of total sulphur in soil by extraction with aqua-regia, potassium bromide/bromate followed by ICP-OES	In-house method	L038B	D	MCERTS
Water Soluble Nitrate (leachate equivalent)	Determination of nitrate by reaction with sodium salicylate and colorimetry	In-house method based on Examination of Water and Wastewatern & Polish Standard Method PN-82/C-04579.08, 2:1 extraction	L078-PL	W	NONE
Nitrate, water soluble, in soil	Determination of nitrate by reaction with sodium salicylate and colorimetry	In-house method based on Examination of Water and Wastewatern & Polish Standard Method PN-82/C-04579.08, 2:1 extraction	L078-PL	W	NONE
Chloride, water soluble, in soil	Determination of Chloride colorimetrically by discrete analyser	In-house method	L082B	D	MCERTS
Ammonium as NH4 in soil	Determination of Ammonium/Ammonia/ Ammoniacal Nitrogen by the colorimetric salicylate/nitroprusside method, 10:1 water extraction.	In-house method based on Examination of Water and Wastewater 20th Edition: Clesceri, Greenberg & Eaton	L082B	W	MCERTS

For method numbers ending in 'UK' or 'A' analysis have been carried out in our laboratory in the United Kingdom (Watford).
For method numbers ending in 'F' analysis have been carried out in our laboratory in the United Kingdom (East Kilbride).
For method numbers ending in 'PL' or 'B' analysis have been carried out in our laboratory in Poland.
Soil analytical results are expressed on a dry weight basis. Where analysis is carried out on as-received the results obtained are multiplied by a moisture correction factor that is determined gravimetrically using the moisture content which is carried out at a maximum of 30oC.
Unless otherwise indicated, site information, order number, project number, sampling date, time, sample reference and depth are provided by the client. The instructed on date indicates the date on which this information was provided to the laboratory.

Quality control parameter failure associated with individual result applies to calculated sum of individuals.
The result for sum should be interpreted with caution

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Analytical Report Number : 25-055739
Project / Site name: Chicklade Service Station Hindon Salisbury SP3 5ST

This deviation report indicates the sample and test deviations that apply to the samples submitted for analysis.Please note that the associated result(s) may be unreliable and should be interpreted with care.

Key: a - No sampling date b - Incorrect container/Insufficient material provided c - Holding time d - Headspace e - Temperature

Sample ID	Other ID	Sample Type	Lab Sample Number	Sample Deviation	Test Name	Test Ref	Test Deviation
BH1	N/A	S	716569	c	pH in soil (automated)	L099-PL	c
BH2	N/A	S	716570	c	pH in soil (automated)	L099-PL	c
BH3	N/A	S	716571	c	pH in soil (automated)	L099-PL	c

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APPENDIX F:
Soil Assessment Criteria

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

Ground and Water Limited

Soil Guideline Values and Generic Assessment Criteria

The Contaminated Land Regime reflects the UK Government's stated objectives of achieving sustainable development through the 'suitable for use approach'. At preliminary risk assessment stage, risks are evaluated qualitatively. As the site investigation progresses to a generic or detailed quantitative risk assessment, data is collected and assessment criteria are utilised to evaluate whether the contaminants represent an unacceptable risk to the identified receptors.

1. Contaminated Land Exposure Assessment Model (CLEA)

Current United Kingdom risk assessment practice is based on the Contaminated Land Exposure Assessment Model (CLEA).

The CLEA Guidance comprises the following documents:

1) EA Science Report SC050021/SR2: Human health toxicological assessment of contaminants in soil.
2) EA Science Report SC050021/SR3: Updated technical background to the CLEA model.
3) EA CLEA Bulletin (2009).
4) CLEA software version 1.07 (2015)
5) Toxicological reports and SGV technical notes.

The CLEA guidance and tools:

- Do not cover other types of risk to humans, such as fire, suffocation or explosion, or short-term and acute exposures;
- Do not cover risks to the environment, such as groundwater, ecosystems or buildings;
- Do not provide a definitive test for telling when human health risks are significant; and
- Are not a legal requirement in assessing land contamination risks. They are not part of the legal regime for Part 2A of the Environmental Protection Act 1990.

The CLEA guidance derives soil concentrations of contaminants above which (in the opinion of the EA) there may be a concern that warrants further investigation. It does not provide a definitive test for establishing that the risk is significant.

1.1. Land-use Scenarios

The CLEA model uses a range of standard land-use scenarios to develop conceptual exposure models outlined in the following sections.

1.1.1. Residential (with home grown produce) (RwHP)

Generic scenario assumes a typical two-storey house built on a ground bearing slab with a private garden having a lawn, flowerbeds and a small fruit and vegetable patch.

- Critical receptor is a young female child (zero to six years old)
- Exposure duration is six years.
- Exposure pathways include direct soil and indoor dust ingestion, consumption of homegrown produce and any adhering soil, skin contact with soils and indoor dust and inhalation of indoor and outdoor dust and vapours.
- Building type is a two-storey small-terraced house.

A sub-set of this land-use is residential apartments with communal landscaped gardens where the consumption of home grown vegetables will not occur. (Residential without homegrown produce (RwoHP)).

1.1.2. Allotments

Provision of open space (about 250sq.m) commonly made available to tenants by the local authority to grow fruit and vegetable for their own consumption. Typically, there are a number of plots to a site which may have a total area of up to 1 hectare. The tenants are assumed to be adults and that young children make occasional accompanied visits.

Although some allotment holders may choose to keep animals including rabbits, hens, and ducks, potential exposure to contaminated meat and eggs is not considered.

- Critical receptor is a young female child (zero to six years old)
- Exposure duration is six years.
- Exposure pathways include direct soil ingestion, consumption of homegrown produce and any adhering soil, skin contact with soils and inhalation of outdoor dust and vapours.
- There is no building.

1.1.3. Commercial/Industrial

The generic scenario assumes a typical commercial or light industrial property comprising a three-storey building at which employees spend most time indoors and are involved in office-based or relatively light physical work.

- Critical receptor is a working female adult (aged 16 to 65 years old).
- Exposure duration is a working lifetime of 49 years.
- Exposure pathways include direct soil and indoor dust ingestion, skin contact with soils and dusts and inhalation of dust and vapours.
- Building type is a three-storey office (pre 1970).

2. LQM/CIEH Suitable 4 Use Levels (S4UL)

For derivation of these S4UL reference must be made to:

Nathanial, P., McCaffrey, C., Gillet, A., Ogden, R., Nathanial, J., *The LQM/CIEH S4UL's for Human Health Risk Assessment*. **Land Quality Press**. 2015

2.1. S4UL Background

The Land Quality Management/Chartered Institute of Environmental Health (LQM/CIEH) S4UL for a given land use is the concentration of the contaminant in soil at which the predicted daily exposure,

as calculated by the CLEA software, equals the Health Criteria Value. The S4ULs have been derived for substances based on various generic land use and soil organic matter contents.

The final output for each contaminant represents a synthesis of new toxicological (and fate and transport) reviews published since the preparation of the 2nd edition LQM/CIEH GAC's (Nathanial et al., 2009).

In the derivation of LQM/CIEH S4UL's the principles of 'minimal' or 'tolerable' risk enshrined in SR2, which has not been withdrawn, has been maintained.

2.2. S4UL Land-use

S4UL's have been derived for the basic CLEA land-uses, as described in section 1.2, and for two new land uses:

- Public Open Spaces near Residential Housing (POSresi).
- Public Park (POSpark).

2.2.1. Public Open Spaces near Residential Housing (POSresi)

Includes the predominantly grassed areas adjacent to high density housing, the central green area on many 1930's – 1970's housing estates, and smaller areas commonly incorporated in newer developments as informal grassed areas or more formal landscaped areas with a mixture of open space and covered soils with planting. It is assumed that the close proximity to the place of residence will allow tracking back of soil to occur.

2.2.2. Public Park (POSpark)

An area of open space, usually owned and maintained by the local authority, provided for recreational uses including family visists and picnics, children's play area, informal sporting activities (not a dedicated sports pitch), and dog walking. It is assumed that tracking back of soils into places of residence will be negligible.

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3. Category 4 Screening Levels (C4SLs)

In the case of Lead, no SGV or GAC has been published to date. This is likely to be due to the toxicity review that is currently being undertaken by the Environment Agency. In the absence of updated toxicity information the SGV derived using CLEA 1.07 methodology and related toxicity will be used.

The overall objective of the C4SLs research project was to assist the provision of technical guidance in support of Defra's revised Statutory Guidance (SG) for Part 2A of the Environmental Protection Act 1990 (Part 2A) (Defra, 2012a). Specifically, the project aimed to deliver:

- A methodology for deriving C4SLs for four generic land-uses comprising residential, commercial, allotments and public open space; and
- A demonstration of the methodology, via the derivation of C4SLs for six substances – arsenic, benzene, benzo(a)pyrene, cadmium, chromium (VI) and lead.

To help achieve a more targeted approach to identifying and managing contaminated land in relation to the risk (or possibility) of harm to human health, the revised SG presented a new four category

system for considering land under Part 2A, ranging from Category 4, where there is no risk that land poses a significant possibility of significant harm (SPOSH), or the level of risk is low, to Category 1, where the risk that land poses a significant possibility of significant harm (SPOSH) is unacceptably high. More specific guidance on what type of land should be considered as Category 4 (Human Health) is provided in Paragraphs 4.21 and 4.22 of the revised SG, as follows:

> *"4.21 The local authority should consider that the following types of land should be placed into Category 4: Human Health:*
> *(a) Land where no relevant contaminant linkage has been established.*
> *(b) Land where there are only normal levels of contaminants in soil, as explained in Section 3 of this Guidance.*
> *(c) Land that has been excluded from the need for further inspection and assessment because contaminant levels do not exceed relevant generic assessment criteria in accordance with Section 3 of this Guidance, or relevant technical tools or advice that may be developed in accordance with paragraph 3.30 of this Guidance.*
> *(d) Land where estimated levels of exposure to contaminants in soil are likely to form only a small proportion of what a receptor might be exposed to anyway through other sources of environmental exposure (e.g. in relation to average estimated national levels of exposure to substances commonly found in the environment, to which receptors are likely to be exposed in the normal course of their lives).*
>
> *4.22 The local authority may consider that land other than the types described in paragraph 4.21 should be placed into Category 4: Human Health if following a detailed quantitative risk assessment it is satisfied that the level of risk posed is sufficiently low."*

The C4SLs are intended as "relevant technical tools" (in relation to Paragraph 4.21(c)) to help local authorities and others when deciding to stop further assessment of a site, on the grounds that it falls within Category 4 (Human Health).

The Impact Assessment (IA), which accompanied the revised SG (Defra, 2012b) provides further information on the nature and potential role of the C4SLs. Paragraph 47(h) of the IA states that:

> *"The new statutory guidance will bring about a situation where the current SGVs/GACs are replaced with more pragmatic (but still strongly precautionary) Category 4 screening levels (C4SLs) which will provide a higher simple test for deciding that land is suitable for use and definitely not contaminated land."*

A key distinction between the Soil Guideline Values (SGVs) and the C4SLs is the level of risk that they describe. As described by the Environment Agency (2009a):

> *"SGVs are guidelines on the level of long-term human exposure to individual chemicals in soil that, unless stated otherwise, are tolerable or pose a minimal risk to human health."*

The implication of Paragraph 47(h) of the IA is that minimal risk is well within Category 4 and that the C4SLs should describe a higher level of risk which, whilst not minimal, can still be considered low enough to allow a judgement to be made that land containing substances at, or below, the C4SLs would typically fall within Category 4. This reflects Paragraph 4.20 of the revised SG, which states:

> *"4.20 The local authority should not assume that land poses a significant possibility of significant harm if it considers that there is no risk or that the level of risk posed is low. For the purposes of this Guidance, such land is referred to as a "Category 4: Human Health" case. The authority may decide that the land is a Category 4: Human Health case as soon as it considers it has evidence to this effect, and this may happen at any stage during risk assessment including the early stages."*

C4SLs, therefore, should not be viewed as "SPOSH levels" and they should not be used as a legal trigger for the determination of land under Part 2A.

The generic screening values referred to before usually take the form of risk-based Soil Guideline Values (SGVs) or other Generic Assessment Criteria (GACs) that are most typically derived using the Environment Agency's Contaminated Land Exposure Assessment (CLEA) model, as described in the Environment Agency's SR2, SR3 and SR7 reports (EA, 2009b & c; EA, 2008). It is anticipated that C4SLs will be used in a similar manner; as generic screening criteria that can be used within a GQRA, albeit describing a higher level of risk than the SGVs.

The suggested approach to the development of C4SLs consists of the retention and use of the CLEA framework, modified according to considerations of the underlying science within the context of Defra's policy objectives relating to the revised SG. Within this context, it is suggested that the development of C4SLs may be achieved in one of three ways, namely:

- By modifying the toxicological parameters used within CLEA (while maintaining current exposure parameters);
- By modifying the exposure parameters embedded within CLEA (while maintaining current toxicological "minimal risk" interpretations); and
- By modifying both toxicological and exposure parameters.

There is also a suggested check on "other considerations" (e.g., background levels, epidemiological data, sources of uncertainty) within the approach, applicable to all three options.

It is suggested that a new term is defined for the toxicological guidance values associated with the derivation of C4SLs – a Low Level of Toxicological Concern (LLTC). A LLTC should represent an intake of low concern that remains suitably protective of health, and definitely does not approach an intake level that could be defined as SPOSH.

4. CL:AIRE Generic Assessment Criteria (GAC)

For derivation of the CL:AIRE Generic Assessment Criteria (GAC) reference should be made to the following report:

> CL:AIRE, *The Soil Generic Assessment Criteria for Human Health Risk Assessment.* **Contaminated Land: Applications in the Real Environment**. 2009.

Within this report, Contaminated Land: Applications in Real Environments (CL:AIRE) provided Generic Assessment Criteria (GAC) in accordance with the CLEA software and the principles outlined previously for a further 35 contaminants sometimes encountered on land affected by contamination.

5. SoBRA Acute GAC

The Society of Brownfield Risk Assessment (SoBRA) identified that most human health risk assessments focus on the chronic risks arising from long-term exposure to specific substances. As chronic risks often occur at lower doses than acute risks, they are often the key drivers, however, in some instances the acute dose may also be an important consideration within risk assessments.

The methodology for deriving the acute GAC were related to two distinct receptor groups:

- Members of the public, where the 'critical' receptor for this group will typically be a female child, which is consistent with CLEA residential and Public Open Space/allotments land-uses;
- Workers involved with excavations. The critical receptor for this group is assumed to be a female working adult, without the use of PPE.

The acute GACs relate to short term exposure of high concentrations of a substance that lead to acute effects. They are not considered to be average exposures across a specific / defined area. As a result, the GACs should be normally be compared with the maximum likely concentration that the individual may be exposed to, and not the average concentration within a specific area.

The SoBRA acute GAC will primarily be used for contaminants that do not currently have any GAC, most notably Cyanide.

6. Detailed Quantitative Risk Assessments (DQRA)

Where the adoption of a GAC is not appropriate, for instance when the intended land-use is at variance the CLEA standard land-uses, then a DQRA may be undertaken to develop site specific values for relevant soil contaminants.

- Establishing the plausibility that generic exposure pathways exist in practice by measurement and observation.
- Developing more accurate parameters using site data.

7. Phytotoxicity

CLEA guidance only addresses human health toxicity; assessment of plant toxicity (phytotoxicity) is based on threshold trigger values obtained from the following source:

- BS3882:2015 – *Specification for Topsoil*

The trigger values are relevant only to those contaminants, where present in excess, have the potential to inhibit plant growth, or kill plants (Cu, Ni and Zn). The criteria have been based on a wide range of planting that are common within a multi-purpose topsoil.

8. Statistical Tests

DEFRA R&D Publication CLR 7 (DOE 1994) addressed the statistical treatment of test results and their comparison to Soil Guideline Values.

Consideration must be given to the appropriate area of land to be considered termed the critical averaging area.

For a communal open space or commercial land-use, the critical averaging area will depend on the proposed layout. For a residential use with private gardens the averaging area is the individual plot.

It may be appropriate to compare the upper 95th percentile concentration with the Soil Guideline Value, subject to applying a statistical test to establish that the range of concentrations are reasonably consistent and belonging to the same underlying distribution of data.

CL:AIRE published guidance in 2020, *Guidance in comparing soil contamination data with a critical concentration,* superseding the CL:AIRE/CIEH 2008 report of the same name. The guidance provides ways to assist land contamination stakeholders to apply statistical methods to their data to enable decisions under the legislative framework; either planning system or Part 2A of the Environmental Protection Act 1990.

The use of the statistical tests should only be applied if the following statements are valid for the datasets:

- Averaging areas, as well as the smallest area of concern have been identified on the basis of the CSM, including the desk study and/or the site walkover;
- The sample locations were chosen using a simple random, stratified random or stratified systematic (square, herringbone or triangular grid) sampling pattern, rather than being targeted to locations suspected of being contaminated;
- The sample locations are relatively evenly spread across the area and are not clustered, to avoid giving undue weight to some parts of the site over others in the calculated statistics;
- The analyses do not suggest a hotspot or outlier of contamination that should be treated as a separate zone. This has been established by a histogram and/or a names statistical test;
- The sample locations are all taken from one population (i.e. the same material);
- Where an averaging zone encompasses several averaging areas, analyses do not show a spatial trend or other spatial pattern across that zone; and
- The number of samples has been shown to be sufficient for a statistical analysis.

Any included statistical spreadsheet is based on an in-house method of statistical analysis, in line with those outlined within the CL:AIRE guidance (2020).

Treatment of Hot-Spots

- A statistical test is applied to establish whether the data is a part of a single set, or whether data outliers are present.
- Provided that the data is based on random sampling and no distinct contamination source was present at the sampling location, the hot-spot(s) may be excluded and the mean of the remaining data assessed.

9. Ground and Water Limited Soil Assessment Criteria

The Soil Assessment Criteria used in the preparation of the Generic Quantitative Risk Assessment are tabulated in the following pages, where the source of each has been outlined in the previous sections.

9.1. Inorganics

SoBRA – Acute Generic Assessment Criteria						
Determinand	**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Cyanide	24	24	24	1400	24	24

9.2. Metals

C4SL Low Level of Toxicological Concern						
Determinand	**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Lead	< 200	< 310	< 80	< 2300	< 630	< 1300

LQM/CIEH Suitable 4 Use Levels – Metals and Semi-metals						
Determinand	**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Arsenic	37	40	43	640	79	170
Beryllium	1.7	1.7	35	12	2.2	63
Boron	290	11000	45	240000	21000	46000
Cadmium	11	85	1.9	190	120	532
Chromium (III)	910	910	18000	8600	1500	33000
Chromium (VI)	6	6	1.8	33	7.7	220
Copper	2400	7100	520	68000	12000	44000
Elemental Mercury	1.2	1.2	21	58	16	30
Inorganic Mercury	40	56	19	1100	120	240
Methylmercury	11	15	6	320	40	68
Nickel	130	180	53	980	230	800
Selenium	250	430	88	12000	1100	1800
Vanadium	410	1200	91	9000	2000	5000
Zinc	3700	40000	620	730000	81000	170000

Phytotoxicity (Harmful to Plants) Threshold Trigger Values			
Determinand	**Soil pH < 6.0 (mg/kg)**	**Soil pH 6.0 – 7.0 (mg/kg)**	**Soil pH > 7.0 (mg/kg)**
Copper	100	135	200
Nickel	60	75	110
Zinc	200	200	300
Notes: BS3882:2015 – *Specification for Topsoil*. Based on a wide range of common plants that will be exposed to multi-purpose topsoil. Toxicity of contaminant may also be impacted by pH of soils. Site observation of plant vitality may give additional guidance.			

CL:AIRE Soil Generic Assessment Criteria				
Determinand	**Residential (mg/kg)**	**Residential without plant uptake (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**
Antimony	ND	550	ND	7500
Barium	ND	1300	ND	22000
Molybdenum	ND	670	ND	17000
ND – Not derived				

9.3. Total Petroleum Hydrocarbons (TPHs)

9.3.1. BTEX Compounds

LQM/CIEH Suitable 4 Use Levels – BTEX Compounds							
Determinand	**Soil Organic Matter**	**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Benzene	1.0% SOM	0.087	0.38	0.017	27	72	90
	2.5% SOM	0.170	0.70	0.034	47	72	100
	6.0% SOM	0.370	1.40	0.075	90	73	110
Toluene	1.0% SOM	130	880	22	56000	56000	87000
	2.5% SOM	290	1900	51	110000	56000	95000
	6.0% SOM	660	3900	120	180000	56000	100000
Ethylbenzene	1.0% SOM	47	83	16	5700	24000	17000
	2.5% SOM	110	190	39	13000	24000	22000
	6.0% SOM	260	440	91	27000	25000	27000
o-Xylene	1.0% SOM	60	88	28	6600	41000	17000
	2.5% SOM	140	210	67	15000	42000	24000
	6.0% SOM	330	480	160	33000	43000	33000
m-Xylene	1.0% SOM	59	82	31	6200	41000	17000
	2.5% SOM	140	190	74	14000	42000	24000
	6.0% SOM	320	450	170	31000	43000	33000
p-Xylene	1.0% SOM	56	79	29	5900	41000	17000
	2.5% SOM	130	180	69	14000	42000	23000
	6.0% SOM	310	430	160	30000	43000	31000
SOM = Soil Organic Matter Content (%)							

9.3.2. Total Petroleum Hydrocarbons – Aliphatic

LQM/CIEH Suitable 4 Use Levels For TPH							
Aliphatic		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
EC 5-6	1.0% SOM	42	42	730	3,200 (304) [sol]	570,000 (304) [sol]	95,000 (304) [sol]
	2.5% SOM	78	78	1,700	5,900 (558) [sol]	590,000	130,000 (558) [sol]
	6.0% SOM	160	160	3,900	12,000 (1150) [sol]	600,000 (1150)[sol]	180,000 (1150) [sol]
EC >6-8	1.0% SOM	100	100	2,300	7,800 (144) [sol]	600,000	150,000 (144) [sol]
	2.5% SOM	230	230	5,600	17,000 (322) [sol]	610,000	220,000 (322) [sol]
	6.0% SOM	530	530	13,000	40,000 (736) [sol]	620,000	320,000 (736) [sol]
EC >8-10	1.0% SOM	27	27	320	2,000 (78) [sol]	13,000	14,000 (78) [sol]
	2.5% SOM	65	65	770	4,800 (118) [vap]	13,000	18,000 (118) [vap]
	6.0% SOM	150	150	1,700	11,000 (451) [vap]	13,000	21,000 (451) [vap]
EC >10-12	1.0% SOM	130 (48) [vap]	130 (48) [vap]	2,200	9,700 (48) [sol]	13,000	21,000 (48) [sol]
	2.5% SOM	330 (118) [vap]	330 (118) [vap]	4,400	23,000 (118) [vap]	13,000	23,000 (118) [vap]
	6.0% SOM	760 (283) [vap]	760 (283) [vap]	7,300	47,000 (283) [vap]	13,000	24,000 (283) [vap]
EC >12-16	1.0% SOM	1,100 (24) [sol]	1,100 (24) [sol]	11,000	59,000 (24) [sol]	13,000	25,000 (24) [sol]
	2.5% SOM	2,400 (59) [sol]	2,400 (59) [sol]	13,000	82,000 (59) [sol]	13,000	25,000 (59) [sol]
	6.0% SOM	4,300 (142) [sol]	4,400 (142) [sol]	13,000	90,000 (142) [sol]	13,000	26,000 (142) [sol]
EC >16-35	1.0% SOM	65,000 (8.48) [sol]	65,000 (8.48) [sol]	260,000	1,600,000	250,000	450,000
	2.5% SOM	92,000 (21) [sol]	92,000 (21) [sol]	270,000	1,700,000	250,000	480,000
	6.0% SOM	110,000	110,000	270,000	1,800,000	250,000	490,000
EC >35-44	1.0% SOM	65,000 (8.48) [sol]	65,000 (8.48) [sol]	260,000	1,600,000	250,000	450,000
	2.5% SOM	92,000 (21) [sol]	92,000 (21) [sol]	270,000	1,700,000	250,000	480,000
	6.0% SOM	110,000	110,000	270,000	1,800,000	250,000	490,000

SOM = Soil Organic Matter Content (%)

[vap] – GAC presented exceeds the vapour saturation limit, which is presented in brackets.

[sol] – GAC presented exceeds the soil saturation limit, which is presented in brackets.

9.3.3. Total Petroleum Hydrocarbons – Aromatic

LQM/CIEH Suitable 4 Use Levels For TPH							
Aromatic		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
EC 5-7 (Benzene)	1.0% SOM	70	370	13	26,000 (1220) [sol]	56,000	76,000 (1220 [sol]
	2.5% SOM	140	690	27	46,000 (2260) [sol]	56,000	84,000 (2260) [sol]
	6.0% SOM	300	1,400	57	86,000 (4710) [sol]	56,000	92,000 (4710) [sol]
EC >7-8 (Toluene)	1.0% SOM	130	860	22	56,000 (869) [vap]	56,000	87,000 (869) [sol]
	2.5% SOM	290	1,800	51	110,000 (1920) [sol]	56,000	95,000 (1920) [sol]
	6.0% SOM	660	3,900	120	180,000 (4360) [vap]	56,000	100,000 (4360) [vap]
EC >8-10	1.0% SOM	34	47	8.6	3,500 (613) [vap]	5,000	7,200 (613) [vap]
	2.5% SOM	83	110	21	8,100 (1500) [vap]	5,000	8,500 (1500) [vap]
	6.0% SOM	190	270	51	17,000 (3850) [vap]	5,000	9,300 (3580) [vap]
EC >10-12	1.0% SOM	74	250	13	16,000 (364) [sol]	5,000	9,200 (364) [sol]
	2.5% SOM	180	590	31	28,000 (899) [sol]	5,000	9,700 (889) [sol]
	6.0% SOM	380	1,200	74	34,000 (2150) [sol]	5,000	10,000
EC >12-16	1.0% SOM	140	1,800	23	36,000 (169) [sol]	5,100	10,000
	2.5% SOM	330	2,300 (419) [sol]	57	37,000	5,100	10,000
	6.0% SOM	660	2,500	130	38,000	5,000	10,000
EC >16-21	1.0% SOM	260	1,900	46	28,000	3,800	7,600
	2.5% SOM	540	1,900	110	28,000	3,800	7,700
	6.0% SOM	930	1,900	260	28,000	3,800	7,800
EC >21-35	1.0% SOM	1,100	1,900	370	28,000	3,800	7,800
	2.5% SOM	1,500	1,900	820	28,000	3,800	7,800
	6.0% SOM	1,700	1,900	1,600	28,000	3,800	7,900
EC >35-44	1.0% SOM	1,100	1,900	370	28,000	3,800	7,800
	2.5% SOM	1,500	1,900	820	28,000	3,800	7,800
	6.0% SOM	1,700	1,900	1,600	28,000	3,800	7,900
EC >44-70	1.0% SOM	1,600	1,900	1,200	28,000	3,800	7,800
	2.5% SOM	1,800	1,900	2,100	28,000	3,800	7,800
	6.0% SOM	1,900	1,900	3,000	28,000	3,800	7,900

SOM = Soil Organic Matter Content (%)
[vap] – GAC presented exceeds the vapour saturation limit, which is presented in brackets.
[sol] – GAC presented exceeds the soil saturation limit, which is presented in brackets.

9.4. Polycyclic Aromatic Hydrocarbons (PAHs)

LQM/CIEH Suitable 4 Use Levels For Polycyclic Aromatic Hydrocarbons (PAHs)							
Determinands		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Acenapthene	1.0% SOM	210	3,000 (57.0) [sol]	34	84,000(57.0) [sol]	15,000	29,000
	2.5% SOM	510	4,700(141) [sol]	85	97,000(141) [sol]	15,000	30,000
	6.0% SOM	1100	6,000(336) [sol]	200	100,000	15,000	30,000
Acenapthylene	1.0% SOM	170	2,900(86.1) [sol]	28	83,000(86.1) [sol]	15,000	29,000
	2.5% SOM	420	4,600(212) [sol]	69	97,000(212) [sol]	15,000	30,000
	6.0% SOM	920	6,000(506) [sol]	160	100,000	15,000	30,000
Anthracene	1.0% SOM	2,400	31,000(1.17) [vap]	380	520,000	74,000	150,000
	2.5% SOM	5,400	35,000	950	540,000	74,000	150,000
	6.0% SOM	11,000	37,000	2,200	540,000	74,000	150,000
Benzo(a)anthracene	1.0% SOM	7.20	11	2.90	170	29	49
	2.5% SOM	11	14	6.50	170	29	56
	6.0% SOM	13	15	13	180	29	62
Benzo(a)pyrene	1.0% SOM	2.20	3.20	0.97	35	5.70	11
	2.5% SOM	2.70	3.20	2.00	35	5.70	12
	6.0% SOM	3.00	3.20	3.50	36	5.70	13
Benzo(b)flouranthene	1.0% SOM	2.60	3.90	0.99	44	7.10	13
	2.5% SOM	3.30	4.00	2.10	44	7.20	15
	6.0% SOM	3.70	4.00	3.90	45	7.20	16
Benzo(ghi)perylene	1.0% SOM	320	360	290	3,900	640	1,400
	2.5% SOM	340	360	470	4,000	640	1,500
	6.0% SOM	350	360	640	4,000	640	1,600
Benzo(k)flouranthene	1.0% SOM	77	110	37	1,200	190	370
	2.5% SOM	93	110	75	1,200	190	410
	6.0% SOM	100	110	130	1,200	190	440
Chrysene	1.0% SOM	15	30	4.10	350	57	93
	2.5% SOM	22	31	9.40	350	57	110
	6.0% SOM	27	32	19	350	57	120
Dibenzo(ah)anthracene	1.0% SOM	0.24	0.31	0.14	3.50	0.57	1.10
	2.5% SOM	0.28	0.32	0.27	3.50	0.57	1.30
	6.0% SOM	0.30	0.32	0.43	3.60	0.58	1.40
Flouranthene	1.0% SOM	280	1,500	52	23,000	3,100	6,300
	2.5% SOM	560	1,600	130	23,000	3,100	6,300
	6.0% SOM	890	1,600	290	23,000	3,100	6,300
Flourene	1.0% SOM	170	2,800 (30.9) [sol]	27	63,000(30.9) [sol]	9,900	20,000
	2.5% SOM	400	3,800(76.5) [sol]	67	68,000	9,900	20,000
	6.0% SOM	860	4,500(183) [sol]	160	71,000	9,900	20,000
Indeno(123-cd)pyrene	1.0% SOM	27	45	9.50	500	82	150
	2.5% SOM	36	46	21	510	82	170
	6.0% SOM	41	46	39	510	82	180
Napthalene	1.0% SOM	15	36	65	1,600	11,000	800
	2.5% SOM	36	36	130	3,700	15,000	1,200
	6.0% SOM	85	85	200	8,400	17,000	1,900
Phenanthrene	1.0% SOM	95	1,300(183) [sol]	15	22,000	3,100	6,200
	2.5% SOM	220	1,500	38	22,000	3,100	6,200
	6.0% SOM	440	1,500	90	23,000	3,100	6,300
Pyrene	1.0% SOM	620	3,700	110	54,000	7,400	15,000
	2.5% SOM	1200	3,800	270	54,000	7,400	15,000
	6.0% SOM	2000	3,800	620	54,000	7,400	15,000
Coal Tar (Benzo(a)pyrene used as marker compound)	1.0% SOM	0.79	1.2	0.32	15	2.20	4.40
	2.5% SOM	0.98	1.2	0.67	15	2.20	4.70
	6.0% SOM	1.10	1.2	1.20	15	2.20	4.80

SOM = Soil Organic Matter Content (%)

[vap] – GAC presented exceeds the vapour saturation limit, which is presented in brackets.

[sol] – GAC presented exceeds the soil saturation limit, which is presented in brackets.

9.5. Volatile and Semi-volatile Organic Compounds

LQM CIEH General Assessment Criteria: Volatile and Semi-Volatile Organic Compounds							
Determinands		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Chloroalkanes & alkenes							
1,2 Dichloroethane	1.0% SOM	0.0071	0.0092	0.0046	0.67	29	21
	2.5% SOM	0.011	0.013	0.0083	0.97	29	24
	6.0% SOM	0.019	0.023	0.016	1.70	29	28
1,1,2,2 Tetrachloroethane	1.0% SOM	1.60	3.90	0.41	270	1,400	1,800
	2.5% SOM	3.40	8.00	0.89	550	1,400	2,100
	6.0% SOM	7.50	17	2.00	1,100	1,400	2,300
1,1,1,2 Tetrachloroethane	1.0% SOM	1.20	1.50	0.79	110	1,400	1,500
	2.5% SOM	2.80	3.50	1.90	250	1,400	1,800
	6.0% SOM	6.40	8.20	4.40	560	1,400	2,100
Tetrachloroethene	1.0% SOM	0.18	0.18	0.65	19	1,400	810 [sol](424)
	2.5% SOM	0.39	0.40	1.50	42	1,400	1,100 [sol](951)
	6.0% SOM	0.90	0.92	3.60	95	1,400	1,500
1,1,1 Trichloroethane	1.0% SOM	8.80	9.00	48	660	140,000	57,000 [vap](1425)
	2.5% SOM	18	18	110	1,300	140,000	76,000 [vap](2915)
	6.0% SOM	39	40	240	3,000	140,000	100,000 [vap] (6392)
Tetrachloromethene	1.0% SOM	0.026	0.026	0.45	2.90	890	190
	2.5% SOM	0.056	0.056	1.00	6.30	920	270
	6.0% SOM	0.130	0.130	2.40	14	950	400
Trichloroethene	1.0% SOM	0.016	0.017	0.041	1.20	120	70
	2.5% SOM	0.034	0.036	0.091	2.60	120	91
	6.0% SOM	0.075	0.080	0.210	5.70	120	120
Trichloromethane	1.0% SOM	0.91	1.20	0.42	99	2,500	2,600
	2.5% SOM	1.70	2.10	0.83	170	2,500	2,800
	6.0% SOM	3.40	4.20	1.70	350	2,500	3,100
Vinyl Chloride	1.0% SOM	0.00064	0.00077	0.00055	0.059	3.50	4.80
	2.5% SOM	0.00087	0.00100	0.00100	0.077	3.50	5.00
	6.0% SOM	0.00014	0.00150	0.00180	0.120	3.50	5.40
Explosives							
2,4,6 Trinitrotoluene	1.0% SOM	1.60	65	0.24	1,000	130	260
	2.5% SOM	3.70	66	0.58	1,000	130	270
	6.0% SOM	8.10	66	1.40	1,000	130	270
RDX (Hexogen/Cyclonite/1,3,5-trinitro-1,3,5-triazacyclohexane)	1.0% SOM	120	13,000	17	210,000	26,000	49,000(18.7)[sol]
	2.5% SOM	250	13,000	38	210,000	26,000	51,000
	6.0% SOM	540	13,000	85	210,000	27,000	53,000
HMX (Octogen/1,3,5,7-tetrenitro-1,3,5,7-tetrazacyclo-octane)	1.0% SOM	5.70	67,00	0.86	110,000	13,000	23,000(0.35)[vap]
	2.5% SOM	13	67,00	1.90	110,000	13,000	23,000(0.39)[vap]
	6.0% SOM	26	67,00	3.90	110,000	13,000	24,000(0.48)[vap]
Atrazine	1.0% SOM	3.30	610	0.50	9,300	1,200	2,300
	2.5% SOM	7.60	620	1.20	9,400	1,200	2,400
	6.0% SOM	17.40	620	2.70	9,400	1,200	2,400
[vap] – GAC presented exceeds the vapour saturation limit, which is presented in brackets.							
[sol] – GAC presented exceeds the soil saturation limit, which is presented in brackets.							

VOC and SVOC table continued overleaf

VOC and SVOC table continued from previous page

LQM CIEH General Assessment Criteria: Volatile and Semi-Volatile Organic Compounds							
Determinands		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Pesticides							
Aldrin	1.0% SOM	5.70	7.30	3.20	170	18	30
	2.5% SOM	6.60	7.40	6.10	170	18	31
	6.0% SOM	7.10	7.50	9.60	170	18	31
Dieldrin	1.0% SOM	0.97	7.00	0.17	170	18	30
	2.5% SOM	2.00	7.30	0.41	170	18	30
	6.0% SOM	3.50	7.40	0.96	170	18	31
Dichlorvos	1.0% SOM	0.032	6.40	0.0049	140	16	26
	2.5% SOM	0.066	6.50	0.0100	140	16	26
	6.0% SOM	0.140	6.60	0.0220	140	16	27
Alpha - Endosulfan	1.0% SOM	7.40	160(0.003)[vap]	1.20	5,600(0.003)[vap]	1,200	2,400
	2.5% SOM	18	280(0.007)[vap]	2.90	7,400(0.007)[vap]	1,200	2,400
	6.0% SOM	41	410(0.016)[vap]	6.80	8,400(0.016)[vap]	1,200	2,400
Beta - Endosulfan	1.0% SOM	7.00	190(0.00007)[vap]	1.10	6,300(0.00007)[vap]	1,200	2,400
	2.5% SOM	17	320(0.0002)[vap]	2.70	7,800(0.0002)[vap]	1,200	2,400
	6.0% SOM	39	440(0.0004)[vap]	6.40	8700	1,200	2,500
Alpha - Hexachlorocyclohexanes	1.0% SOM	0.23	6.90	0.035	170	24	47
	2.5% SOM	0.55	9.20	0.087	180	24	48
	6.0% SOM	1.20	11	0.210	180	24	48
Beta - Hexachlorocyclohexanes	1.0% SOM	0.085	3.70	0.013	65	8.10	15
	2.5% SOM	0.200	3.80	0.032	65	8.10	15
	6.0% SOM	0.460	3.80	0.077	65	8.10	16
Gamma - Hexachlorocyclohexanes	1.0% SOM	0.06	2.90	0.0092	67	8.2	14
	2.5% SOM	0.14	3.30	0.0230	69	8.2	15
	6.0% SOM	0.33	3.50	0.0540	70	8.2	15
Chlorobenzenes							
Chlorobenzene	1.0% SOM	0.46	0.46	5.90	56	11,000	1,300(675)[sol]
	2.5% SOM	1.00	1.00	14	130	13,000	2,000(1520)[sol]
	6.0% SOM	2.40	2.40	32	290	14,000	2,900
1,2-Dichlorobenzene	1.0% SOM	23	24	94	2,000 (571) [sol]	90,000	24,000(571)[sol]
	2.5% SOM	55	57	230	4,800 (1370) [sol]	95,000	36,000(1370)[sol]
	6.0% SOM	130	130	540	11,000 (3240) [sol]	98,000	51,000(3240)[sol]
1,3-Dichlorobenzene	1.0% SOM	0.40	0.44	0.25	30	300	390
	2.5% SOM	1.00	1.10	0.60	73	300	440
	6.0% SOM	2.30	2.50	1.50	170	300	470
1,4-Dichlorobenzene	1.0% SOM	61	61	15	4,400 (224)[vap]	17,000[g]	36,000 (224)[vap]
	2.5% SOM	150	150	37	10,000 (540)[vap]	17,000[g]	36,000 (540)[vap]
	6.0% SOM	350	350	88[g]	25,000 (1280)[vap]	17,000[g]	36,000 (1280)[vap]
1,2,3,-Trichlorobenzene	1.0% SOM	1.50	1.50	4.70	102	1,800	770(134)[vap]
	2.5% SOM	3.60	3.70	12	250	1,800	1,100(330)[vap]
	6.0% SOM	8.60	8.80	28	590	1,800	1,600(789)[vap]
1,2,4,-Trichlorobenzene	1.0% SOM	2.60	2.60	55	220	15,000	1,700(318)[vap]
	2.5% SOM	6.40	6.40	140	530	17,000	2,600(786)[vap]
	6.0% SOM	15	15	320	1,300	19,000	4,000(1880)[vap]
1,3,5,-Trichlorobenzene	1.0% SOM	0.33	0.33	4.70	23	1,700	380(36.7)[vap]
	2.5% SOM	0.81	0.81	12	55	1,700	590(90.8)[vap]
	6.0% SOM	1.90	1.90	140	130	1,800	860(217)[vap]

[vap] – GAC presented exceeds the vapour saturation limit, which is presented in brackets.
[sol] – GAC presented exceeds the soil saturation limit, which is presented in brackets.

VOC and SVOC table continued overleaf

VOC and SVOC table continued from previous page

LQM CIEH General Assessment Criteria: Volatile and Semi-Volatile Organic Compounds							
Determinands		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**	**POSresi (mg/kg)**	**POSpark (mg/kg)**
Chlorobenzenes (cont.)							
1,2,3,4,- Tetrachlorobenzene	1.0% SOM	15	24	4.40	1,700(122)^{vap}	830	1,500(122)^{vap}
	2.5% SOM	36	56	11	3,080(304)^{vap}	830	1,600
	6.0% SOM	78	120	26	4,400(728)^{vap}	830	1,600
1,2,3,5,- Tetrachlobenzene	1.0% SOM	0.66	0.75	0.38	49(39.4)^{vap}	78	110(39)^{vap}
	2.5% SOM	1.60	1.90	0.90	120(98.1)^{vap}	79	120
	6.0% SOM	3.70	4.30	2.20	240(235)^{vap}	79	130
1,2,4, 5,- Tetrachlobenzene	1.0% SOM	0.33	0.73	0.06	42(19.7)^{sol}	13	25
	2.5% SOM	0.77	1.70	0.16	72(49.1)^{sol}	13	26
	6.0% SOM	1.60	3.50	0.37	96	13	26
Pentachlrobenzene	1.0% SOM	5.80	19	1.20	640(43.0)^{sol}	100	190
	2.5% SOM	12	30	3.10	770(107)^{sol}	100	190
	6.0% SOM	22	38	7.00	830	100	190
Hexachlorobenzene	1.0% SOM	1.80(0.20)^{vap}	4.10 (0.20)^{vap}	0.47	110(0.20)^{vap}	16	30
	2.5% SOM	3.30(0.50)^{vap}	5.70 (0.50)^{vap}	1.10	120	16	30
	6.0% SOM	4.90	6.70 (1.2)^{vap}	2.50	120	16	30
Phenols & Chlorophenols							
BTEX	1.0% SOM	280	750	66	760^{dir}(31,000)	760^{dir}(11,000)	760^{dir}(8,600)
	2.5% SOM	550	1,300	140	1,500^{dir}(35,000)	1,500^{dir}(11,000)	1,500^{dir}(9,700)
	6.0% SOM	1100	2,300	280	3,200^{dir}(37,000)	3,200^{dir}(11,000)	3,200^{dir}(11,000)
Chlorophenols (4 Congeners)	1.0% SOM	0.87	94	0.13	3,500	620	1,100
	2.5% SOM	2.00	150	0.30	4,000	620	1,100
	6.0% SOM	4.50	210	0.70	4,300	620	1,100
Pentachlorophenols	1.0% SOM	0.22	27(16.4)^{vap}	0.03	400	60	110
	2.5% SOM	0.52	29	0.08	400	60	120
	6.0% SOM	1.20	31	0.19	400	60	120
Others							
Carbon Disulphide	1.0% SOM	0.14	0.14	4.80	11	11,000	1,300
	2.5% SOM	0.29	0.29	10	22	11,000	1,900
	6.0% SOM	0.62	0.62	23	47	12,000	2,700
Hexachloro-1,3-Butadiene	1.0% SOM	0.29	0.32	0.25	31	25	48
	2.5% SOM	0.70	0.78	0.61	68	25	50
	6.0% SOM	1.60	1.80	1.40	120	25	51
^{vap} – GAC presented exceeds the vapour saturation limit, which is presented in brackets. ^{sol} – GAC presented exceeds the soil saturation limit, which is presented in brackets.							

VOC and SVOC table continued overleaf

VOC and SVOC table continued from previous page

CL:AIRE General Assessment Criteria: Volatile and Semi-Volatile Organic Compounds					
Determinands		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**
1,1,2 Trichloroethane	1.0% SOM	0.60	0.88	0.28	94
	2.5% SOM	1.20	1.8	0.61	190
	6.0% SOM	2.70	3.9	1.40	400
1,1-Dichloroethane	1.0% SOM	2.40	2.50	9.20	280
	2.5% SOM	3.90	4.10	17	450
	6.0% SOM	7.40	7.70	35	850
1,1-Dichloroethene	1.0% SOM	0.23	0.23	2.80	26
	2.5% SOM	0.40	0.41	5.60	46
	6.0% SOM	0.82	0.82	12	92
1,2,4-Trimethylbenzene	1.0% SOM	0.35	0.41	0.38	42
	2.5% SOM	0.85	0.99	0.93	99
	6.0% SOM	2.00	2.30	2.20	220
1,2-Dichloropropane	1.0% SOM	0.024	0.024	0.62	3.3
	2.5% SOM	0.042	0.042	1.20	5.9
	6.0% SOM	0.084	0.085	2.60	12
2,4-Dimethylphenol	1.0% SOM	19	210	3.10	16000*
	2.5% SOM	43	410	7.20	24000*
	6.0% SOM	97	730	17	30000*
2,4-Dinitrotoluene	1.0% SOM	1.50	170*	0.22	3700*
	2.5% SOM	3.20	170	0.49	3700*
	6.0% SOM	7.20	170	1.10	3800*
2,6-Dinitrotoluene	1.0% SOM	0.78	78	0.12	1900*
	2.5% SOM	1.70	84	0.27	1900*
	6.0% SOM	3.90	87	0.61	1900*
2-Chloronapthalene	1.0% SOM	3.70	3.80	40	390*
	2.5% SOM	9.20	9.30	98	960*
	6.0% SOM	22	22	230	2200*
Biphenyl	1.0% SOM	66*	220*	14	18000*
	2.5% SOM	160	500*	35	33000*
	6.0% SOM	360	980*	83	48000*
Bis (2-ethylhexyl) phthalate	1.0% SOM	280*	2700*	47*	85000*
	2.5% SOM	610*	2800*	120*	86000*
	6.0% SOM	1100*	2800*	280*	86000*
Bromobenzene	1.0% SOM	0.87	0.91	3.2	97
	2.5% SOM	2.0	2.1	7.6	220
	6.0% SOM	4.7	4.9	18	520
Bromodichloromethane	1.0% SOM	0.016	0.019	0.016	2.1
	2.5% SOM	0.030	0.034	0.032	3.7
	6.0% SOM	0.061	0.070	0.068	7.6
Bromoform	1.0% SOM	2.8	5.2	0.95	760
	2.5% SOM	5.9	11	2.1	1500
	6.0% SOM	13	23	4.6	3100
Butyl benzyl phthalate	1.0% SOM	1400*	42000*	220*	940000*
	2.5% SOM	3300*	44000*	550*	940000*
	6.0% SOM	7200*	44000*	1300*	950000*

*soil concentration above saturation limit

VOC and SVOC table continued overleaf

VOC and SVOC table continued from previous page

CL:AIRE General Assessment Criteria: Volatile and Semi-Volatile Organic Compounds					
Determinands		**RwHP (mg/kg)**	**RwoHP (mg/kg)**	**Allotment (mg/kg)**	**Commercial (mg/kg)**
Chloroethane	1.0% SOM	8.3	8.4	110	960
	2.5% SOM	11	11	200	1300
	6.0% SOM	18	18	380	2100
Chloromethane	1.0% SOM	0.0083	0.0085	0.066	1.0
	2.5% SOM	0.0098	0.0099	0.13	1.2
	6.0% SOM	0.013	0.013	0.23	1.6
***Cis* 1,2 Dichloroethene**	1.0% SOM	0.11	0.12	0.26	14
	2.5% SOM	0.19	0.20	0.50	24
	6.0% SOM	0.37	0.39	1.0	47
Dichloromethane	1.0% SOM	0.58	2.10	0.10	270
	2.5% SOM	0.98	2.80	0.19	360
	6.0% SOM	1.70	4.50	0.34	560
Diethyl Phthalate	1.0% SOM	120*	1800*	19*	150000*
	2.5% SOM	260*	3500*	41*	220000*
	6.0% SOM	570*	6300*	94*	290000*
Di-*n*-butyl phthalate	1.0% SOM	13*	450*	2.00	15000*
	2.5% SOM	31*	450*	5.00	15000*
	6.0% SOM	67*	450*	12	15000*
Di-*n*-octyl phthalate	1.0% SOM	2300*	3400*	940*	89000*
	2.5% SOM	2800*	3400*	2100*	89000*
	6.0% SOM	3100*	3400*	3900*	89000*
Hexachloroethane	1.0% SOM	0.20	0.22	0.27	22*
	2.5% SOM	0.48	0.54	0.67	53*
	6.0% SOM	1.10	1.30	1.60	120*
Isopropylbenzene	1.0% SOM	11	12	32	1400*
	2.5% SOM	27	28	79	3300*
	6.0% SOM	64	67	190	7700*
Methyl *tert*-butyl ether (MTBE)	1.0% SOM	49	73	23	7900
	2.5% SOM	84	120	44	13000
	6.0% SOM	160	220	90	24000
Propylbenzene	1.0% SOM	34	40	34	4100*
	2.5% SOM	82	97	83	9700*
	6.0% SOM	190	230	200	21000*
Styrene	1.0% SOM	8.10	35	1.60	3300*
	2.5% SOM	19	78	3.70	6500*
	6.0% SOM	43	170	8.70	11000*
Total Cresols (2-, 3-, and 4-methylphenol)	1.0% SOM	80	3700	12	160000
	2.5% SOM	180	5400	27	180000*
	6.0% SOM	400	6900	63	180000*
***Trans* 1,2 Dichloroethene**	1.0% SOM	0.19	0.19	0.93	22
	2.5% SOM	0.34	0.35	1.90	40
	6.0% SOM	0.70	0.71	0.24	81
Tributyl tin oxide	1.0% SOM	0.25	1.40	0.042	130*
	2.5% SOM	0.59	3.10	0.100	180*
	6.0% SOM	1.30	5.70	0.240	200*

*soil concentration above saturation limit

C4SL Low Level of Toxicological Concern							
Determinands		RwHP (mg/kg)	RwoHP (mg/kg)	Allotment (mg/kg)	Commercial (mg/kg)	POSresi (mg/kg)	POSpark (mg/kg)
1,2-Dichloroethane (Ethylene Dichloride)	1.0% SOM	0.11	0.16	0.054	12	300	300
	2.5% SOM	0.18	0.24	0.10	17	310	330
	6.0% SOM	0.31	0.41	0.19	29	310	380
Cis-1,2-Dichloroethene	1.0% SOM	0.46	0.50	0.89	38	3,800	2,000
	2.5% SOM	0.78	0.84	1.7	64	3,800	2,400
	6.0% SOM	1.5	1.6	3.6	120	3,900	3,100
Tetrachloroethene (PCE)	1.0% SOM	0.31	0.32	2	24	3,200	1,400
	2.5% SOM	0.70	0.71	4.8	55	3,300	1,900
	6.0% SOM	1.60	1.60	11	130	3,400	2,500
Trans-1,2-Dichloroethene	1.0% SOM	0.90	0.93	3.70	69	13,000	5,600
	2.5% SOM	1.60	1.70	7.50	120	13,000	7,000
	6.0% SOM	3.30	3.40	16	260	13,000	9,100
Trichloroethene (TCE)	1.0% SOM	0.0093	0.0097	0.032	0.73	76	41
	2.5% SOM	0.020	0.020	0.072	1.5	78	54
	6.0% SOM	0.043	0.045	0.16	3.4	79	69
Vinyl Chloride (Chloroethene)	1.0% SOM	0.0064	0.015	0.0017	1.1	7.8	18
	2.5% SOM	0.010	0.019	0.0031	1.4	7.8	19
	6.0% SOM	0.017	0.029	0.0058	2.2	7.8	19

9.6. Per- and Polyfluoroalkyl Substances (PFAS)

Interim C4SLs for PFOA, PFNA, PFHxS and PFOS						
Determinand	RwHP (mg/kg)	RwoHP (mg/kg)	Allotment (mg/kg)	Commercial (mg/kg)	POSresi (mg/kg)	POSpark (mg/kg)
PFOA	0.076	0.041	0.0014	0.60	0.079	0.17
PFOS	0.013	0.041	0.0027	0.60	0.079	0.17
PFHxS	0.0081	0.041	0.0015	0.60	0.079	0.17
PFNA	0.0073	0.041	0.0013	0.60	0.079	0.17

9.7. Asbestos

No asbestos or asbestos containing materials (ACM's) are considered acceptable on-site from a human health perspective. Therefore the GAC for asbestos & ACM's within any imported material should be none detected (ND).

APPENDIX G: Waste Hazard Assessment

2 The Long Barn, Norton Farm, Selborne Road, Alton, Hampshire GU34 3NB
0333 600 1221 enquiries@groundandwater.co.uk **groundandwater.co.uk**

Registered Office: Kineton House, 31 Horse Fair, Banbury, Oxfordshire OX16 0AE Registered in England No. 07032001

Waste Classification Report

HazWasteOnline™ classifies waste as either **hazardous** or **non-hazardous** based on its chemical composition, related legislation and the rules and data defined in the current UK or EU technical guidance (Appendix C) (note that HP 9 Infectious is not assessed). It is the responsibility of the classifier named below to:

a) understand the origin of the waste
b) select the correct List of Waste code(s)
c) confirm that the list of determinands, results and sampling plan are fit for purpose
d) select and justify the chosen metal species (Appendix B)
e) correctly apply moisture correction and other available corrections
f) add the meta data for their user-defined substances (Appendix A)
g) check that the classification engine is suitable with respect to the national destination of the waste (Appendix C)

To aid the reviewer, the laboratory results, assumptions and justifications managed by the classifier are highlighted in pale yellow.

27CSF-QS1H2-3WZVS

Report is invalid if pages are removed.

Job name

GWPR6571 the Chicklade Service Station, Hindon, Salisbury SP3 5ST

Description/Comments

Project

GWPR6571

Site

the Chicklade Service Station, Hindon, Salisbury SP3 5ST

Classified by

Name:
Roger Foord
Date:
03 Nov 2025 17:08 GMT
Telephone:
0333 600 1221

Company:
Ground and Water
2 The Long Barn, Norton Farm
Selborne Road
Alton
GU34 3NB

HazWasteOnline™ provides a two day, hazardous waste classification course that covers the use of the software and both basic and advanced waste classification techniques. Certification has to be renewed every 3 years.

HazWasteOnline™ Certification: -

Course	Date
Hazardous Waste Classification	-

Purpose of classification

2 - Material Characterisation

Address of the waste

the Chicklade Service Station, Hindon, Salisbury SP3 5ST **Post Code** SP 5ST

SIC for the process giving rise to the waste

41201 Construction of commercial buildings

Description of industry/producer giving rise to the waste

Redevelopment of former petrol station

Description of the specific process, sub-process and/or activity that created the waste

Waste created during foundation excavations

Description of the waste

Made Ground comprising red/grey sandy GRAVEL. Sand is fine to medium. Gravel was fine to coarse, sub-angular to sub-rounded brick (60%) and concrete (40%).

Job summary

#	Sample name	Depth [m]	Classification Result	Hazard properties	Page
1	TP11--22102025-0.50		Hazardous	HP 7, HP 11	3
2	CBR01--22102025-0.50		Non Hazardous		6
3	CBR02--22102025-0.50		Non Hazardous		8
4	CBR03--22102025-0.50		Non Hazardous		11
5	TP09--22102025-0.50		Non Hazardous		13
6	TP10--22102025-0.20		Non Hazardous		16

Related documents

#	Name	Description
1	25-057486_HWOL[2].hwol	i2 Analytical .hwol file used to populate the Job

Report

Created by: Roger Foord

Created date: 03 Nov 2025 17:08 GMT

Appendices	Page
Appendix A: Classifier defined and non GB MCL determinands	18
Appendix B: Rationale for selection of metal species	19
Appendix C: Version	20

Classification of sample: TP11--22102025-0.50

Hazardous Waste
Classified as **17 05 03 ***
in the List of Waste

Sample details

Sample name:
TP11--22102025-0.50
Moisture content:
9%
(wet weight correction)

LoW Code:
Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Entry: 17 05 03 * (Soil and stones containing hazardous substances)

Hazard properties

HP 7: Carcinogenic "waste which induces cancer or increases its incidence"

Hazard Statements hit:

Carc. 1B; H350 "May cause cancer [state route of exposure if it is conclusively proven that no other routes of exposure cause the hazard]."

Because of determinand:

TPH (C6 to C40) petroleum group (conc.: 0.118%)

HP 11: Mutagenic "waste which may cause a mutation, that is a permanent change in the amount or structure of the genetic material in a cell"

Hazard Statements hit:

Muta. 1B; H340 "May cause genetic defects [state route of exposure if it is conclusively proven that no other routes of exposure cause the hazard]."

Because of determinand:

TPH (C6 to C40) petroleum group (conc.: 0.118%)

Determinands

Moisture content: 9% Wet Weight Moisture Correction applied (MC)

#		Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
1		benzene	601-020-00-8	200-753-7	71-43-2		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<**LOD**
2		ethylbenzene	601-023-00-4	202-849-4	100-41-4		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<**LOD**
3		tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane	603-181-00-X	216-653-1	1634-04-4		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<**LOD**
4		xylene	601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<**LOD**
5		toluene	601-021-00-3	203-625-9	108-88-3		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<**LOD**
6		chromium in Cr(VI) compounds { chromium(VI) oxide }	024-001-00-0	215-607-8	1333-82-0		<1.8 mg/kg	1.923	<3.462 mg/kg	<0.000346 %		<**LOD**
7		monohydric phenols			P1186		<1 mg/kg		<1 mg/kg	<0.0001 %		<**LOD**
8		cyanides { salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides,					<1 mg/kg	1.884	<1.884 mg/kg	<0.000188 %		<**LOD**

#		Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
		ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex }	006-007-00-5											
9		pH			PH		8.2	pH		8.2	pH	8.2 pH		
10		boron { boron tribromide }	005-003-00-0	233-657-9	10294-33-4		1.1	mg/kg	23.173	23.196	mg/kg	0.00232 %	✓	
11		arsenic { arsenic }	033-001-00-X	231-148-6	7440-38-2		14	mg/kg		12.74	mg/kg	0.00127 %	✓	
12		cadmium { cadmium sulfide }	048-010-00-4	215-147-8	1306-23-6	1	1.1	mg/kg	1.285	1.287	mg/kg	0.0001 %	✓	
13		copper { dicopper oxide; copper (I) oxide }	029-002-00-X	215-270-7	1317-39-1		19	mg/kg	1.126	19.467	mg/kg	0.00195 %	✓	
14		mercury { mercury }	080-001-00-0	231-106-7	7439-97-6		<0.3	mg/kg		<0.3	mg/kg	<0.00003 %		<**LOD**
15		nickel { nickel dihydroxide }	028-008-00-X	235-008-5 [1] 234-348-1 [2]	12054-48-7 [1] 11113-74-9 [2]		11	mg/kg	1.579	15.811	mg/kg	0.00158 %	✓	
16		lead { lead chromate }	082-004-00-2	231-846-0	7758-97-6	1	53	mg/kg	1.56	75.23	mg/kg	0.00482 %	✓	
17		selenium { selenium }	034-001-00-2	231-957-4	7782-49-2		<1	mg/kg		<1	mg/kg	<0.0001 %		<**LOD**
18		vanadium { divanadium pentaoxide; vanadium pentoxide }	023-001-00-8	215-239-8	1314-62-1		20	mg/kg	1.785	32.49	mg/kg	0.00325 %	✓	
19		zinc { zinc sulphate }	030-006-00-9	231-793-3 [1] 231-793-3 [2]	7446-19-7 [1] 7733-02-0 [2]		310	mg/kg	2.469	696.589	mg/kg	0.0697 %	✓	
20		acenaphthylene		205-917-1	208-96-8		0.32	mg/kg		0.291	mg/kg	0.0000291 %	✓	
21		acenaphthene		201-469-6	83-32-9		1.7	mg/kg		1.547	mg/kg	0.000155 %	✓	
22		anthracene		204-371-1	120-12-7		4.1	mg/kg		3.731	mg/kg	0.000373 %	✓	
23		benzo[a]anthracene	601-033-00-9	200-280-6	56-55-3		11	mg/kg		10.01	mg/kg	0.001 %	✓	
24		benzo[a]pyrene; benzo[def]chrysene	601-032-00-3	200-028-5	50-32-8		13	mg/kg		11.83	mg/kg	0.00118 %	✓	
25		benzo[b]fluoranthene	601-034-00-4	205-911-9	205-99-2		14	mg/kg		12.74	mg/kg	0.00127 %	✓	
26		benzo[k]fluoranthene	601-036-00-5	205-916-6	207-08-9		6.3	mg/kg		5.733	mg/kg	0.000573 %	✓	
27		benzo[ghi]perylene		205-883-8	191-24-2		8	mg/kg		7.28	mg/kg	0.000728 %	✓	
28		chrysene	601-048-00-0	205-923-4	218-01-9		11	mg/kg		10.01	mg/kg	0.001 %	✓	
29		dibenz[a,h]anthracene	601-041-00-2	200-181-8	53-70-3		0.75	mg/kg		0.683	mg/kg	0.0000683 %	✓	
30		fluoranthene		205-912-4	206-44-0		23	mg/kg		20.93	mg/kg	0.00209 %	✓	
31		fluorene		201-695-5	86-73-7		1.1	mg/kg		1.001	mg/kg	0.0001 %	✓	
32		indeno[123-cd]pyrene		205-893-2	193-39-5		6.6	mg/kg		6.006	mg/kg	0.000601 %	✓	
33		naphthalene	601-052-00-2	202-049-5	91-20-3		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
34		phenanthrene		201-581-5	85-01-8		11	mg/kg		10.01	mg/kg	0.001 %	✓	
35		pyrene		204-927-3	129-00-0		23	mg/kg		20.93	mg/kg	0.00209 %	✓	

#		Determinand			CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
		EU CLP index number	EC Number	CAS Number									
36		TPH (C6 to C40) petroleum group		TPH		1300	mg/kg		1183	mg/kg	0.118 %	✓	
37		chromium in Cr(III) compounds { chromium(III) oxide }	215-160-9	1308-38-9		16	mg/kg	1.462	21.28	mg/kg	0.00213 %	✓	
										Total:	0.218 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
	Hazardous result
	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Deteminand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
CLP: Note 1	Only the metal concentration has been used for classification

Supplementary Hazardous Property Information

HP 3(i): Flammable "flammable liquid waste: liquid waste having a flash point below 60°C or waste gas oil, diesel and light heating oils having a flash point > 55°C and <= 75°C"
Force this Hazardous Property to non-hazardous for cumulative determinand results below the threshold of: 100000 mg/kg (10%) because: heavy end hydrocarbons unlikely to be flammable at the concentrations noted

Hazard Statements hit:

Flam. Liq. 3; H226 "Flammable liquid and vapour."

Because of determinand:

TPH (C6 to C40) petroleum group (conc.: 0.118%)

Classification of sample: CBR01--22102025-0.50

Non Hazardous Waste
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:
CBR01--22102025-0.50
Moisture content:
3.8%
(wet weight correction)

LoW Code:
Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)

Hazard properties

None identified

Determinands

Moisture content: 3.8% Wet Weight Moisture Correction applied (MC)

#	Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
1	chromium in Cr(VI) compounds { chromium(VI) oxide }	024-001-00-0	215-607-8	1333-82-0		<1.8 mg/kg	1.923	<3.462 mg/kg	<0.000346 %		<**LOD**
2	monohydric phenols			P1186		<1 mg/kg		<1 mg/kg	<0.0001 %		<**LOD**
3	cyanides { salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex }	006-007-00-5				<1 mg/kg	1.884	<1.884 mg/kg	<0.000188 %		<**LOD**
4	pH			PH		8.5 pH		8.5 pH	8.5 pH		
5	boron { boron tribromide }	005-003-00-0	233-657-9	10294-33-4		0.3 mg/kg	23.173	6.688 mg/kg	0.000669 %	✓	
6	arsenic { arsenic }	033-001-00-X	231-148-6	7440-38-2		12 mg/kg		11.544 mg/kg	0.00115 %	✓	
7	cadmium { cadmium sulfide }	048-010-00-4	215-147-8	1306-23-6	1	1.1 mg/kg	1.285	1.36 mg/kg	0.000106 %	✓	
8	copper { dicopper oxide; copper (I) oxide }	029-002-00-X	215-270-7	1317-39-1		11 mg/kg	1.126	11.914 mg/kg	0.00119 %	✓	
9	mercury { mercury }	080-001-00-0	231-106-7	7439-97-6		<0.3 mg/kg		<0.3 mg/kg	<0.00003 %		<**LOD**
10	nickel { nickel dihydroxide }	028-008-00-X	235-008-5 [1] 234-348-1 [2]	12054-48-7 [1] 11113-74-9 [2]		17 mg/kg	1.579	25.831 mg/kg	0.00258 %	✓	
11	lead { lead chromate }	082-004-00-2	231-846-0	7758-97-6	1	50 mg/kg	1.56	75.027 mg/kg	0.00481 %	✓	
12	selenium { selenium }	034-001-00-2	231-957-4	7782-49-2		1 mg/kg		0.962 mg/kg	0.0000962 %	✓	
13	vanadium { divanadium pentaoxide; vanadium pentoxide }	023-001-00-8	215-239-8	1314-62-1		17 mg/kg	1.785	29.195 mg/kg	0.00292 %	✓	
14	zinc { zinc sulphate }	030-006-00-9	231-793-3 [1] 231-793-3 [2]	7446-19-7 [1] 7733-02-0 [2]		210 mg/kg	2.469	498.847 mg/kg	0.0499 %	✓	
15	acenaphthylene		205-917-1	208-96-8		0.31 mg/kg		0.298 mg/kg	0.0000298 %	✓	

#		Determinand			CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
		EU CLP index number	EC Number	CAS Number									
16		acenaphthene				0.74	mg/kg		0.712	mg/kg	0.0000712 %	✓	
			201-469-6	83-32-9									
17		anthracene				1.4	mg/kg		1.347	mg/kg	0.000135 %	✓	
			204-371-1	120-12-7									
18		benzo[a]anthracene				7.7	mg/kg		7.407	mg/kg	0.000741 %	✓	
		601-033-00-9	200-280-6	56-55-3									
19		benzo[a]pyrene; benzo[def]chrysene				19	mg/kg		18.278	mg/kg	0.00183 %	✓	
		601-032-00-3	200-028-5	50-32-8									
20		benzo[b]fluoranthene				25	mg/kg		24.05	mg/kg	0.00241 %	✓	
		601-034-00-4	205-911-9	205-99-2									
21		benzo[k]fluoranthene				8.1	mg/kg		7.792	mg/kg	0.000779 %	✓	
		601-036-00-5	205-916-6	207-08-9									
22		benzo[ghi]perylene				15	mg/kg		14.43	mg/kg	0.00144 %	✓	
			205-883-8	191-24-2									
23		chrysene				9.6	mg/kg		9.235	mg/kg	0.000924 %	✓	
		601-048-00-0	205-923-4	218-01-9									
24		dibenz[a,h]anthracene				3.1	mg/kg		2.982	mg/kg	0.000298 %	✓	
		601-041-00-2	200-181-8	53-70-3									
25		fluoranthene				14	mg/kg		13.468	mg/kg	0.00135 %	✓	
			205-912-4	206-44-0									
26		fluorene				0.55	mg/kg		0.529	mg/kg	0.0000529 %	✓	
			201-695-5	86-73-7									
27		indeno[123-cd]pyrene				13	mg/kg		12.506	mg/kg	0.00125 %	✓	
			205-893-2	193-39-5									
28		naphthalene				<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
		601-052-00-2	202-049-5	91-20-3									
29		phenanthrene				3	mg/kg		2.886	mg/kg	0.000289 %	✓	
			201-581-5	85-01-8									
30		pyrene				13	mg/kg		12.506	mg/kg	0.00125 %	✓	
			204-927-3	129-00-0									
31		chromium in Cr(III) compounds { chromium(III) oxide }				18	mg/kg	1.462	25.308	mg/kg	0.00253 %	✓	
			215-160-9	1308-38-9									
										Total:	0.0788 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Deteminand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
CLP: Note 1	Only the metal concentration has been used for classification

Classification of sample: CBR02--22102025-0.50

Non Hazardous Waste
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:
CBR02--22102025-0.50
Moisture content:
6%
(wet weight correction)

LoW Code:
Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)

Hazard properties

None identified

Determinands

Moisture content: 6% Wet Weight Moisture Correction applied (MC)

#		Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
1		benzene	601-020-00-8	200-753-7	71-43-2		<5	µg/kg		<0.005	mg/kg	<0.0000005 %		<**LOD**
2		ethylbenzene	601-023-00-4	202-849-4	100-41-4		<5	µg/kg		<0.005	mg/kg	<0.0000005 %		<**LOD**
3		tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane	603-181-00-X	216-653-1	1634-04-4		<5	µg/kg		<0.005	mg/kg	<0.0000005 %		<**LOD**
4		xylene	601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<5	µg/kg		<0.005	mg/kg	<0.0000005 %		<**LOD**
5		toluene	601-021-00-3	203-625-9	108-88-3		<5	µg/kg		<0.005	mg/kg	<0.0000005 %		<**LOD**
6		chromium in Cr(VI) compounds { chromium(VI) oxide }	024-001-00-0	215-607-8	1333-82-0		<1.8	mg/kg	1.923	<3.462	mg/kg	<0.000346 %		<**LOD**
7		monohydric phenols			P1186		<1	mg/kg		<1	mg/kg	<0.0001 %		<**LOD**
8		cyanides { salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex }	006-007-00-5				<1	mg/kg	1.884	<1.884	mg/kg	<0.000188 %		<**LOD**
9		pH			PH		8.2	pH		8.2	pH	8.2 pH		
10		boron { boron tribromide }	005-003-00-0	233-657-9	10294-33-4		1.3	mg/kg	23.173	28.317	mg/kg	0.00283 %	✓	
11		arsenic { arsenic }	033-001-00-X	231-148-6	7440-38-2		10	mg/kg		9.4	mg/kg	0.00094 %	✓	
12		cadmium { cadmium sulfide }	048-010-00-4	215-147-8	1306-23-6	1	0.8	mg/kg	1.285	0.967	mg/kg	0.0000752 %	✓	
13		copper { dicopper oxide; copper (I) oxide }	029-002-00-X	215-270-7	1317-39-1		6.5	mg/kg	1.126	6.879	mg/kg	0.000688 %	✓	
14		mercury { mercury }	080-001-00-0	231-106-7	7439-97-6		<0.3	mg/kg		<0.3	mg/kg	<0.00003 %		<**LOD**

#		Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
15		nickel { nickel dihydroxide }	028-008-00-X	235-008-5 [1] 234-348-1 [2]	12054-48-7 [1] 11113-74-9 [2]		14	mg/kg	1.579	20.786	mg/kg	0.00208 %	✓	
16		lead { lead chromate }	082-004-00-2	231-846-0	7758-97-6	1	60	mg/kg	1.56	87.974	mg/kg	0.00564 %	✓	
17		selenium { selenium }	034-001-00-2	231-957-4	7782-49-2		1	mg/kg		0.94	mg/kg	0.000094 %	✓	
18		vanadium { divanadium pentaoxide; vanadium pentoxide }	023-001-00-8	215-239-8	1314-62-1		19	mg/kg	1.785	31.883	mg/kg	0.00319 %	✓	
19		zinc { zinc sulphate }	030-006-00-9	231-793-3 [1] 231-793-3 [2]	7446-19-7 [1] 7733-02-0 [2]		170	mg/kg	2.469	394.594	mg/kg	0.0395 %	✓	
20		acenaphthylene		205-917-1	208-96-8		0.3	mg/kg		0.282	mg/kg	0.0000282 %	✓	
21		acenaphthene		201-469-6	83-32-9		0.77	mg/kg		0.724	mg/kg	0.0000724 %	✓	
22		anthracene		204-371-1	120-12-7		1.9	mg/kg		1.786	mg/kg	0.000179 %	✓	
23		benzo[a]anthracene	601-033-00-9	200-280-6	56-55-3		7.7	mg/kg		7.238	mg/kg	0.000724 %	✓	
24		benzo[a]pyrene; benzo[def]chrysene	601-032-00-3	200-028-5	50-32-8		19	mg/kg		17.86	mg/kg	0.00179 %	✓	
25		benzo[b]fluoranthene	601-034-00-4	205-911-9	205-99-2		24	mg/kg		22.56	mg/kg	0.00226 %	✓	
26		benzo[k]fluoranthene	601-036-00-5	205-916-6	207-08-9		8.4	mg/kg		7.896	mg/kg	0.00079 %	✓	
27		benzo[ghi]perylene		205-883-8	191-24-2		14	mg/kg		13.16	mg/kg	0.00132 %	✓	
28		chrysene	601-048-00-0	205-923-4	218-01-9		8.2	mg/kg		7.708	mg/kg	0.000771 %	✓	
29		dibenz[a,h]anthracene	601-041-00-2	200-181-8	53-70-3		2.8	mg/kg		2.632	mg/kg	0.000263 %	✓	
30		fluoranthene		205-912-4	206-44-0		13	mg/kg		12.22	mg/kg	0.00122 %	✓	
31		fluorene		201-695-5	86-73-7		0.72	mg/kg		0.677	mg/kg	0.0000677 %	✓	
32		indeno[123-cd]pyrene		205-893-2	193-39-5		13	mg/kg		12.22	mg/kg	0.00122 %	✓	
33		naphthalene	601-052-00-2	202-049-5	91-20-3		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
34		phenanthrene		201-581-5	85-01-8		4.5	mg/kg		4.23	mg/kg	0.000423 %	✓	
35		pyrene		204-927-3	129-00-0		12	mg/kg		11.28	mg/kg	0.00113 %	✓	
36		TPH (C6 to C40) petroleum group			TPH		630	mg/kg		592.2	mg/kg	0.0592 %	✓	
37		chromium in Cr(III) compounds { chromium(III) oxide }		215-160-9	1308-38-9		15	mg/kg	1.462	20.608	mg/kg	0.00206 %	✓	
											Total:	0.129 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Deteminand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
CLP: Note 1	Only the metal concentration has been used for classification

Supplementary Hazardous Property Information

HP 3(i): Flammable "flammable liquid waste: liquid waste having a flash point below 60°C or waste gas oil, diesel and light heating oils having a flash point > 55°C and <= 75°C"
Force this Hazardous Property to non-hazardous for cumulative determinand results below the threshold of: 100000 mg/kg (10%) because: heavy end hydrocarbons unlikely to be flammable at the concentrations noted

Hazard Statements hit:

Flam. Liq. 3; H226 "Flammable liquid and vapour."

Because of determinand:

TPH (C6 to C40) petroleum group (conc.: 0.0592%)

Classification of sample: CBR03--22102025-0.50

Non Hazardous Waste
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:
CBR03--22102025-0.50
Moisture content:
3.1%
(wet weight correction)

LoW Code:
Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)

Hazard properties

None identified

Determinands

Moisture content: 3.1% Wet Weight Moisture Correction applied (MC)

#	Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
1	chromium in Cr(VI) compounds { chromium(VI) oxide }	024-001-00-0	215-607-8	1333-82-0		<1.8 mg/kg	1.923	<3.462 mg/kg	<0.000346 %		<**LOD**
2	monohydric phenols			P1186		<1 mg/kg		<1 mg/kg	<0.0001 %		<**LOD**
3	cyanides { salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex }	006-007-00-5				<1 mg/kg	1.884	<1.884 mg/kg	<0.000188 %		<**LOD**
4	pH			PH		8.2 pH		8.2 pH	8.2 pH		
5	boron { boron tribromide }	005-003-00-0	233-657-9	10294-33-4		15 mg/kg	23.173	336.819 mg/kg	0.0337 %	✓	
6	arsenic { arsenic }	033-001-00-X	231-148-6	7440-38-2		12 mg/kg		11.628 mg/kg	0.00116 %	✓	
7	cadmium { cadmium sulfide }	048-010-00-4	215-147-8	1306-23-6	1	1 mg/kg	1.285	1.245 mg/kg	0.0000969 %	✓	
8	copper { dicopper oxide; copper (I) oxide }	029-002-00-X	215-270-7	1317-39-1		9.1 mg/kg	1.126	9.928 mg/kg	0.000993 %	✓	
9	mercury { mercury }	080-001-00-0	231-106-7	7439-97-6		<0.3 mg/kg		<0.3 mg/kg	<0.00003 %		<**LOD**
10	nickel { nickel dihydroxide }	028-008-00-X	235-008-5 [1] 234-348-1 [2]	12054-48-7 [1] 11113-74-9 [2]		13 mg/kg	1.579	19.897 mg/kg	0.00199 %	✓	
11	lead { lead chromate }	082-004-00-2	231-846-0	7758-97-6	1	71 mg/kg	1.56	107.314 mg/kg	0.00688 %	✓	
12	selenium { selenium }	034-001-00-2	231-957-4	7782-49-2		<1 mg/kg		<1 mg/kg	<0.0001 %		<**LOD**
13	vanadium { divanadium pentaoxide; vanadium pentoxide }	023-001-00-8	215-239-8	1314-62-1		17 mg/kg	1.785	29.407 mg/kg	0.00294 %	✓	
14	zinc { zinc sulphate }	030-006-00-9	231-793-3 [1] 231-793-3 [2]	7446-19-7 [1] 7733-02-0 [2]		160 mg/kg	2.469	382.84 mg/kg	0.0383 %	✓	
15	acenaphthylene		205-917-1	208-96-8		<0.05 mg/kg		<0.05 mg/kg	<0.000005 %		<**LOD**

#		Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
16		acenaphthene		201-469-6	83-32-9		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
17		anthracene		204-371-1	120-12-7		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
18		benzo[a]anthracene	601-033-00-9	200-280-6	56-55-3		0.1	mg/kg		0.0969	mg/kg	0.00000969 %	✓	
19		benzo[a]pyrene; benzo[def]chrysene	601-032-00-3	200-028-5	50-32-8		0.11	mg/kg		0.107	mg/kg	0.0000107 %	✓	
20		benzo[b]fluoranthene	601-034-00-4	205-911-9	205-99-2		0.16	mg/kg		0.155	mg/kg	0.0000155 %	✓	
21		benzo[k]fluoranthene	601-036-00-5	205-916-6	207-08-9		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
22		benzo[ghi]perylene		205-883-8	191-24-2		0.06	mg/kg		0.0581	mg/kg	0.00000581 %	✓	
23		chrysene	601-048-00-0	205-923-4	218-01-9		0.09	mg/kg		0.0872	mg/kg	0.00000872 %	✓	
24		dibenz[a,h]anthracene	601-041-00-2	200-181-8	53-70-3		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
25		fluoranthene		205-912-4	206-44-0		0.16	mg/kg		0.155	mg/kg	0.0000155 %	✓	
26		fluorene		201-695-5	86-73-7		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
27		indeno[123-cd]pyrene		205-893-2	193-39-5		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
28		naphthalene	601-052-00-2	202-049-5	91-20-3		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
29		phenanthrene		201-581-5	85-01-8		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
30		pyrene		204-927-3	129-00-0		0.16	mg/kg		0.155	mg/kg	0.0000155 %	✓	
31		chromium in Cr(III) compounds { chromium(III) oxide }		215-160-9	1308-38-9		16	mg/kg	1.462	22.66	mg/kg	0.00227 %	✓	
											Total:	0.0884 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Deteminand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
CLP: Note 1	Only the metal concentration has been used for classification

Classification of sample: TP09--22102025-0.50

Non Hazardous Waste
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:
TP09--22102025-0.50
Moisture content:
9.3%
(wet weight correction)

LoW Code:
Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)

Hazard properties

None identified

Determinands

Moisture content: 9.3% Wet Weight Moisture Correction applied (MC)

#	Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
1	benzene	601-020-00-8	200-753-7	71-43-2		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<LOD
2	ethylbenzene	601-023-00-4	202-849-4	100-41-4		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<LOD
3	tert-butyl methyl ether; MTBE; 2-methoxy-2-methylpropane	603-181-00-X	216-653-1	1634-04-4		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<LOD
4	xylene	601-022-00-9	202-422-2 [1] 203-396-5 [2] 203-576-3 [3] 215-535-7 [4]	95-47-6 [1] 106-42-3 [2] 108-38-3 [3] 1330-20-7 [4]		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<LOD
5	toluene	601-021-00-3	203-625-9	108-88-3		<5 µg/kg		<0.005 mg/kg	<0.0000005 %		<LOD
6	chromium in Cr(VI) compounds { chromium(VI) oxide }	024-001-00-0	215-607-8	1333-82-0		<1.8 mg/kg	1.923	<3.462 mg/kg	<0.000346 %		<LOD
7	monohydric phenols			P1186		<1 mg/kg		<1 mg/kg	<0.0001 %		<LOD
8	cyanides { salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex }	006-007-00-5				<1 mg/kg	1.884	<1.884 mg/kg	<0.000188 %		<LOD
9	pH			PH		8.4 pH		8.4 pH	8.4 pH		
10	boron { boron tribromide }	005-003-00-0	233-657-9	10294-33-4		0.8 mg/kg	23.173	16.814 mg/kg	0.00168 %	✓	
11	arsenic { arsenic }	033-001-00-X	231-148-6	7440-38-2		8.8 mg/kg		7.982 mg/kg	0.000798 %	✓	
12	cadmium { cadmium sulfide }	048-010-00-4	215-147-8	1306-23-6	1	1 mg/kg	1.285	1.166 mg/kg	0.0000907 %	✓	
13	copper { dicopper oxide; copper (I) oxide }	029-002-00-X	215-270-7	1317-39-1		7.9 mg/kg	1.126	8.067 mg/kg	0.000807 %	✓	
14	mercury { mercury }	080-001-00-0	231-106-7	7439-97-6		<0.3 mg/kg		<0.3 mg/kg	<0.00003 %		<LOD

#		Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
15		nickel { nickel dihydroxide }	028-008-00-X	235-008-5 [1] 234-348-1 [2]	12054-48-7 [1] 11113-74-9 [2]		8.6	mg/kg	1.579	12.32	mg/kg	0.00123 %	✓	
16		lead { lead chromate }	082-004-00-2	231-846-0	7758-97-6	1	70	mg/kg	1.56	99.033	mg/kg	0.00635 %	✓	
17		selenium { selenium }	034-001-00-2	231-957-4	7782-49-2		<1	mg/kg		<1	mg/kg	<0.0001 %		<**LOD**
18		vanadium { divanadium pentaoxide; vanadium pentoxide }	023-001-00-8	215-239-8	1314-62-1		14	mg/kg	1.785	22.668	mg/kg	0.00227 %	✓	
19		zinc { zinc sulphate }	030-006-00-9	231-793-3 [1] 231-793-3 [2]	7446-19-7 [1] 7733-02-0 [2]		160	mg/kg	2.469	358.344	mg/kg	0.0358 %	✓	
20		acenaphthylene		205-917-1	208-96-8		0.33	mg/kg		0.299	mg/kg	0.0000299 %	✓	
21		acenaphthene		201-469-6	83-32-9		0.87	mg/kg		0.789	mg/kg	0.0000789 %	✓	
22		anthracene		204-371-1	120-12-7		2.6	mg/kg		2.358	mg/kg	0.000236 %	✓	
23		benzo[a]anthracene	601-033-00-9	200-280-6	56-55-3		19	mg/kg		17.233	mg/kg	0.00172 %	✓	
24		benzo[a]pyrene; benzo[def]chrysene	601-032-00-3	200-028-5	50-32-8		23	mg/kg		20.861	mg/kg	0.00209 %	✓	
25		benzo[b]fluoranthene	601-034-00-4	205-911-9	205-99-2		32	mg/kg		29.024	mg/kg	0.0029 %	✓	
26		benzo[k]fluoranthene	601-036-00-5	205-916-6	207-08-9		13	mg/kg		11.791	mg/kg	0.00118 %	✓	
27		benzo[ghi]perylene		205-883-8	191-24-2		12	mg/kg		10.884	mg/kg	0.00109 %	✓	
28		chrysene	601-048-00-0	205-923-4	218-01-9		19	mg/kg		17.233	mg/kg	0.00172 %	✓	
29		dibenz[a,h]anthracene	601-041-00-2	200-181-8	53-70-3		3.2	mg/kg		2.902	mg/kg	0.00029 %	✓	
30		fluoranthene		205-912-4	206-44-0		31	mg/kg		28.117	mg/kg	0.00281 %	✓	
31		fluorene		201-695-5	86-73-7		0.53	mg/kg		0.481	mg/kg	0.0000481 %	✓	
32		indeno[123-cd]pyrene		205-893-2	193-39-5		12	mg/kg		10.884	mg/kg	0.00109 %	✓	
33		naphthalene	601-052-00-2	202-049-5	91-20-3		<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
34		phenanthrene		201-581-5	85-01-8		6.5	mg/kg		5.896	mg/kg	0.00059 %	✓	
35		pyrene		204-927-3	129-00-0		29	mg/kg		26.303	mg/kg	0.00263 %	✓	
36		TPH (C6 to C40) petroleum group			TPH		300	mg/kg		272.1	mg/kg	0.0272 %	✓	
37		chromium in Cr(III) compounds { chromium(III) oxide }		215-160-9	1308-38-9		11	mg/kg	1.462	14.582	mg/kg	0.00146 %	✓	
											Total:	0.0962 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Deteminand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
CLP: Note 1	Only the metal concentration has been used for classification

Supplementary Hazardous Property Information

HP 3(i): Flammable "flammable liquid waste: liquid waste having a flash point below 60°C or waste gas oil, diesel and light heating oils having a flash point > 55°C and <= 75°C"
Force this Hazardous Property to non-hazardous for cumulative determinand results below the threshold of: 100000 mg/kg (10%) because: heavy end hydrocarbons unlikely to be flammable at the concentrations noted

Hazard Statements hit:

Flam. Liq. 3; H226 "Flammable liquid and vapour."

Because of determinand:

TPH (C6 to C40) petroleum group (conc.: 0.0272%)

Classification of sample: TP10--22102025-0.20

Non Hazardous Waste
Classified as **17 05 04**
in the List of Waste

Sample details

Sample name:
TP10--22102025-0.20
Moisture content:
4.3%
(wet weight correction)

LoW Code:
Chapter: 17: Construction and Demolition Wastes (including excavated soil from contaminated sites)
Entry: 17 05 04 (Soil and stones other than those mentioned in 17 05 03)

Hazard properties

None identified

Determinands

Moisture content: 4.3% Wet Weight Moisture Correction applied (MC)

#	Determinand	EU CLP index number	EC Number	CAS Number	CLP Note	User entered data	Conv. Factor	Compound conc.	Classification value	MC Applied	Conc. Not Used
1	chromium in Cr(VI) compounds { chromium(VI) oxide }	024-001-00-0	215-607-8	1333-82-0		<1.8 mg/kg	1.923	<3.462 mg/kg	<0.000346 %		<LOD
2	monohydric phenols			P1186		<1 mg/kg		<1 mg/kg	<0.0001 %		<LOD
3	cyanides { salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex }	006-007-00-5				<1 mg/kg	1.884	<1.884 mg/kg	<0.000188 %		<LOD
4	pH			PH		8.4 pH		8.4 pH	8.4 pH		
5	boron { boron tribromide }	005-003-00-0	233-657-9	10294-33-4		0.6 mg/kg	23.173	13.306 mg/kg	0.00133 %	✓	
6	arsenic { arsenic }	033-001-00-X	231-148-6	7440-38-2		13 mg/kg		12.441 mg/kg	0.00124 %	✓	
7	cadmium { cadmium sulfide }	048-010-00-4	215-147-8	1306-23-6	1	1.3 mg/kg	1.285	1.599 mg/kg	0.000124 %	✓	
8	copper { dicopper oxide; copper (I) oxide }	029-002-00-X	215-270-7	1317-39-1		9.3 mg/kg	1.126	10.021 mg/kg	0.001 %	✓	
9	mercury { mercury }	080-001-00-0	231-106-7	7439-97-6		<0.3 mg/kg		<0.3 mg/kg	<0.00003 %		<LOD
10	nickel { nickel dihydroxide }	028-008-00-X	235-008-5 [1] 234-348-1 [2]	12054-48-7 [1] 11113-74-9 [2]		13 mg/kg	1.579	19.651 mg/kg	0.00197 %	✓	
11	lead { lead chromate }	082-004-00-2	231-846-0	7758-97-6	1	82 mg/kg	1.56	122.405 mg/kg	0.00785 %	✓	
12	selenium { selenium }	034-001-00-2	231-957-4	7782-49-2		1.1 mg/kg		1.053 mg/kg	0.000105 %	✓	
13	vanadium { divanadium pentaoxide; vanadium pentoxide }	023-001-00-8	215-239-8	1314-62-1		16 mg/kg	1.785	27.335 mg/kg	0.00273 %	✓	
14	zinc { zinc sulphate }	030-006-00-9	231-793-3 [1] 231-793-3 [2]	7446-19-7 [1] 7733-02-0 [2]		250 mg/kg	2.469	590.779 mg/kg	0.0591 %	✓	
15	acenaphthylene		205-917-1	208-96-8		0.48 mg/kg		0.459 mg/kg	0.0000459 %	✓	

#		Determinand			CLP Note	User entered data		Conv. Factor	Compound conc.		Classification value	MC Applied	Conc. Not Used
		EU CLP index number	EC Number	CAS Number									
16		acenaphthene				0.69	mg/kg		0.66	mg/kg	0.000066 %	✓	
			201-469-6	83-32-9									
17		anthracene				2	mg/kg		1.914	mg/kg	0.000191 %	✓	
			204-371-1	120-12-7									
18		benzo[a]anthracene				14	mg/kg		13.398	mg/kg	0.00134 %	✓	
		601-033-00-9	200-280-6	56-55-3									
19		benzo[a]pyrene; benzo[def]chrysene				31	mg/kg		29.667	mg/kg	0.00297 %	✓	
		601-032-00-3	200-028-5	50-32-8									
20		benzo[b]fluoranthene				43	mg/kg		41.151	mg/kg	0.00412 %	✓	
		601-034-00-4	205-911-9	205-99-2									
21		benzo[k]fluoranthene				11	mg/kg		10.527	mg/kg	0.00105 %	✓	
		601-036-00-5	205-916-6	207-08-9									
22		benzo[ghi]perylene				27	mg/kg		25.839	mg/kg	0.00258 %	✓	
			205-883-8	191-24-2									
23		chrysene				14	mg/kg		13.398	mg/kg	0.00134 %	✓	
		601-048-00-0	205-923-4	218-01-9									
24		dibenz[a,h]anthracene				5.1	mg/kg		4.881	mg/kg	0.000488 %	✓	
		601-041-00-2	200-181-8	53-70-3									
25		fluoranthene				23	mg/kg		22.011	mg/kg	0.0022 %	✓	
			205-912-4	206-44-0									
26		fluorene				0.55	mg/kg		0.526	mg/kg	0.0000526 %	✓	
			201-695-5	86-73-7									
27		indeno[123-cd]pyrene				24	mg/kg		22.968	mg/kg	0.0023 %	✓	
			205-893-2	193-39-5									
28		naphthalene				<0.05	mg/kg		<0.05	mg/kg	<0.000005 %		<**LOD**
		601-052-00-2	202-049-5	91-20-3									
29		phenanthrene				4	mg/kg		3.828	mg/kg	0.000383 %	✓	
			201-581-5	85-01-8									
30		pyrene				21	mg/kg		20.097	mg/kg	0.00201 %	✓	
			204-927-3	129-00-0									
31		chromium in Cr(III) compounds { chromium(III) oxide }				14	mg/kg	1.462	19.582	mg/kg	0.00196 %	✓	
			215-160-9	1308-38-9									
										Total:	0.0985 %		

Key

	User supplied data
	Determinand values ignored for classification, see column 'Conc. Not Used' for reason
	Determinand defined or amended by HazWasteOnline (see Appendix A)
	Speciated Deteminand - Unless the Determinand is Note 1, the Conversion Factor is used to calculate the compound concentration
<LOD	Below limit of detection
CLP: Note 1	Only the metal concentration has been used for classification

Appendix A: Classifier defined and non GB MCL determinands

ethylbenzene (EC Number: 202-849-4, CAS Number: 100-41-4)

GB MCL index number: 601-023-00-4
Description/Comments:
Additional Hazard Statement(s): Carc. 2; H351
Reason for additional Hazards Statement(s):
20 Nov 2021 - Carc. 2; H351 hazard statement sourced from: IARC Group 2B (77) 2000

monohydric phenols (CAS Number: P1186)

Description/Comments: Combined hazards statements from harmonised entries in CLP for phenol, cresols and xylenols (604-001-00-2, 604-004-00-9, 604-006-00-X)
Data source: CLP combined data
Data source date: 26 Mar 2019
Hazard Statements: Muta. 2; H341 , Acute Tox. 3; H331 , Acute Tox. 3; H311 , Acute Tox. 3; H301 , STOT RE 2; H373 , Skin Corr. 1B; H314 , Skin Corr. 1B; H314 >= 3 % , Skin Irrit. 2; H315 1 <= conc. < 3 % , Eye Irrit. 2; H319 1 <= conc. < 3 % , Aquatic Chronic 2; H411

salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex

GB MCL index number: 006-007-00-5
Description/Comments: Conversion factor based on a worst case compound: sodium cyanide
Additional Hazard Statement(s): EUH032 >= 0.2 %
Reason for additional Hazards Statement(s):
20 Nov 2021 - EUH032 >= 0.2 % hazard statement sourced from: WM3, Table C12.2

pH (CAS Number: PH)

Description/Comments: Appendix C4
Data source: WM3 1st Edition 2015
Data source date: 25 May 2015
Hazard Statements: None.

arsenic (EC Number: 231-148-6, CAS Number: 7440-38-2)

GB MCL index number: 033-001-00-X
Description/Comments: Worst Case: IARC considers arsenic Group 1; Carcinogenic to humans
Additional Hazard Statement(s): Carc. 1A; H350
Reason for additional Hazards Statement(s):
20 Nov 2021 - Carc. 1A; H350 hazard statement sourced from: IARC Group 1 (23, Sup 7, 100C) 2012

acenaphthylene (EC Number: 205-917-1, CAS Number: 208-96-8)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 17 Jul 2015
Hazard Statements: Acute Tox. 4; H302 , Acute Tox. 1; H330 , Acute Tox. 1; H310 , Eye Irrit. 2; H319 , STOT SE 3; H335 , Skin Irrit. 2; H315

acenaphthene (EC Number: 201-469-6, CAS Number: 83-32-9)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 17 Jul 2015
Hazard Statements: Eye Irrit. 2; H319 , STOT SE 3; H335 , Skin Irrit. 2; H315 , Aquatic Acute 1; H400 , Aquatic Chronic 1; H410 , Aquatic Chronic 2; H411

anthracene (EC Number: 204-371-1, CAS Number: 120-12-7)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 17 Jul 2015
Hazard Statements: Eye Irrit. 2; H319 , STOT SE 3; H335 , Skin Irrit. 2; H315 , Skin Sens. 1; H317 , Aquatic Acute 1; H400 , Aquatic Chronic 1; H410

benzo[ghi]perylene (EC Number: 205-883-8, CAS Number: 191-24-2)

Description/Comments: Data from C&L Inventory Database; SDS Sigma Aldrich 28/02/2015
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 23 Jul 2015
Hazard Statements: Aquatic Acute 1; H400 , Aquatic Chronic 1; H410

fluoranthene (EC Number: 205-912-4, CAS Number: 206-44-0)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 21 Aug 2015
Hazard Statements: Acute Tox. 4; H302 , Aquatic Acute 1; H400 , Aquatic Chronic 1; H410

fluorene (EC Number: 201-695-5, CAS Number: 86-73-7)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 06 Aug 2015
Hazard Statements: Aquatic Acute 1; H400 , Aquatic Chronic 1; H410

indeno[123-cd]pyrene (EC Number: 205-893-2, CAS Number: 193-39-5)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 06 Aug 2015
Hazard Statements: Carc. 2; H351

phenanthrene (EC Number: 201-581-5, CAS Number: 85-01-8)

Description/Comments: Data from C&L Inventory Database
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 06 Aug 2015
Hazard Statements: Acute Tox. 4; H302 , Eye Irrit. 2; H319 , STOT SE 3; H335 , Carc. 2; H351 , Skin Sens. 1; H317 , Aquatic Acute 1; H400 , Aquatic Chronic 1; H410 , Skin Irrit. 2; H315

pyrene (EC Number: 204-927-3, CAS Number: 129-00-0)

Description/Comments: Data from C&L Inventory Database; SDS Sigma Aldrich 2014
Data source: http://echa.europa.eu/web/guest/information-on-chemicals/cl-inventory-database
Data source date: 21 Aug 2015
Hazard Statements: Skin Irrit. 2; H315 , Eye Irrit. 2; H319 , STOT SE 3; H335 , Aquatic Acute 1; H400 , Aquatic Chronic 1; H410

TPH (C6 to C40) petroleum group (CAS Number: TPH)

Description/Comments: Unknown Oil

Hazard statements taken from WM3 1st Edition 2015
Data source: WM3 1st Edition 2015
Data source date: 25 May 2015
Hazard Statements: Flam. Liq. 3; H226 , Asp. Tox. 1; H304 , STOT RE 2; H373 , Muta. 1B; H340 , Carc. 1B; H350 , Repr. 2; H361d , Aquatic Chronic 2; H411

chromium(III) oxide (EC Number: 215-160-9, CAS Number: 1308-38-9)

Description/Comments: Data from ECHA's C&L inventory database
Data source: https://echa.europa.eu/information-on-chemicals/cl-inventory-database/-/discli/details/33806
Data source date: 30 Apr 2020
Hazard Statements: Acute Tox. 4; H302 , Skin Sens. 1; H317 , Eye Irrit. 2; H319

Appendix B: Rationale for selection of metal species

chromium in Cr(VI) compounds {chromium(VI) oxide}

Most likely worst case.

cyanides {salts of hydrogen cyanide with the exception of complex cyanides such as ferrocyanides, ferricyanides and mercuric oxycyanide and those specified elsewhere in this Annex}

Most likely worst case.

boron {boron tribromide}

Most likely worst case.

arsenic {arsenic}

Most likely worst case.

cadmium {cadmium sulfide}

Most likely worst case.

copper {dicopper oxide; copper (I) oxide}

Most likely worst case.

mercury {mercury}

Most likely worst case.

nickel {nickel dihydroxide}

Most likely worst case.

lead {lead chromate}

Most likely worst case.

selenium {selenium}

Most likely worst case.

vanadium {divanadium pentaoxide; vanadium pentoxide}

Most likely worst case.

zinc {zinc sulphate}

Most likely worst case.

chromium in Cr(III) compounds {chromium(III) oxide}

Most likely worst case.

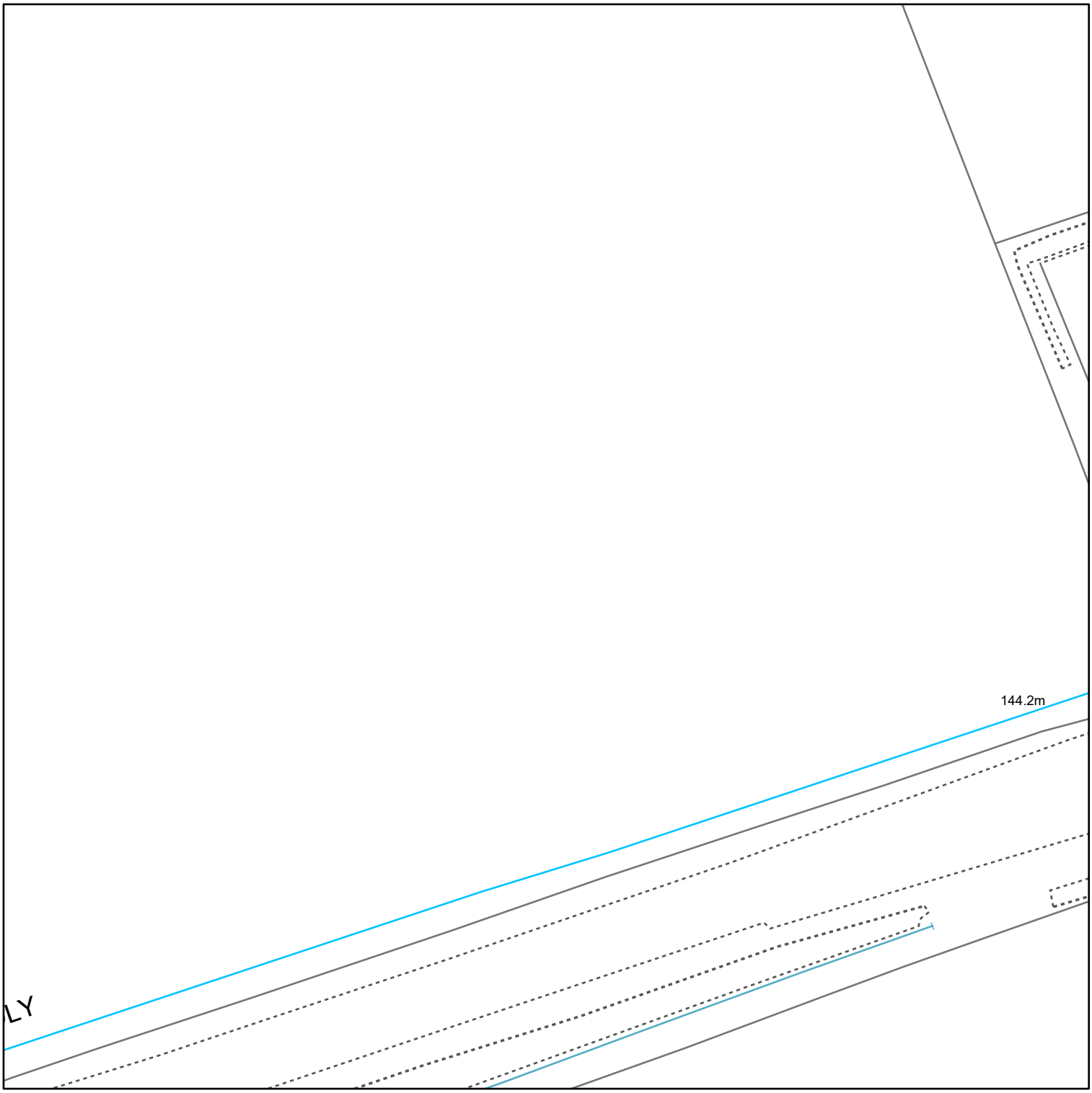
Appendix C: Version

HazWasteOnline Classification Engine: WM3 1st Edition v1.2.GB - Oct 2021
HazWasteOnline Classification Engine Version: 2025.303.6844.12384 (30 Oct 2025)
HazWasteOnline Database: 2025.303.6844.12384 (30 Oct 2025)

This classification utilises the following guidance and legislation:
WM3 v1.2.GB - Waste Classification - 1st Edition v1.2.GB - Oct 2021
CLP Regulation - Regulation 1272/2008/EC of 16 December 2008
1st ATP - Regulation 790/2009/EC of 10 August 2009
2nd ATP - Regulation 286/2011/EC of 10 March 2011
3rd ATP - Regulation 618/2012/EU of 10 July 2012
4th ATP - Regulation 487/2013/EU of 8 May 2013
Correction to 1st ATP - Regulation 758/2013/EU of 7 August 2013
5th ATP - Regulation 944/2013/EU of 2 October 2013
6th ATP - Regulation 605/2014/EU of 5 June 2014
WFD Annex III replacement - Regulation 1357/2014/EU of 18 December 2014
Revised List of Waste 2014 - Decision 2014/955/EU of 18 December 2014
7th ATP - Regulation 2015/1221/EU of 24 July 2015
8th ATP - Regulation (EU) 2016/918 of 19 May 2016
9th ATP - Regulation (EU) 2016/1179 of 19 July 2016
10th ATP - Regulation (EU) 2017/776 of 4 May 2017
HP14 amendment - Regulation (EU) 2017/997 of 8 June 2017
13th ATP - Regulation (EU) 2018/1480 of 4 October 2018
14th ATP - Regulation (EU) 2020/217 of 4 October 2019
15th ATP - Regulation (EU) 2020/1182 of 19 May 2020
The Chemicals (Health and Safety) and Genetically Modified Organisms (Contained Use)(Amendment etc.) (EU Exit) Regulations 2020 - UK: 2020 No. 1567 of 16th December 2020
The Waste and Environmental Permitting etc. (Legislative Functions and Amendment etc.) (EU Exit) Regulations 2020 - UK: 2020 No. 1540 of 16th December 2020
GB MCL List - version 1.1 of 09 June 2021
GB MCL List v2.0 - version 2.0 of 20th October 2023
GB MCL List v3.0 - version 3.0 of 11th January 2024
GB MCL List v4.0 - version 4.0 of 2nd March 2024
GB MCL List v5.0 - version 5.0 of 26th June 2024
GB MCL List v6.0 - version 6.0 of 15th February 2025

Appendix C: Sewer Asset Plan

Wessex Water Network Map



Reproduced from the Ordnance Survey map with the permission of the Controller of His Majesty's Stationery Office. © Crown copyright and database rights (2026) OS AC0000848304

WATER MAINS
- Distribution
- Washout
- Raw Water
- Abandoned
- Private

FITTINGS
- Hydrant
- Other

SEWERS — STRATEGIC / PUBLIC / PRIVATE / SECTION 104
- Foul
- Surface
- Combined
- Abandoned

Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow).
Some styles of line and symbol are shown on the key in sample/typical colours.

STRUCTURES
- Manhole - Foul
- Manhole - Surface
- Manhole - Combined
- Inlet
- Outfall
- Lamphole
- Bifurcation - Foul
- Bifurcation - Surface
- Bifurcation - Combined
- Combined Sewage Overflow
- Pumping Station - Surface
- Pumping Stn - Foul/Combined
- Gully
- Vent Column
- Rodding Eye
- Catchpit
- Flushing Chamber
- Soakaway
- Non Return Valve
- Air Valve
- Hatch Box
- X Washout

OTHER WESSEX PIPES
- Rising Mains
- Standby Rising Mains
- EDM — Effluent Disposal
- Overflow
- S — Syphon

NON-WESSEX / UNKNOWN
- Private Rising Mains
- CW — Culverted Watercourse
- H — Highway Drain
- ? — Use Unknown
- SU — Status Unknown

OTHER STRUCTURES
- Attenuation Tank
- Storage Tank
- Chamber
- Tunnel
- Interceptor

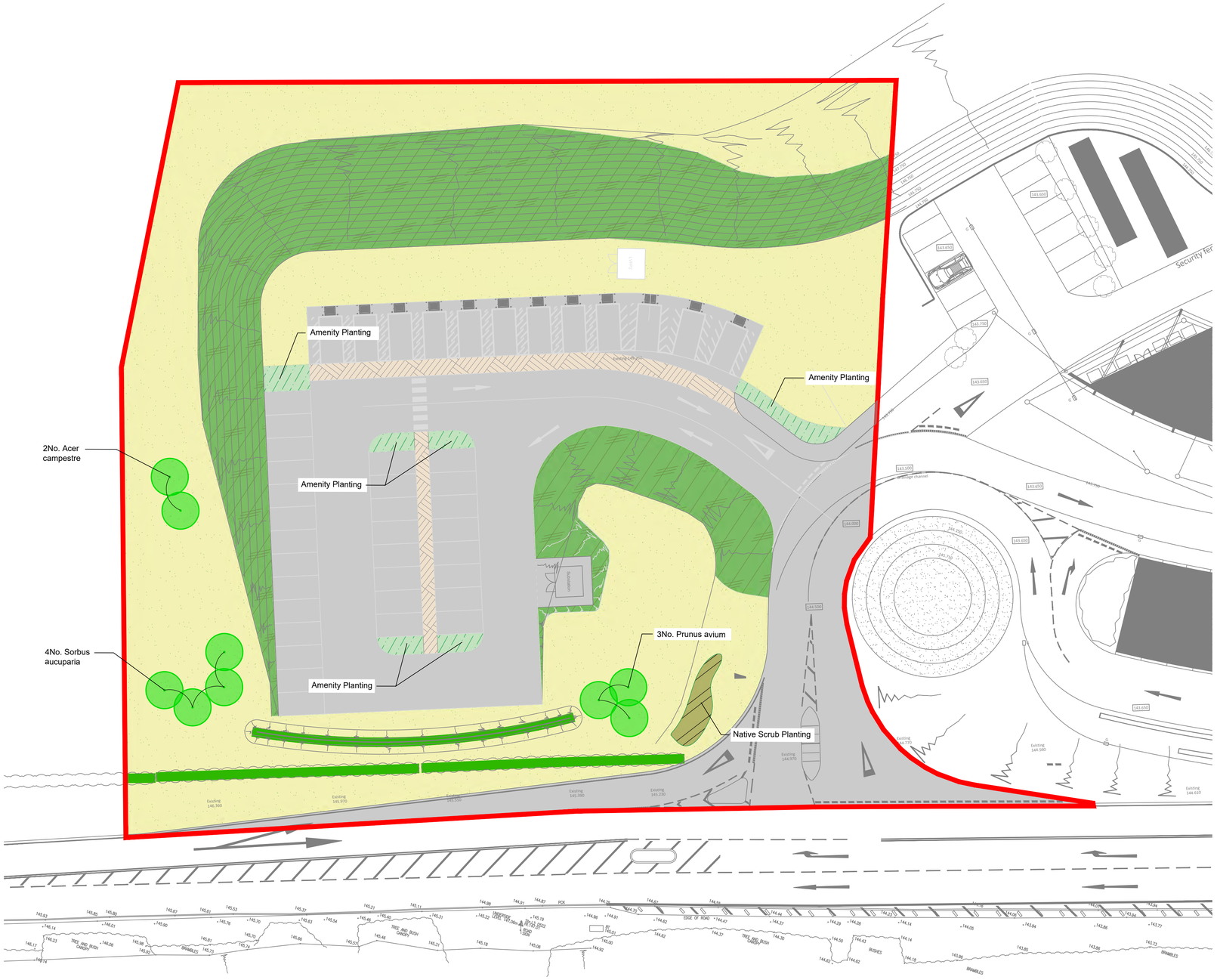
Wessex Water
YTL GROUP

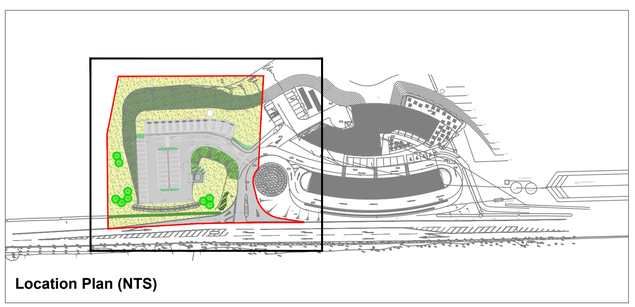
Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526333 or e-mail: developer.enquiries@wessexwater.co.uk to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.

Date: 13/07/2026
Centre: 390367E, 134438N
Scale: 1:625
(when printed at A4 size)

N W E S

Appendix D: Landscape & Site Plans





N

0m 5m 10m 15m 20m 25m

SCALE 1:250

Planting Schedule

UK Native Tree Planting

Species	Specification	Girth (cm)	Height (cm)	Clear Stem (cm)
Acer campestre	Selected Standard	10-12	300-350	min. 175
Prunus avium	Selected Standard	10-12	300-350	175-200
Sorbus aucuparia	Selected Standard	10-12	300-350	175-200

Native Scrub Planting

Species		Form	Times Transplanted	Height (cm)	Root condition	Spacing	% Mix
Acer campestre	Field maple	Transplant	1+2	80-100	Bare root	1m centres. To be planted in groups of 1-3 single species. Not to be planted on a regular grid.	10
Corylus avellana	Hazel	Transplant	1+2	80-100	Bare root		25
Euonymus europaeus	Spindle	Transplant	1+2	80-100	Bare root		15
Ligustrum vulgare	Wild Privet	Transplant	1+2	80-100	Bare root		15
Rosa canina	Dog Rose	Transplant	1+2	80-100	Bare root		15
Viburnum opulus	Guelder Rose	Transplant	1+2	80-100	Bare root		20

Amenity Planting

Species	Pot size	Plant size (cm)	Specification	Density (per m2)
Cornus var.	3L	40-60	Branched, 4 Breaks	3
Echniacea purpurea	2L	-	-	5
Hebe var.	2L	20-30	Bushy, 3 Breaks	5
Lavandula var.	3L	20-30	Bushy, 5 Breaks	5
Lonicera nitida	3L	30-40	Bushy, 6 Breaks	3
Prunus laurocerasus 'Otto Luyken'	3L	60-80	Bushy, 3 Breaks	3
Rudbeckia fulgida var. sullivantii 'Goldsturm'	2L	-	-	5
Sesleria autumnalis	2L	-	-	4

Mixed Native Hedge Planting

Species	Height (cm)	Specification	% Mix
Acer campestre	60 - 80	1+2 Transplant, Whip, BR	10
Carpinus betulus	60 - 80	1+2 Transplant, Whip, BR	10
Corylus avellana	60 - 80	1+2 Transplant, Whip, BR	15
Crataegus monogyna	60 - 80	1+2 Transplant, Whip, BR	30
Euonymous europaeus	60 - 80	1+2 Transplant, Whip, BR	10
Rosa canina	60 - 80	3L Container, Branched, 5 Breaks	10
Sambucus nigra	60 - 80	1+2 Transplant, Whip, BR	15

BR=bare root.

KEY

- Site Boundary
- Proposed Tree
- Proposed Mixed Native Hedge
- Proposed Earth Banking
- Amenity Planting
- Native Scrub Planting
- Grass Seeding - Species Rich Grassland.
  To be Emorsgate EG6 Meadow Grass Mixture for Chalk & Limestone Soils (or similar and approved)
- Flowering Wildflower Seeding to Banking.
  To be Emorsgate EM6 Meadow Mixture for Chalk and Limestone Soils (or similar and approved)
- Standard Tarmac Asphalt
  To Highway Engineer's Specification & build-up. Road markings to be thermoplastic white lining (or similar and approved)
- Non Skid Surface to Tarmac to denote path.
  To be Buff Colour Resin Bonded Gravel (or similar and approved)

REV	DATE	COMMENTS	AUTHOR / CHECKED
P1	19.06.26	ISSUED FOR INFORMATION	DE / AS

PROJECT TITLE
Chicklade Services Car Park Extension

DETAIL
Hard and Soft Landscape

DRAWING NUMBER
(PROJECT-ORIGINATOR-ZONE-LEVEL-TYPE-ROLE-NUMBER)
25098 - LHC - 00 - XX- DR - L - 9410

STATUS	STATUS DESCRIPTION	
S2	FOR INFORMATION	
REVISION	DATE	SCALE
P1	JUNE 2026	1:250 @A1

CONTRACTORS MUST CHECK ALL DIMENSIONS ON SITE - ONLY FIGURED DIMENSIONS ARE TO BE WORKED FROM . DISCREPANCIES MUST BE REPORTED TO THE ARCHITECT BEFORE PROCEEDING
© THIS DRAWING IS COPYRIGHT

LHC PROJECT NUMBER
25098

lhc design
EXETER 01392 444334
PLYMOUTH 01752 669368
lhc.net

Appendix E: Greenfield Runoff Calculations

Greenfield runoff rate estimation tool

www.uksuds.com | Greenfield runoff rate estimation tool (https://www.uksuds.com/)

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	14/07/2026
Calculated by	Craig Bobbett
Reference	12718
Model version	2.2.3

Location

Site name	Chicklade, Phase 2
Site location	Chicklade, Wiltshire



Site easting (British National Grid)	390389
Site northing (British National Grid)	134429

Site details

Total site area (ha)	0.61 ha

Greenfield runoff

Method

Method	FEH statistical (2025)

FEH statistical (2025)

	My value		Map value
SAAR9120 (mm)	1017 mm		
BFIHOST19scaled	0.928		
QMed-QBar conversion	1.136		1.136
QMed (l/s)	0.4 l/s		
QBar (FEH statistical 2025) (l/s)	0.5 l/s		

Growth curve factors

	My value		Map value
Hydrological region	7		7
1 year growth factor	0.85		
2 year growth factor	0.88		
10 year growth factor	1.62		
30 year growth factor	2.3		
100 year growth factor	3.19		
200 year growth factor	3.74		

Results

Method	FEH statistical (2025)
Flow rate 1 year (l/s)	0.4 l/s
Flow rate 2 year (l/s)	0.4 l/s
Flow rate 10 years (l/s)	0.8 l/s
Flow rate 30 years (l/s)	1.1 l/s
Flow rate 100 years (l/s)	1.6 l/s
Flow rate 200 years (l/s)	1.9 l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent 'zero' figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.3) developed by HR Wallingford and available at uksuds.com (https://www.uksuds.com/). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at uksuds.com/terms-conditions (https://www.uksuds.com/terms-conditions). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

Appendix F: SuDS Water Quality assessment

ANDREW WARING ASSOCIATES
(01794) 524447
mail@awaengineers.com
www.awaengineers.com

WATER QUALITY RISK MANAGEMENT: Design using Simple Index Approach (CIRIA SuDS Manual 2015)

Job Name:	Chicklade Services, Phase 2 Car Park	Job No.	12718	By:	CJB
Element:	Car Park Aisle run-off to Bio Retention	Date:	09/07/2026	Chk:	
Site Address	51.10926618566098, -2.1374827929736573	Link:	Click for Google Map		

SIMPLE INDEX APPROACH DESIGN

STEP 1: Determine the Pollution Hazard Index for the runoff area discharging to the proposed SuDS scheme
Allocate suitable pollution hazard indices for the proposed land use

If the land use varies across the 'runoff area', either:
- use the land use type with the highest Pollution Hazard Index
- apply the approach for each of the land use types to determine whether the proposed SuDS design is sufficient for all.
If it is not, consider collecting more hazardous runoff separately and providing additional treatment.
If the generic land use types suggested are not applicable, select 'Other' and enter a description of the land use of the runoff area and agreed user defined indices in the row below the drop down lists.

			Pollution Hazard Indices		
	Runoff Area Land Use Description	Pollution Hazard Level	Total Suspended Solids	Metals	Hydrocarbons
Select land use type from the drop down list (or 'Other' if none applicable):	Non-residential car parking with frequent change (eg hospitals, retail)	Medium	0.7	0.6	0.7
If the generic land use types in the drop down list above are not applicable, select 'Other' and enter a description of the land use of the runoff area and agreed user defined indices in this row:					
	Land use Pollution Hazard Index	Medium	0.7	0.6	0.7

STEP 2A: Determine the Pollution Mitigation Index for the proposed SuDS components

Select the proposed SuDS components that will be used to treat runoff - before it is discharged to a receiving surface waterbody or downstream infiltration component
components and move
to Step 2B
This step should be applied to evaluate the water quality protection provided by proposed SuDS components for discharges to receiving surface waters or downstream infiltration components (note: in England and Wales this will include components that allow any amount of infiltration, however small, even where infiltration is not specifically accounted for in the design).
If fewer than 3 components, select 'None' for the components that are not required
If the proposed component is bespoke and/or a proprietary treatment product and not generically described by the suggested components, then 'Proprietary treatment system' or 'User defined indices' should be selected and a description of the component and agreed user defined indices should be entered in the rows below the drop down lists

		Pollution Mitigation Indices		
	SuDS Component Description	Total Suspended Solids	Metals	Hydrocarbons
Select SuDS Component 1 (i.e. the upstream SuDS component) from the drop down list:	None	0	0	0
Select SuDS Component 2 (i.e. the second SuDS component in a series) from the drop down list:	None	0	0	0
Select SuDS Component 3 (i.e. the third SuDS component in a series) from the drop down list:	None	0	0	0
If the proposed SuDS components are bespoke/proprietary and/or the generic indices above are not considered appropriate, select 'Proprietary treatment system' or 'User defined indices' and enter component descriptions and agreed user defined indices in these rows:				
	Aggregated Surface Water Pollution Mitigation Index	0	0	0

Is the runoff now discharged to an infiltration component?
Yes ? Go to Step 2B
No ? Go to Step 2C

STEP 2B: Determine the Pollution Mitigation Index for the proposed Groundwater Protection

Type of groundwater protection that is either part of the SuDS component or lie between the component and groundwater

This step should be applied where a SuDS component is specifically designed to infiltrate runoff (note: in England and Wales this will include components that allow any amount of infiltration, however small, even where infiltration is not specifically accounted for in the design).

'Groundwater protection' describes the proposed depth of soil or other material through which runoff will flow between the runoff surface and the underlying groundwater.

Where the discharge is to surface waters and risks to groundwater need not be considered, select 'None'

If the proposed groundwater protection is bespoke and/or a proprietary product and not generically described by the suggested measures, then a description of the protection and agreed user defined indices should be entered in the row below the drop down list.

	Groundwater Pollution mitigation	Pollution Mitigation Indices		
		Total Suspended Solids	Metals	Hydrocarbons
Select type of groundwater protection from the drop down list:	Bioretention component underlain by 300 mm minimum depth of soils with good contamination attenuation potential	0.8	0.8	0.8
bespoke/proprietary and/or the generic indices above are not considered appropriate, select 'Proprietary product' or 'User defined indices' and enter a description of the protection and agreed user defined indices in this row:				
	Groundwater Protection Pollution Mitigation Index	0.8	0.8	0.8

STEP 2C: Determine the Combined Pollution Mitigation Indices for the Runoff Area

Combine the proposed SuDS Pollution Mitigation Indices with any Groundwater Protection Pollution Mitigation Indices

Nb. Comined Mitigation indices are limited to max 0.95

Combined Pollution Mitigation Indices for the Runoff Area	0.8	0.8	0.8

STEP 2D: Determine Sufficiency of Pollution Mitigation Indices for Selected SuDS Components

When the combined mitigation index exceeds the land use pollution hazard index, then the proposed components are considered sufficient in providing pollution risk mitigation.

In England and Wales, where the discharge is to protected surface waters or groundwater, an additional treatment component (ie over and above that required for standard discharges), or other equivalent protection, is required that provides environmental protection in the event of an unexpected pollution event or poor system performance. Protected surface waters are those designated for drinking water abstraction.

In England and Wales, protected groundwater resources are defined as Source Protection Zone 1.

	Discharges to Protected Waters or SPZ 1	Sufficiency of Pollution Mitigation Indices		
		Total Suspended Solids	Metals	Hydrocarbons
Is the surface water designated for drinking water abstraction?	No			
Is the groundwater discharge in Source Protection Zone 1?	No	Sufficient	Sufficient	Sufficient

ANDREW WARING ASSOCIATES
(01794) 524447
mail@awaengineers.com
www.awaengineers.com

Job Name:	Chicklade Services, Phase 2 Car Park	Job No.	12718	By:	CJB
Element:	Car parking and Aisle run-off to Permeable Paving	Date:	09/07/2026	Chk:	
Site Address	51.10926618566098, -2.1374827929736573	Link:	Click for Google Map		

SIMPLE INDEX APPROACH DESIGN

STEP 1: Determine the Pollution Hazard Index for the runoff area discharging to the proposed SuDS scheme
Allocate suitable pollution hazard indices for the proposed land use

If the land use varies across the 'runoff area', either:
- use the land use type with the highest Pollution Hazard Index
- apply the approach for each of the land use types to determine whether the proposed SuDS design is sufficient for all.
If it is not, consider collecting more hazardous runoff separately and providing additional treatment.
If the generic land use types suggested are not applicable, select 'Other' and enter a description of the land use of the runoff area and agreed user defined indices in the row below the drop down lists.

			Pollution Hazard Indices		
	Runoff Area Land Use Description	Pollution Hazard Level	Total Suspended Solids	Metals	Hydrocarbons
Select land use type from the drop down list (or 'Other' if none applicable):	Non-residential car parking with frequent change (eg hospitals, retail)	Medium	0.7	0.6	0.7
If the generic land use types in the drop down list above are not applicable, select 'Other' and enter a description of the land use of the runoff area and agreed user defined indices in this row:					
	Land use Pollution Hazard Index	Medium	0.7	0.6	0.7

STEP 2A: Determine the Pollution Mitigation Index for the proposed SuDS components

Select the proposed SuDS components that will be used to treat runoff - before it is discharged to a receiving surface waterbody or downstream infiltration component
components and move
to Step 2B
This step should be applied to evaluate the water quality protection provided by proposed SuDS components for discharges to receiving surface waters or downstream infiltration components (note: in England and Wales this will include components that allow any amount of infiltration, however small, even where infiltration is not specifically accounted for in the design).
If fewer than 3 components, select 'None' for the components that are not required
If the proposed component is bespoke and/or a proprietary treatment product and not generically described by the suggested components, then 'Proprietary treatment system' or 'User defined indices' should be selected and a description of the component and agreed user defined indices should be entered in the rows below the drop down lists

		Pollution Mitigation Indices		
	SuDS Component Description	Total Suspended Solids	Metals	Hydrocarbons
Select SuDS Component 1 (i.e. the upstream SuDS component) from the drop down list:	None	0	0	0
Select SuDS Component 2 (i.e. the second SuDS component in a series) from the drop down list:	None	0	0	0
Select SuDS Component 3 (i.e. the third SuDS component in a series) from the drop down list:	None	0	0	0
If the proposed SuDS components are bespoke/proprietary and/or the generic indices above are not considered appropriate, select 'Proprietary treatment system' or 'User defined indices' and enter component descriptions and agreed user defined indices in these rows:				
	Aggregated Surface Water Pollution Mitigation Index	0	0	0

Is the runoff now discharged to an infiltration component?
Yes ? Go to Step 2B
No ? Go to Step 2C

STEP 2B: Determine the Pollution Mitigation Index for the proposed Groundwater Protection

Type of groundwater protection that is either part of the SuDS component or lie between the component and groundwater

This step should be applied where a SuDS component is specifically designed to infiltrate runoff (note: in England and Wales this will include components that allow any amount of infiltration, however small, even where infiltration is not specifically accounted for in the design).

'Groundwater protection' describes the proposed depth of soil or other material through which runoff will flow between the runoff surface and the underlying groundwater.

Where the discharge is to surface waters and risks to groundwater need not be considered, select 'None'

If the proposed groundwater protection is bespoke and/or a proprietary product and not generically described by the suggested measures, then a description of the protection and agreed user defined indices should be entered in the row below the drop down list.

	Groundwater Pollution mitigation	Pollution Mitigation Indices		
		Total Suspended Solids	Metals	Hydrocarbons
Select type of groundwater protection from the drop down list:	Pervious pavement underlain by 300 mm minimum depth of soils with good contamination attenuation potential	0.7	0.6	0.7
bespoke/proprietary and/or the generic indices above are not considered appropriate, select 'Proprietary product' or 'User defined indices' and enter a description of the protection and agreed user defined indices in this row:				
	Groundwater Protection Pollution Mitigation Index	0.7	0.6	0.7

STEP 2C: Determine the Combined Pollution Mitigation Indices for the Runoff Area

Combine the proposed SuDS Pollution Mitigation Indices with any Groundwater Protection Pollution Mitigation Indices

Nb. Comined Mitigation indices are limited to max 0.95

Combined Pollution Mitigation Indices for the Runoff Area	0.7	0.6	0.7

STEP 2D: Determine Sufficiency of Pollution Mitigation Indices for Selected SuDS Components

When the combined mitigation index exceeds the land use pollution hazard index, then the proposed components are considered sufficient in providing pollution risk mitigation.

In England and Wales, where the discharge is to protected surface waters or groundwater, an additional treatment component (ie over and above that required for standard discharges), or other equivalent protection, is required that provides environmental protection in the event of an unexpected pollution event or poor system performance. Protected surface waters are those designated for drinking water abstraction.

In England and Wales, protected groundwater resources are defined as Source Protection Zone 1.

	Discharges to Protected Waters or SPZ 1	Sufficiency of Pollution Mitigation Indices		
		Total Suspended Solids	Metals	Hydrocarbons
Is the surface water designated for drinking water abstraction?	No			
Is the groundwater discharge in Source Protection Zone 1?	No			
		Sufficient	Sufficient	Sufficient

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(01794) 524447
mail@awaengineers.com
www.awaengineers.com

Job Name:	Chicklade Services, Phase 2 Car Park	Job No.	12718	By:	CJB
Element:	Car Park Aisle Run-off to SPEL ESR	Date:	09/07/2026	Chk:	
Site Address	51.10926618566098, -2.1374827929736573	Link:	Click for Google Map		

SIMPLE INDEX APPROACH DESIGN

STEP 1: Determine the Pollution Hazard Index for the runoff area discharging to the proposed SuDS scheme
Allocate suitable pollution hazard indices for the proposed land use

If the land use varies across the 'runoff area', either:
- use the land use type with the highest Pollution Hazard Index
- apply the approach for each of the land use types to determine whether the proposed SuDS design is sufficient for all.
If it is not, consider collecting more hazardous runoff separately and providing additional treatment.
If the generic land use types suggested are not applicable, select 'Other' and enter a description of the land use of the runoff area and agreed user defined indices in the row below the drop down lists.

			Pollution Hazard Indices		
	Runoff Area Land Use Description	Pollution Hazard Level	Total Suspended Solids	Metals	Hydrocarbons
Select land use type from the drop down list (or 'Other' if none applicable):	Non-residential car parking with frequent change (eg hospitals, retail)	Medium	0.7	0.6	0.7
If the generic land use types in the drop down list above are not applicable, select 'Other' and enter a description of the land use of the runoff area and agreed user defined indices in this row:					
	Land use Pollution Hazard Index	Medium	0.7	0.6	0.7

STEP 2A: Determine the Pollution Mitigation Index for the proposed SuDS components

Select the proposed SuDS components that will be used to treat runoff - before it is discharged to a receiving surface waterbody or downstream infiltration component
components and move
to Step 2B
This step should be applied to evaluate the water quality protection provided by proposed SuDS components for discharges to receiving surface waters or downstream infiltration components (note: in England and Wales this will include components that allow any amount of infiltration, however small, even where infiltration is not specifically accounted for in the design).
If fewer than 3 components, select 'None' for the components that are not required
If the proposed component is bespoke and/or a proprietary treatment product and not generically described by the suggested components, then 'Proprietary treatment system' or 'User defined indices' should be selected and a description of the component and agreed user defined indices should be entered in the rows below the drop down lists

			Pollution Mitigation Indices		
	SuDS Component Description		Total Suspended Solids	Metals	Hydrocarbons
Select SuDS Component 1 (i.e. the upstream SuDS component) from the drop down list:	User defined indices		0	0	0
Select SuDS Component 2 (i.e. the second SuDS component in a series) from the drop down list:	None		0	0	0
Select SuDS Component 3 (i.e. the third SuDS component in a series) from the drop down list:	None		0	0	0
If the proposed SuDS components are bespoke/proprietary and/or the generic indices above are not considered appropriate, select 'Proprietary treatment system' or 'User defined indices' and enter component descriptions and agreed user defined indices in these rows:	SPEL 210C1/ESR	SuDS Component 1	0.8	0.6	0.9
	Aggregated Surface Water Pollution Mitigation Index		0.8	0.6	0.9

Is the runoff now discharged to an infiltration component?

Yes ? Go to Step 2B
No ? Go to Step 2C

STEP 2B: Determine the Pollution Mitigation Index for the proposed Groundwater Protection

Type of groundwater protection that is either part of the SuDS component or lie between the component and groundwater

This step should be applied where a SuDS component is specifically designed to infiltrate runoff (note: in England and Wales this will include components that allow any amount of infiltration, however small, even where infiltration is not specifically accounted for in the design).

'Groundwater protection' describes the proposed depth of soil or other material through which runoff will flow between the runoff surface and the underlying groundwater.

Where the discharge is to surface waters and risks to groundwater need not be considered, select 'None'

If the proposed groundwater protection is bespoke and/or a proprietary product and not generically described by the suggested measures, then a description of the protection and agreed user defined indices should be entered in the row below the drop down list.

	Groundwater Pollution mitigation	Pollution Mitigation Indices		
		Total Suspended Solids	Metals	Hydrocarbons
Select type of groundwater protection from the drop down list:	None	0	0	0
bespoke/proprietary and/or the generic indices above are not considered appropriate, select 'Proprietary product' or 'User defined indices' and enter a description of the protection and agreed user defined indices in this row:				
	Groundwater Protection Pollution Mitigation Index	0	0	0

STEP 2C: Determine the Combined Pollution Mitigation Indices for the Runoff Area

Combine the proposed SuDS Pollution Mitigation Indices with any Groundwater Protection Pollution Mitigation Indices

Nb. Comined Mitigation indices are limited to max 0.95

Combined Pollution Mitigation Indices for the Runoff Area	0.8	0.6	0.9

STEP 2D: Determine Sufficiency of Pollution Mitigation Indices for Selected SuDS Components

When the combined mitigation index exceeds the land use pollution hazard index, then the proposed components are considered sufficient in providing pollution risk mitigation.

In England and Wales, where the discharge is to protected surface waters or groundwater, an additional treatment component (ie over and above that required for standard discharges), or other equivalent protection, is required that provides environmental protection in the event of an unexpected pollution event or poor system performance. Protected surface waters are those designated for drinking water abstraction.

In England and Wales, protected groundwater resources are defined as Source Protection Zone 1.

	Discharges to Protected Waters or SPZ 1	Sufficiency of Pollution Mitigation Indices		
		Total Suspended Solids	Metals	Hydrocarbons
Is the surface water designated for drinking water abstraction?	No			
Is the groundwater discharge in Source Protection Zone 1?	No	Sufficient	Sufficient	Sufficient

Appendix G: SuDS Maintenance

ANDREW WARING ASSOCIATES

(01794) 524447
mail@awaengineers.com
www.awaengineers.com

DRAINAGE: Surface Water Maintenance Summary

Job Name:	**Chicklade Services - Phase 2 Car Park**
Job No.	**12718**
Date:	09/07/2026

	Activity	Frequency
	Regular Inspection	
0	Bio Infiltration (rain garden)	
0.1	Inspect infiltration surfaces for silting and ponding, record de-watering time of the facilit and assess standing water levels in underdrain (if appropriate) to determine if maintenance is necessary	Quarterly
0.2	Check operation of underdrains by inspection of flows after rain	Annually
0.3	Assess plants for disease infection, poor growth, invaisive species etc and replace as necessary	Quarterly
0.4	Inspect inlets and outlets for blockages	Quarterly
	Regular Maintenance	
1	Bio Infiltration (rain garden)	
1.1	Remove litter and surface debris and weeds	Min quarterly (increase frequency if desired for aestheics)
1.2	Replace and plants, to maintain planting density	As required
1.3	Remove sediment, litter and debris build-up from around inlets or forebays	Quarterly to half yearly
2	Porous Pavements	
2.1	Brush regularly and remove litter and sweepings from all hard surfaces - pay attention to areas where adjacent impermeable surfaces drain as silt accumulation is more likely in these locations	3 Monthly, then reduced based on site specific observations
2.2	Maintain adjacent planted areas to ensure washout of soil onto permeable surface does not occur. Clean and remediate any spillage to prevent clogging	3 Monthly, then as required under general landscape maintenance
2.3	Snow removal, standard road salt used as de-icing medium; no abrasives such as sands, grits or cinders shall be applied	As required in winter months
3	Chambers and Pipes	
3.1	Remove cover and inspect chambers, ensuring water is flowing freely and exit routes are unobstructed. Remove and litter and debris.	3 monthly and following leaf fall
4	Linear Channel (ACO)	
4.1	Pick up all litter and leaf fall inhibiting flow into channels. Inspect after leaf fall in Autumn	3 monthly
4.2	Check grating is free from silt build up and flow into channels is not inhibited. Check grating is aligned and not displaced	6 monthly, or as required
4.3	Inspect silt trap for silt levels and blockages (pour water down channel to check flow if blockage is suspected)	6 monthly

5	SPEL ESR By-Pass Treatment System	
5.1	Inspect during the construction phase to check for evidence of pollution and contamination. Undertake further inspection if a pollution event is known to have occurred on site	Monthly, or as required. To be maintained following handover
5.2	Test alarm system in line with manufacturer's guidance	Monthly
5.3	Monitor alarm system and act when activated	As required
5.4	Inspect coalescer guide rails for alignment and damage	Monthly, when testing alarm
6	Soakaway	
6.1	Inspect for sediment and debris in pre-treatment components and inside of upstream manholes and inspection chambers	Annually
6.2	Cleaning of gutters and and any filters on downpipes	Annually or as required based on inspections
	Occasional tasks	
7	Bio Infiltration (rain garden)	
7.1	Infill any holes or scour in the filter medium, improve erosion protection if required	As required
7.2	Repair minor accumulations of silt by raking away surface mulch, scarifying the surface medium and replacing mulch	As required
8	Porous Pavements	
8.1	Brush and vacuum surface once a year to prevent silt blockage and enhance design life. Replace any displaced jointing material by sweeping in 2/6mm clean stone	Annually following leaf fall
8.2	Inspect diffuser units with CCTV to check for siltation; review upstream silt management if required	Every 5 years, or as required
9	Chambers and Pipes	
9.1	Inspect channel bases, sumps, gullies and inspection chambers for silt accumulation. Vaccum suction removal of silt in road gullies. Jet wash pipes as required to maintain flow.	Annually
10	Linear Channel (ACO)	
10.1	Clean grating of silt using manual mechanical methods, or apply a pressure washer to clear the grating	As required, based on regular maintenance inspections
10.2	Clean channel of silt and debris using manual methods (trowel) or jet wash using 25mm jetting hose and suction hose at silt trap	Min. every 5 years, or as required, based on regular maintenance inspections
11	SPEL ESR By-Pass Treatment System	
11.1	Extract coalescer and carry out cleaning in accordance with manufacturer's recommendations. Ensure cleaning is carried out upstream of the ESR unit to ensure oil is retained within the tank.	6 monthly, frequency to be reviewed following inspection
11.2	Remove oil and sediment from ground level using vacuum tanker - no man entry to unit	6 monthly
11.3	Review emptying frequency based on silt and pollution levels - significant increases shall trigger a review of site wide drainage components for any failure	Annually
12	Soakaway	
12.1	Remove sediment and debris in pre-treatment components	As required, based on inspections

	Remedial Work	
13	Bio Infiltration (rain garden)	
13.1	Remove and replace filter medium and vegetation	As required (circa > 20 years)
14	Porous Pavements - Asphalt	
14.1	Remedial works to correct rutting, depressions which are hazardous. Small areas of patch repair can be carried out with porous mix or standard asphalt	As required
14.2	Rehabilitation of surface and upper substructure layers - jet washing and suction cleaning to improve pavement efficiency. Monitor effectiveness and advise client of any areas which do not infiltrate immediately	10 - 15 years depending on performance and efficiency of regular maintenance regime
15	Chambers and Pipes	
15.1	Repair physical damage to below ground pipes, manholes and inspection chambers	As required
16	Linear Channel (ACO)	
16.1	Repair physical damage to channel and gratings	As required
17	SPEL ESR By-Pass Treatment System	
17.1	Repair physical damage to manhole & cover	As required
17.2	Replace coalescer units	As required
18	Soakaway	
18.1	Reconstruct soakaway and/or replace if performance deteriorates or failure occurs	As required
18.2	Replacement of clogged geotextile (requires reconstruction of soakaway)	As required

Appendix H: Drainage Strategy Drawings, Details, Supporting Calculations

Car Park Extension: Drainage Calculations



Chicklade Services, Wiltshire

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ANDREW WARING ASSOCIATES

Document Information	
Project no.	12718
Project title	Chicklade Services, Wiltshire
Client	Instavolt Ltd
Document title	Car Park Extension: Drainage Calculations
Document ref.	12718-DC

Issue Status				
Rev	Date	By	Approved	Description
-	22/07/2026	CB	SW	First issue

Offices

Romsey The Old Brewery House, Portersbridge St, Romsey, Hampshire, SO51 8DJ

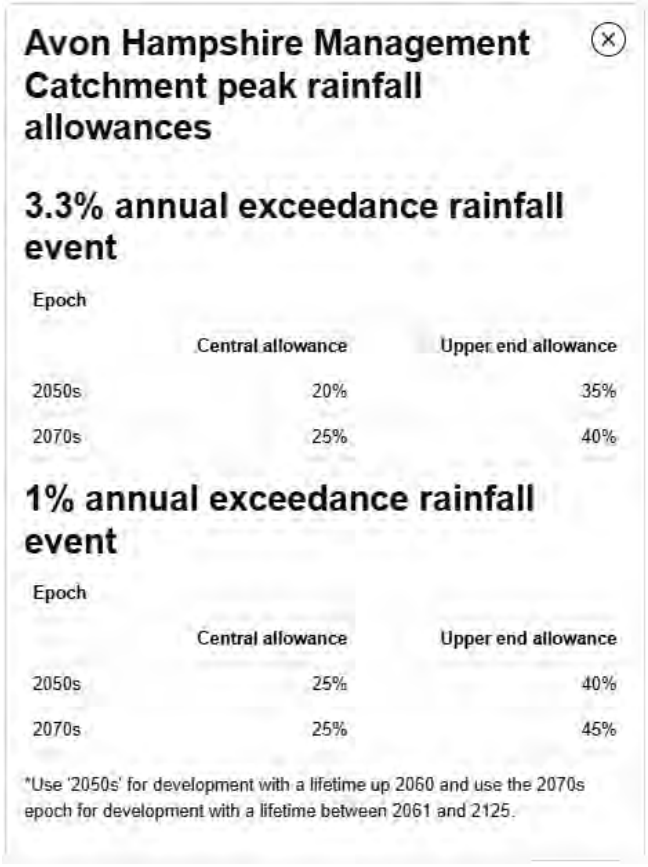
Bath 2nd Floor, Unit 7, Riverside Court, Bath BA2 3DZ

Calculation Design Notes

Rainfall data – FH2022
The rainfall data location is: ST 90479 34457
FEH22 data has been used for the following return periods and durations:

Rainfall Data					
FEH Parameters		Return Period (years)	Increase rainfall (%)	Storm Durations (mins)	Run time (mins)
bfihost19scaled	0.928	2	0	15	30
propwet	0.35	10		30	60
saar9120	1017	30	40	60	120
X	390479	100		120	240
Y	134457	100	45	240	360
				360	480
				480	720
				960	960
				1440	1200

Climate Change allowances
Climate change has been set for the 2070s upper end allowance for the Avon Hampshire catchment, based on the DEFRA Hydrology Data Explorer.



Avon Hampshire Management Catchment peak rainfall allowances

3.3% annual exceedance rainfall event

Epoch

	Central allowance	Upper end allowance
2050s	20%	35%
2070s	25%	40%

1% annual exceedance rainfall event

Epoch

	Central allowance	Upper end allowance
2050s	25%	40%
2070s	25%	45%

*Use '2050s' for development with a lifetime up 2060 and use the 2070s epoch for development with a lifetime between 2061 and 2125.

Runoff Co-efficient

The following co-efficients have been applied globally to the catchments.

Rainfall period	Volumetric Runoff Coefficient
Summer	1.0
Winter	1.0

Urban Creep
No allowance is made for urban creep, as any future extensions would be subject to a new planning application, and a new surface water system would be sized appropriately for said extension.

Infiltration Rates
Soakage testing was undertaken during site investigation works and results indicate favourable rates of infiltration at 1.0m BGL. Owing to the reduced ground levels proposed in the application, it is anticipated that infiltration results will, as a minimum, be equivalent, but will in all likelihood exceed the tested values.

The infiltration rate for the design of all stormwater controls is:
3.88 x 10-5 m/s (0.1396mm/hr)

Strategy Summary
Rate and volume control for the development will be achieved through storage and infiltration, with bio-infiltration (rain gardens), soakaways and permeable paving sized for the 1:100yr (1% AEP) + 45% climate change event.

The landscape design provides small planting areas in the middle of the car park, between the circulation space and the bays. These have been incorporated as rain gardens and accept lateral flows from portions of the impermeable aisles directly onto the planted surface soil medium, over a granular storage layer below. Flows up to the 10yr event are contained within the rain garden; flows beyond this are designed to crest the kerb and overflow to the channels and gullies downstream.

Impermeable asphalt concrete forms the entrance and circulation routes of the car park, with permeable asphalt used for all parking bays. The central bays accept lateral flows from the western aisle, but in general the levels required to tie into the wider site make it inefficient to drain the aisles to the rest of the bays. The permeable paving will offer approximately 71.3m^3 of storage.

Surface level channels and gullies convey the remaining flows from the impermeable areas to a below ground network of pipes before passing through an interceptor and discharging to a geocellular soakaway matrix. The soakaway offers approximately 50.3m^3 of storage.

Greenfield Rates
Greenfield runoff was assessed using the FEH Statistical Method using the HR Wallingford Online Tool. As infiltration is viable the rates have no consequence on the design, as no flows are released off-site. A copy of the results are included in the calculations.

Factors of Safety (FOS)
Infiltration designs are based on CIRIA methodology (Bettess 1996) and the applied FOS are derived from Table 25.1 of the SuDS Manual:

Size of area to be drained	Consequence of failure		
	No damage or inconvenience	Minor damage to external areas or inconvenience (eg surface water on car parking)	Damage to buildings or structures, or major inconvenience (eg flooding of roads)
<100m²	1.5	2	10
100-1000m²	1.5	3	10
>1000m²	1.5	5	10

Permeable Surfaces
No permeable surfaces drain an area greater than 200m² and the risk of damage or inconvenience is minimal. However, owing to the absence of an exact infiltration test at this location, a conservative but acceptable FOS of **5.0** has been used.

Rain garden
As above, the areas served and consequence of failure are minimal but a FOS of **5.0** has been used.

Soakaways
The total area served by Soakaway SA1 is 805m², below the 1000m² in Table 25.1. As the failure consequence is low, and to account for the inexact infiltration rate, a FOS of **5.0** is applied.

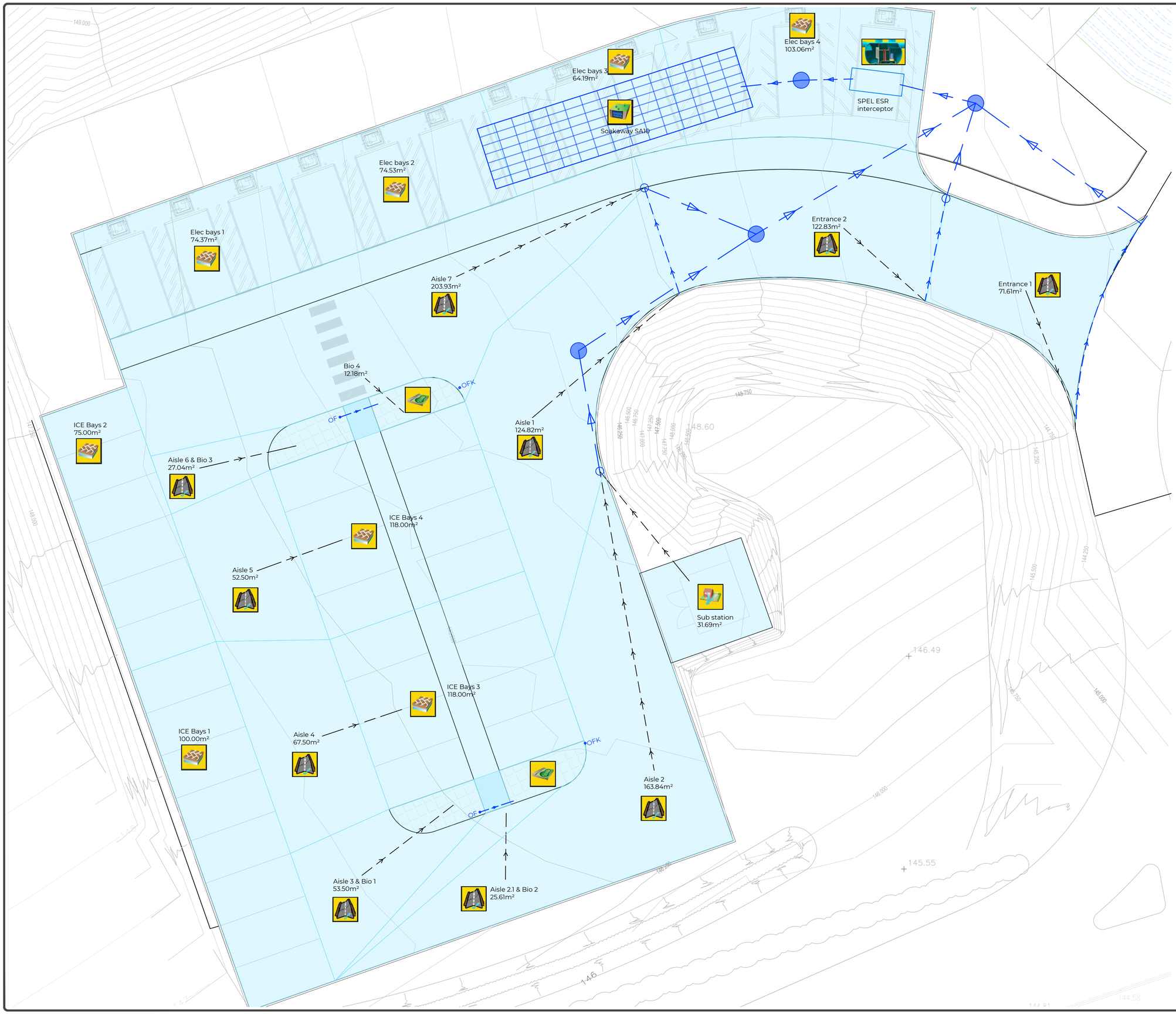
Calculation Summary

Full calculations have been carried out using AutoDesk InfoDrainage software, V2026.4.
The following demonstrate:

- No flooding of the permeable paving or soakaways occurs during the analysed return periods.
- Rain gardens are designed to contain the 2 year events, with events beyond this routed by design to the gullies and channels downstream. The kerb overflows have been modelled as weirs, with the connections to the gullies as lagged flows to account for the flow paths across the surface.
- ACO channel drains have been modelled as small junctions with custom profiled connections in the shape of the channels (data supplied by ACO). This enables a more accurate simulation of the channel's performance receiving a lateral flow.
- No flooding of the pipes occurs during the 2, 10, 30 & 100 year return periods, with surcharge only in the 100 year +45%.
- No flooding of the junctions is reported during the analysed periods.
- Catchments are shown on drawing 12718-Z2
- Overland flows are routed away from buildings and will be dealt with by the wider surface water network on the site. See plan 12718-116

Compiled calculations

- Catchment Plan Z2
- Stormwater Control Parameters 1
- Connection Parameters 19
- Rainfall input 22
- Junction Results 24
- Control Results 204
- Connection Results 90 pages
- Drainage Plan 12718-D-116
- Drainage Manhole Tables 12718-D-117
- Details Sheet 1 - 12718-D-129
- Details Sheet 2 – 12718-D-130



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- Inflow: Impermeable road
- Inflow: Roof or hard surfacing
- Control item & inflow: Permeable paving
- Control item & inflow: Bio-infiltration
- Control item: Crate soakaway
- Treatment item: Interceptor

Name	Connected to	Area (m²)	Fixed coefficient	PIMP(%)	Urban Creep (%)
Aisle 1	M150D20	124.82	1.0	100	0.00
Aisle 2	RG1	163.84	1.0	100	0.00
Aisle 2.1 & Bio 2	Bio 2	25.61	1.0	100	0.00
Aisle 3 & Bio 1	Bio 1	53.50	1.0	100	0.00
Aisle 4	ICE Bays 3	67.50	1.0	100	0.00
Aisle 5	ICE Bays 4	52.50	1.0	100	0.00
Aisle 6 & Bio 3	Bio 3	27.04	1.0	100	0.00
Aisle 7	RG2 ACO	203.93	1.0	100	0.00
Bio 4	Bio 4	12.18	1.0	100	0.00
Elec bays 1	Elec bays 1	74.37	1.0	100	0.00
Elec bays 2	Elec bays 2	74.53	1.0	100	0.00
Elec bays 3	Elec bays 3	64.19	1.0	100	0.00
Elec bays 4	Elec bays 4	103.06	1.0	100	0.00
Entrance 1	M100D30.1	71.61	1.0	100	0.00
Entrance 2	M100D30	122.83	1.0	100	0.00
ICE Bays 1	ICE Bays 1	100.00	1.0	100	0.00
ICE Bays 2	ICE Bays 2	75.00	1.0	100	0.00
ICE Bays 3	ICE Bays 3	118.00	1.0	100	0.00
ICE Bays 4	ICE Bays 4	118.00	1.0	100	0.00
Sub station	RG1	31.69	1.0	100	0.00
TOTAL		**1684.19**			

THIS DRAWING TO BE VIEWED AND PRINTED IN COLOUR

M 1:100
0 1 2 3 4 5 6 7 8 9 10 [m]

REV	DATE	REVISIONS	REV BY	CHK BY
P1	22/7/26	Calculation pack Planning Issue	CJB	SW

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awa ANDREW WARING ASSOCIATES
Consulting Engineers
STRUCTURAL - CIVIL - MARINE
Romsey, Hampshire - Bath, Somerset
Romsey: 01794 526447
Bath: 01225 259088
mail@awaengineers.com
www.awaengineers.com

ARCHITECT: Studio Four Architects

CLIENT: InstaVolt

JOB TITLE: CHICKLADE SERVICES

DRG TITLE: CAR PARK EXTENSION: CATCHMENT PLAN

STATUS: INFORMATIVE

DRG NO	REV
12718-Z-2	P1

SCALE	DATE	DRAWN BY	CHECKED BY
1:100 @ A1	JULY 2026	CJB	SW

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



Bio 1

Type : Bioretention

Ponding Area

Exceedance Level (m)	146.290
Depth (m)	0.050
Base Level (m)	146.240
Top Area (m²)	9.15
Side Slope (1:X)	0.00
Base Area (m²)	9.15
Freeboard (mm)	150
Porosity (%)	100
Length (m)	5.000
Long. Slope (1:X)	30.00
Filtration Rate (m/hr)	1.0
Friction Scheme	Manning's n
n	0.025
Total Volume (m³)	0.915

Filter Area

Base Level (m)	145.440

Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☑	Soil	400	20	0.35	Soil Type
	Storage	400	30	50.0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Advanced

Safety Factor	5.0

Ponding Area

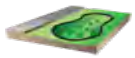
Base Perimeter (m)	13.660
Top Perimeter (m)	13.660

Filter Area

Base Infiltration Rate (m/hr)	0.135

Pollutant Removals

Name	Aspect	Background Concentration (mg/L)	Method	Percentage Removal (%)	T (mins)
TSS	Ponding Pollution	0.0	Percentage Removal	40	
Heavy Metals	Ponding Pollution	0.0	Percentage Removal	40	
Hydrocarbons	Ponding Pollution	0.0	Percentage Removal	40	
BOD	Ponding Pollution	0.0	Percentage Removal	0	
Nitrogen	Ponding Pollution	0.0	Percentage Removal	0	
TSS	Storage Filter Pollution	0.0	Percentage Removal	40	
Heavy Metals	Storage Filter Pollution	0.0	Percentage Removal	40	
Hydrocarbons	Storage Filter Pollution	0.0	Percentage Removal	40	
BOD	Storage Filter Pollution	0.0	Percentage Removal	0	
Nitrogen	Storage Filter Pollution	0.0	Percentage Removal	0	



Bio 4

Type : Bioretention

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Ponding Area

Exceedance Level (m)	145.790
Depth (m)	0.050
Base Level (m)	145.740
Top Area (m²)	9.15
Side Slope (1:X)	0.00
Base Area (m²)	9.15
Freeboard (mm)	150
Porosity (%)	100
Length (m)	5.000
Long. Slope (1:X)	20.00
Filtration Rate (m/hr)	1.0
Friction Scheme	Manning's n
n	0.025
Total Volume (m³)	0.915

Filter Area

Base Level (m)	144.940

Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☑	Soil	400	20	0.35	Soil Type
	Storage	400	30	50.0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Advanced

Safety Factor	5.0

Ponding Area

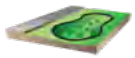
Base Perimeter (m)	13.660
Top Perimeter (m)	13.660

Filter Area

Base Infiltration Rate (m/hr)	0.135

Pollutant Removals

Name	Aspect	Background Concentration (mg/L)	Method	Percentage Removal (%)	T (mins)
TSS	Ponding Pollution	0.0	Percentage Removal	40	
Heavy Metals	Ponding Pollution	0.0	Percentage Removal	40	
Hydrocarbons	Ponding Pollution	0.0	Percentage Removal	40	
BOD	Ponding Pollution	0.0	Percentage Removal	0	
Nitrogen	Ponding Pollution	0.0	Percentage Removal	0	
TSS	Storage Filter Pollution	0.0	Percentage Removal	0	
Heavy Metals	Storage Filter Pollution	0.0	Percentage Removal	0	
Hydrocarbons	Storage Filter Pollution	0.0	Percentage Removal	0	
BOD	Storage Filter Pollution	0.0	Percentage Removal	0	
Nitrogen	Storage Filter Pollution	0.0	Percentage Removal	0	



Bio 2

Type : Bioretention

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Ponding Area

Exceedance Level (m)	146.080
Depth (m)	0.050
Base Level (m)	146.030
Top Area (m²)	9.15
Side Slope (1:X)	0.00
Base Area (m²)	9.15
Freeboard (mm)	150
Porosity (%)	100
Length (m)	5.000
Long. Slope (1:X)	40.00
Filtration Rate (m/hr)	1.0
Friction Scheme	Manning's n
n	0.025
Total Volume (m³)	0.915

Filter Area

Base Level (m)	145.230

Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☑	Soil	400	20	0.35	Soil Type
	Storage	400	30	50.0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Advanced

Safety Factor	5.0

Ponding Area

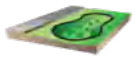
Base Perimeter (m)	13.660
Top Perimeter (m)	13.660

Filter Area

Base Infiltration Rate (m/hr)	0.135

Pollutant Removals

Name	Aspect	Background Concentration (mg/L)	Method	Percentage Removal (%)	T (mins)
TSS	Ponding Pollution	0.0	Percentage Removal	40	
Heavy Metals	Ponding Pollution	0.0	Percentage Removal	40	
Hydrocarbons	Ponding Pollution	0.0	Percentage Removal	40	
BOD	Ponding Pollution	0.0	Percentage Removal	0	
Nitrogen	Ponding Pollution	0.0	Percentage Removal	0	
TSS	Storage Filter Pollution	0.0	Percentage Removal	0	
Heavy Metals	Storage Filter Pollution	0.0	Percentage Removal	0	
Hydrocarbons	Storage Filter Pollution	0.0	Percentage Removal	0	
BOD	Storage Filter Pollution	0.0	Percentage Removal	0	
Nitrogen	Storage Filter Pollution	0.0	Percentage Removal	0	



Bio 3

Type : Bioretention

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Ponding Area

Exceedance Level (m)	146.063
Depth (m)	0.050
Base Level (m)	146.013
Top Area (m²)	9.14
Side Slope (1:X)	0.00
Base Area (m²)	9.14
Freeboard (mm)	150
Porosity (%)	100
Length (m)	5.000
Long. Slope (1:X)	20.00
Filtration Rate (m/hr)	1.0
Friction Scheme	Manning's n
n	0.025
Total Volume (m³)	0.914

Filter Area

Base Level (m)	145.213

Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☑	Soil	400	20	0.093	Soil Type
	Storage	400	30	50.0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Advanced

Safety Factor	5.0

Ponding Area

Base Perimeter (m)	13.656
Top Perimeter (m)	13.656

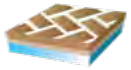
Filter Area

Base Infiltration Rate (m/hr)	0.135

Pollutant Removals

Name	Aspect	Background Concentration (mg/L)	Method	Percentage Removal (%)	T (mins)
TSS	Ponding Pollution	0.0	Percentage Removal	40	
Heavy Metals	Ponding Pollution	0.0	Percentage Removal	40	
Hydrocarbons	Ponding Pollution	0.0	Percentage Removal	40	
BOD	Ponding Pollution	0.0	Percentage Removal	0	
Nitrogen	Ponding Pollution	0.0	Percentage Removal	0	
TSS	Storage Filter Pollution	0.0	Percentage Removal	0	
Heavy Metals	Storage Filter Pollution	0.0	Percentage Removal	0	
Hydrocarbons	Storage Filter Pollution	0.0	Percentage Removal	0	
BOD	Storage Filter Pollution	0.0	Percentage Removal	0	
Nitrogen	Storage Filter Pollution	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



ICE Bays 1

Type : Porous Paving

Dimensions

Exceedance Level (m)	146.516
Depth (m)	0.380
Base Level (m)	146.136
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	20.616
Long. Slope (1:X)	112.00
Width (m)	4.851
Total Volume (m³)	7.500

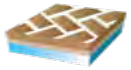
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



ICE Bays 2

Type : Porous Paving

Dimensions

Exceedance Level (m)	146.460
Depth (m)	0.380
Base Level (m)	146.080
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	15.811
Long. Slope (1:X)	105.00
Width (m)	4.743
Total Volume (m³)	5.625

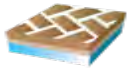
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



ICE Bays 3

Type : Porous Paving

Dimensions

Exceedance Level (m)	145.875
Depth (m)	0.450
Base Level (m)	145.425
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	15.240
Long. Slope (1:X)	28.00
Width (m)	7.546
Total Volume (m³)	11.040

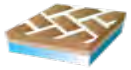
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



ICE Bays 4

Type : Porous Paving

Dimensions

Exceedance Level (m)	145.692
Depth (m)	0.450
Base Level (m)	145.242
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	15.240
Long. Slope (1:X)	22.00
Width (m)	7.546
Total Volume (m³)	11.040

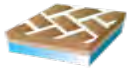
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



Elec bays 1

Type : Porous Paving

Dimensions

Exceedance Level (m)	145.860
Depth (m)	0.450
Base Level (m)	145.410
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	13.508
Long. Slope (1:X)	22.00
Width (m)	5.506
Total Volume (m³)	7.140

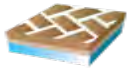
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



Elec bays 2

Type : Porous Paving

Dimensions

Exceedance Level (m)	145.270
Depth (m)	0.450
Base Level (m)	144.820
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	13.541
Long. Slope (1:X)	22.00
Width (m)	5.503
Total Volume (m³)	7.154

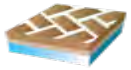
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



Elec bays 3

Type : Porous Paving

Dimensions

Exceedance Level (m)	144.856
Depth (m)	0.450
Base Level (m)	144.406
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	12.115
Long. Slope (1:X)	22.00
Width (m)	5.277
Total Volume (m³)	6.138

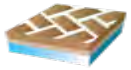
Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



Elec bays 4

Type : Porous Paving

Dimensions

Exceedance Level (m)	144.218
Depth (m)	0.450
Base Level (m)	143.768
Paving Layer Depth (mm)	130
Membrane Percolation (m/hr)	50.0
Porosity (%)	30
Length (m)	15.973
Long. Slope (1:X)	22.00
Width (m)	6.460
Total Volume (m³)	9.905

Advanced

Base Infiltration Rate (m/hr)	0.135
Safety Factor	5.0
Conductivity (m/hr)	100.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	70	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	70	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



SPEL ESR Ph II

Type : Chamber

Dimensions

Exceedance Level (m)	144.250
Depth (m)	1.240
Base Level (m)	142.928
Number of Chambers	1
Number of Rows	1
Distance Between Rows (mm)	0
Total Volume (m³)	3.413

Chamber Shape

Type	Circular Chamber
Chamber Length (m)	2.920
Wall Thickness (mm)	10
Diameter / Base Width (mm)	1220

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	80	
Heavy Metals	0.0	Percentage Removal	60	
Hydrocarbons	0.0	Percentage Removal	90	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



Soakaway SA1

Type : Cellular Storage

Dimensions

Exceedance Level (m)	143.125
Depth (m)	1.200
Base Level (m)	141.900
Number of Crates Long	15
Number of Crates Wide	7
Number of Crates High	3
Porosity (%)	95
Crate Length (m)	1
Crate Width (m)	0.5
Crate Height (m)	0.4
Total Volume (m³)	59.875

Advanced

Base Infiltration Rate (m/hr)	0.135
Side Infiltration Rate (m/hr)	0.135
Safety Factor	5.0

Pollutant Removals

Name	Background Concentration (mg/L)	Method	Percentage Removal (%)	τ (mins)
TSS	0.0	Percentage Removal	0	
Heavy Metals	0.0	Percentage Removal	0	
Hydrocarbons	0.0	Percentage Removal	0	
BOD	0.0	Percentage Removal	0	
Nitrogen	0.0	Percentage Removal	0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Name	From	To	Length (m)	Connection Type	Slope (1:X)	Manning's n	Colebrook-White Roughness (mm)	Diameter / Base Width (mm)	Height (mm)	Cross Section	Upstream Cover Level (m)	Upstream Invert Level (m)
Aco M100D30	M100D30	RG3	5.821	Custom	38.294	0.013		100	260		144.370	144.110
Aco M100D30.1	M100D30.1	M100D30.2	11.449	Custom	57.244	0.013		100	260		143.900	143.640
Aco M150D20	M150D20	RG2 ACO	6.189	Custom	884.136	0.011		150	265		144.986	144.721
SW2.000	RG2 ACO	SW2	6.730	Pipe	9.162		0.6	150			144.978	144.098
SW2.001	RG3	SW3	5.559	Pipe	23.458		0.6	150			144.217	143.337
SW2.002	M100D30.2	SW3	11.352	Pipe	98.978		0.6	150			143.700	143.140
SW1.003	SW3	SPEL ESR Ph II	4.318	Pipe	200.000		0.6	225			144.036	143.000
SW1.004	SPEL ESR Ph II	SW4	2.872	Pipe	115.139		0.6	225			144.446	142.928
SW1.001	SW1	SW2	11.828	Pipe	13.611		0.6	150			145.432	144.232
SW1.000	RG1	SW1	6.804	Pipe	39.865		0.6	150			145.637	144.980
Overflow Bio4	Bio 4	M150D20	13.521	Lagged Flow								
Overflow Bio 2	Bio 2	RG1	15.108	Lagged Flow								
Overflow2	Bio 3	Bio 4	2.228	No Delay								
SW1.005	SW4	Soakaway SA1	3.365	Pipe	120.162		0.6	225			144.603	142.903
Overflow1	Bio 1	Bio 2	1.967	No Delay								
SW1.002	SW2	SW3	14.223	Pipe	39.139		0.6	150			144.721	143.363

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Name	Downstream Cover Level (m)	Downstream Invert Level (m)	Part Family	Lock	Min. Cover Depth (m)	Flow	Velocity (m/s)	Time Of Travel (mins)	Wall Thickness (mm)	Material	Trench Width (mm)	Bedding Material
Aco M100D30	144.217	143.958		Levels	0.000				0		0	
Aco M100D30.1	143.700	143.440		Levels	0.000				0		0	
Aco M150D20	144.978	144.714		Levels	0.000				0		0	
SW2.000	144.721	143.363		Levels	0.000				0		0	
SW2.001	144.036	143.100		Levels	0.000				0		0	
SW2.002	144.036	143.025		Levels	0.000				0		0	
SW1.003	144.289	142.978		Levels	0.000				0		0	
SW1.004	144.603	142.903		Levels	0.000				0		0	
SW1.001	144.721	143.363		None	0.000				0		0	
SW1.000	145.432	144.809		Levels	0.000				0		0	
Overflow Bio4							0.818	0.3				
Overflow Bio 2							0.515	0.5				
Overflow2												
SW1.005	144.760	142.875		Levels	0.000				0		0	
Overflow1												
SW1.002	144.036	143.000		None	0.000				0		0	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Name	Surrounding Material
Aco M100D30	
Aco M100D30.1	
Aco M150D20	
SW2.000	
SW2.001	
SW2.002	
SW1.003	
SW1.004	
SW1.001	
SW1.000	
Overflow Bio4	
Overflow Bio 2	
Overflow2	
SW1.005	
Overflow1	
SW1.002	

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Title: Rainfall Analysis Criteria	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Use Catchment Values
Junction Flood Risk Margin (mm)	300
Prefill Manhole Sumps	☑
Perform No Discharge Analysis	☐

Rainfall

FEH2022

Type: FEH

Site Location	GB 390479 134457 ST 90479 34457
Rainfall Version	2022
Summer	☑
Winter	☑

Return Period

Return Period (years)	Increase Rainfall (%)
2.0	0.000
10.0	0.000
30.0	40.000
100.0	0.000
100.0	45.000

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Title: Rainfall Analysis Criteria	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Storm Durations

Duration (mins)	Run Time (mins)
15	30
30	60
60	120
120	240
240	480
360	720
480	960
960	1920
1440	2880

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 15 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.018	0.596	3.7	0.095	0.000	3.6	1.609	OK	145.018
RG3	144.217	142.897	143.462	0.565	2.2	0.090	0.000	2.2	1.151	OK	143.462
M100D30	144.370	144.110	144.161	0.051	2.3	0.004	0.000	2.2	1.009	Flood Risk	144.161
M100D30.1	143.900	143.640	143.680	0.040	1.4	0.003	0.000	1.3	0.586	Flood Risk	143.680
M100D30.2	143.700	142.990	143.166	0.176	1.3	0.013	0.000	1.2	0.583	OK	143.166
RG2 ACO	144.978	143.658	144.223	0.565	6.1	0.090	0.000	6.0	2.827	OK	144.223
M150D20	144.986	144.721	144.794	0.073	2.4	0.022	0.000	2.2	1.012	Flood Risk	144.794
SW3	144.036	142.500	143.098	0.598	12.7	0.380	0.000	12.1	6.180	OK	143.098
SW1	145.432	144.232	144.260	0.028	3.6	0.018	0.000	3.6	1.608	OK	144.260
SW4	144.603	142.903	142.989	0.086	11.9	0.054	0.000	11.6	6.180	OK	142.989
SW2	144.721	143.363	143.423	0.059	9.6	0.038	0.000	9.3	4.427	OK	143.423

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.619
0.755
0.209
0.220
0.534
0.755
0.191
0.938
1.172
1.615
1.298

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 15 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.017	0.595	3.5	0.095	0.000	3.4	1.606	OK	145.017
RG3	144.217	142.897	143.462	0.565	2.1	0.090	0.000	2.0	1.148	OK	143.462
M100D30	144.370	144.110	144.159	0.049	2.2	0.004	0.000	2.1	1.006	Flood Risk	144.159
M100D30.1	143.900	143.640	143.679	0.039	1.3	0.003	0.000	1.2	0.583	Flood Risk	143.679
M100D30.2	143.700	142.990	143.165	0.175	1.2	0.013	0.000	1.2	0.580	OK	143.165
RG2 ACO	144.978	143.658	144.223	0.565	5.7	0.090	0.000	5.7	2.833	OK	144.223
M150D20	144.986	144.721	144.792	0.071	2.2	0.021	0.000	2.1	1.018	Flood Risk	144.792
SW3	144.036	142.500	143.095	0.595	12.0	0.378	0.000	11.4	6.179	OK	143.095
SW1	145.432	144.232	144.259	0.027	3.4	0.017	0.000	3.4	1.605	OK	144.259
SW4	144.603	142.903	142.986	0.083	11.2	0.053	0.000	11.0	6.179	OK	142.986
SW2	144.721	143.363	143.421	0.057	9.0	0.036	0.000	8.8	4.430	OK	143.421

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.620
0.755
0.211
0.221
0.535
0.755
0.194
0.941
1.173
1.617
1.300

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 30 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.011	0.589	2.5	0.094	0.000	2.5	2.131	OK	145.011
RG3	144.217	142.897	143.462	0.565	1.5	0.090	0.000	1.5	1.473	OK	143.462
M100D30	144.370	144.110	144.151	0.041	1.5	0.003	0.000	1.5	1.331	Flood Risk	144.151
M100D30.1	143.900	143.640	143.673	0.033	0.9	0.002	0.000	0.9	0.778	Flood Risk	143.673
M100D30.2	143.700	142.990	143.162	0.172	0.9	0.013	0.000	0.9	0.777	OK	143.162
RG2 ACO	144.978	143.658	144.223	0.565	4.1	0.090	0.000	4.1	3.700	OK	144.223
M150D20	144.986	144.721	144.782	0.061	1.6	0.018	0.000	1.6	1.345	Flood Risk	144.782
SW3	144.036	142.500	143.082	0.582	9.0	0.370	0.000	9.0	8.096	OK	143.082
SW1	145.432	144.232	144.255	0.023	2.5	0.015	0.000	2.5	2.131	OK	144.255
SW4	144.603	142.903	142.977	0.074	9.0	0.047	0.000	9.0	8.123	OK	142.977
SW2	144.721	143.363	143.412	0.049	6.6	0.031	0.000	6.6	5.824	OK	143.412

Freeboard (m)
0.626
0.755
0.219
0.227
0.538
0.755
0.204
0.954
1.177
1.626
1.308

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 30 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.010	0.588	2.3	0.093	0.000	2.3	2.119	OK	145.010
RG3	144.217	142.897	143.462	0.565	1.4	0.090	0.000	1.4	1.467	OK	143.462
M100D30	144.370	144.110	144.149	0.039	1.4	0.003	0.000	1.4	1.325	Flood Risk	144.149
M100D30.1	143.900	143.640	143.671	0.031	0.8	0.002	0.000	0.8	0.773	Flood Risk	143.671
M100D30.2	143.700	142.990	143.162	0.172	0.8	0.013	0.000	0.8	0.771	OK	143.162
RG2 ACO	144.978	143.658	144.223	0.565	3.9	0.090	0.000	3.9	3.701	OK	144.223
M150D20	144.986	144.721	144.781	0.060	1.5	0.018	0.000	1.5	1.351	Flood Risk	144.781
SW3	144.036	142.500	143.080	0.580	8.4	0.369	0.000	8.4	8.072	OK	143.080
SW1	145.432	144.232	144.255	0.022	2.3	0.014	0.000	2.3	2.119	OK	144.255
SW4	144.603	142.903	142.974	0.071	8.4	0.045	0.000	8.4	8.102	OK	142.974
SW2	144.721	143.363	143.411	0.047	6.1	0.030	0.000	6.1	5.812	OK	143.411

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.627
0.755
0.221
0.229
0.538
0.755
0.205
0.957
1.178
1.629
1.310

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 60 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.009	0.587	2.2	0.093	0.000	2.2	2.713	OK	145.009
RG3	144.217	142.897	143.462	0.565	1.4	0.090	0.000	1.4	1.839	OK	143.462
M100D30	144.370	144.110	144.148	0.038	1.4	0.003	0.000	1.4	1.697	Flood Risk	144.148
M100D30.1	143.900	143.640	143.670	0.030	0.8	0.002	0.000	0.8	0.989	Flood Risk	143.670
M100D30.2	143.700	142.990	143.161	0.171	0.8	0.013	0.000	0.8	0.987	OK	143.161
RG2 ACO	144.978	143.658	144.223	0.565	3.6	0.090	0.000	3.6	4.693	OK	144.223
M150D20	144.986	144.721	144.779	0.058	1.4	0.017	0.000	1.4	1.725	Flood Risk	144.779
SW3	144.036	142.500	143.077	0.577	7.9	0.367	0.000	7.9	10.246	OK	143.077
SW1	145.432	144.232	144.254	0.022	2.2	0.014	0.000	2.2	2.713	OK	144.254
SW4	144.603	142.903	142.972	0.069	7.9	0.044	0.000	7.9	10.280	OK	142.972
SW2	144.721	143.363	143.409	0.046	5.8	0.029	0.000	5.8	7.399	OK	143.409

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.628
0.755
0.222
0.230
0.539
0.755
0.207
0.959
1.178
1.631
1.311

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 60 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.006	0.584	1.7	0.093	0.000	1.7	2.707	OK	145.006
RG3	144.217	142.897	143.462	0.565	1.1	0.090	0.000	1.1	1.839	OK	143.462
M100D30	144.370	144.110	144.143	0.033	1.1	0.002	0.000	1.1	1.697	Flood Risk	144.143
M100D30.1	143.900	143.640	143.667	0.027	0.6	0.002	0.000	0.6	0.995	Flood Risk	143.667
M100D30.2	143.700	142.990	143.159	0.169	0.6	0.013	0.000	0.6	0.993	OK	143.159
RG2 ACO	144.978	143.658	144.223	0.565	2.9	0.090	0.000	2.9	4.687	OK	144.223
M150D20	144.986	144.721	144.773	0.052	1.1	0.016	0.000	1.1	1.719	Flood Risk	144.773
SW3	144.036	142.500	143.068	0.568	6.4	0.362	0.000	6.4	10.239	OK	143.068
SW1	145.432	144.232	144.252	0.020	1.7	0.012	0.000	1.7	2.707	OK	144.252
SW4	144.603	142.903	142.965	0.061	6.4	0.039	0.000	6.4	10.270	OK	142.965
SW2	144.721	143.363	143.405	0.041	4.7	0.026	0.000	4.7	7.387	OK	143.405

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.631
0.755
0.227
0.233
0.541
0.755
0.213
0.968
1.180
1.639
1.316

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 120 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.006	0.584	1.8	0.093	0.000	1.8	3.781	OK	145.006
RG3	144.217	142.897	143.462	0.565	1.1	0.090	0.000	1.1	2.511	OK	143.462
M100D30	144.370	144.110	144.144	0.034	1.1	0.003	0.000	1.1	2.369	Flood Risk	144.144
M100D30.1	143.900	143.640	143.667	0.027	0.7	0.002	0.000	0.7	1.385	Flood Risk	143.667
M100D30.2	143.700	142.990	143.159	0.169	0.7	0.013	0.000	0.7	1.383	OK	143.159
RG2 ACO	144.978	143.658	144.223	0.565	3.0	0.090	0.000	3.0	6.476	OK	144.223
M150D20	144.986	144.721	144.774	0.053	1.1	0.016	0.000	1.1	2.404	Flood Risk	144.774
SW3	144.036	142.500	143.070	0.570	6.6	0.362	0.000	6.6	14.166	OK	143.070
SW1	145.432	144.232	144.252	0.020	1.8	0.013	0.000	1.8	3.781	OK	144.252
SW4	144.603	142.903	142.965	0.062	6.6	0.040	0.000	6.6	14.204	OK	142.965
SW2	144.721	143.363	143.405	0.042	4.8	0.027	0.000	4.8	10.251	OK	143.405

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.631
0.755
0.226
0.233
0.541
0.755
0.212
0.967
1.180
1.638
1.315

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 120 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.002	0.580	1.3	0.092	0.000	1.3	3.775	OK	145.002
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	2.505	OK	143.462
M100D30	144.370	144.110	144.138	0.028	0.8	0.002	0.000	0.8	2.363	Flood Risk	144.138
M100D30.1	143.900	143.640	143.663	0.023	0.5	0.002	0.000	0.5	1.373	Flood Risk	143.663
M100D30.2	143.700	142.990	143.157	0.167	0.5	0.012	0.000	0.5	1.371	OK	143.157
RG2 ACO	144.978	143.658	144.223	0.565	2.2	0.090	0.000	2.2	6.488	OK	144.223
M150D20	144.986	144.721	144.766	0.045	0.8	0.013	0.000	0.8	2.416	Flood Risk	144.766
SW3	144.036	142.500	143.058	0.558	4.7	0.355	0.000	4.7	14.153	OK	143.058
SW1	145.432	144.232	144.249	0.017	1.3	0.011	0.000	1.3	3.775	OK	144.249
SW4	144.603	142.903	142.955	0.052	4.7	0.033	0.000	4.7	14.197	OK	142.955
SW2	144.721	143.363	143.399	0.035	3.4	0.022	0.000	3.4	10.257	OK	143.399

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.232
0.237
0.543
0.755
0.220
0.978
1.183
1.648
1.322

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 240 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.002	0.580	1.3	0.092	0.000	1.3	4.975	OK	145.002
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	3.267	OK	143.462
M100D30	144.370	144.110	144.138	0.028	0.8	0.002	0.000	0.8	3.125	Flood Risk	144.138
M100D30.1	143.900	143.640	143.663	0.023	0.5	0.002	0.000	0.5	1.823	Flood Risk	143.663
M100D30.2	143.700	142.990	143.157	0.167	0.5	0.012	0.000	0.5	1.821	OK	143.157
RG2 ACO	144.978	143.658	144.223	0.565	2.2	0.090	0.000	2.2	8.528	OK	144.223
M150D20	144.986	144.721	144.766	0.045	0.8	0.013	0.000	0.8	3.178	Flood Risk	144.766
SW3	144.036	142.500	143.058	0.558	4.7	0.355	0.000	4.7	18.606	OK	143.058
SW1	145.432	144.232	144.249	0.017	1.3	0.011	0.000	1.3	4.975	OK	144.249
SW4	144.603	142.903	142.955	0.052	4.7	0.033	0.000	4.7	18.652	OK	142.955
SW2	144.721	143.363	143.399	0.035	3.5	0.023	0.000	3.5	13.498	OK	143.399

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.232
0.237
0.543
0.755
0.220
0.978
1.183
1.648
1.322

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 240 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.998	0.577	0.9	0.092	0.000	0.9	4.981	OK	144.998
RG3	144.217	142.897	143.462	0.565	0.6	0.090	0.000	0.6	3.273	OK	143.462
M100D30	144.370	144.110	144.132	0.022	0.6	0.002	0.000	0.6	3.131	Flood Risk	144.132
M100D30.1	143.900	143.640	143.658	0.018	0.3	0.001	0.000	0.3	1.835	Flood Risk	143.658
M100D30.2	143.700	142.990	143.154	0.164	0.3	0.012	0.000	0.3	1.833	OK	143.154
RG2 ACO	144.978	143.658	144.223	0.565	1.5	0.090	0.000	1.5	8.528	OK	144.223
M150D20	144.986	144.721	144.758	0.037	0.6	0.011	0.000	0.5	3.184	Flood Risk	144.758
SW3	144.036	142.500	143.048	0.548	3.2	0.348	0.000	3.2	18.629	OK	143.048
SW1	145.432	144.232	144.246	0.014	0.9	0.009	0.000	0.9	4.981	OK	144.246
SW4	144.603	142.903	142.946	0.043	3.2	0.027	0.000	3.2	18.677	OK	142.946
SW2	144.721	143.363	143.393	0.029	2.3	0.019	0.000	2.3	13.504	OK	143.393

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.638
0.755
0.238
0.242
0.546
0.755
0.228
0.989
1.186
1.658
1.328

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 360 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.000	0.578	1.0	0.092	0.000	1.0	5.737	OK	145.000
RG3	144.217	142.897	143.462	0.565	0.6	0.090	0.000	0.6	3.735	OK	143.462
M100D30	144.370	144.110	144.134	0.024	0.6	0.002	0.000	0.6	3.593	Flood Risk	144.134
M100D30.1	143.900	143.640	143.660	0.020	0.4	0.001	0.000	0.4	2.093	Flood Risk	143.660
M100D30.2	143.700	142.990	143.155	0.165	0.4	0.012	0.000	0.4	2.091	OK	143.155
RG2 ACO	144.978	143.658	144.223	0.565	1.7	0.090	0.000	1.7	9.746	OK	144.223
M150D20	144.986	144.721	144.761	0.040	0.6	0.012	0.000	0.6	3.646	Flood Risk	144.761
SW3	144.036	142.500	143.052	0.552	3.7	0.351	0.000	3.7	21.324	OK	143.052
SW1	145.432	144.232	144.248	0.015	1.0	0.010	0.000	1.0	5.737	OK	144.248
SW4	144.603	142.903	142.949	0.046	3.7	0.029	0.000	3.7	21.372	OK	142.949
SW2	144.721	143.363	143.395	0.031	2.7	0.020	0.000	2.7	15.478	OK	143.395

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.637
0.755
0.236
0.240
0.545
0.755
0.225
0.985
1.185
1.654
1.326

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 360 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.996	0.574	0.7	0.091	0.000	0.7	5.725	OK	144.996
RG3	144.217	142.897	143.462	0.565	0.4	0.090	0.000	0.4	3.717	OK	143.462
M100D30	144.370	144.110	144.129	0.019	0.4	0.001	0.000	0.4	3.575	Flood Risk	144.129
M100D30.1	143.900	143.640	143.656	0.016	0.2	0.001	0.000	0.2	2.081	Flood Risk	143.656
M100D30.2	143.700	142.990	143.152	0.162	0.2	0.012	0.000	0.2	2.079	OK	143.152
RG2 ACO	144.978	143.658	144.223	0.565	1.1	0.090	0.000	1.1	9.746	OK	144.223
M150D20	144.986	144.721	144.754	0.033	0.4	0.010	0.000	0.4	3.664	Flood Risk	144.754
SW3	144.036	142.500	143.042	0.542	2.5	0.344	0.000	2.5	21.281	OK	143.042
SW1	145.432	144.232	144.245	0.012	0.7	0.008	0.000	0.7	5.725	OK	144.245
SW4	144.603	142.903	142.940	0.037	2.5	0.024	0.000	2.5	21.342	OK	142.940
SW2	144.721	143.363	143.389	0.026	1.8	0.016	0.000	1.8	15.466	OK	143.389

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.641
0.755
0.241
0.244
0.548
0.755
0.232
0.995
1.188
1.663
1.332

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 480 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.998	0.576	0.9	0.092	0.000	0.9	6.241	OK	144.998
RG3	144.217	142.897	143.462	0.565	0.5	0.090	0.000	0.5	4.059	OK	143.462
M100D30	144.370	144.110	144.132	0.022	0.5	0.002	0.000	0.5	3.917	Flood Risk	144.132
M100D30.1	143.900	143.640	143.658	0.018	0.3	0.001	0.000	0.3	2.279	Flood Risk	143.658
M100D30.2	143.700	142.990	143.154	0.164	0.3	0.012	0.000	0.3	2.277	OK	143.154
RG2 ACO	144.978	143.658	144.223	0.565	1.4	0.090	0.000	1.4	10.628	OK	144.223
M150D20	144.986	144.721	144.758	0.037	0.5	0.011	0.000	0.5	3.988	Flood Risk	144.758
SW3	144.036	142.500	143.047	0.547	3.1	0.348	0.000	3.1	23.220	OK	143.047
SW1	145.432	144.232	144.246	0.014	0.9	0.009	0.000	0.9	6.241	OK	144.246
SW4	144.603	142.903	142.945	0.042	3.1	0.027	0.000	3.1	23.265	OK	142.945
SW2	144.721	143.363	143.392	0.029	2.3	0.018	0.000	2.3	16.864	OK	143.392

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.639
0.755
0.238
0.242
0.546
0.755
0.228
0.990
1.186
1.658
1.329

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 480 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.995	0.573	0.6	0.091	0.000	0.6	6.265	OK	144.995
RG3	144.217	142.897	143.462	0.565	0.3	0.090	0.000	0.3	4.047	OK	143.462
M100D30	144.370	144.110	144.127	0.017	0.3	0.001	0.000	0.3	3.905	Flood Risk	144.127
M100D30.1	143.900	143.640	143.654	0.014	0.2	0.001	0.000	0.2	2.303	Flood Risk	143.654
M100D30.2	143.700	142.990	143.151	0.161	0.2	0.012	0.000	0.2	2.301	OK	143.151
RG2 ACO	144.978	143.658	144.223	0.565	0.9	0.090	0.000	0.9	10.628	OK	144.223
M150D20	144.986	144.721	144.751	0.030	0.3	0.009	0.000	0.3	3.982	Flood Risk	144.751
SW3	144.036	142.500	143.038	0.538	2.0	0.342	0.000	2.0	23.255	OK	143.038
SW1	145.432	144.232	144.244	0.011	0.6	0.007	0.000	0.6	6.265	OK	144.244
SW4	144.603	142.903	142.937	0.034	2.0	0.021	0.000	2.0	23.334	OK	142.937
SW2	144.721	143.363	143.387	0.023	1.5	0.015	0.000	1.5	16.888	OK	143.387

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.642
0.755
0.243
0.246
0.549
0.755
0.235
0.999
1.189
1.667
1.334

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 960 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.994	0.572	0.5	0.091	0.000	0.5	7.651	OK	144.994
RG3	144.217	142.897	143.462	0.565	0.3	0.090	0.000	0.3	4.923	OK	143.462
M100D30	144.370	144.110	144.127	0.017	0.3	0.001	0.000	0.3	4.781	Flood Risk	144.127
M100D30.1	143.900	143.640	143.654	0.014	0.2	0.001	0.000	0.2	2.819	Flood Risk	143.654
M100D30.2	143.700	142.990	143.151	0.161	0.2	0.012	0.000	0.2	2.817	OK	143.151
RG2 ACO	144.978	143.658	144.223	0.565	0.9	0.090	0.000	0.9	12.938	OK	144.223
M150D20	144.986	144.721	144.750	0.029	0.3	0.009	0.000	0.3	4.853	Flood Risk	144.750
SW3	144.036	142.500	143.036	0.536	1.9	0.341	0.000	1.9	28.344	OK	143.036
SW1	145.432	144.232	144.243	0.011	0.5	0.007	0.000	0.5	7.651	OK	144.243
SW4	144.603	142.903	142.936	0.033	1.9	0.021	0.000	1.9	28.567	OK	142.936
SW2	144.721	143.363	143.386	0.023	1.4	0.014	0.000	1.4	20.584	OK	143.386

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.643
0.755
0.243
0.246
0.549
0.755
0.236
1.000
1.189
1.668
1.335

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 960 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.992	0.570	0.3	0.091	0.000	0.3	7.603	OK	144.992
RG3	144.217	142.897	143.462	0.565	0.2	0.090	0.000	0.2	4.947	OK	143.462
M100D30	144.370	144.110	144.123	0.013	0.2	0.001	0.000	0.2	4.805	Flood Risk	144.123
M100D30.1	143.900	143.640	143.651	0.011	0.1	0.001	0.000	0.1	2.771	Flood Risk	143.651
M100D30.2	143.700	142.990	143.149	0.159	0.1	0.012	0.000	0.1	2.769	OK	143.149
RG2 ACO	144.978	143.658	144.223	0.565	0.6	0.090	0.000	0.6	12.932	OK	144.223
M150D20	144.986	144.721	144.745	0.024	0.2	0.007	0.000	0.2	4.858	Flood Risk	144.745
SW3	144.036	142.500	143.029	0.529	1.2	0.337	0.000	1.2	28.265	OK	143.029
SW1	145.432	144.232	144.241	0.009	0.3	0.006	0.000	0.3	7.603	OK	144.241
SW4	144.603	142.903	142.929	0.026	1.2	0.017	0.000	1.2	28.430	OK	142.929
SW2	144.721	143.363	143.382	0.018	0.9	0.012	0.000	0.9	20.530	OK	143.382

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.645
0.755
0.247
0.249
0.551
0.755
0.241
1.007
1.191
1.674
1.339

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 1440 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.992	0.570	0.4	0.091	0.000	0.4	8.683	OK	144.992
RG3	144.217	142.897	143.462	0.565	0.2	0.090	0.000	0.2	5.445	OK	143.462
M100D30	144.370	144.110	144.124	0.014	0.2	0.001	0.000	0.2	5.303	Flood Risk	144.124
M100D30.1	143.900	143.640	143.652	0.012	0.1	0.001	0.000	0.1	3.059	Flood Risk	143.652
M100D30.2	143.700	142.990	143.150	0.160	0.1	0.012	0.000	0.1	3.058	OK	143.150
RG2 ACO	144.978	143.658	144.223	0.565	0.6	0.090	0.000	0.6	14.432	OK	144.223
M150D20	144.986	144.721	144.747	0.026	0.3	0.008	0.000	0.3	5.411	Flood Risk	144.747
SW3	144.036	142.500	143.031	0.531	1.4	0.338	0.000	1.4	31.632	OK	143.031
SW1	145.432	144.232	144.242	0.010	0.4	0.006	0.000	0.4	8.683	OK	144.242
SW4	144.603	142.903	142.931	0.028	1.4	0.018	0.000	1.4	32.258	OK	142.931
SW2	144.721	143.363	143.383	0.020	1.0	0.012	0.000	1.0	23.110	OK	143.383

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.645
0.755
0.246
0.248
0.550
0.755
0.239
1.005
1.190
1.672
1.338

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 1440 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.990	0.568	0.3	0.090	0.000	0.3	8.623	OK	144.990
RG3	144.217	142.897	143.462	0.565	0.2	0.090	0.000	0.2	5.505	OK	143.462
M100D30	144.370	144.110	144.122	0.012	0.2	0.001	0.000	0.2	5.363	Flood Risk	144.122
M100D30.1	143.900	143.640	143.650	0.010	0.1	0.001	0.000	0.1	3.167	Flood Risk	143.650
M100D30.2	143.700	142.990	143.148	0.158	0.1	0.012	0.000	0.1	3.165	OK	143.148
RG2 ACO	144.978	143.658	144.223	0.565	0.4	0.090	0.000	0.4	14.492	OK	144.223
M150D20	144.986	144.721	144.742	0.021	0.2	0.006	0.000	0.2	5.470	Flood Risk	144.742
SW3	144.036	142.500	143.025	0.525	0.9	0.334	0.000	0.9	31.799	OK	143.025
SW1	145.432	144.232	144.240	0.008	0.3	0.005	0.000	0.3	8.623	OK	144.240
SW4	144.603	142.903	142.926	0.023	0.9	0.014	0.000	0.9	32.324	OK	142.926
SW2	144.721	143.363	143.379	0.016	0.7	0.010	0.000	0.7	23.110	OK	143.379

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.647
0.755
0.248
0.250
0.552
0.755
0.244
1.011
1.192
1.678
1.341

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 15 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.030	0.609	6.1	0.097	0.000	6.1	2.677	OK	145.030
RG3	144.217	142.897	143.462	0.565	3.7	0.090	0.000	3.7	1.816	OK	143.462
M100D30	144.370	144.110	144.181	0.071	3.9	0.005	0.000	3.7	1.675	Flood Risk	144.181
M100D30.1	143.900	143.640	143.696	0.056	2.3	0.004	0.000	2.1	0.975	Flood Risk	143.696
M100D30.2	143.700	142.990	143.174	0.184	2.1	0.014	0.000	2.1	0.973	OK	143.174
RG2 ACO	144.978	143.658	144.223	0.565	10.2	0.090	0.000	10.1	4.621	OK	144.223
M150D20	144.986	144.721	144.816	0.095	3.9	0.029	0.000	3.8	1.693	Flood Risk	144.816
SW3	144.036	142.500	143.135	0.635	21.4	0.404	0.000	20.5	10.101	OK	143.135
SW1	145.432	144.232	144.268	0.036	6.1	0.023	0.000	6.0	2.676	OK	144.268
SW4	144.603	142.903	143.019	0.116	20.0	0.074	0.000	19.7	10.099	OK	143.019
SW2	144.721	143.363	143.445	0.082	16.1	0.052	0.000	15.6	7.288	OK	143.445

Freeboard (m)
0.606
0.755
0.189
0.204
0.526
0.755
0.169
0.902
1.164
1.584
1.276

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 15 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.029	0.607	5.8	0.096	0.000	5.7	2.674	OK	145.029
RG3	144.217	142.897	143.462	0.565	3.5	0.090	0.000	3.4	1.816	OK	143.462
M100D30	144.370	144.110	144.178	0.068	3.6	0.005	0.000	3.5	1.675	Flood Risk	144.178
M100D30.1	143.900	143.640	143.694	0.054	2.1	0.004	0.000	2.0	0.975	Flood Risk	143.694
M100D30.2	143.700	142.990	143.173	0.183	2.0	0.014	0.000	2.0	0.973	OK	143.173
RG2 ACO	144.978	143.658	144.223	0.565	9.6	0.090	0.000	9.5	4.615	OK	144.223
M150D20	144.986	144.721	144.814	0.093	3.7	0.028	0.000	3.5	1.693	Flood Risk	144.814
SW3	144.036	142.500	143.130	0.630	20.1	0.401	0.000	19.4	10.092	OK	143.130
SW1	145.432	144.232	144.267	0.035	5.7	0.022	0.000	5.6	2.673	OK	144.267
SW4	144.603	142.903	143.015	0.112	18.9	0.071	0.000	18.6	10.091	OK	143.015
SW2	144.721	143.363	143.442	0.078	15.1	0.050	0.000	14.7	7.279	OK	143.442

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.608
0.755
0.192
0.206
0.527
0.755
0.172
0.906
1.165
1.588
1.279

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 30 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.021	0.599	4.1	0.095	0.000	4.1	3.571	OK	145.021
RG3	144.217	142.897	143.462	0.565	2.6	0.090	0.000	2.6	2.372	OK	143.462
M100D30	144.370	144.110	144.166	0.056	2.6	0.004	0.000	2.6	2.230	Flood Risk	144.166
M100D30.1	143.900	143.640	143.685	0.045	1.5	0.003	0.000	1.5	1.294	Flood Risk	143.685
M100D30.2	143.700	142.990	143.169	0.179	1.5	0.013	0.000	1.5	1.293	OK	143.169
RG2 ACO	144.978	143.658	144.223	0.565	6.9	0.090	0.000	6.9	6.112	OK	144.223
M150D20	144.986	144.721	144.800	0.079	2.6	0.024	0.000	2.6	2.263	Flood Risk	144.800
SW3	144.036	142.500	143.111	0.611	15.1	0.389	0.000	15.1	13.364	OK	143.111
SW1	145.432	144.232	144.262	0.030	4.1	0.019	0.000	4.1	3.571	OK	144.262
SW4	144.603	142.903	143.002	0.099	15.1	0.063	0.000	15.1	13.394	OK	143.002
SW2	144.721	143.363	143.428	0.065	11.0	0.041	0.000	11.0	9.676	OK	143.428

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.616
0.755
0.204
0.215
0.531
0.755
0.186
0.925
1.170
1.601
1.292

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 30 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.019	0.597	3.8	0.095	0.000	3.8	3.559	OK	145.019
RG3	144.217	142.897	143.462	0.565	2.4	0.090	0.000	2.4	2.373	OK	143.462
M100D30	144.370	144.110	144.164	0.054	2.4	0.004	0.000	2.4	2.230	Flood Risk	144.164
M100D30.1	143.900	143.640	143.683	0.043	1.4	0.003	0.000	1.4	1.300	Flood Risk	143.683
M100D30.2	143.700	142.990	143.168	0.178	1.4	0.013	0.000	1.4	1.299	OK	143.168
RG2 ACO	144.978	143.658	144.223	0.565	6.5	0.090	0.000	6.5	6.118	OK	144.223
M150D20	144.986	144.721	144.798	0.077	2.5	0.023	0.000	2.5	2.263	Flood Risk	144.798
SW3	144.036	142.500	143.107	0.607	14.2	0.386	0.000	14.2	13.365	OK	143.107
SW1	145.432	144.232	144.261	0.029	3.8	0.018	0.000	3.8	3.559	OK	144.261
SW4	144.603	142.903	142.998	0.095	14.2	0.061	0.000	14.2	13.394	OK	142.998
SW2	144.721	143.363	143.426	0.063	10.3	0.040	0.000	10.3	9.670	OK	143.426

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.618
0.755
0.206
0.217
0.532
0.755
0.188
0.930
1.171
1.605
1.295

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 60 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.018	0.596	3.7	0.095	0.000	3.7	4.567	OK	145.018
RG3	144.217	142.897	143.462	0.565	2.3	0.090	0.000	2.3	3.015	OK	143.462
M100D30	144.370	144.110	144.162	0.052	2.3	0.004	0.000	2.3	2.873	Flood Risk	144.162
M100D30.1	143.900	143.640	143.682	0.042	1.3	0.003	0.000	1.3	1.673	Flood Risk	143.682
M100D30.2	143.700	142.990	143.167	0.177	1.3	0.013	0.000	1.3	1.671	OK	143.167
RG2 ACO	144.978	143.658	144.223	0.565	6.1	0.090	0.000	6.1	7.813	OK	144.223
M150D20	144.986	144.721	144.796	0.075	2.3	0.022	0.000	2.3	2.907	Flood Risk	144.796
SW3	144.036	142.500	143.104	0.604	13.4	0.384	0.000	13.4	17.081	OK	143.104
SW1	145.432	144.232	144.260	0.028	3.7	0.018	0.000	3.7	4.567	OK	144.260
SW4	144.603	142.903	142.996	0.092	13.4	0.059	0.000	13.4	17.115	OK	142.996
SW2	144.721	143.363	143.424	0.061	9.8	0.039	0.000	9.8	12.373	OK	143.424

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.619
0.755
0.208
0.218
0.533
0.755
0.190
0.933
1.172
1.608
1.296

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 60 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.014	0.592	2.9	0.094	0.000	2.9	4.573	OK	145.014
RG3	144.217	142.897	143.462	0.565	1.9	0.090	0.000	1.9	3.015	OK	143.462
M100D30	144.370	144.110	144.156	0.046	1.9	0.003	0.000	1.9	2.873	Flood Risk	144.156
M100D30.1	143.900	143.640	143.676	0.036	1.1	0.003	0.000	1.1	1.685	Flood Risk	143.676
M100D30.2	143.700	142.990	143.164	0.174	1.1	0.013	0.000	1.1	1.683	OK	143.164
RG2 ACO	144.978	143.658	144.223	0.565	4.9	0.090	0.000	4.9	7.813	OK	144.223
M150D20	144.986	144.721	144.788	0.067	1.9	0.020	0.000	1.9	2.913	Flood Risk	144.788
SW3	144.036	142.500	143.091	0.591	10.8	0.376	0.000	10.8	17.098	OK	143.091
SW1	145.432	144.232	144.258	0.025	2.9	0.016	0.000	2.9	4.573	OK	144.258
SW4	144.603	142.903	142.985	0.082	10.8	0.052	0.000	10.8	17.130	OK	142.985
SW2	144.721	143.363	143.417	0.054	7.9	0.034	0.000	7.9	12.379	OK	143.417

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.623
0.755
0.214
0.224
0.536
0.755
0.198
0.945
1.175
1.618
1.303

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 120 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.013	0.591	2.8	0.094	0.000	2.8	5.857	OK	145.013
RG3	144.217	142.897	143.462	0.565	1.8	0.090	0.000	1.8	3.819	OK	143.462
M100D30	144.370	144.110	144.154	0.044	1.8	0.003	0.000	1.8	3.677	Flood Risk	144.154
M100D30.1	143.900	143.640	143.675	0.035	1.0	0.003	0.000	1.0	2.141	Flood Risk	143.675
M100D30.2	143.700	142.990	143.164	0.174	1.0	0.013	0.000	1.0	2.139	OK	143.164
RG2 ACO	144.978	143.658	144.223	0.565	4.7	0.090	0.000	4.7	9.986	OK	144.223
M150D20	144.986	144.721	144.786	0.065	1.8	0.020	0.000	1.8	3.730	Flood Risk	144.786
SW3	144.036	142.500	143.089	0.589	10.2	0.374	0.000	10.2	21.817	OK	143.089
SW1	145.432	144.232	144.257	0.024	2.8	0.016	0.000	2.8	5.857	OK	144.257
SW4	144.603	142.903	142.982	0.079	10.2	0.050	0.000	10.2	21.857	OK	142.982
SW2	144.721	143.363	143.416	0.052	7.4	0.033	0.000	7.4	15.837	OK	143.416

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.624
0.755
0.216
0.225
0.536
0.755
0.200
0.948
1.176
1.621
1.305

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 120 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.008	0.586	2.0	0.093	0.000	2.0	5.863	OK	145.008
RG3	144.217	142.897	143.462	0.565	1.3	0.090	0.000	1.3	3.831	OK	143.462
M100D30	144.370	144.110	144.146	0.036	1.3	0.003	0.000	1.3	3.689	Flood Risk	144.146
M100D30.1	143.900	143.640	143.669	0.029	0.7	0.002	0.000	0.7	2.135	Flood Risk	143.669
M100D30.2	143.700	142.990	143.160	0.170	0.7	0.013	0.000	0.7	2.133	OK	143.160
RG2 ACO	144.978	143.658	144.223	0.565	3.4	0.090	0.000	3.4	9.992	OK	144.223
M150D20	144.986	144.721	144.777	0.056	1.3	0.017	0.000	1.3	3.748	Flood Risk	144.777
SW3	144.036	142.500	143.074	0.574	7.4	0.365	0.000	7.4	21.833	OK	143.074
SW1	145.432	144.232	144.253	0.021	2.0	0.013	0.000	2.0	5.863	OK	144.253
SW4	144.603	142.903	142.969	0.066	7.4	0.042	0.000	7.4	21.875	OK	142.969
SW2	144.721	143.363	143.408	0.044	5.4	0.028	0.000	5.4	15.849	OK	143.408

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.629
0.755
0.224
0.231
0.540
0.755
0.209
0.963
1.179
1.634
1.313

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 240 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.007	0.585	1.9	0.093	0.000	1.9	7.291	OK	145.007
RG3	144.217	142.897	143.462	0.565	1.2	0.090	0.000	1.2	4.725	OK	143.462
M100D30	144.370	144.110	144.145	0.035	1.2	0.003	0.000	1.2	4.583	Flood Risk	144.145
M100D30.1	143.900	143.640	143.668	0.028	0.7	0.002	0.000	0.7	2.663	Flood Risk	143.668
M100D30.2	143.700	142.990	143.160	0.170	0.7	0.013	0.000	0.7	2.661	OK	143.160
RG2 ACO	144.978	143.658	144.223	0.565	3.2	0.090	0.000	3.2	12.434	OK	144.223
M150D20	144.986	144.721	144.775	0.054	1.2	0.016	0.000	1.2	4.672	Flood Risk	144.775
SW3	144.036	142.500	143.072	0.572	6.9	0.364	0.000	6.9	27.127	OK	143.072
SW1	145.432	144.232	144.253	0.020	1.9	0.013	0.000	1.9	7.291	OK	144.253
SW4	144.603	142.903	142.967	0.064	6.9	0.041	0.000	6.9	27.170	OK	142.967
SW2	144.721	143.363	143.406	0.043	5.1	0.027	0.000	5.1	19.720	OK	143.406

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.630
0.755
0.225
0.232
0.540
0.755
0.211
0.965
1.180
1.636
1.314

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 240 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.002	0.580	1.3	0.092	0.000	1.3	7.351	OK	145.002
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	4.731	OK	143.462
M100D30	144.370	144.110	144.137	0.027	0.8	0.002	0.000	0.8	4.589	Flood Risk	144.137
M100D30.1	143.900	143.640	143.663	0.023	0.5	0.002	0.000	0.5	2.681	Flood Risk	143.663
M100D30.2	143.700	142.990	143.157	0.167	0.5	0.012	0.000	0.5	2.679	OK	143.157
RG2 ACO	144.978	143.658	144.223	0.565	2.1	0.090	0.000	2.1	12.410	OK	144.223
M150D20	144.986	144.721	144.765	0.044	0.8	0.013	0.000	0.8	4.642	Flood Risk	144.765
SW3	144.036	142.500	143.058	0.558	4.7	0.355	0.000	4.7	27.185	OK	143.058
SW1	145.432	144.232	144.249	0.017	1.3	0.011	0.000	1.3	7.351	OK	144.249
SW4	144.603	142.903	142.955	0.052	4.7	0.033	0.000	4.7	27.232	OK	142.955
SW2	144.721	143.363	143.399	0.035	3.4	0.022	0.000	3.4	19.756	OK	143.399

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.233
0.237
0.543
0.755
0.220
0.978
1.183
1.648
1.322

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 360 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.004	0.582	1.4	0.092	0.000	1.4	8.167	OK	145.004
RG3	144.217	142.897	143.462	0.565	0.9	0.090	0.000	0.9	5.295	OK	143.462
M100D30	144.370	144.110	144.140	0.030	0.9	0.002	0.000	0.9	5.153	Flood Risk	144.140
M100D30.1	143.900	143.640	143.664	0.024	0.5	0.002	0.000	0.5	2.999	Flood Risk	143.664
M100D30.2	143.700	142.990	143.158	0.168	0.5	0.013	0.000	0.5	2.997	OK	143.158
RG2 ACO	144.978	143.658	144.223	0.565	2.4	0.090	0.000	2.4	13.910	OK	144.223
M150D20	144.986	144.721	144.768	0.047	0.9	0.014	0.000	0.9	5.224	Flood Risk	144.768
SW3	144.036	142.500	143.062	0.562	5.3	0.357	0.000	5.3	30.384	OK	143.062
SW1	145.432	144.232	144.250	0.018	1.4	0.011	0.000	1.4	8.167	OK	144.250
SW4	144.603	142.903	142.959	0.056	5.3	0.035	0.000	5.3	30.427	OK	142.959
SW2	144.721	143.363	143.401	0.037	3.9	0.024	0.000	3.9	22.072	OK	143.401

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.633
0.755
0.230
0.236
0.542
0.755
0.218
0.974
1.182
1.645
1.320

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 360 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.999	0.577	0.9	0.092	0.000	0.9	8.185	OK	144.999
RG3	144.217	142.897	143.462	0.565	0.6	0.090	0.000	0.6	5.289	OK	143.462
M100D30	144.370	144.110	144.133	0.023	0.6	0.002	0.000	0.6	5.147	Flood Risk	144.133
M100D30.1	143.900	143.640	143.659	0.019	0.3	0.001	0.000	0.3	2.987	Flood Risk	143.659
M100D30.2	143.700	142.990	143.155	0.165	0.3	0.012	0.000	0.3	2.985	OK	143.155
RG2 ACO	144.978	143.658	144.223	0.565	1.6	0.090	0.000	1.6	13.892	OK	144.223
M150D20	144.986	144.721	144.760	0.039	0.6	0.012	0.000	0.6	5.218	Flood Risk	144.760
SW3	144.036	142.500	143.050	0.550	3.5	0.350	0.000	3.5	30.365	OK	143.050
SW1	145.432	144.232	144.247	0.015	0.9	0.009	0.000	0.9	8.185	OK	144.247
SW4	144.603	142.903	142.948	0.045	3.5	0.028	0.000	3.5	30.418	OK	142.948
SW2	144.721	143.363	143.394	0.030	2.6	0.019	0.000	2.6	22.072	OK	143.394

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.638
0.755
0.237
0.241
0.545
0.755
0.226
0.987
1.185
1.656
1.327

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 480 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.001	0.580	1.2	0.092	0.000	1.2	8.839	OK	145.001
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	5.667	OK	143.462
M100D30	144.370	144.110	144.136	0.026	0.8	0.002	0.000	0.8	5.525	Flood Risk	144.136
M100D30.1	143.900	143.640	143.662	0.022	0.4	0.002	0.000	0.4	3.233	Flood Risk	143.662
M100D30.2	143.700	142.990	143.156	0.166	0.4	0.012	0.000	0.4	3.231	OK	143.156
RG2 ACO	144.978	143.658	144.223	0.565	2.0	0.090	0.000	2.0	14.990	OK	144.223
M150D20	144.986	144.721	144.764	0.043	0.8	0.013	0.000	0.8	5.626	Flood Risk	144.764
SW3	144.036	142.500	143.056	0.556	4.4	0.354	0.000	4.4	32.742	OK	143.056
SW1	145.432	144.232	144.249	0.016	1.2	0.010	0.000	1.2	8.839	OK	144.249
SW4	144.603	142.903	142.953	0.050	4.4	0.032	0.000	4.4	32.785	OK	142.953
SW2	144.721	143.363	143.397	0.034	3.2	0.022	0.000	3.2	23.824	OK	143.397

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.234
0.238
0.544
0.755
0.222
0.980
1.184
1.650
1.323

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 480 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.997	0.576	0.8	0.092	0.000	0.8	8.809	OK	144.997
RG3	144.217	142.897	143.462	0.565	0.5	0.090	0.000	0.5	5.697	OK	143.462
M100D30	144.370	144.110	144.131	0.021	0.5	0.002	0.000	0.5	5.555	Flood Risk	144.131
M100D30.1	143.900	143.640	143.657	0.017	0.3	0.001	0.000	0.3	3.227	Flood Risk	143.657
M100D30.2	143.700	142.990	143.153	0.163	0.3	0.012	0.000	0.3	3.225	OK	143.153
RG2 ACO	144.978	143.658	144.223	0.565	1.3	0.090	0.000	1.3	14.966	OK	144.223
M150D20	144.986	144.721	144.756	0.035	0.5	0.011	0.000	0.5	5.638	Flood Risk	144.756
SW3	144.036	142.500	143.045	0.545	2.9	0.347	0.000	2.9	32.711	OK	143.045
SW1	145.432	144.232	144.246	0.013	0.8	0.009	0.000	0.8	8.809	OK	144.246
SW4	144.603	142.903	142.943	0.040	2.9	0.026	0.000	2.8	32.778	OK	142.943
SW2	144.721	143.363	143.391	0.028	2.1	0.018	0.000	2.1	23.770	OK	143.391

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.639
0.755
0.239
0.243
0.547
0.755
0.230
0.992
1.187
1.660
1.330

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 960 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.997	0.575	0.7	0.091	0.000	0.7	10.471	OK	144.997
RG3	144.217	142.897	143.462	0.565	0.4	0.090	0.000	0.4	6.699	OK	143.462
M100D30	144.370	144.110	144.130	0.020	0.4	0.001	0.000	0.4	6.557	Flood Risk	144.130
M100D30.1	143.900	143.640	143.656	0.016	0.3	0.001	0.000	0.3	3.809	Flood Risk	143.656
M100D30.2	143.700	142.990	143.153	0.163	0.3	0.012	0.000	0.3	3.807	OK	143.153
RG2 ACO	144.978	143.658	144.223	0.565	1.2	0.090	0.000	1.2	17.726	OK	144.223
M150D20	144.986	144.721	144.755	0.034	0.4	0.010	0.000	0.4	6.677	Flood Risk	144.755
SW3	144.036	142.500	143.043	0.543	2.6	0.345	0.000	2.6	38.718	OK	143.043
SW1	145.432	144.232	144.245	0.013	0.7	0.008	0.000	0.7	10.471	OK	144.245
SW4	144.603	142.903	142.942	0.038	2.6	0.024	0.000	2.6	38.766	OK	142.942
SW2	144.721	143.363	143.390	0.026	1.9	0.017	0.000	1.9	28.192	OK	143.390

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.640
0.755
0.240
0.244
0.547
0.755
0.231
0.993
1.187
1.662
1.331

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 960 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.994	0.572	0.5	0.091	0.000	0.5	10.453	OK	144.994
RG3	144.217	142.897	143.462	0.565	0.3	0.090	0.000	0.3	6.693	OK	143.462
M100D30	144.370	144.110	144.126	0.016	0.3	0.001	0.000	0.3	6.551	Flood Risk	144.126
M100D30.1	143.900	143.640	143.653	0.013	0.2	0.001	0.000	0.2	3.839	Flood Risk	143.653
M100D30.2	143.700	142.990	143.151	0.161	0.2	0.012	0.000	0.2	3.837	OK	143.151
RG2 ACO	144.978	143.658	144.223	0.565	0.8	0.090	0.000	0.8	17.684	OK	144.223
M150D20	144.986	144.721	144.749	0.028	0.3	0.008	0.000	0.3	6.670	Flood Risk	144.749
SW3	144.036	142.500	143.034	0.534	1.7	0.340	0.000	1.7	38.681	OK	143.034
SW1	145.432	144.232	144.243	0.010	0.5	0.007	0.000	0.5	10.453	OK	144.243
SW4	144.603	142.903	142.934	0.031	1.7	0.020	0.000	1.7	38.824	OK	142.934
SW2	144.721	143.363	143.385	0.021	1.2	0.014	0.000	1.2	28.132	OK	143.385

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.643
0.755
0.244
0.247
0.549
0.755
0.237
1.002
1.190
1.670
1.336

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 1440 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.994	0.573	0.5	0.091	0.000	0.5	11.635	OK	144.994
RG3	144.217	142.897	143.462	0.565	0.3	0.090	0.000	0.3	7.413	OK	143.462
M100D30	144.370	144.110	144.127	0.017	0.3	0.001	0.000	0.3	7.271	Flood Risk	144.127
M100D30.1	143.900	143.640	143.654	0.014	0.2	0.001	0.000	0.2	4.265	Flood Risk	143.654
M100D30.2	143.700	142.990	143.151	0.161	0.2	0.012	0.000	0.2	4.263	OK	143.151
RG2 ACO	144.978	143.658	144.223	0.565	0.9	0.090	0.000	0.9	19.592	OK	144.223
M150D20	144.986	144.721	144.750	0.029	0.3	0.009	0.000	0.3	7.379	Flood Risk	144.750
SW3	144.036	142.500	143.037	0.537	1.9	0.341	0.000	1.9	42.918	OK	143.037
SW1	145.432	144.232	144.244	0.011	0.5	0.007	0.000	0.5	11.635	OK	144.244
SW4	144.603	142.903	142.936	0.033	1.9	0.021	0.000	1.9	43.222	OK	142.936
SW2	144.721	143.363	143.386	0.023	1.4	0.015	0.000	1.4	31.222	OK	143.386

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.642
0.755
0.243
0.246
0.549
0.755
0.236
1.000
1.189
1.667
1.334

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 1440 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.992	0.570	0.3	0.091	0.000	0.3	11.509	OK	144.992
RG3	144.217	142.897	143.462	0.565	0.2	0.090	0.000	0.2	7.437	OK	143.462
M100D30	144.370	144.110	144.123	0.013	0.2	0.001	0.000	0.2	7.295	Flood Risk	144.123
M100D30.1	143.900	143.640	143.651	0.011	0.1	0.001	0.000	0.1	4.181	Flood Risk	143.651
M100D30.2	143.700	142.990	143.149	0.159	0.1	0.012	0.000	0.1	4.179	OK	143.149
RG2 ACO	144.978	143.658	144.223	0.565	0.6	0.090	0.000	0.6	19.562	OK	144.223
M150D20	144.986	144.721	144.745	0.024	0.2	0.007	0.000	0.2	7.426	Flood Risk	144.745
SW3	144.036	142.500	143.029	0.529	1.2	0.337	0.000	1.2	42.701	OK	143.029
SW1	145.432	144.232	144.241	0.009	0.3	0.006	0.000	0.3	11.509	OK	144.241
SW4	144.603	142.903	142.929	0.026	1.2	0.017	0.000	1.2	42.906	OK	142.929
SW2	144.721	143.363	143.382	0.018	0.9	0.012	0.000	0.9	31.066	OK	143.382

Freeboard (m)
0.645
0.755
0.247
0.249
0.551
0.755
0.240
1.007
1.191
1.674
1.339

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 15 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.052	0.630	11.2	0.100	0.000	11.1	4.944	OK	145.052
RG3	144.217	142.897	143.462	0.565	6.8	0.090	0.000	6.8	3.202	OK	143.462
M100D30	144.370	144.110	144.215	0.105	7.1	0.008	0.000	6.8	3.060	Flood Risk	144.215
M100D30.1	143.900	143.640	143.722	0.082	4.1	0.006	0.000	3.9	1.785	Flood Risk	143.722
M100D30.2	143.700	142.990	143.202	0.212	3.9	0.016	0.000	3.7	1.782	OK	143.202
RG2 ACO	144.978	143.658	144.223	0.565	18.7	0.090	0.000	18.6	8.340	OK	144.223
M150D20	144.986	144.721	144.851	0.130	7.2	0.039	0.000	7.0	3.108	Flood Risk	144.851
SW3	144.036	142.500	143.193	0.693	36.6	0.440	0.000	35.8	18.287	OK	143.193
SW1	145.432	144.232	144.280	0.048	11.1	0.030	0.000	11.1	4.942	OK	144.280
SW4	144.603	142.903	143.070	0.167	35.1	0.106	0.000	34.8	18.284	OK	143.070
SW2	144.721	143.363	143.544	0.181	29.7	0.115	0.000	26.2	13.273	Surcharged	143.544

Freeboard (m)
0.585
0.755
0.155
0.178
0.498
0.755
0.135
0.844
1.152
1.534
1.176

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 15 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.049	0.627	10.5	0.100	0.000	10.4	4.918	OK	145.049
RG3	144.217	142.897	143.462	0.565	6.4	0.090	0.000	6.4	3.205	OK	143.462
M100D30	144.370	144.110	144.211	0.101	6.6	0.008	0.000	6.4	3.063	Flood Risk	144.211
M100D30.1	143.900	143.640	143.719	0.079	3.9	0.006	0.000	3.7	1.782	Flood Risk	143.719
M100D30.2	143.700	142.990	143.198	0.208	3.7	0.016	0.000	3.6	1.779	OK	143.198
RG2 ACO	144.978	143.658	144.223	0.565	17.5	0.090	0.000	17.5	8.337	OK	144.223
M150D20	144.986	144.721	144.847	0.126	6.7	0.038	0.000	6.5	3.109	Flood Risk	144.847
SW3	144.036	142.500	143.188	0.688	35.1	0.437	0.000	34.7	18.259	OK	143.188
SW1	145.432	144.232	144.280	0.047	10.4	0.030	0.000	10.3	4.917	OK	144.280
SW4	144.603	142.903	143.067	0.163	34.2	0.104	0.000	33.9	18.256	OK	143.067
SW2	144.721	143.363	143.501	0.138	27.8	0.088	0.000	25.1	13.244	OK	143.501

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.588
0.755
0.159
0.181
0.502
0.755
0.139
0.849
1.153
1.537
1.219

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 30 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.037	0.615	7.6	0.098	0.000	7.6	6.746	OK	145.037
RG3	144.217	142.897	143.462	0.565	4.8	0.090	0.000	4.8	4.256	OK	143.462
M100D30	144.370	144.110	144.193	0.083	4.8	0.006	0.000	4.8	4.114	Flood Risk	144.193
M100D30.1	143.900	143.640	143.705	0.065	2.8	0.005	0.000	2.8	2.392	Flood Risk	143.705
M100D30.2	143.700	142.990	143.179	0.189	2.8	0.014	0.000	2.8	2.391	OK	143.179
RG2 ACO	144.978	143.658	144.223	0.565	12.7	0.090	0.000	12.7	11.140	OK	144.223
M150D20	144.986	144.721	144.829	0.108	4.8	0.032	0.000	4.8	4.170	Flood Risk	144.829
SW3	144.036	142.500	143.162	0.662	27.8	0.421	0.000	27.8	24.552	OK	143.162
SW1	145.432	144.232	144.273	0.040	7.6	0.025	0.000	7.6	6.746	OK	144.273
SW4	144.603	142.903	143.046	0.143	27.8	0.091	0.000	27.8	24.579	OK	143.046
SW2	144.721	143.363	143.462	0.099	20.3	0.063	0.000	20.3	17.878	OK	143.462

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.600
0.755
0.177
0.195
0.521
0.755
0.157
0.875
1.160
1.558
1.258

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 30 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.035	0.613	7.1	0.097	0.000	7.1	6.716	OK	145.035
RG3	144.217	142.897	143.462	0.565	4.4	0.090	0.000	4.4	4.250	OK	143.462
M100D30	144.370	144.110	144.190	0.080	4.4	0.006	0.000	4.4	4.108	Flood Risk	144.190
M100D30.1	143.900	143.640	143.703	0.063	2.6	0.005	0.000	2.6	2.398	Flood Risk	143.703
M100D30.2	143.700	142.990	143.177	0.187	2.6	0.014	0.000	2.6	2.397	OK	143.177
RG2 ACO	144.978	143.658	144.223	0.565	11.9	0.090	0.000	11.9	11.146	OK	144.223
M150D20	144.986	144.721	144.825	0.104	4.5	0.031	0.000	4.5	4.177	Flood Risk	144.825
SW3	144.036	142.500	143.155	0.655	26.1	0.417	0.000	26.1	24.528	OK	143.155
SW1	145.432	144.232	144.271	0.039	7.1	0.025	0.000	7.1	6.716	OK	144.271
SW4	144.603	142.903	143.040	0.137	26.1	0.087	0.000	26.1	24.557	OK	143.040
SW2	144.721	143.363	143.458	0.094	19.0	0.060	0.000	19.0	17.854	OK	143.458

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.602
0.755
0.180
0.197
0.523
0.755
0.160
0.881
1.161
1.563
1.263

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 60 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.033	0.611	6.8	0.097	0.000	6.8	8.670	OK	145.033
RG3	144.217	142.897	143.462	0.565	4.2	0.090	0.000	4.2	5.438	OK	143.462
M100D30	144.370	144.110	144.187	0.077	4.2	0.006	0.000	4.2	5.296	Flood Risk	144.187
M100D30.1	143.900	143.640	143.701	0.061	2.5	0.005	0.000	2.5	3.082	Flood Risk	143.701
M100D30.2	143.700	142.990	143.177	0.187	2.5	0.014	0.000	2.5	3.081	OK	143.177
RG2 ACO	144.978	143.658	144.223	0.565	11.4	0.090	0.000	11.4	14.322	OK	144.223
M150D20	144.986	144.721	144.823	0.102	4.3	0.031	0.000	4.3	5.385	Flood Risk	144.823
SW3	144.036	142.500	143.151	0.651	24.8	0.414	0.000	24.8	31.529	OK	143.151
SW1	145.432	144.232	144.270	0.038	6.8	0.024	0.000	6.8	8.670	OK	144.270
SW4	144.603	142.903	143.036	0.133	24.8	0.085	0.000	24.8	31.560	OK	143.036
SW2	144.721	143.363	143.454	0.091	18.1	0.058	0.000	18.1	22.985	OK	143.454

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.604
0.755
0.183
0.199
0.523
0.755
0.163
0.886
1.162
1.567
1.267

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 60 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.027	0.605	5.4	0.096	0.000	5.4	8.566	OK	145.027
RG3	144.217	142.897	143.462	0.565	3.4	0.090	0.000	3.4	5.439	OK	143.462
M100D30	144.370	144.110	144.177	0.067	3.4	0.005	0.000	3.4	5.297	Flood Risk	144.177
M100D30.1	143.900	143.640	143.693	0.053	2.0	0.004	0.000	2.0	3.088	Flood Risk	143.693
M100D30.2	143.700	142.990	143.173	0.183	2.0	0.014	0.000	2.0	3.087	OK	143.173
RG2 ACO	144.978	143.658	144.223	0.565	9.1	0.090	0.000	9.1	14.323	OK	144.223
M150D20	144.986	144.721	144.812	0.091	3.5	0.027	0.000	3.5	5.385	Flood Risk	144.812
SW3	144.036	142.500	143.132	0.632	20.0	0.402	0.000	20.0	31.431	OK	143.132
SW1	145.432	144.232	144.266	0.034	5.4	0.022	0.000	5.4	8.566	OK	144.266
SW4	144.603	142.903	143.019	0.116	20.0	0.074	0.000	20.0	31.463	OK	143.019
SW2	144.721	143.363	143.441	0.077	14.6	0.049	0.000	14.6	22.882	OK	143.441

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.610
0.755
0.193
0.207
0.527
0.755
0.174
0.905
1.166
1.584
1.280

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 120 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.025	0.603	4.9	0.096	0.000	4.9	10.770	OK	145.025
RG3	144.217	142.897	143.462	0.565	3.1	0.090	0.000	3.1	6.693	OK	143.462
M100D30	144.370	144.110	144.173	0.063	3.1	0.005	0.000	3.1	6.551	Flood Risk	144.173
M100D30.1	143.900	143.640	143.690	0.050	1.8	0.004	0.000	1.8	3.815	Flood Risk	143.690
M100D30.2	143.700	142.990	143.171	0.181	1.8	0.014	0.000	1.8	3.813	OK	143.171
RG2 ACO	144.978	143.658	144.223	0.565	8.3	0.090	0.000	8.3	17.690	OK	144.223
M150D20	144.986	144.721	144.808	0.087	3.2	0.026	0.000	3.2	6.658	Flood Risk	144.808
SW3	144.036	142.500	143.124	0.624	18.2	0.397	0.000	18.2	38.983	OK	143.124
SW1	145.432	144.232	144.265	0.032	4.9	0.021	0.000	4.9	10.770	OK	144.265
SW4	144.603	142.903	143.013	0.110	18.2	0.070	0.000	18.2	39.034	OK	143.013
SW2	144.721	143.363	143.436	0.072	13.3	0.046	0.000	13.3	28.453	OK	143.436

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.612
0.755
0.197
0.210
0.529
0.755
0.178
0.912
1.168
1.590
1.285

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 120 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.018	0.596	3.6	0.095	0.000	3.6	10.709	OK	145.018
RG3	144.217	142.897	143.462	0.565	2.2	0.090	0.000	2.2	6.705	OK	143.462
M100D30	144.370	144.110	144.161	0.051	2.2	0.004	0.000	2.2	6.563	Flood Risk	144.161
M100D30.1	143.900	143.640	143.681	0.041	1.3	0.003	0.000	1.3	3.821	Flood Risk	143.681
M100D30.2	143.700	142.990	143.167	0.177	1.3	0.013	0.000	1.3	3.819	OK	143.167
RG2 ACO	144.978	143.658	144.223	0.565	6.0	0.090	0.000	6.0	17.684	OK	144.223
M150D20	144.986	144.721	144.795	0.074	2.3	0.022	0.000	2.3	6.658	Flood Risk	144.795
SW3	144.036	142.500	143.102	0.602	13.1	0.383	0.000	13.1	38.932	OK	143.102
SW1	145.432	144.232	144.260	0.028	3.6	0.018	0.000	3.6	10.708	OK	144.260
SW4	144.603	142.903	142.994	0.091	13.1	0.058	0.000	13.1	38.984	OK	142.994
SW2	144.721	143.363	143.423	0.060	9.6	0.038	0.000	9.6	28.386	OK	143.423

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.619
0.755
0.209
0.219
0.533
0.755
0.191
0.934
1.172
1.609
1.297

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 240 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.016	0.594	3.3	0.094	0.000	3.3	13.379	OK	145.016
RG3	144.217	142.897	143.462	0.565	2.1	0.090	0.000	2.1	8.139	OK	143.462
M100D30	144.370	144.110	144.159	0.049	2.1	0.004	0.000	2.1	7.997	Flood Risk	144.159
M100D30.1	143.900	143.640	143.679	0.039	1.2	0.003	0.000	1.2	4.673	Flood Risk	143.679
M100D30.2	143.700	142.990	143.166	0.176	1.2	0.013	0.000	1.2	4.671	OK	143.166
RG2 ACO	144.978	143.658	144.223	0.565	5.5	0.090	0.000	5.5	21.518	OK	144.223
M150D20	144.986	144.721	144.792	0.071	2.1	0.021	0.000	2.1	8.104	Flood Risk	144.792
SW3	144.036	142.500	143.098	0.598	12.1	0.380	0.000	12.1	47.724	OK	143.098
SW1	145.432	144.232	144.259	0.027	3.3	0.017	0.000	3.3	13.379	OK	144.259
SW4	144.603	142.903	142.990	0.087	12.1	0.055	0.000	12.1	47.765	OK	142.990
SW2	144.721	143.363	143.421	0.057	8.8	0.037	0.000	8.8	34.892	OK	143.421

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.621
0.755
0.211
0.221
0.534
0.755
0.194
0.939
1.173
1.613
1.300

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 240 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.009	0.588	2.2	0.093	0.000	2.2	13.329	OK	145.009
RG3	144.217	142.897	143.462	0.565	1.4	0.090	0.000	1.4	8.133	OK	143.462
M100D30	144.370	144.110	144.148	0.038	1.4	0.003	0.000	1.4	7.991	Flood Risk	144.148
M100D30.1	143.900	143.640	143.671	0.031	0.8	0.002	0.000	0.8	4.643	Flood Risk	143.671
M100D30.2	143.700	142.990	143.161	0.171	0.8	0.013	0.000	0.8	4.641	OK	143.161
RG2 ACO	144.978	143.658	144.223	0.565	3.7	0.090	0.000	3.7	21.524	OK	144.223
M150D20	144.986	144.721	144.779	0.058	1.4	0.017	0.000	1.4	8.122	Flood Risk	144.779
SW3	144.036	142.500	143.078	0.578	8.1	0.368	0.000	8.1	47.641	OK	143.078
SW1	145.432	144.232	144.254	0.022	2.2	0.014	0.000	2.2	13.328	OK	144.254
SW4	144.603	142.903	142.973	0.070	8.1	0.045	0.000	8.1	47.689	OK	142.973
SW2	144.721	143.363	143.410	0.047	5.9	0.030	0.000	5.9	34.847	OK	143.410

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.627
0.755
0.222
0.229
0.539
0.755
0.207
0.958
1.178
1.630
1.311

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 360 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.011	0.589	2.5	0.094	0.000	2.5	14.858	OK	145.011
RG3	144.217	142.897	143.462	0.565	1.6	0.090	0.000	1.6	9.039	OK	143.462
M100D30	144.370	144.110	144.151	0.041	1.6	0.003	0.000	1.6	8.897	Flood Risk	144.151
M100D30.1	143.900	143.640	143.673	0.033	0.9	0.002	0.000	0.9	5.195	Flood Risk	143.673
M100D30.2	143.700	142.990	143.163	0.173	0.9	0.013	0.000	0.9	5.193	OK	143.163
RG2 ACO	144.978	143.658	144.223	0.565	4.2	0.090	0.000	4.2	23.936	OK	144.223
M150D20	144.986	144.721	144.783	0.062	1.6	0.019	0.000	1.6	9.046	Flood Risk	144.783
SW3	144.036	142.500	143.084	0.584	9.2	0.371	0.000	9.2	53.043	OK	143.084
SW1	145.432	144.232	144.256	0.023	2.5	0.015	0.000	2.5	14.857	OK	144.256
SW4	144.603	142.903	142.978	0.075	9.2	0.048	0.000	9.2	53.083	OK	142.978
SW2	144.721	143.363	143.413	0.050	6.7	0.032	0.000	6.7	38.788	OK	143.413

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.626
0.755
0.219
0.227
0.537
0.755
0.203
0.953
1.177
1.625
1.308

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 360 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.005	0.583	1.6	0.093	0.000	1.6	14.820	OK	145.005
RG3	144.217	142.897	143.462	0.565	1.0	0.090	0.000	1.0	9.015	OK	143.462
M100D30	144.370	144.110	144.142	0.032	1.0	0.002	0.000	1.0	8.873	Flood Risk	144.142
M100D30.1	143.900	143.640	143.666	0.026	0.6	0.002	0.000	0.6	5.165	Flood Risk	143.666
M100D30.2	143.700	142.990	143.158	0.168	0.6	0.013	0.000	0.6	5.163	OK	143.158
RG2 ACO	144.978	143.658	144.223	0.565	2.8	0.090	0.000	2.8	23.930	OK	144.223
M150D20	144.986	144.721	144.771	0.050	1.0	0.015	0.000	1.0	9.034	Flood Risk	144.771
SW3	144.036	142.500	143.067	0.567	6.1	0.360	0.000	6.1	52.942	OK	143.067
SW1	145.432	144.232	144.251	0.019	1.6	0.012	0.000	1.6	14.819	OK	144.251
SW4	144.603	142.903	142.963	0.060	6.1	0.038	0.000	6.1	52.987	OK	142.963
SW2	144.721	143.363	143.403	0.040	4.4	0.025	0.000	4.4	38.744	OK	143.403

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.632
0.755
0.228
0.234
0.542
0.755
0.214
0.970
1.181
1.641
1.317

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 480 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.008	0.586	2.1	0.093	0.000	2.1	15.831	OK	145.008
RG3	144.217	142.897	143.462	0.565	1.3	0.090	0.000	1.3	9.699	OK	143.462
M100D30	144.370	144.110	144.147	0.037	1.3	0.003	0.000	1.3	9.557	Flood Risk	144.147
M100D30.1	143.900	143.640	143.670	0.030	0.8	0.002	0.000	0.8	5.561	Flood Risk	143.670
M100D30.2	143.700	142.990	143.161	0.171	0.8	0.013	0.000	0.8	5.559	OK	143.161
RG2 ACO	144.978	143.658	144.223	0.565	3.5	0.090	0.000	3.5	25.742	OK	144.223
M150D20	144.986	144.721	144.777	0.056	1.3	0.017	0.000	1.3	9.718	Flood Risk	144.777
SW3	144.036	142.500	143.075	0.575	7.6	0.366	0.000	7.6	56.848	OK	143.075
SW1	145.432	144.232	144.254	0.021	2.1	0.013	0.000	2.1	15.831	OK	144.254
SW4	144.603	142.903	142.970	0.067	7.6	0.043	0.000	7.6	56.889	OK	142.970
SW2	144.721	143.363	143.408	0.045	5.5	0.029	0.000	5.5	41.568	OK	143.408

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.629
0.755
0.223
0.230
0.539
0.755
0.208
0.961
1.179
1.633
1.312

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 480 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.003	0.581	1.3	0.092	0.000	1.3	15.786	OK	145.003
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	9.705	OK	143.462
M100D30	144.370	144.110	144.138	0.028	0.8	0.002	0.000	0.8	9.563	Flood Risk	144.138
M100D30.1	143.900	143.640	143.663	0.023	0.5	0.002	0.000	0.5	5.603	Flood Risk	143.663
M100D30.2	143.700	142.990	143.157	0.167	0.5	0.013	0.000	0.5	5.601	OK	143.157
RG2 ACO	144.978	143.658	144.223	0.565	2.2	0.090	0.000	2.2	25.730	OK	144.223
M150D20	144.986	144.721	144.767	0.046	0.9	0.014	0.000	0.8	9.712	Flood Risk	144.767
SW3	144.036	142.500	143.059	0.559	4.9	0.356	0.000	4.9	56.836	OK	143.059
SW1	145.432	144.232	144.250	0.017	1.3	0.011	0.000	1.3	15.785	OK	144.250
SW4	144.603	142.903	142.956	0.053	4.9	0.034	0.000	4.9	56.888	OK	142.956
SW2	144.721	143.363	143.399	0.036	3.6	0.023	0.000	3.6	41.510	OK	143.399

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.634
0.755
0.232
0.237
0.543
0.755
0.219
0.977
1.183
1.647
1.321

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 960 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.002	0.580	1.2	0.092	0.000	1.2	18.227	OK	145.002
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	11.445	OK	143.462
M100D30	144.370	144.110	144.137	0.027	0.8	0.002	0.000	0.8	11.303	Flood Risk	144.137
M100D30.1	143.900	143.640	143.662	0.022	0.4	0.002	0.000	0.4	6.587	Flood Risk	143.662
M100D30.2	143.700	142.990	143.156	0.166	0.4	0.012	0.000	0.5	6.585	OK	143.156
RG2 ACO	144.978	143.658	144.223	0.565	2.1	0.090	0.000	2.1	30.446	OK	144.223
M150D20	144.986	144.721	144.765	0.044	0.8	0.013	0.000	0.8	11.524	Flood Risk	144.765
SW3	144.036	142.500	143.057	0.557	4.5	0.354	0.000	4.5	66.718	OK	143.057
SW1	145.432	144.232	144.249	0.017	1.2	0.011	0.000	1.2	18.227	OK	144.249
SW4	144.603	142.903	142.954	0.051	4.5	0.032	0.000	4.5	66.762	OK	142.954
SW2	144.721	143.363	143.398	0.035	3.3	0.022	0.000	3.3	48.668	OK	143.398

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.233
0.238
0.544
0.755
0.221
0.980
1.183
1.649
1.323

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 960 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.998	0.576	0.8	0.092	0.000	0.8	18.037	OK	144.998
RG3	144.217	142.897	143.462	0.565	0.5	0.090	0.000	0.5	11.481	OK	143.462
M100D30	144.370	144.110	144.131	0.021	0.5	0.002	0.000	0.5	11.339	Flood Risk	144.131
M100D30.1	143.900	143.640	143.657	0.017	0.3	0.001	0.000	0.3	6.596	Flood Risk	143.657
M100D30.2	143.700	142.990	143.153	0.163	0.3	0.012	0.000	0.3	6.594	OK	143.153
RG2 ACO	144.978	143.658	144.223	0.565	1.3	0.090	0.000	1.3	30.488	OK	144.223
M150D20	144.986	144.721	144.757	0.036	0.5	0.011	0.000	0.5	11.530	Flood Risk	144.757
SW3	144.036	142.500	143.045	0.545	2.9	0.347	0.000	2.9	66.614	OK	143.045
SW1	145.432	144.232	144.246	0.013	0.8	0.009	0.000	0.8	18.037	OK	144.246
SW4	144.603	142.903	142.944	0.041	2.9	0.026	0.000	2.9	66.712	OK	142.944
SW2	144.721	143.363	143.391	0.028	2.1	0.018	0.000	2.1	48.520	OK	143.391

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.639
0.755
0.239
0.243
0.547
0.755
0.229
0.991
1.187
1.660
1.329

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 1440 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.999	0.577	0.9	0.092	0.000	0.9	19.807	OK	144.999
RG3	144.217	142.897	143.462	0.565	0.6	0.090	0.000	0.6	12.609	OK	143.462
M100D30	144.370	144.110	144.133	0.023	0.6	0.002	0.000	0.6	12.467	Flood Risk	144.133
M100D30.1	143.900	143.640	143.659	0.019	0.3	0.001	0.000	0.3	7.289	Flood Risk	143.659
M100D30.2	143.700	142.990	143.154	0.164	0.3	0.012	0.000	0.3	7.287	OK	143.154
RG2 ACO	144.978	143.658	144.223	0.565	1.5	0.090	0.000	1.5	33.536	OK	144.223
M150D20	144.986	144.721	144.759	0.038	0.6	0.011	0.000	0.6	12.665	Flood Risk	144.759
SW3	144.036	142.500	143.049	0.549	3.3	0.349	0.000	3.3	73.254	OK	143.049
SW1	145.432	144.232	144.247	0.014	0.9	0.009	0.000	0.9	19.807	OK	144.247
SW4	144.603	142.903	142.947	0.044	3.3	0.028	0.000	3.3	73.299	OK	142.947
SW2	144.721	143.363	143.393	0.030	2.4	0.019	0.000	2.4	53.338	OK	143.393

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.638
0.755
0.237
0.241
0.546
0.755
0.227
0.988
1.186
1.657
1.328

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 1440 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.995	0.573	0.6	0.091	0.000	0.6	19.963	OK	144.995
RG3	144.217	142.897	143.462	0.565	0.4	0.090	0.000	0.4	12.561	OK	143.462
M100D30	144.370	144.110	144.128	0.018	0.4	0.001	0.000	0.4	12.419	Flood Risk	144.128
M100D30.1	143.900	143.640	143.655	0.015	0.2	0.001	0.000	0.2	7.301	Flood Risk	143.655
M100D30.2	143.700	142.990	143.152	0.162	0.2	0.012	0.000	0.2	7.299	OK	143.152
RG2 ACO	144.978	143.658	144.223	0.565	1.0	0.090	0.000	1.0	33.698	OK	144.223
M150D20	144.986	144.721	144.752	0.031	0.4	0.009	0.000	0.4	12.730	Flood Risk	144.752
SW3	144.036	142.500	143.039	0.539	2.1	0.343	0.000	2.1	73.535	OK	143.039
SW1	145.432	144.232	144.244	0.012	0.6	0.007	0.000	0.6	19.963	OK	144.244
SW4	144.603	142.903	142.938	0.035	2.1	0.022	0.000	2.1	73.688	OK	142.938
SW2	144.721	143.363	143.387	0.024	1.6	0.015	0.000	1.6	53.656	OK	143.387

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.642
0.755
0.242
0.245
0.548
0.755
0.234
0.998
1.188
1.666
1.333

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 15 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.047	0.625	10.0	0.099	0.000	9.9	4.369	OK	145.047
RG3	144.217	142.897	143.462	0.565	6.1	0.090	0.000	6.0	2.878	OK	143.462
M100D30	144.370	144.110	144.208	0.098	6.3	0.007	0.000	6.1	2.736	Flood Risk	144.208
M100D30.1	143.900	143.640	143.717	0.077	3.7	0.006	0.000	3.5	1.596	Flood Risk	143.717
M100D30.2	143.700	142.990	143.195	0.205	3.5	0.015	0.000	3.2	1.593	OK	143.195
RG2 ACO	144.978	143.658	144.223	0.565	16.7	0.090	0.000	16.6	7.470	OK	144.223
M150D20	144.986	144.721	144.844	0.123	6.4	0.037	0.000	6.2	2.779	Flood Risk	144.844
SW3	144.036	142.500	143.185	0.685	34.1	0.436	0.000	33.9	16.328	OK	143.185
SW1	145.432	144.232	144.278	0.046	9.9	0.029	0.000	9.8	4.368	OK	144.278
SW4	144.603	142.903	143.063	0.160	33.1	0.101	0.000	32.6	16.325	OK	143.063
SW2	144.721	143.363	143.483	0.119	26.4	0.076	0.000	24.9	11.827	OK	143.483

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.590
0.755
0.162
0.183
0.505
0.755
0.142
0.852
1.154
1.541
1.238

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 15 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.045	0.623	9.4	0.099	0.000	9.3	4.365	OK	145.045
RG3	144.217	142.897	143.462	0.565	5.7	0.090	0.000	5.7	2.881	OK	143.462
M100D30	144.370	144.110	144.204	0.094	5.9	0.007	0.000	5.7	2.739	Flood Risk	144.204
M100D30.1	143.900	143.640	143.714	0.074	3.5	0.006	0.000	3.3	1.593	Flood Risk	143.714
M100D30.2	143.700	142.990	143.189	0.199	3.3	0.015	0.000	3.0	1.591	OK	143.189
RG2 ACO	144.978	143.658	144.223	0.565	15.7	0.090	0.000	15.6	7.476	OK	144.223
M150D20	144.986	144.721	144.840	0.119	6.0	0.036	0.000	5.9	2.782	Flood Risk	144.840
SW3	144.036	142.500	143.179	0.679	33.0	0.432	0.000	32.2	16.331	OK	143.179
SW1	145.432	144.232	144.277	0.044	9.3	0.028	0.000	9.2	4.365	OK	144.277
SW4	144.603	142.903	143.057	0.154	31.4	0.098	0.000	30.9	16.328	OK	143.057
SW2	144.721	143.363	143.476	0.112	24.8	0.071	0.000	24.4	11.830	OK	143.476

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.592
0.755
0.166
0.186
0.511
0.755
0.146
0.858
1.156
1.547
1.245

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 30 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.034	0.612	6.8	0.097	0.000	6.8	5.956	OK	145.034
RG3	144.217	142.897	143.462	0.565	4.3	0.090	0.000	4.3	3.872	OK	143.462
M100D30	144.370	144.110	144.188	0.078	4.3	0.006	0.000	4.3	3.730	Flood Risk	144.188
M100D30.1	143.900	143.640	143.702	0.062	2.5	0.005	0.000	2.5	2.170	Flood Risk	143.702
M100D30.2	143.700	142.990	143.177	0.187	2.5	0.014	0.000	2.5	2.169	OK	143.177
RG2 ACO	144.978	143.658	144.223	0.565	11.5	0.090	0.000	11.5	10.126	OK	144.223
M150D20	144.986	144.721	144.824	0.103	4.4	0.031	0.000	4.4	3.793	Flood Risk	144.824
SW3	144.036	142.500	143.152	0.652	25.2	0.415	0.000	25.2	22.142	OK	143.152
SW1	145.432	144.232	144.271	0.038	6.8	0.024	0.000	6.8	5.956	OK	144.271
SW4	144.603	142.903	143.037	0.134	25.2	0.085	0.000	25.2	22.171	OK	143.037
SW2	144.721	143.363	143.455	0.092	18.4	0.058	0.000	18.4	16.073	OK	143.455

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.603
0.755
0.182
0.198
0.523
0.755
0.162
0.884
1.162
1.566
1.265

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 30 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.032	0.610	6.4	0.097	0.000	6.4	5.941	OK	145.032
RG3	144.217	142.897	143.462	0.565	4.0	0.090	0.000	4.0	3.866	OK	143.462
M100D30	144.370	144.110	144.185	0.075	4.0	0.006	0.000	4.0	3.724	Flood Risk	144.185
M100D30.1	143.900	143.640	143.699	0.059	2.4	0.004	0.000	2.4	2.176	Flood Risk	143.699
M100D30.2	143.700	142.990	143.176	0.186	2.4	0.014	0.000	2.4	2.175	OK	143.176
RG2 ACO	144.978	143.658	144.223	0.565	10.8	0.090	0.000	10.8	10.120	OK	144.223
M150D20	144.986	144.721	144.820	0.099	4.1	0.030	0.000	4.1	3.787	Flood Risk	144.820
SW3	144.036	142.500	143.146	0.646	23.6	0.411	0.000	23.6	22.121	OK	143.146
SW1	145.432	144.232	144.269	0.037	6.4	0.024	0.000	6.4	5.940	OK	144.269
SW4	144.603	142.903	143.032	0.129	23.6	0.082	0.000	23.6	22.147	OK	143.032
SW2	144.721	143.363	143.451	0.087	17.3	0.056	0.000	17.3	16.053	OK	143.451

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.605
0.755
0.185
0.201
0.524
0.755
0.165
0.890
1.163
1.571
1.270

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 60 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.031	0.609	6.2	0.097	0.000	6.2	7.747	OK	145.031
RG3	144.217	142.897	143.462	0.565	3.9	0.090	0.000	3.9	4.989	OK	143.462
M100D30	144.370	144.110	144.183	0.073	3.9	0.005	0.000	3.9	4.846	Flood Risk	144.183
M100D30.1	143.900	143.640	143.698	0.058	2.3	0.004	0.000	2.3	2.824	Flood Risk	143.698
M100D30.2	143.700	142.990	143.175	0.185	2.3	0.014	0.000	2.3	2.823	OK	143.175
RG2 ACO	144.978	143.658	144.223	0.565	10.4	0.090	0.000	10.4	13.093	OK	144.223
M150D20	144.986	144.721	144.818	0.097	3.9	0.029	0.000	3.9	4.917	Flood Risk	144.818
SW3	144.036	142.500	143.143	0.643	22.7	0.409	0.000	22.7	28.668	OK	143.143
SW1	145.432	144.232	144.269	0.036	6.2	0.023	0.000	6.2	7.747	OK	144.269
SW4	144.603	142.903	143.029	0.126	22.7	0.080	0.000	22.7	28.702	OK	143.029
SW2	144.721	143.363	143.448	0.085	16.5	0.054	0.000	16.5	20.832	OK	143.448

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.606
0.755
0.187
0.202
0.525
0.755
0.168
0.894
1.164
1.575
1.273

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 60 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.025	0.603	5.0	0.096	0.000	5.0	7.711	OK	145.025
RG3	144.217	142.897	143.462	0.565	3.1	0.090	0.000	3.1	4.989	OK	143.462
M100D30	144.370	144.110	144.174	0.064	3.1	0.005	0.000	3.1	4.847	Flood Risk	144.174
M100D30.1	143.900	143.640	143.690	0.050	1.8	0.004	0.000	1.8	2.825	Flood Risk	143.690
M100D30.2	143.700	142.990	143.171	0.181	1.8	0.014	0.000	1.8	2.823	OK	143.171
RG2 ACO	144.978	143.658	144.223	0.565	8.3	0.090	0.000	8.3	13.111	OK	144.223
M150D20	144.986	144.721	144.808	0.087	3.2	0.026	0.000	3.2	4.923	Flood Risk	144.808
SW3	144.036	142.500	143.125	0.625	18.2	0.397	0.000	18.2	28.649	OK	143.125
SW1	145.432	144.232	144.265	0.032	5.0	0.021	0.000	5.0	7.711	OK	144.265
SW4	144.603	142.903	143.013	0.110	18.2	0.070	0.000	18.2	28.680	OK	143.013
SW2	144.721	143.363	143.436	0.072	13.3	0.046	0.000	13.3	20.815	OK	143.436

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.612
0.755
0.196
0.210
0.529
0.755
0.178
0.912
1.168
1.590
1.285

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 120 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.022	0.600	4.4	0.095	0.000	4.4	9.307	OK	145.022
RG3	144.217	142.897	143.462	0.565	2.8	0.090	0.000	2.8	6.003	OK	143.462
M100D30	144.370	144.110	144.169	0.059	2.8	0.004	0.000	2.8	5.861	Flood Risk	144.169
M100D30.1	143.900	143.640	143.687	0.047	1.6	0.004	0.000	1.6	3.413	Flood Risk	143.687
M100D30.2	143.700	142.990	143.170	0.180	1.6	0.013	0.000	1.6	3.411	OK	143.170
RG2 ACO	144.978	143.658	144.223	0.565	7.4	0.090	0.000	7.4	15.806	OK	144.223
M150D20	144.986	144.721	144.803	0.082	2.8	0.025	0.000	2.8	5.950	Flood Risk	144.803
SW3	144.036	142.500	143.116	0.616	16.3	0.392	0.000	16.3	34.544	OK	143.116
SW1	145.432	144.232	144.263	0.031	4.4	0.019	0.000	4.4	9.307	OK	144.263
SW4	144.603	142.903	143.006	0.103	16.3	0.066	0.000	16.3	34.585	OK	143.006
SW2	144.721	143.363	143.431	0.068	11.9	0.043	0.000	11.9	25.107	OK	143.431

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.615
0.755
0.201
0.213
0.530
0.755
0.183
0.920
1.169
1.597
1.290

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 120 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.015	0.594	3.2	0.094	0.000	3.2	9.325	OK	145.015
RG3	144.217	142.897	143.462	0.565	2.0	0.090	0.000	2.0	6.003	OK	143.462
M100D30	144.370	144.110	144.158	0.048	2.0	0.004	0.000	2.0	5.861	Flood Risk	144.158
M100D30.1	143.900	143.640	143.678	0.038	1.2	0.003	0.000	1.2	3.413	Flood Risk	143.678
M100D30.2	143.700	142.990	143.165	0.175	1.2	0.013	0.000	1.2	3.411	OK	143.165
RG2 ACO	144.978	143.658	144.223	0.565	5.4	0.090	0.000	5.4	15.794	OK	144.223
M150D20	144.986	144.721	144.791	0.070	2.0	0.021	0.000	2.0	5.950	Flood Risk	144.791
SW3	144.036	142.500	143.096	0.596	11.7	0.379	0.000	11.7	34.548	OK	143.096
SW1	145.432	144.232	144.259	0.026	3.2	0.017	0.000	3.2	9.325	OK	144.259
SW4	144.603	142.903	142.989	0.086	11.7	0.054	0.000	11.7	34.585	OK	142.989
SW2	144.721	143.363	143.420	0.056	8.5	0.036	0.000	8.5	25.113	OK	143.420

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.621
0.755
0.212
0.222
0.535
0.755
0.195
0.941
1.174
1.615
1.301

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 240 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.014	0.592	2.9	0.094	0.000	2.9	11.403	OK	145.014
RG3	144.217	142.897	143.462	0.565	1.8	0.090	0.000	1.8	7.191	OK	143.462
M100D30	144.370	144.110	144.155	0.045	1.8	0.003	0.000	1.8	7.049	Flood Risk	144.155
M100D30.1	143.900	143.640	143.676	0.036	1.1	0.003	0.000	1.1	4.115	Flood Risk	143.676
M100D30.2	143.700	142.990	143.164	0.174	1.1	0.013	0.000	1.1	4.113	OK	143.164
RG2 ACO	144.978	143.658	144.223	0.565	4.9	0.090	0.000	4.9	19.046	OK	144.223
M150D20	144.986	144.721	144.788	0.067	1.9	0.020	0.000	1.9	7.174	Flood Risk	144.788
SW3	144.036	142.500	143.091	0.591	10.7	0.376	0.000	10.7	41.770	OK	143.091
SW1	145.432	144.232	144.257	0.025	2.9	0.016	0.000	2.9	11.403	OK	144.257
SW4	144.603	142.903	142.984	0.081	10.7	0.052	0.000	10.7	41.814	OK	142.984
SW2	144.721	143.363	143.417	0.054	7.8	0.034	0.000	7.8	30.444	OK	143.417

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.623
0.755
0.215
0.224
0.536
0.755
0.198
0.946
1.175
1.619
1.303

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 240 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.008	0.586	2.0	0.093	0.000	2.0	11.357	OK	145.008
RG3	144.217	142.897	143.462	0.565	1.2	0.090	0.000	1.2	7.203	OK	143.462
M100D30	144.370	144.110	144.145	0.035	1.2	0.003	0.000	1.2	7.061	Flood Risk	144.145
M100D30.1	143.900	143.640	143.669	0.029	0.7	0.002	0.000	0.7	4.127	Flood Risk	143.669
M100D30.2	143.700	142.990	143.160	0.170	0.7	0.013	0.000	0.7	4.125	OK	143.160
RG2 ACO	144.978	143.658	144.223	0.565	3.3	0.090	0.000	3.3	19.040	OK	144.223
M150D20	144.986	144.721	144.776	0.055	1.3	0.016	0.000	1.3	7.180	Flood Risk	144.776
SW3	144.036	142.500	143.073	0.573	7.2	0.364	0.000	7.2	41.740	OK	143.073
SW1	145.432	144.232	144.253	0.021	2.0	0.013	0.000	2.0	11.357	OK	144.253
SW4	144.603	142.903	142.969	0.066	7.2	0.042	0.000	7.2	41.785	OK	142.969
SW2	144.721	143.363	143.407	0.044	5.3	0.028	0.000	5.3	30.392	OK	143.407

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.629
0.755
0.225
0.231
0.540
0.755
0.210
0.963
1.179
1.635
1.314

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 360 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.009	0.588	2.2	0.093	0.000	2.2	12.711	OK	145.009
RG3	144.217	142.897	143.462	0.565	1.4	0.090	0.000	1.4	8.019	OK	143.462
M100D30	144.370	144.110	144.148	0.038	1.4	0.003	0.000	1.4	7.877	Flood Risk	144.148
M100D30.1	143.900	143.640	143.671	0.031	0.8	0.002	0.000	0.8	4.595	Flood Risk	143.671
M100D30.2	143.700	142.990	143.161	0.171	0.8	0.013	0.000	0.8	4.593	OK	143.161
RG2 ACO	144.978	143.658	144.223	0.565	3.7	0.090	0.000	3.7	21.194	OK	144.223
M150D20	144.986	144.721	144.780	0.059	1.4	0.018	0.000	1.4	7.990	Flood Risk	144.780
SW3	144.036	142.500	143.078	0.578	8.2	0.368	0.000	8.2	46.533	OK	143.078
SW1	145.432	144.232	144.254	0.022	2.2	0.014	0.000	2.2	12.711	OK	144.254
SW4	144.603	142.903	142.973	0.070	8.2	0.045	0.000	8.2	46.574	OK	142.973
SW2	144.721	143.363	143.410	0.047	6.0	0.030	0.000	6.0	33.900	OK	143.410

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.627
0.755
0.222
0.229
0.539
0.755
0.206
0.958
1.178
1.630
1.311

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 360 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.004	0.582	1.5	0.093	0.000	1.5	12.668	OK	145.004
RG3	144.217	142.897	143.462	0.565	0.9	0.090	0.000	0.9	8.013	OK	143.462
M100D30	144.370	144.110	144.140	0.030	0.9	0.002	0.000	0.9	7.871	Flood Risk	144.140
M100D30.1	143.900	143.640	143.664	0.024	0.5	0.002	0.000	0.5	4.577	Flood Risk	143.664
M100D30.2	143.700	142.990	143.158	0.168	0.5	0.013	0.000	0.5	4.575	OK	143.158
RG2 ACO	144.978	143.658	144.223	0.565	2.5	0.090	0.000	2.5	21.212	OK	144.223
M150D20	144.986	144.721	144.769	0.048	0.9	0.014	0.000	0.9	7.996	Flood Risk	144.769
SW3	144.036	142.500	143.062	0.562	5.4	0.358	0.000	5.4	46.482	OK	143.062
SW1	145.432	144.232	144.250	0.018	1.5	0.011	0.000	1.5	12.668	OK	144.250
SW4	144.603	142.903	142.959	0.056	5.4	0.036	0.000	5.4	46.528	OK	142.959
SW2	144.721	143.363	143.401	0.038	3.9	0.024	0.000	3.9	33.874	OK	143.401

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.633
0.755
0.230
0.236
0.542
0.755
0.217
0.974
1.182
1.644
1.320

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 480 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.007	0.585	1.8	0.093	0.000	1.8	13.684	OK	145.007
RG3	144.217	142.897	143.462	0.565	1.2	0.090	0.000	1.2	8.661	OK	143.462
M100D30	144.370	144.110	144.144	0.034	1.2	0.003	0.000	1.2	8.519	Flood Risk	144.144
M100D30.1	143.900	143.640	143.667	0.027	0.7	0.002	0.000	0.7	4.961	Flood Risk	143.667
M100D30.2	143.700	142.990	143.159	0.169	0.7	0.013	0.000	0.7	4.959	OK	143.159
RG2 ACO	144.978	143.658	144.223	0.565	3.1	0.090	0.000	3.1	22.946	OK	144.223
M150D20	144.986	144.721	144.774	0.053	1.2	0.016	0.000	1.2	8.668	Flood Risk	144.774
SW3	144.036	142.500	143.071	0.571	6.8	0.363	0.000	6.8	50.265	OK	143.071
SW1	145.432	144.232	144.252	0.020	1.8	0.013	0.000	1.8	13.683	OK	144.252
SW4	144.603	142.903	142.966	0.063	6.8	0.040	0.000	6.8	50.311	OK	142.966
SW2	144.721	143.363	143.406	0.042	4.9	0.027	0.000	4.9	36.624	OK	143.406

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.630
0.755
0.226
0.233
0.541
0.755
0.212
0.966
1.180
1.637
1.315

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 480 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.001	0.580	1.2	0.092	0.000	1.2	13.585	OK	145.001
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	8.649	OK	143.462
M100D30	144.370	144.110	144.136	0.026	0.8	0.002	0.000	0.8	8.507	Flood Risk	144.136
M100D30.1	143.900	143.640	143.662	0.022	0.4	0.002	0.000	0.4	4.973	Flood Risk	143.662
M100D30.2	143.700	142.990	143.156	0.166	0.4	0.012	0.000	0.4	4.971	OK	143.156
RG2 ACO	144.978	143.658	144.223	0.565	2.0	0.090	0.000	2.0	22.928	OK	144.223
M150D20	144.986	144.721	144.764	0.043	0.8	0.013	0.000	0.8	8.632	Flood Risk	144.764
SW3	144.036	142.500	143.056	0.556	4.4	0.354	0.000	4.4	50.147	OK	143.056
SW1	145.432	144.232	144.249	0.016	1.2	0.010	0.000	1.2	13.585	OK	144.249
SW4	144.603	142.903	142.953	0.050	4.4	0.032	0.000	4.4	50.204	OK	142.953
SW2	144.721	143.363	143.397	0.034	3.2	0.022	0.000	3.2	36.508	OK	143.397

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.234
0.238
0.544
0.755
0.222
0.980
1.184
1.650
1.323

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 960 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.001	0.579	1.1	0.092	0.000	1.1	16.513	OK	145.001
RG3	144.217	142.897	143.462	0.565	0.7	0.090	0.000	0.7	10.461	OK	143.462
M100D30	144.370	144.110	144.135	0.025	0.7	0.002	0.000	0.7	10.319	Flood Risk	144.135
M100D30.1	143.900	143.640	143.661	0.021	0.4	0.002	0.000	0.4	6.017	Flood Risk	143.661
M100D30.2	143.700	142.990	143.156	0.166	0.4	0.012	0.000	0.4	6.015	OK	143.156
RG2 ACO	144.978	143.658	144.223	0.565	1.9	0.090	0.000	1.9	27.788	OK	144.223
M150D20	144.986	144.721	144.763	0.042	0.7	0.013	0.000	0.7	10.505	Flood Risk	144.763
SW3	144.036	142.500	143.054	0.554	4.1	0.352	0.000	4.1	60.792	OK	143.054
SW1	145.432	144.232	144.248	0.016	1.1	0.010	0.000	1.1	16.513	OK	144.248
SW4	144.603	142.903	142.952	0.049	4.1	0.031	0.000	4.1	60.839	OK	142.952
SW2	144.721	143.363	143.396	0.033	3.0	0.021	0.000	3.0	44.296	OK	143.396

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.636
0.755
0.235
0.239
0.544
0.755
0.223
0.982
1.184
1.652
1.324

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 960 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.997	0.575	0.7	0.091	0.000	0.7	16.471	OK	144.997
RG3	144.217	142.897	143.462	0.565	0.4	0.090	0.000	0.4	10.491	OK	143.462
M100D30	144.370	144.110	144.130	0.020	0.4	0.001	0.000	0.4	10.349	Flood Risk	144.130
M100D30.1	143.900	143.640	143.656	0.016	0.3	0.001	0.000	0.3	6.047	Flood Risk	143.656
M100D30.2	143.700	142.990	143.153	0.163	0.3	0.012	0.000	0.3	6.045	OK	143.153
RG2 ACO	144.978	143.658	144.223	0.565	1.2	0.090	0.000	1.2	27.776	OK	144.223
M150D20	144.986	144.721	144.755	0.034	0.5	0.010	0.000	0.5	10.498	Flood Risk	144.755
SW3	144.036	142.500	143.043	0.543	2.6	0.345	0.000	2.6	60.797	OK	143.043
SW1	145.432	144.232	144.245	0.013	0.7	0.008	0.000	0.7	16.471	OK	144.245
SW4	144.603	142.903	142.942	0.039	2.6	0.025	0.000	2.6	60.900	OK	142.942
SW2	144.721	143.363	143.390	0.027	1.9	0.017	0.000	1.9	44.242	OK	143.390

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.640
0.755
0.240
0.244
0.547
0.755
0.231
0.993
1.187
1.662
1.331

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 1440 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.998	0.576	0.8	0.092	0.000	0.8	18.289	OK	144.998
RG3	144.217	142.897	143.462	0.565	0.5	0.090	0.000	0.5	11.655	OK	143.462
M100D30	144.370	144.110	144.132	0.022	0.5	0.002	0.000	0.5	11.513	Flood Risk	144.132
M100D30.1	143.900	143.640	143.658	0.018	0.3	0.001	0.000	0.3	6.761	Flood Risk	143.658
M100D30.2	143.700	142.990	143.154	0.164	0.3	0.012	0.000	0.3	6.759	OK	143.154
RG2 ACO	144.978	143.658	144.223	0.565	1.4	0.090	0.000	1.4	31.034	OK	144.223
M150D20	144.986	144.721	144.757	0.036	0.5	0.011	0.000	0.5	11.735	Flood Risk	144.757
SW3	144.036	142.500	143.046	0.546	3.0	0.348	0.000	3.0	67.752	OK	143.046
SW1	145.432	144.232	144.246	0.014	0.8	0.009	0.000	0.8	18.289	OK	144.246
SW4	144.603	142.903	142.945	0.042	3.0	0.026	0.000	3.0	67.802	OK	142.945
SW2	144.721	143.363	143.392	0.028	2.2	0.018	0.000	2.2	49.318	OK	143.392

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.639
0.755
0.238
0.242
0.546
0.755
0.229
0.990
1.186
1.659
1.329

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 1440 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.995	0.573	0.5	0.091	0.000	0.5	18.409	OK	144.995
RG3	144.217	142.897	143.462	0.565	0.3	0.090	0.000	0.3	11.685	OK	143.462
M100D30	144.370	144.110	144.127	0.017	0.3	0.001	0.000	0.3	11.543	Flood Risk	144.127
M100D30.1	143.900	143.640	143.654	0.014	0.2	0.001	0.000	0.2	6.671	Flood Risk	143.654
M100D30.2	143.700	142.990	143.151	0.161	0.2	0.012	0.000	0.2	6.669	OK	143.151
RG2 ACO	144.978	143.658	144.223	0.565	0.9	0.090	0.000	0.9	31.010	OK	144.223
M150D20	144.986	144.721	144.751	0.030	0.3	0.009	0.000	0.3	11.692	Flood Risk	144.751
SW3	144.036	142.500	143.037	0.537	2.0	0.342	0.000	2.0	67.787	OK	143.037
SW1	145.432	144.232	144.244	0.011	0.5	0.007	0.000	0.5	18.409	OK	144.244
SW4	144.603	142.903	142.936	0.033	2.0	0.021	0.000	2.0	67.942	OK	142.936
SW2	144.721	143.363	143.386	0.023	1.4	0.015	0.000	1.4	49.414	OK	143.386

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.642
0.755
0.243
0.246
0.549
0.755
0.235
0.999
1.189
1.667
1.334

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 15 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.065	0.643	14.6	0.102	0.000	14.4	6.882	OK	145.065
RG3	144.217	142.897	143.462	0.565	8.9	0.090	0.000	8.9	4.116	OK	143.462
M100D30	144.370	144.110	144.234	0.124	9.2	0.009	0.000	8.9	3.975	Flood Risk	144.234
M100D30.1	143.900	143.640	143.737	0.097	5.3	0.007	0.000	5.1	2.316	Flood Risk	143.737
M100D30.2	143.700	142.990	143.251	0.261	5.1	0.020	0.000	4.2	2.313	OK	143.251
RG2 ACO	144.978	143.658	144.223	0.565	24.3	0.090	0.000	24.2	10.772	OK	144.223
M150D20	144.986	144.721	144.870	0.149	9.3	0.045	0.000	9.1	4.032	Flood Risk	144.870
SW3	144.036	142.500	143.242	0.742	47.0	0.472	0.000	45.8	24.098	Surcharged	143.242
SW1	145.432	144.232	144.288	0.055	14.4	0.035	0.000	14.3	6.880	OK	144.288
SW4	144.603	142.903	143.105	0.202	43.7	0.128	0.000	43.0	24.095	OK	143.105
SW2	144.721	143.363	143.839	0.476	38.5	0.302	0.000	34.0	17.642	Surcharged	143.839

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.572
0.755
0.136
0.163
0.449
0.755
0.116
0.795
1.145
1.498
0.882

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 15 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.061	0.639	13.7	0.102	0.000	13.5	6.854	OK	145.061
RG3	144.217	142.897	143.462	0.565	8.4	0.090	0.000	8.3	4.113	OK	143.462
M100D30	144.370	144.110	144.229	0.119	8.6	0.009	0.000	8.4	3.972	Flood Risk	144.229
M100D30.1	143.900	143.640	143.733	0.093	5.0	0.007	0.000	4.8	2.313	Flood Risk	143.733
M100D30.2	143.700	142.990	143.236	0.246	4.8	0.018	0.000	4.0	2.310	OK	143.236
RG2 ACO	144.978	143.658	144.223	0.565	22.8	0.090	0.000	22.7	10.772	OK	144.223
M150D20	144.986	144.721	144.865	0.144	8.7	0.043	0.000	8.5	4.032	Flood Risk	144.865
SW3	144.036	142.500	143.228	0.728	44.5	0.463	0.000	43.6	24.066	Surcharged	143.228
SW1	145.432	144.232	144.286	0.053	13.5	0.034	0.000	13.5	6.852	OK	144.286
SW4	144.603	142.903	143.095	0.192	41.8	0.122	0.000	41.1	24.062	OK	143.095
SW2	144.721	143.363	143.769	0.405	36.1	0.258	0.000	32.2	17.615	Surcharged	143.769

Freeboard (m)
0.576
0.755
0.141
0.167
0.464
0.755
0.121
0.808
1.147
1.508
0.952

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 30 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.059	0.637	13.0	0.101	0.000	13.0	9.491	OK	145.059
RG3	144.217	142.897	143.462	0.565	6.2	0.090	0.000	6.2	5.546	OK	143.462
M100D30	144.370	144.110	144.209	0.099	6.2	0.007	0.000	6.2	5.404	Flood Risk	144.209
M100D30.1	143.900	143.640	143.718	0.078	3.6	0.006	0.000	3.6	3.154	Flood Risk	143.718
M100D30.2	143.700	142.990	143.211	0.221	3.6	0.017	0.000	3.5	3.152	OK	143.211
RG2 ACO	144.978	143.658	144.223	0.565	16.7	0.090	0.000	16.7	14.613	OK	144.223
M150D20	144.986	144.721	144.845	0.124	6.3	0.037	0.000	6.3	5.490	Flood Risk	144.845
SW3	144.036	142.500	143.203	0.703	38.7	0.447	0.000	38.5	32.826	OK	143.203
SW1	145.432	144.232	144.285	0.052	13.0	0.033	0.000	13.0	9.491	OK	144.285
SW4	144.603	142.903	143.081	0.178	38.2	0.113	0.000	38.0	32.854	OK	143.081
SW2	144.721	143.363	143.631	0.268	29.7	0.170	0.000	28.9	24.096	Surcharged	143.631

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.578
0.755
0.161
0.182
0.489
0.755
0.141
0.833
1.148
1.522
1.089

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 30 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.055	0.634	12.1	0.101	0.000	12.1	9.479	OK	145.055
RG3	144.217	142.897	143.462	0.565	5.9	0.090	0.000	5.9	5.552	OK	143.462
M100D30	144.370	144.110	144.205	0.095	5.9	0.007	0.000	5.9	5.410	Flood Risk	144.205
M100D30.1	143.900	143.640	143.715	0.075	3.4	0.006	0.000	3.4	3.154	Flood Risk	143.715
M100D30.2	143.700	142.990	143.200	0.210	3.4	0.016	0.000	3.3	3.153	OK	143.200
RG2 ACO	144.978	143.658	144.223	0.565	15.7	0.090	0.000	15.7	14.608	OK	144.223
M150D20	144.986	144.721	144.841	0.120	6.0	0.036	0.000	6.0	5.484	Flood Risk	144.841
SW3	144.036	142.500	143.192	0.692	36.1	0.440	0.000	35.8	32.813	OK	143.192
SW1	145.432	144.232	144.282	0.050	12.1	0.032	0.000	12.1	9.478	OK	144.282
SW4	144.603	142.903	143.071	0.168	35.5	0.107	0.000	35.3	32.839	OK	143.071
SW2	144.721	143.363	143.557	0.194	27.8	0.123	0.000	27.0	24.078	Surcharged	143.557

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.581
0.755
0.165
0.185
0.500
0.755
0.145
0.844
1.150
1.532
1.163

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 60 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.053	0.632	11.6	0.100	0.000	11.6	12.199	OK	145.053
RG3	144.217	142.897	143.462	0.565	5.6	0.090	0.000	5.6	7.160	OK	143.462
M100D30	144.370	144.110	144.202	0.092	5.6	0.007	0.000	5.6	7.018	Flood Risk	144.202
M100D30.1	143.900	143.640	143.713	0.073	3.3	0.005	0.000	3.3	4.096	Flood Risk	143.713
M100D30.2	143.700	142.990	143.197	0.207	3.3	0.016	0.000	3.2	4.095	OK	143.197
RG2 ACO	144.978	143.658	144.223	0.565	15.0	0.090	0.000	15.0	18.930	OK	144.223
M150D20	144.986	144.721	144.838	0.117	5.7	0.035	0.000	5.7	7.136	Flood Risk	144.838
SW3	144.036	142.500	143.189	0.689	35.1	0.438	0.000	34.9	42.405	OK	143.189
SW1	145.432	144.232	144.282	0.049	11.6	0.031	0.000	11.6	12.199	OK	144.282
SW4	144.603	142.903	143.069	0.166	34.7	0.105	0.000	34.6	42.436	OK	143.069
SW2	144.721	143.363	143.536	0.172	26.7	0.110	0.000	26.2	31.122	Surcharged	143.536

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.583
0.755
0.168
0.187
0.503
0.755
0.148
0.848
1.151
1.535
1.185

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 60 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.044	0.622	9.1	0.099	0.000	9.1	12.139	OK	145.044
RG3	144.217	142.897	143.462	0.565	4.5	0.090	0.000	4.5	7.160	OK	143.462
M100D30	144.370	144.110	144.190	0.080	4.5	0.006	0.000	4.5	7.018	Flood Risk	144.190
M100D30.1	143.900	143.640	143.703	0.063	2.6	0.005	0.000	2.6	4.090	Flood Risk	143.703
M100D30.2	143.700	142.990	143.178	0.188	2.6	0.014	0.000	2.6	4.089	OK	143.178
RG2 ACO	144.978	143.658	144.223	0.565	12.1	0.090	0.000	12.1	18.930	OK	144.223
M150D20	144.986	144.721	144.826	0.105	4.6	0.032	0.000	4.6	7.137	Flood Risk	144.826
SW3	144.036	142.500	143.164	0.664	28.4	0.422	0.000	28.4	42.336	OK	143.164
SW1	145.432	144.232	144.277	0.044	9.1	0.028	0.000	9.1	12.139	OK	144.277
SW4	144.603	142.903	143.048	0.145	28.4	0.092	0.000	28.4	42.376	OK	143.048
SW2	144.721	143.363	143.466	0.103	21.2	0.065	0.000	21.2	31.062	OK	143.466

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.593
0.755
0.180
0.197
0.522
0.755
0.160
0.872
1.156
1.556
1.255

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 120 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.039	0.617	8.0	0.098	0.000	8.0	14.965	OK	145.039
RG3	144.217	142.897	143.462	0.565	4.0	0.090	0.000	4.0	8.625	OK	143.462
M100D30	144.370	144.110	144.185	0.075	4.0	0.006	0.000	4.0	8.483	Flood Risk	144.185
M100D30.1	143.900	143.640	143.699	0.059	2.3	0.004	0.000	2.3	4.948	Flood Risk	143.699
M100D30.2	143.700	142.990	143.176	0.186	2.3	0.014	0.000	2.4	4.947	OK	143.176
RG2 ACO	144.978	143.658	144.223	0.565	10.8	0.090	0.000	10.8	22.874	OK	144.223
M150D20	144.986	144.721	144.820	0.099	4.1	0.030	0.000	4.1	8.626	Flood Risk	144.820
SW3	144.036	142.500	143.152	0.652	25.1	0.414	0.000	25.1	51.429	OK	143.152
SW1	145.432	144.232	144.274	0.041	8.0	0.026	0.000	8.0	14.965	OK	144.274
SW4	144.603	142.903	143.036	0.133	24.9	0.085	0.000	24.8	51.485	OK	143.036
SW2	144.721	143.363	143.457	0.093	18.8	0.059	0.000	18.8	37.833	OK	143.457

Freeboard (m)
0.598
0.755
0.185
0.201
0.524
0.755
0.166
0.885
1.159
1.567
1.264

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 120 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.024	0.603	4.8	0.096	0.000	4.9	14.953	OK	145.024
RG3	144.217	142.897	143.462	0.565	2.9	0.090	0.000	2.9	8.637	OK	143.462
M100D30	144.370	144.110	144.171	0.061	2.9	0.005	0.000	2.9	8.495	Flood Risk	144.171
M100D30.1	143.900	143.640	143.688	0.048	1.7	0.004	0.000	1.7	4.943	Flood Risk	143.688
M100D30.2	143.700	142.990	143.170	0.180	1.7	0.014	0.000	1.7	4.941	OK	143.170
RG2 ACO	144.978	143.658	144.223	0.565	7.8	0.090	0.000	7.8	22.856	OK	144.223
M150D20	144.986	144.721	144.805	0.084	3.0	0.025	0.000	3.0	8.620	Flood Risk	144.805
SW3	144.036	142.500	143.119	0.619	17.0	0.394	0.000	17.0	51.402	OK	143.119
SW1	145.432	144.232	144.265	0.032	4.9	0.020	0.000	4.9	14.953	OK	144.265
SW4	144.603	142.903	143.009	0.106	17.0	0.067	0.000	17.0	51.457	OK	143.009
SW2	144.721	143.363	143.433	0.069	12.4	0.044	0.000	12.4	37.803	OK	143.433

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.612
0.755
0.199
0.212
0.530
0.755
0.181
0.917
1.168
1.594
1.288

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 240 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.022	0.600	4.4	0.095	0.000	4.4	18.272	OK	145.022
RG3	144.217	142.897	143.462	0.565	2.6	0.090	0.000	2.6	10.389	OK	143.462
M100D30	144.370	144.110	144.167	0.057	2.6	0.004	0.000	2.6	10.247	Flood Risk	144.167
M100D30.1	143.900	143.640	143.686	0.046	1.6	0.003	0.000	1.6	5.975	Flood Risk	143.686
M100D30.2	143.700	142.990	143.169	0.179	1.6	0.013	0.000	1.6	5.973	OK	143.169
RG2 ACO	144.978	143.658	144.223	0.565	7.1	0.090	0.000	7.1	27.554	OK	144.223
M150D20	144.986	144.721	144.802	0.081	2.7	0.024	0.000	2.7	10.408	Flood Risk	144.802
SW3	144.036	142.500	143.113	0.613	15.5	0.390	0.000	15.5	62.207	OK	143.113
SW1	145.432	144.232	144.263	0.031	4.4	0.019	0.000	4.4	18.272	OK	144.263
SW4	144.603	142.903	143.004	0.100	15.5	0.064	0.000	15.5	62.232	OK	143.004
SW2	144.721	143.363	143.429	0.066	11.3	0.042	0.000	11.3	45.821	OK	143.429

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.615
0.755
0.203
0.214
0.531
0.755
0.184
0.924
1.169
1.600
1.291

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 240 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.016	0.594	3.2	0.094	0.000	3.2	18.279	OK	145.016
RG3	144.217	142.897	143.462	0.565	1.8	0.090	0.000	1.8	10.377	OK	143.462
M100D30	144.370	144.110	144.155	0.045	1.8	0.003	0.000	1.8	10.235	Flood Risk	144.155
M100D30.1	143.900	143.640	143.676	0.036	1.0	0.003	0.000	1.0	5.969	Flood Risk	143.676
M100D30.2	143.700	142.990	143.164	0.174	1.0	0.013	0.000	1.0	5.967	OK	143.164
RG2 ACO	144.978	143.658	144.223	0.565	4.8	0.090	0.000	4.8	27.560	OK	144.223
M150D20	144.986	144.721	144.787	0.066	1.8	0.020	0.000	1.8	10.414	Flood Risk	144.787
SW3	144.036	142.500	143.090	0.590	10.4	0.375	0.000	10.4	62.199	OK	143.090
SW1	145.432	144.232	144.259	0.026	3.2	0.017	0.000	3.2	18.279	OK	144.259
SW4	144.603	142.903	142.985	0.082	10.4	0.052	0.000	10.4	62.233	OK	142.985
SW2	144.721	143.363	143.416	0.053	7.6	0.034	0.000	7.6	45.834	OK	143.416

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.621
0.755
0.215
0.224
0.536
0.755
0.199
0.947
1.174
1.619
1.304

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 360 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.018	0.596	3.6	0.095	0.000	3.6	20.293	OK	145.018
RG3	144.217	142.897	143.462	0.565	2.0	0.090	0.000	2.0	11.541	OK	143.462
M100D30	144.370	144.110	144.158	0.048	2.0	0.004	0.000	2.0	11.399	Flood Risk	144.158
M100D30.1	143.900	143.640	143.678	0.038	1.2	0.003	0.000	1.2	6.647	Flood Risk	143.678
M100D30.2	143.700	142.990	143.165	0.175	1.2	0.013	0.000	1.2	6.645	OK	143.165
RG2 ACO	144.978	143.658	144.223	0.565	5.4	0.090	0.000	5.4	30.710	OK	144.223
M150D20	144.986	144.721	144.791	0.070	2.0	0.021	0.000	2.0	11.608	Flood Risk	144.791
SW3	144.036	142.500	143.096	0.596	11.8	0.379	0.000	11.8	69.203	OK	143.096
SW1	145.432	144.232	144.260	0.028	3.6	0.018	0.000	3.6	20.293	OK	144.260
SW4	144.603	142.903	143.036	0.133	11.8	0.085	0.000	11.8	69.275	OK	143.036
SW2	144.721	143.363	143.420	0.057	8.6	0.036	0.000	8.6	50.998	OK	143.420

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.619
0.755
0.212
0.222
0.535
0.755
0.195
0.940
1.172
1.567
1.300

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 360 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.012	0.590	2.6	0.094	0.000	2.6	20.256	OK	145.012
RG3	144.217	142.897	143.462	0.565	1.3	0.090	0.000	1.3	11.565	OK	143.462
M100D30	144.370	144.110	144.147	0.037	1.3	0.003	0.000	1.3	11.423	Flood Risk	144.147
M100D30.1	143.900	143.640	143.670	0.030	0.8	0.002	0.000	0.8	6.653	Flood Risk	143.670
M100D30.2	143.700	142.990	143.161	0.171	0.8	0.013	0.000	0.8	6.651	OK	143.161
RG2 ACO	144.978	143.658	144.223	0.565	3.6	0.090	0.000	3.6	30.698	OK	144.223
M150D20	144.986	144.721	144.778	0.057	1.4	0.017	0.000	1.4	11.596	Flood Risk	144.778
SW3	144.036	142.500	143.076	0.576	7.8	0.366	0.000	7.8	69.181	OK	143.076
SW1	145.432	144.232	144.256	0.024	2.6	0.015	0.000	2.6	20.256	OK	144.256
SW4	144.603	142.903	143.041	0.138	7.8	0.088	0.000	7.8	69.241	OK	143.041
SW2	144.721	143.363	143.410	0.046	5.8	0.029	0.000	5.8	50.950	OK	143.410

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.625
0.755
0.223
0.230
0.539
0.755
0.208
0.960
1.176
1.562
1.311

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 480 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.014	0.592	3.0	0.094	0.000	3.0	21.748	OK	145.014
RG3	144.217	142.897	143.462	0.565	1.7	0.090	0.000	1.7	12.501	OK	143.462
M100D30	144.370	144.110	144.153	0.043	1.7	0.003	0.000	1.7	12.359	Flood Risk	144.153
M100D30.1	143.900	143.640	143.674	0.034	1.0	0.003	0.000	1.0	7.199	Flood Risk	143.674
M100D30.2	143.700	142.990	143.163	0.173	1.0	0.013	0.000	1.0	7.197	OK	143.163
RG2 ACO	144.978	143.658	144.223	0.565	4.5	0.090	0.000	4.5	33.212	OK	144.223
M150D20	144.986	144.721	144.785	0.064	1.7	0.019	0.000	1.7	12.556	Flood Risk	144.785
SW3	144.036	142.500	143.087	0.587	9.8	0.373	0.000	9.8	74.680	OK	143.087
SW1	145.432	144.232	144.258	0.025	3.0	0.016	0.000	3.0	21.748	OK	144.258
SW4	144.603	142.903	143.061	0.158	9.8	0.100	0.000	9.8	74.750	OK	143.061
SW2	144.721	143.363	143.415	0.051	7.2	0.033	0.000	7.2	54.957	OK	143.415

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.623
0.755
0.217
0.226
0.537
0.755
0.201
0.950
1.175
1.542
1.306

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 480 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.009	0.587	2.1	0.093	0.000	2.1	21.731	OK	145.009
RG3	144.217	142.897	143.462	0.565	1.1	0.090	0.000	1.1	12.495	OK	143.462
M100D30	144.370	144.110	144.143	0.033	1.1	0.002	0.000	1.1	12.353	Flood Risk	144.143
M100D30.1	143.900	143.640	143.667	0.027	0.6	0.002	0.000	0.6	7.181	Flood Risk	143.667
M100D30.2	143.700	142.990	143.159	0.169	0.6	0.013	0.000	0.6	7.179	OK	143.159
RG2 ACO	144.978	143.658	144.223	0.565	2.9	0.090	0.000	2.9	33.194	OK	144.223
M150D20	144.986	144.721	144.773	0.052	1.1	0.015	0.000	1.1	12.532	Flood Risk	144.773
SW3	144.036	142.500	143.069	0.569	6.5	0.362	0.000	6.5	74.610	OK	143.069
SW1	145.432	144.232	144.254	0.022	2.1	0.014	0.000	2.1	21.731	OK	144.254
SW4	144.603	142.903	143.067	0.164	6.5	0.104	0.000	6.5	74.672	OK	143.067
SW2	144.721	143.363	143.406	0.042	4.9	0.027	0.000	4.9	54.922	OK	143.406

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.628
0.755
0.227
0.233
0.541
0.755
0.213
0.967
1.178
1.536
1.315

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 960 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.006	0.584	1.8	0.093	0.000	1.8	25.513	OK	145.006
RG3	144.217	142.897	143.462	0.565	1.0	0.090	0.000	1.0	15.135	OK	143.462
M100D30	144.370	144.110	144.142	0.032	1.0	0.002	0.000	1.0	14.993	Flood Risk	144.142
M100D30.1	143.900	143.640	143.666	0.026	0.6	0.002	0.000	0.6	8.741	Flood Risk	143.666
M100D30.2	143.700	142.990	143.158	0.168	0.6	0.013	0.000	0.6	8.739	OK	143.158
RG2 ACO	144.978	143.658	144.223	0.565	2.7	0.090	0.000	2.7	40.214	OK	144.223
M150D20	144.986	144.721	144.771	0.050	1.0	0.015	0.000	1.0	15.226	Flood Risk	144.771
SW3	144.036	142.500	143.067	0.567	6.0	0.361	0.000	6.0	89.612	OK	143.067
SW1	145.432	144.232	144.252	0.020	1.8	0.012	0.000	1.7	25.513	OK	144.252
SW4	144.603	142.903	143.067	0.164	6.0	0.104	0.000	6.0	89.694	OK	143.067
SW2	144.721	143.363	143.403	0.040	4.3	0.025	0.000	4.3	65.723	OK	143.403

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.631
0.755
0.228
0.234
0.542
0.755
0.215
0.970
1.180
1.537
1.318

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 960 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.001	0.580	1.2	0.092	0.000	1.2	25.318	OK	145.001
RG3	144.217	142.897	143.462	0.565	0.7	0.090	0.000	0.7	15.177	OK	143.462
M100D30	144.370	144.110	144.135	0.025	0.7	0.002	0.000	0.7	15.035	Flood Risk	144.135
M100D30.1	143.900	143.640	143.660	0.020	0.4	0.002	0.000	0.4	8.741	Flood Risk	143.660
M100D30.2	143.700	142.990	143.155	0.165	0.4	0.012	0.000	0.4	8.739	OK	143.155
RG2 ACO	144.978	143.658	144.223	0.565	1.8	0.090	0.000	1.8	40.232	OK	144.223
M150D20	144.986	144.721	144.762	0.041	0.7	0.012	0.000	0.7	15.232	Flood Risk	144.762
SW3	144.036	142.500	143.068	0.568	3.8	0.361	0.000	3.8	89.476	OK	143.068
SW1	145.432	144.232	144.249	0.016	1.2	0.010	0.000	1.2	25.318	OK	144.249
SW4	144.603	142.903	143.068	0.165	3.8	0.105	0.000	3.8	89.573	OK	143.068
SW2	144.721	143.363	143.395	0.032	2.8	0.020	0.000	2.8	65.547	OK	143.395

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.235
0.240
0.545
0.755
0.224
0.968
1.184
1.535
1.325

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 1440 mins: Summer

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	145.002	0.580	1.2	0.092	0.000	1.2	27.982	OK	145.002
RG3	144.217	142.897	143.462	0.565	0.8	0.090	0.000	0.8	16.803	OK	143.462
M100D30	144.370	144.110	144.137	0.027	0.8	0.002	0.000	0.8	16.661	Flood Risk	144.137
M100D30.1	143.900	143.640	143.662	0.022	0.4	0.002	0.000	0.4	9.737	Flood Risk	143.662
M100D30.2	143.700	142.990	143.156	0.166	0.4	0.012	0.000	0.4	9.735	OK	143.156
RG2 ACO	144.978	143.658	144.223	0.565	2.0	0.090	0.000	2.0	44.846	OK	144.223
M150D20	144.986	144.721	144.764	0.043	0.8	0.013	0.000	0.8	16.949	Flood Risk	144.764
SW3	144.036	142.500	143.056	0.556	4.4	0.354	0.000	4.4	99.378	OK	143.056
SW1	145.432	144.232	144.249	0.016	1.2	0.010	0.000	1.2	27.982	OK	144.249
SW4	144.603	142.903	143.033	0.130	4.4	0.083	0.000	4.4	99.407	OK	143.033
SW2	144.721	143.363	143.398	0.034	3.2	0.022	0.000	3.2	72.823	OK	143.398

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.635
0.755
0.233
0.238
0.544
0.755
0.221
0.980
1.184
1.570
1.323

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 1440 mins: Winter

Junction	Cover Level (m)	Invert Level (m)	Max. Level (m)	Max. Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Status	HGL (m)
RG1	145.637	144.422	144.997	0.576	0.8	0.092	0.000	0.8	27.340	OK	144.997
RG3	144.217	142.897	143.462	0.565	0.5	0.090	0.000	0.5	16.887	OK	143.462
M100D30	144.370	144.110	144.131	0.021	0.5	0.002	0.000	0.5	16.745	Flood Risk	144.131
M100D30.1	143.900	143.640	143.657	0.017	0.3	0.001	0.000	0.3	9.740	Flood Risk	143.657
M100D30.2	143.700	142.990	143.153	0.163	0.3	0.012	0.000	0.3	9.738	OK	143.153
RG2 ACO	144.978	143.658	144.223	0.565	1.3	0.090	0.000	1.3	44.882	OK	144.223
M150D20	144.986	144.721	144.756	0.035	0.5	0.011	0.000	0.5	17.050	Flood Risk	144.756
SW3	144.036	142.500	143.045	0.545	2.9	0.347	0.000	2.9	98.859	OK	143.045
SW1	145.432	144.232	144.246	0.013	0.8	0.009	0.000	0.8	27.340	OK	144.246
SW4	144.603	142.903	143.027	0.124	2.9	0.079	0.000	2.9	98.952	OK	143.027
SW2	144.721	143.363	143.391	0.028	2.1	0.018	0.000	2.1	72.216	OK	143.391

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Junctions Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

Freeboard (m)
0.639
0.755
0.239
0.243
0.547
0.755
0.229
0.991
1.187
1.576
1.330

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 15 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.328	146.145	0.008	0.009	1.9	0.255	0.000	0.819		96.601	OK
ICE Bays 2	146.239	146.089	0.008	0.009	1.4	0.193	0.000	0.618		96.566	OK
ICE Bays 3	145.990	145.449	0.021	0.024	3.5	0.769	0.000	1.406		93.030	OK
ICE Bays 4	145.953	145.263	0.018	0.021	3.2	0.666	0.000	1.350		93.971	OK
Elec bays 1	146.032	145.419	0.008	0.009	1.4	0.190	0.000	0.609		97.335	OK
Elec bays 2	145.443	144.829	0.008	0.009	1.4	0.190	0.000	0.609		97.348	OK
Elec bays 3	144.965	144.416	0.008	0.010	1.2	0.167	0.000	0.528		97.283	OK
Elec bays 4	144.502	143.777	0.008	0.009	1.9	0.262	0.000	0.843		97.353	OK
SPEL ESR Ph II	143.019	143.019	0.091	0.091	12.1	0.074	0.000	0.000		97.828	OK
Soakaway SA1	142.011	142.011	0.111	0.111	11.6	5.552	0.000	0.739		90.728	OK
Bio 1	146.408	146.259	0.801	0.819	1.0	0.740	0.000	0.027		19.120	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.410	0.000	0.000		55.182	OK
Bio 2	145.355	145.230	0.000	0.000	0.5	0.524	0.000	0.000		42.730	OK
Bio 3	145.463	145.213	0.000	0.000	0.5	0.530	0.000	0.000		42.041	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 15 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.328	146.144	0.008	0.008	1.8	0.237	0.000	0.822		96.840	OK
ICE Bays 2	146.238	146.088	0.008	0.008	1.3	0.176	0.000	0.615		96.864	OK
ICE Bays 3	145.989	145.448	0.020	0.023	3.3	0.736	0.000	1.417		93.334	OK
ICE Bays 4	145.952	145.262	0.017	0.020	3.0	0.634	0.000	1.355		94.254	OK
Elec bays 1	146.031	145.419	0.007	0.009	1.3	0.176	0.000	0.612		97.537	OK
Elec bays 2	145.443	144.829	0.007	0.009	1.3	0.175	0.000	0.612		97.549	OK
Elec bays 3	144.964	144.415	0.007	0.009	1.1	0.153	0.000	0.528		97.508	OK
Elec bays 4	144.501	143.776	0.007	0.008	1.8	0.241	0.000	0.843		97.563	OK
SPEL ESR Ph II	143.016	143.016	0.088	0.088	11.4	0.069	0.000	0.000		97.978	OK
Soakaway SA1	142.011	142.011	0.111	0.111	11.0	5.543	0.000	0.739		90.742	OK
Bio 1	146.408	146.257	0.801	0.817	0.9	0.746	0.000	0.027		18.524	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.413	0.000	0.000		54.854	OK
Bio 2	145.355	145.230	0.000	0.000	0.5	0.521	0.000	0.000		43.056	OK
Bio 3	145.463	145.213	0.000	0.000	0.5	0.533	0.000	0.000		41.711	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 30 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.327	146.143	0.007	0.007	1.3	0.206	0.000	1.086		97.254	OK
ICE Bays 2	146.237	146.087	0.007	0.007	0.9	0.155	0.000	0.810		97.244	OK
ICE Bays 3	145.991	145.451	0.021	0.026	2.3	0.810	0.000	2.017		92.663	OK
ICE Bays 4	145.952	145.264	0.018	0.022	2.1	0.690	0.000	1.855		93.749	OK
Elec bays 1	146.030	145.418	0.006	0.008	0.9	0.153	0.000	0.804		97.860	OK
Elec bays 2	145.441	144.828	0.006	0.008	0.9	0.152	0.000	0.804		97.876	OK
Elec bays 3	144.963	144.414	0.006	0.008	0.8	0.133	0.000	0.696		97.838	OK
Elec bays 4	144.500	143.776	0.006	0.008	1.3	0.213	0.000	1.122		97.854	OK
SPEL ESR Ph II	143.005	143.005	0.077	0.077	9.0	0.052	0.000	0.000		98.477	OK
Soakaway SA1	142.043	142.043	0.143	0.143	9.0	7.116	0.000	1.510		88.115	OK
Bio 1	146.407	146.252	0.801	0.812	0.7	0.841	0.000	0.135		8.135	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.443	0.000	0.000		51.574	OK
Bio 2	145.355	145.230	0.000	0.000	0.3	0.593	0.000	0.000		35.184	OK
Bio 3	145.463	145.213	0.000	0.000	0.3	0.605	0.000	0.000		33.829	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 30 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.326	146.142	0.006	0.006	1.2	0.176	0.000	1.086		97.657	OK
ICE Bays 2	146.236	146.086	0.006	0.006	0.9	0.132	0.000	0.810		97.647	OK
ICE Bays 3	145.989	145.449	0.020	0.024	2.2	0.764	0.000	2.011		93.083	OK
ICE Bays 4	145.951	145.263	0.017	0.021	2.0	0.646	0.000	1.849		94.151	OK
Elec bays 1	146.029	145.417	0.005	0.007	0.9	0.130	0.000	0.804		98.177	OK
Elec bays 2	145.441	144.827	0.005	0.007	0.9	0.129	0.000	0.804		98.192	OK
Elec bays 3	144.962	144.413	0.005	0.007	0.8	0.114	0.000	0.696		98.145	OK
Elec bays 4	144.499	143.775	0.005	0.007	1.2	0.182	0.000	1.116		98.159	OK
SPEL ESR Ph II	143.002	143.002	0.074	0.074	8.4	0.047	0.000	0.000		98.610	OK
Soakaway SA1	142.042	142.042	0.142	0.142	8.4	7.094	0.000	1.509		88.152	OK
Bio 1	146.407	146.248	0.801	0.808	0.6	0.839	0.000	0.142		8.267	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.1	0.443	0.000	0.000		51.573	OK
Bio 2	145.355	145.230	0.000	0.000	0.3	0.593	0.000	0.000		35.183	OK
Bio 3	145.463	145.213	0.000	0.000	0.3	0.605	0.000	0.000		33.828	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 60 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.325	146.141	0.005	0.005	1.1	0.155	0.000	1.386		97.932	OK
ICE Bays 2	146.236	146.085	0.005	0.005	0.8	0.115	0.000	1.044		97.947	OK
ICE Bays 3	145.989	145.448	0.019	0.023	2.1	0.733	0.000	2.569		93.360	OK
ICE Bays 4	145.950	145.262	0.016	0.020	1.9	0.610	0.000	2.359		94.478	OK
Elec bays 1	146.028	145.416	0.004	0.006	0.8	0.111	0.000	1.020		98.442	OK
Elec bays 2	145.440	144.826	0.004	0.006	0.8	0.112	0.000	1.032		98.428	OK
Elec bays 3	144.961	144.412	0.004	0.006	0.7	0.099	0.000	0.888		98.391	OK
Elec bays 4	144.498	143.774	0.004	0.006	1.1	0.156	0.000	1.422		98.426	OK
SPEL ESR Ph II	143.000	143.000	0.072	0.072	7.9	0.044	0.000	0.000		98.722	OK
Soakaway SA1	142.071	142.071	0.171	0.171	7.9	8.527	0.000	3.066		85.759	OK
Bio 1	146.407	146.245	0.800	0.805	0.6	0.930	0.000	0.300		-1.604	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.1	0.479	0.000	0.000		47.638	OK
Bio 2	145.355	145.230	0.000	0.000	0.3	0.665	0.000	0.000		27.312	OK
Bio 3	145.463	145.213	0.000	0.000	0.3	0.689	0.000	0.000		24.634	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 60 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.322	146.138	0.002	0.002	0.9	0.053	0.000	1.386		99.295	OK
ICE Bays 2	146.232	146.082	0.002	0.002	0.7	0.042	0.000	1.044		99.260	OK
ICE Bays 3	145.984	145.445	0.015	0.020	1.6	0.580	0.000	2.563		94.745	OK
ICE Bays 4	145.946	145.258	0.012	0.016	1.5	0.457	0.000	2.365		95.858	OK
Elec bays 1	146.026	145.412	0.002	0.002	0.7	0.038	0.000	1.032		99.471	OK
Elec bays 2	145.437	144.822	0.002	0.002	0.7	0.037	0.000	1.032		99.479	OK
Elec bays 3	144.958	144.408	0.002	0.002	0.6	0.034	0.000	0.894		99.439	OK
Elec bays 4	144.496	143.770	0.002	0.002	0.9	0.056	0.000	1.434		99.438	OK
SPEL ESR Ph II	142.992	142.992	0.064	0.064	6.4	0.032	0.000	0.000		99.065	OK
Soakaway SA1	142.071	142.071	0.171	0.171	6.4	8.528	0.000	3.066		85.757	OK
Bio 1	146.015	145.848	0.408	0.408	0.5	0.935	0.000	0.311		-2.157	OK
Bio 4	145.190	144.940	0.000	0.000	0.1	0.473	0.000	0.000		48.293	OK
Bio 2	145.355	145.230	0.000	0.000	0.2	0.671	0.000	0.000		26.655	OK
Bio 3	145.463	145.213	0.000	0.000	0.2	0.683	0.000	0.000		25.290	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 120 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.322	146.139	0.002	0.003	0.9	0.074	0.000	1.926		99.017	OK
ICE Bays 2	146.233	146.083	0.002	0.003	0.7	0.055	0.000	1.446		99.024	OK
ICE Bays 3	145.986	145.447	0.016	0.022	1.7	0.657	0.000	3.577		94.052	OK
ICE Bays 4	145.948	145.260	0.013	0.018	1.6	0.507	0.000	3.295		95.405	OK
Elec bays 1	146.026	145.413	0.002	0.003	0.7	0.053	0.000	1.434		99.262	OK
Elec bays 2	145.438	144.823	0.002	0.003	0.7	0.052	0.000	1.434		99.275	OK
Elec bays 3	144.959	144.409	0.002	0.003	0.6	0.048	0.000	1.242		99.223	OK
Elec bays 4	144.496	143.771	0.002	0.003	0.9	0.071	0.000	1.992		99.285	OK
SPEL ESR Ph II	142.993	142.993	0.065	0.065	6.6	0.033	0.000	0.000		99.028	OK
Soakaway SA1	142.119	142.119	0.219	0.219	6.6	10.905	0.000	6.256		81.788	OK
Bio 1	146.015	145.848	0.408	0.408	0.5	1.093	0.000	0.602		-19.481	OK
Bio 4	145.190	144.940	0.000	0.000	0.1	0.539	0.000	0.000		41.079	OK
Bio 2	145.757	145.632	0.402	0.402	0.2	0.700	0.000	0.108		23.466	OK
Bio 3	145.865	145.615	0.402	0.402	0.2	0.710	0.000	0.138		22.290	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 120 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.7	0.000	0.000	1.926		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	1.446		100.000	OK
ICE Bays 3	145.978	145.437	0.009	0.012	1.2	0.350	0.000	3.565		96.827	OK
ICE Bays 4	145.940	145.250	0.006	0.008	1.1	0.229	0.000	3.282		97.922	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	1.440		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	1.440		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	1.242		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.7	0.000	0.000	1.980		100.000	OK
SPEL ESR Ph II	142.982	142.982	0.054	0.054	4.7	0.021	0.000	0.000		99.387	OK
Soakaway SA1	142.119	142.119	0.219	0.219	4.7	10.947	0.000	6.255		81.717	OK
Bio 1	146.013	145.847	0.407	0.407	0.4	1.095	0.000	0.610		-19.638	OK
Bio 4	145.190	144.940	0.000	0.000	0.1	0.545	0.000	0.000		40.423	OK
Bio 2	145.757	145.632	0.402	0.402	0.2	0.701	0.000	0.108		23.390	OK
Bio 3	145.865	145.615	0.402	0.402	0.2	0.712	0.000	0.132		22.129	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 240 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.7	0.000	0.000	2.550		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	1.908		100.000	OK
ICE Bays 3	145.977	145.435	0.008	0.010	1.2	0.311	0.000	4.740		97.181	OK
ICE Bays 4	145.940	145.249	0.005	0.007	1.1	0.189	0.000	4.356		98.288	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	1.896		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	1.902		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	1.632		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.7	0.000	0.000	2.616		100.000	OK
SPEL ESR Ph II	142.982	142.982	0.054	0.054	4.7	0.021	0.000	0.000		99.385	OK
Soakaway SA1	142.146	142.146	0.246	0.246	4.7	12.291	0.000	12.565		79.472	OK
Bio 1	146.013	145.847	0.407	0.407	0.4	1.219	0.000	0.966		-33.199	OK
Bio 4	145.190	144.940	0.000	0.000	0.1	0.635	0.000	0.000		30.587	OK
Bio 2	145.758	145.633	0.403	0.403	0.2	0.743	0.000	0.276		18.839	OK
Bio 3	145.866	145.616	0.403	0.403	0.2	0.759	0.000	0.312		16.944	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 240 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	2.544		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	1.914		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.8	0.000	0.000	4.704		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.7	0.000	0.000	4.320		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	1.890		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	1.890		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	1.638		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.5	0.000	0.000	2.616		100.000	OK
SPEL ESR Ph II	142.971	142.971	0.043	0.043	3.2	0.012	0.000	0.000		99.637	OK
Soakaway SA1	142.149	142.149	0.249	0.249	3.2	12.440	0.000	12.568		79.223	OK
Bio 1	146.012	145.845	0.405	0.405	0.2	1.195	0.000	0.978		-30.564	OK
Bio 4	145.190	144.940	0.000	0.000	0.0	0.617	0.000	0.000		32.556	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.725	0.000	0.270		20.788	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.738	0.000	0.294		19.200	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 360 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.5	0.000	0.000	2.934		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.4	0.000	0.000	2.202		100.000	OK
ICE Bays 3	145.971	145.427	0.001	0.002	1.0	0.055	0.000	5.412		99.503	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.9	0.006	0.000	4.980		99.949	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.4	0.000	0.000	2.178		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.4	0.000	0.000	2.190		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	1.872		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.5	0.000	0.000	3.018		100.000	OK
SPEL ESR Ph II	142.975	142.975	0.047	0.047	3.7	0.015	0.000	0.000		99.555	OK
Soakaway SA1	142.147	142.147	0.247	0.247	3.7	12.338	0.000	18.604		79.394	OK
Bio 1	146.012	145.846	0.406	0.406	0.3	1.258	0.000	1.182		-37.502	OK
Bio 4	145.590	145.340	0.400	0.400	0.1	0.696	0.000	0.006		23.988	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.739	0.000	0.342		19.260	OK
Bio 3	145.866	145.616	0.403	0.403	0.1	0.765	0.000	0.402		16.263	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 360 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.3	0.000	0.000	2.928		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	2.202		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.6	0.000	0.000	5.418		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.6	0.000	0.000	4.986		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	2.166		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	2.166		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	1.884		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.3	0.000	0.000	2.994		100.000	OK
SPEL ESR Ph II	142.966	142.966	0.038	0.038	2.5	0.009	0.000	0.000		99.743	OK
Soakaway SA1	142.148	142.148	0.248	0.248	2.5	12.374	0.000	18.545		79.334	OK
Bio 1	146.011	145.844	0.404	0.404	0.2	1.184	0.000	1.164		-29.422	OK
Bio 4	145.190	144.940	0.000	0.000	0.0	0.683	0.000	0.000		25.341	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.720	0.000	0.390		21.319	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.727	0.000	0.426		20.432	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 480 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	3.198		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	2.400		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.8	0.000	0.000	5.922		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.7	0.000	0.000	5.430		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	2.382		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	2.382		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	2.040		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	3.300		100.000	OK
SPEL ESR Ph II	142.971	142.971	0.043	0.043	3.1	0.012	0.000	0.000		99.650	OK
Soakaway SA1	142.145	142.145	0.245	0.245	3.1	12.200	0.000	23.268		79.624	OK
Bio 1	146.011	145.845	0.405	0.405	0.2	1.241	0.000	1.326		-35.595	OK
Bio 4	145.590	145.340	0.400	0.400	0.0	0.696	0.000	0.012		23.988	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.738	0.000	0.420		19.396	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.760	0.000	0.468		16.803	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 480 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.3	0.000	0.000	3.204		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	2.394		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.5	0.000	0.000	5.928		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.5	0.000	0.000	5.472		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	2.388		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	2.388		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	2.040		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.3	0.000	0.000	3.264		100.000	OK
SPEL ESR Ph II	142.962	142.962	0.034	0.034	2.0	0.007	0.000	0.000		99.801	OK
Soakaway SA1	142.143	142.143	0.243	0.243	2.0	12.140	0.000	23.337		79.725	OK
Bio 1	146.010	145.843	0.403	0.403	0.2	1.138	0.000	1.344		-24.392	OK
Bio 4	145.590	145.340	0.400	0.400	0.0	0.696	0.000	0.024		23.988	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.698	0.000	0.426		23.723	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.704	0.000	0.456		22.967	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 960 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.3	0.000	0.000	3.864		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	2.964		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.5	0.000	0.000	7.296		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.4	0.000	0.000	6.642		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	2.928		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	2.934		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	2.514		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.3	0.000	0.000	4.020		100.000	OK
SPEL ESR Ph II	142.961	142.961	0.033	0.033	1.9	0.006	0.000	0.000		99.815	OK
Soakaway SA1	142.124	142.124	0.224	0.224	1.9	11.194	0.000	28.573		81.305	OK
Bio 1	146.010	145.843	0.403	0.403	0.1	1.051	0.000	1.678		-14.887	OK
Bio 4	145.590	145.340	0.400	0.400	0.0	0.696	0.000	0.006		23.988	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.697	0.000	0.700		23.838	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.697	0.000	0.731		23.726	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 960 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.2	0.000	0.000	3.894		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.1	0.000	0.000	2.898		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.3	0.000	0.000	7.254		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.3	0.000	0.000	6.720		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.1	0.000	0.000	2.856		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.1	0.000	0.000	2.880		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.1	0.000	0.000	2.508		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.2	0.000	0.000	4.008		100.000	OK
SPEL ESR Ph II	142.954	142.954	0.026	0.026	1.2	0.004	0.000	0.000		99.895	OK
Soakaway SA1	142.108	142.108	0.208	0.208	1.2	10.398	0.000	28.434		82.633	OK
Bio 1	146.009	145.842	0.402	0.402	0.1	0.844	0.000	1.745		7.795	OK
Bio 4	145.590	145.340	0.400	0.400	0.0	0.696	0.000	0.018		23.988	OK
Bio 2	145.756	145.631	0.401	0.401	0.0	0.696	0.000	0.635		23.980	OK
Bio 3	145.864	145.614	0.401	0.401	0.1	0.695	0.000	0.690		23.968	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 1440 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.2	0.000	0.000	4.410		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	3.216		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.4	0.000	0.000	7.956		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.3	0.000	0.000	7.338		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	3.180		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	3.198		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.1	0.000	0.000	2.754		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.2	0.000	0.000	4.518		100.000	OK
SPEL ESR Ph II	142.956	142.956	0.028	0.028	1.4	0.004	0.000	0.000		99.873	OK
Soakaway SA1	142.100	142.100	0.200	0.200	1.4	9.955	0.000	32.269		83.374	OK
Bio 1	146.009	145.843	0.403	0.403	0.1	0.877	0.000	1.933		4.129	OK
Bio 4	145.590	145.340	0.400	0.400	0.0	0.696	0.000	0.012		23.988	OK
Bio 2	145.756	145.631	0.401	0.401	0.0	0.696	0.000	0.730		23.978	OK
Bio 3	145.864	145.614	0.401	0.401	0.1	0.695	0.000	0.832		23.969	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 1440 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.1	0.000	0.000	4.302		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.1	0.000	0.000	3.306		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.2	0.000	0.000	8.178		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.2	0.000	0.000	7.422		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.1	0.000	0.000	3.276		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.1	0.000	0.000	3.282		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.1	0.000	0.000	2.706		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.1	0.000	0.000	4.434		100.000	OK
SPEL ESR Ph II	142.951	142.951	0.023	0.023	0.9	0.002	0.000	0.000		99.927	OK
Soakaway SA1	142.066	142.066	0.166	0.166	0.9	8.271	0.000	32.335		86.186	OK
Bio 1	146.009	145.842	0.402	0.402	0.1	0.703	0.000	1.911		23.207	OK
Bio 4	145.590	145.340	0.400	0.400	0.0	0.696	0.000	0.018		23.988	OK
Bio 2	145.756	145.631	0.401	0.401	0.0	0.696	0.000	0.819		23.982	OK
Bio 3	145.864	145.614	0.401	0.401	0.0	0.695	0.000	0.856		23.972	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 15 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.343	146.160	0.023	0.024	3.1	0.705	0.000	1.229		90.603	OK
ICE Bays 2	146.254	146.104	0.023	0.024	2.4	0.526	0.000	0.921		90.641	OK
ICE Bays 3	146.014	145.477	0.045	0.052	5.8	1.647	0.000	1.466		85.080	OK
ICE Bays 4	145.974	145.289	0.039	0.047	5.4	1.466	0.000	1.459		86.723	OK
Elec bays 1	146.045	145.436	0.021	0.026	2.3	0.523	0.000	0.909		92.673	OK
Elec bays 2	145.457	144.846	0.022	0.026	2.3	0.526	0.000	0.911		92.654	OK
Elec bays 3	144.978	144.432	0.021	0.026	2.0	0.453	0.000	0.779		92.616	OK
Elec bays 4	144.516	143.793	0.022	0.025	3.2	0.727	0.000	1.268		92.665	OK
SPEL ESR Ph II	143.053	143.053	0.125	0.125	20.5	0.139	0.000	0.000		95.926	OK
Soakaway SA1	142.089	142.089	0.189	0.189	19.7	9.445	0.000	0.764		84.225	OK
Bio 1	146.409	146.286	0.802	0.846	1.7	0.919	0.000	0.043		-0.438	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.4	0.479	0.000	0.000		47.644	OK
Bio 2	146.156	146.036	0.801	0.806	0.8	0.741	0.000	0.051		18.997	Flood Risk
Bio 3	146.264	146.022	0.801	0.809	0.8	0.675	0.000	0.018		26.161	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 15 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.342	146.159	0.022	0.023	3.0	0.678	0.000	1.244		90.962	OK
ICE Bays 2	146.253	146.103	0.022	0.023	2.2	0.508	0.000	0.933		90.960	OK
ICE Bays 3	146.013	145.476	0.044	0.051	5.5	1.609	0.000	1.476		85.423	OK
ICE Bays 4	145.973	145.288	0.038	0.046	5.0	1.427	0.000	1.470		87.070	OK
Elec bays 1	146.045	145.435	0.021	0.025	2.2	0.502	0.000	0.917		92.968	OK
Elec bays 2	145.456	144.844	0.021	0.024	2.2	0.501	0.000	0.918		92.990	OK
Elec bays 3	144.977	144.431	0.021	0.025	1.9	0.435	0.000	0.786		92.909	OK
Elec bays 4	144.515	143.792	0.021	0.024	3.0	0.697	0.000	1.279		92.968	OK
SPEL ESR Ph II	143.049	143.049	0.121	0.121	19.4	0.130	0.000	0.000		96.186	OK
Soakaway SA1	142.089	142.089	0.189	0.189	18.6	9.436	0.000	0.764		84.240	OK
Bio 1	146.409	146.286	0.802	0.846	1.6	0.937	0.000	0.052		-2.401	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.4	0.479	0.000	0.000		47.643	OK
Bio 2	146.156	146.034	0.801	0.804	0.8	0.726	0.000	0.045		20.639	Flood Risk
Bio 3	146.264	146.020	0.801	0.807	0.8	0.677	0.000	0.015		25.880	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 30 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.345	146.162	0.025	0.026	2.1	0.757	0.000	1.818		89.910	OK
ICE Bays 2	146.255	146.106	0.025	0.026	1.6	0.571	0.000	1.368		89.855	OK
ICE Bays 3	146.019	145.488	0.050	0.063	3.9	1.894	0.000	2.890		82.846	OK
ICE Bays 4	145.978	145.299	0.043	0.057	3.6	1.667	0.000	2.764		84.902	OK
Elec bays 1	146.046	145.439	0.022	0.029	1.6	0.563	0.000	1.350		92.120	OK
Elec bays 2	145.458	144.849	0.022	0.029	1.6	0.568	0.000	1.356		92.065	OK
Elec bays 3	144.979	144.435	0.022	0.029	1.4	0.489	0.000	1.170		92.035	OK
Elec bays 4	144.517	143.796	0.023	0.028	2.2	0.783	0.000	1.878		92.091	OK
SPEL ESR Ph II	143.033	143.033	0.105	0.105	15.1	0.100	0.000	0.000		97.079	OK
Soakaway SA1	142.147	142.147	0.247	0.247	15.1	12.328	0.000	1.582		79.410	OK
Bio 1	146.408	146.289	0.802	0.849	1.1	1.110	0.000	0.167		-21.299	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.3	0.533	0.000	0.000		41.739	OK
Bio 2	146.155	146.032	0.800	0.802	1.1	0.830	0.000	0.149		9.245	Flood Risk
Bio 3	146.263	146.017	0.800	0.804	0.6	0.757	0.000	0.108		17.197	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 30 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.343	146.160	0.023	0.024	2.0	0.717	0.000	1.818		90.438	OK
ICE Bays 2	146.254	146.105	0.023	0.025	1.5	0.538	0.000	1.362		90.434	OK
ICE Bays 3	146.018	145.488	0.049	0.063	3.7	1.892	0.000	2.892		82.860	OK
ICE Bays 4	145.977	145.299	0.042	0.057	3.4	1.650	0.000	2.775		85.055	OK
Elec bays 1	146.045	145.437	0.021	0.027	1.5	0.536	0.000	1.356		92.492	OK
Elec bays 2	145.457	144.847	0.021	0.027	1.5	0.535	0.000	1.356		92.520	OK
Elec bays 3	144.978	144.434	0.021	0.028	1.3	0.466	0.000	1.170		92.414	OK
Elec bays 4	144.515	143.795	0.021	0.027	2.0	0.738	0.000	1.873		92.552	OK
SPEL ESR Ph II	143.029	143.029	0.101	0.101	14.2	0.092	0.000	0.000		97.295	OK
Soakaway SA1	142.147	142.147	0.247	0.247	14.2	12.330	0.000	1.582		79.407	OK
Bio 1	146.408	146.287	0.802	0.847	1.0	1.130	0.000	0.168		-23.544	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.533	0.000	0.000		41.738	OK
Bio 2	146.155	146.030	0.800	0.800	0.7	0.811	0.000	0.141		11.411	Flood Risk
Bio 3	146.263	146.014	0.800	0.801	0.5	0.756	0.000	0.108		17.283	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 60 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.343	146.160	0.023	0.024	1.9	0.698	0.000	2.341		90.696	OK
ICE Bays 2	146.253	146.104	0.023	0.024	1.4	0.521	0.000	1.752		90.736	OK
ICE Bays 3	146.018	145.492	0.049	0.067	3.5	1.941	0.000	4.339		82.414	OK
ICE Bays 4	145.976	145.302	0.042	0.060	3.2	1.681	0.000	3.985		84.776	OK
Elec bays 1	146.044	145.437	0.020	0.027	1.4	0.516	0.000	1.734		92.774	OK
Elec bays 2	145.456	144.847	0.020	0.027	1.4	0.518	0.000	1.740		92.753	OK
Elec bays 3	144.977	144.434	0.020	0.028	1.2	0.449	0.000	1.500		92.690	OK
Elec bays 4	144.515	143.794	0.021	0.026	1.9	0.718	0.000	2.407		92.750	OK
SPEL ESR Ph II	143.026	143.026	0.098	0.098	13.4	0.087	0.000	0.000		97.463	OK
Soakaway SA1	142.206	142.206	0.306	0.306	13.4	15.267	0.000	3.260		74.501	OK
Bio 1	146.408	146.286	0.801	0.846	1.0	1.342	0.000	0.373		-46.614	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.599	0.000	0.000		34.524	OK
Bio 2	145.758	145.633	0.404	0.404	0.5	0.870	0.000	0.283		4.942	OK
Bio 3	146.263	146.014	0.800	0.801	0.5	0.829	0.000	0.246		9.278	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 60 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.339	146.156	0.018	0.020	1.5	0.575	0.000	2.341		92.335	OK
ICE Bays 2	146.249	146.100	0.018	0.020	1.1	0.433	0.000	1.758		92.294	OK
ICE Bays 3	146.015	145.490	0.046	0.065	2.8	1.840	0.000	4.321		83.331	OK
ICE Bays 4	145.973	145.301	0.038	0.059	2.6	1.587	0.000	3.979		85.627	OK
Elec bays 1	146.040	145.433	0.016	0.023	1.1	0.426	0.000	1.734		94.035	OK
Elec bays 2	145.452	144.844	0.016	0.024	1.1	0.430	0.000	1.740		93.986	OK
Elec bays 3	144.973	144.430	0.016	0.024	1.0	0.368	0.000	1.488		94.000	OK
Elec bays 4	144.511	143.791	0.017	0.023	1.6	0.593	0.000	2.413		94.009	OK
SPEL ESR Ph II	143.014	143.014	0.086	0.086	10.8	0.066	0.000	0.000		98.060	OK
Soakaway SA1	142.207	142.207	0.307	0.307	10.8	15.295	0.000	3.260		74.456	OK
Bio 1	146.408	146.281	0.801	0.841	0.8	1.407	0.000	0.379		-53.788	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.593	0.000	0.000		35.179	OK
Bio 2	145.759	145.634	0.404	0.404	0.4	0.803	0.000	0.210		12.241	OK
Bio 3	146.202	145.953	0.739	0.740	0.4	0.838	0.000	0.252		8.278	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 120 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.337	146.154	0.016	0.018	1.4	0.512	0.000	3.000		93.167	OK
ICE Bays 2	146.247	146.098	0.016	0.018	1.1	0.385	0.000	2.250		93.162	OK
ICE Bays 3	146.013	145.490	0.044	0.065	2.6	1.807	0.000	5.539		83.634	OK
ICE Bays 4	145.971	145.300	0.036	0.058	2.4	1.525	0.000	5.113		86.187	OK
Elec bays 1	146.038	145.431	0.014	0.021	1.1	0.385	0.000	2.232		94.608	OK
Elec bays 2	145.450	144.841	0.014	0.021	1.1	0.384	0.000	2.232		94.639	OK
Elec bays 3	144.971	144.428	0.014	0.022	0.9	0.331	0.000	1.920		94.603	OK
Elec bays 4	144.509	143.788	0.015	0.020	1.5	0.525	0.000	3.084		94.700	OK
SPEL ESR Ph II	143.011	143.011	0.083	0.083	10.2	0.062	0.000	0.000		98.196	OK
Soakaway SA1	142.269	142.269	0.369	0.369	10.2	18.384	0.000	6.693		69.296	OK
Bio 1	146.408	146.273	0.801	0.833	0.8	1.612	0.000	0.778		-76.224	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.2	0.683	0.000	0.000		25.342	OK
Bio 2	145.760	145.635	0.405	0.405	0.4	0.879	0.000	0.384		3.930	OK
Bio 3	146.263	146.017	0.800	0.804	0.4	0.905	0.000	0.420		0.918	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 120 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.328	146.145	0.008	0.009	1.0	0.249	0.000	2.988		96.681	OK
ICE Bays 2	146.239	146.089	0.008	0.009	0.8	0.194	0.000	2.256		96.546	OK
ICE Bays 3	146.005	145.483	0.035	0.058	1.9	1.529	0.000	5.575		86.150	OK
ICE Bays 4	145.962	145.291	0.027	0.049	1.7	1.226	0.000	5.119		88.895	OK
Elec bays 1	146.031	145.420	0.007	0.010	0.8	0.186	0.000	2.226		97.392	OK
Elec bays 2	145.442	144.830	0.007	0.010	0.8	0.185	0.000	2.232		97.419	OK
Elec bays 3	144.964	144.417	0.007	0.011	0.7	0.169	0.000	1.926		97.251	OK
Elec bays 4	144.501	143.778	0.007	0.010	1.1	0.258	0.000	3.084		97.394	OK
SPEL ESR Ph II	142.997	142.997	0.069	0.069	7.4	0.039	0.000	0.000		98.858	OK
Soakaway SA1	142.269	142.269	0.369	0.369	7.4	18.425	0.000	6.693		69.227	OK
Bio 1	146.407	146.244	0.800	0.804	0.6	1.613	0.000	0.791		-76.272	Flood Risk
Bio 4	145.190	144.940	0.000	0.000	0.1	0.671	0.000	0.000		26.653	OK
Bio 2	145.759	145.634	0.404	0.404	0.3	0.879	0.000	0.390		3.969	OK
Bio 3	146.049	145.800	0.586	0.587	0.3	0.905	0.000	0.420		0.998	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 240 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.325	146.141	0.005	0.005	1.0	0.150	0.000	3.744		98.006	OK
ICE Bays 2	146.235	146.085	0.005	0.005	0.7	0.114	0.000	2.802		97.977	OK
ICE Bays 3	145.998	145.473	0.029	0.048	1.8	1.245	0.000	6.931		88.724	OK
ICE Bays 4	145.956	145.281	0.022	0.039	1.6	0.971	0.000	6.379		91.203	OK
Elec bays 1	146.028	145.416	0.004	0.006	0.7	0.112	0.000	2.778		98.426	OK
Elec bays 2	145.440	144.826	0.004	0.006	0.7	0.111	0.000	2.784		98.444	OK
Elec bays 3	144.961	144.413	0.004	0.006	0.6	0.099	0.000	2.400		98.386	OK
Elec bays 4	144.499	143.774	0.004	0.006	1.0	0.156	0.000	3.864		98.429	OK
SPEL ESR Ph II	142.995	142.995	0.067	0.067	6.9	0.036	0.000	0.000		98.948	OK
Soakaway SA1	142.309	142.309	0.409	0.409	6.9	20.413	0.000	13.519		65.907	OK
Bio 1	146.407	146.266	0.800	0.826	0.5	1.740	0.000	1.494		-90.155	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.696	0.000	0.060		23.986	OK
Bio 2	145.759	145.634	0.404	0.404	0.2	0.927	0.000	0.564		-1.354	OK
Bio 3	145.981	145.732	0.518	0.519	0.3	0.969	0.000	0.624		-6.027	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 240 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.6	0.000	0.000	3.732		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	2.808		100.000	OK
ICE Bays 3	145.983	145.450	0.014	0.025	1.2	0.629	0.000	6.948		94.301	OK
ICE Bays 4	145.943	145.258	0.008	0.016	1.1	0.384	0.000	6.390		96.522	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	2.778		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	2.796		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	2.394		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.7	0.000	0.000	3.846		100.000	OK
SPEL ESR Ph II	142.981	142.981	0.053	0.053	4.7	0.021	0.000	0.000		99.394	OK
Soakaway SA1	142.313	142.313	0.413	0.413	4.7	20.601	0.000	13.525		65.594	OK
Bio 1	146.013	146.259	0.407	0.819	0.4	1.703	0.000	1.497		-86.150	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.696	0.000	0.066		23.984	OK
Bio 2	145.758	145.633	0.403	0.403	0.2	0.902	0.000	0.576		1.434	OK
Bio 3	145.866	145.616	0.403	0.403	0.2	0.940	0.000	0.636		-2.836	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 360 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.7	0.000	0.000	4.188		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.6	0.000	0.000	3.150		100.000	OK
ICE Bays 3	145.985	145.452	0.016	0.027	1.4	0.704	0.000	7.741		93.621	OK
ICE Bays 4	145.946	145.261	0.011	0.019	1.3	0.484	0.000	7.128		95.620	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.6	0.000	0.000	3.102		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.6	0.000	0.000	3.108		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.5	0.000	0.000	2.676		99.998	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.8	0.000	0.000	4.314		100.000	OK
SPEL ESR Ph II	142.985	142.985	0.057	0.057	5.3	0.025	0.000	0.000		99.281	OK
Soakaway SA1	142.311	142.311	0.411	0.411	5.3	20.493	0.000	20.099		65.774	OK
Bio 1	146.014	146.266	0.407	0.826	0.4	1.743	0.000	1.854		-90.501	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.696	0.000	0.132		23.983	OK
Bio 2	145.758	145.633	0.403	0.403	0.2	0.935	0.000	0.708		-2.216	OK
Bio 3	145.867	145.617	0.404	0.404	0.2	0.967	0.000	0.756		-5.821	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 360 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.5	0.000	0.000	4.200		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.4	0.000	0.000	3.144		100.000	OK
ICE Bays 3	145.970	145.426	0.001	0.001	0.9	0.038	0.000	7.788		99.654	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.8	0.000	0.000	7.170		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.4	0.000	0.000	3.114		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.4	0.000	0.000	3.126		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	2.688		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.5	0.000	0.000	4.314		100.000	OK
SPEL ESR Ph II	142.974	142.974	0.046	0.046	3.5	0.014	0.000	0.000		99.590	OK
Soakaway SA1	142.315	142.315	0.415	0.415	3.5	20.698	0.000	20.088		65.432	OK
Bio 1	146.012	146.260	0.405	0.820	0.3	1.710	0.000	1.878		-86.869	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.696	0.000	0.126		23.982	OK
Bio 2	145.758	145.633	0.403	0.403	0.1	0.867	0.000	0.690		5.267	OK
Bio 3	145.866	145.616	0.403	0.403	0.1	0.895	0.000	0.732		2.013	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 480 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.6	0.000	0.000	4.518		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	3.384		100.000	OK
ICE Bays 3	145.977	145.436	0.007	0.011	1.1	0.313	0.000	8.364		97.169	OK
ICE Bays 4	145.939	145.248	0.004	0.006	1.0	0.161	0.000	7.704		98.540	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	3.354		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	3.372		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	2.898		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.6	0.000	0.000	4.638		100.000	OK
SPEL ESR Ph II	142.979	142.979	0.052	0.052	4.4	0.019	0.000	0.000		99.446	OK
Soakaway SA1	142.302	142.302	0.402	0.402	4.4	20.041	0.000	26.366		66.528	OK
Bio 1	146.013	146.263	0.406	0.823	0.3	1.723	0.000	2.046		-88.270	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.696	0.000	0.192		23.981	OK
Bio 2	145.758	145.633	0.403	0.403	0.2	0.903	0.000	0.768		1.300	OK
Bio 3	145.866	145.616	0.403	0.403	0.2	0.940	0.000	0.834		-2.845	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 480 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	4.506		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	3.378		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.7	0.000	0.000	8.346		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.7	0.000	0.000	7.686		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	3.342		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	3.354		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	2.862		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	4.626		100.000	OK
SPEL ESR Ph II	142.969	142.969	0.041	0.041	2.9	0.011	0.000	0.000		99.688	OK
Soakaway SA1	142.304	142.304	0.404	0.404	2.8	20.168	0.000	26.287		66.317	OK
Bio 1	146.011	146.247	0.404	0.807	0.2	1.638	0.000	2.040		-79.070	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.186		23.982	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.817	0.000	0.792		10.664	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.845	0.000	0.840		7.518	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 960 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	5.316		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	4.026		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.7	0.000	0.000	9.960		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.6	0.000	0.000	9.186		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	3.984		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	3.990		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	3.402		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	5.520		100.000	OK
SPEL ESR Ph II	142.967	142.967	0.039	0.039	2.6	0.010	0.000	0.000		99.721	OK
Soakaway SA1	142.267	142.267	0.367	0.367	2.6	18.310	0.000	38.769		69.419	OK
Bio 1	146.011	145.844	0.404	0.404	0.2	1.458	0.000	2.508		-59.359	OK
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.228		23.984	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.757	0.000	0.996		17.309	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.781	0.000	1.050		14.528	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 960 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.2	0.000	0.000	5.358		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	4.014		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.4	0.000	0.000	9.978		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.4	0.000	0.000	9.114		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	3.972		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	3.984		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	3.474		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.2	0.000	0.000	5.460		100.000	OK
SPEL ESR Ph II	142.959	142.959	0.031	0.031	1.7	0.005	0.000	0.000		99.841	OK
Soakaway SA1	142.258	142.258	0.358	0.358	1.7	17.851	0.000	38.827		70.186	OK
Bio 1	146.010	145.843	0.403	0.403	0.1	1.241	0.000	2.442		-35.635	OK
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.318		23.984	OK
Bio 2	145.756	145.631	0.401	0.401	0.1	0.696	0.000	0.984		23.977	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.695	0.000	1.038		23.967	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 1440 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.3	0.000	0.000	5.910		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	4.476		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.5	0.000	0.000	11.070		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.5	0.000	0.000	10.038		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	4.446		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	4.464		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	3.786		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.3	0.000	0.000	6.102		100.000	OK
SPEL ESR Ph II	142.961	142.961	0.033	0.033	1.9	0.006	0.000	0.000		99.811	OK
Soakaway SA1	142.236	142.236	0.336	0.336	1.9	16.770	0.000	43.230		71.992	OK
Bio 1	146.010	145.843	0.403	0.403	0.1	1.233	0.000	2.746		-34.710	OK
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.216		23.984	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.698	0.000	1.246		23.749	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.699	0.000	1.312		23.536	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 1440 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.2	0.000	0.000	5.892		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.1	0.000	0.000	4.428		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.3	0.000	0.000	11.016		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.3	0.000	0.000	10.188		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.1	0.000	0.000	4.374		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.1	0.000	0.000	4.374		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.1	0.000	0.000	3.804		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.2	0.000	0.000	6.078		100.000	OK
SPEL ESR Ph II	142.954	142.954	0.026	0.026	1.2	0.004	0.000	0.000		99.894	OK
Soakaway SA1	142.209	142.209	0.309	0.309	1.2	15.422	0.000	42.911		74.243	OK
Bio 1	146.009	145.842	0.402	0.402	0.1	0.931	0.000	2.837		-1.716	OK
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.240		23.986	OK
Bio 2	145.756	145.631	0.401	0.401	0.0	0.696	0.000	1.151		23.980	OK
Bio 3	145.864	145.614	0.401	0.401	0.1	0.695	0.000	1.266		23.968	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 15 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.376	146.193	0.056	0.057	5.7	1.692	0.000	1.284		77.434	OK
ICE Bays 2	146.286	146.137	0.056	0.057	4.3	1.269	0.000	0.962		77.432	OK
ICE Bays 3	146.066	145.543	0.096	0.118	10.7	3.642	0.000	1.505		67.007	OK
ICE Bays 4	146.020	145.352	0.086	0.110	9.8	3.272	0.000	1.501		70.365	OK
Elec bays 1	146.076	145.475	0.052	0.065	4.3	1.257	0.000	0.955		82.399	OK
Elec bays 2	145.487	144.885	0.052	0.065	4.3	1.259	0.000	0.956		82.402	OK
Elec bays 3	145.008	144.473	0.051	0.067	3.7	1.088	0.000	0.821		82.273	OK
Elec bays 4	144.546	143.832	0.052	0.064	5.9	1.745	0.000	1.324		82.382	OK
SPEL ESR Ph II	143.115	143.115	0.187	0.187	35.8	0.281	0.000	0.000		91.776	OK
Soakaway SA1	142.253	142.253	0.353	0.353	34.8	17.590	0.000	0.816		70.622	OK
Bio 1	146.411	146.296	0.804	0.856	3.1	0.995	0.026	0.064		-8.771	Flood
Bio 4	145.991	145.742	0.801	0.802	0.7	0.635	0.000	0.036		30.571	Flood Risk
Bio 2	146.157	146.071	0.802	0.841	3.9	1.477	0.000	0.085		-61.385	Flood Risk
Bio 3	146.265	146.066	0.802	0.853	1.6	0.938	0.015	0.039		-2.626	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 15 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.375	146.193	0.055	0.057	5.4	1.660	0.000	1.292		77.866	OK
ICE Bays 2	146.285	146.137	0.054	0.057	4.0	1.241	0.000	0.968		77.940	OK
ICE Bays 3	146.064	145.543	0.095	0.118	10.0	3.637	0.000	1.511		67.058	OK
ICE Bays 4	146.019	145.351	0.084	0.109	9.2	3.266	0.000	1.507		70.420	OK
Elec bays 1	146.074	145.475	0.051	0.065	4.0	1.231	0.000	0.960		82.758	OK
Elec bays 2	145.486	144.885	0.051	0.065	4.0	1.233	0.000	0.962		82.760	OK
Elec bays 3	145.007	144.472	0.050	0.066	3.5	1.063	0.000	0.826		82.675	OK
Elec bays 4	144.545	143.831	0.051	0.063	5.6	1.707	0.000	1.332		82.770	OK
SPEL ESR Ph II	143.111	143.111	0.183	0.183	34.7	0.270	0.000	0.000		92.079	OK
Soakaway SA1	142.252	142.252	0.352	0.352	33.9	17.562	0.000	0.815		70.668	OK
Bio 1	146.410	146.295	0.804	0.855	2.9	1.009	0.023	0.065		-10.280	Flood
Bio 4	145.990	145.741	0.800	0.801	0.7	0.621	0.000	0.020		32.109	Flood Risk
Bio 2	146.157	146.071	0.802	0.841	3.6	1.479	0.000	0.086		-61.666	Flood Risk
Bio 3	146.265	146.065	0.802	0.852	1.5	0.946	0.009	0.046		-3.517	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 30 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.385	146.206	0.065	0.070	3.9	2.021	0.000	2.575		73.058	OK
ICE Bays 2	146.295	146.150	0.065	0.070	2.9	1.519	0.000	1.931		73.001	OK
ICE Bays 3	146.084	145.583	0.114	0.158	7.2	4.561	0.000	3.016		58.690	OK
ICE Bays 4	146.035	145.391	0.100	0.149	6.6	4.082	0.000	3.007		63.025	OK
Elec bays 1	146.082	145.494	0.058	0.084	2.9	1.499	0.000	1.890		79.009	OK
Elec bays 2	145.494	144.904	0.058	0.084	2.9	1.503	0.000	1.894		78.987	OK
Elec bays 3	145.014	144.493	0.058	0.087	2.5	1.301	0.000	1.613		78.800	OK
Elec bays 4	144.553	143.849	0.059	0.081	4.0	2.081	0.000	2.655		78.990	OK
SPEL ESR Ph II	143.084	143.084	0.156	0.156	27.8	0.208	0.000	0.000		93.911	OK
Soakaway SA1	142.369	142.369	0.469	0.469	27.8	23.412	0.000	1.736		60.898	OK
Bio 1	146.410	146.293	0.803	0.853	2.1	1.281	0.014	0.179		-40.005	Flood
Bio 4	145.591	145.341	0.401	0.401	0.5	0.764	0.000	0.107		16.534	OK
Bio 2	146.157	146.073	0.802	0.843	2.6	1.636	0.000	0.183		-78.830	Flood Risk
Bio 3	146.264	146.069	0.801	0.856	1.0	1.063	0.027	0.156		-16.294	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 30 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.385	146.206	0.065	0.070	3.6	2.029	0.000	2.574		72.943	OK
ICE Bays 2	146.296	146.150	0.065	0.070	2.7	1.521	0.000	1.929		72.951	OK
ICE Bays 3	146.083	145.583	0.114	0.158	6.7	4.565	0.000	3.014		58.647	OK
ICE Bays 4	146.033	145.391	0.098	0.149	6.2	4.085	0.000	3.006		63.001	OK
Elec bays 1	146.081	145.494	0.057	0.084	2.7	1.510	0.000	1.893		78.851	OK
Elec bays 2	145.492	144.904	0.057	0.084	2.7	1.509	0.000	1.896		78.913	OK
Elec bays 3	145.013	144.493	0.057	0.087	2.3	1.307	0.000	1.615		78.712	OK
Elec bays 4	144.552	143.849	0.058	0.081	3.7	2.090	0.000	2.655		78.903	OK
SPEL ESR Ph II	143.077	143.077	0.149	0.149	26.1	0.192	0.000	0.000		94.366	OK
Soakaway SA1	142.369	142.369	0.469	0.469	26.1	23.389	0.000	1.735		60.937	OK
Bio 1	146.409	146.293	0.803	0.853	1.9	1.294	0.012	0.180		-41.371	Flood
Bio 4	145.591	145.341	0.401	0.401	0.4	0.761	0.000	0.104		16.851	OK
Bio 2	146.157	146.073	0.802	0.843	2.4	1.659	0.000	0.183		-81.307	Flood Risk
Bio 3	146.264	146.069	0.801	0.856	1.0	1.071	0.028	0.160		-17.173	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 60 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.388	146.209	0.067	0.073	3.5	2.097	0.000	4.314		72.040	OK
ICE Bays 2	146.298	146.153	0.067	0.073	2.6	1.574	0.000	3.240		72.012	OK
ICE Bays 3	146.089	145.617	0.120	0.192	6.4	4.966	0.000	5.850		55.014	OK
ICE Bays 4	146.037	145.426	0.103	0.184	5.9	4.386	0.000	5.538		60.270	OK
Elec bays 1	146.081	145.507	0.057	0.097	2.6	1.563	0.000	2.963		78.106	OK
Elec bays 2	145.493	144.917	0.057	0.097	2.6	1.562	0.000	2.971		78.171	OK
Elec bays 3	145.014	144.508	0.057	0.102	2.2	1.351	0.000	2.524		77.989	OK
Elec bays 4	144.553	143.860	0.059	0.092	3.6	2.163	0.000	4.184		78.161	OK
SPEL ESR Ph II	143.072	143.072	0.144	0.144	24.8	0.181	0.000	0.000		94.696	OK
Soakaway SA1	142.492	142.492	0.592	0.592	24.8	29.531	0.000	3.670		50.678	OK
Bio 1	146.409	146.292	0.803	0.852	1.8	1.689	0.010	0.399		-84.555	Flood
Bio 4	145.593	145.343	0.403	0.403	0.5	0.892	0.000	0.296		2.528	OK
Bio 2	146.157	146.074	0.802	0.844	2.2	1.800	0.000	0.372		-96.775	Flood Risk
Bio 3	146.265	146.071	0.802	0.858	0.9	1.181	0.038	0.326		-29.218	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 60 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.385	146.208	0.065	0.072	2.8	2.053	0.000	4.321		72.631	OK
ICE Bays 2	146.295	146.152	0.065	0.072	2.1	1.536	0.000	3.240		72.696	OK
ICE Bays 3	146.087	145.619	0.118	0.194	5.1	5.024	0.000	5.821		54.496	OK
ICE Bays 4	146.035	145.427	0.100	0.185	4.7	4.431	0.000	5.526		59.868	OK
Elec bays 1	146.078	145.506	0.054	0.096	2.1	1.525	0.000	2.976		78.644	OK
Elec bays 2	145.490	144.916	0.054	0.096	2.1	1.523	0.000	2.980		78.715	OK
Elec bays 3	145.010	144.506	0.053	0.100	1.8	1.317	0.000	2.532		78.550	OK
Elec bays 4	144.550	143.859	0.056	0.091	2.9	2.111	0.000	4.201		78.688	OK
SPEL ESR Ph II	143.053	143.053	0.125	0.125	20.0	0.139	0.000	0.000		95.921	OK
Soakaway SA1	142.490	142.490	0.590	0.590	20.0	29.448	0.000	3.668		50.817	OK
Bio 1	146.409	146.291	0.802	0.851	1.5	1.744	0.004	0.403		-90.646	Flood
Bio 4	145.593	145.343	0.403	0.403	0.5	0.863	0.000	0.259		5.734	OK
Bio 2	146.158	146.074	0.803	0.844	1.7	1.861	0.000	0.377		-103.367	Flood Risk
Bio 3	146.264	146.070	0.801	0.857	0.8	1.205	0.032	0.339		-31.815	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 120 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.380	146.202	0.060	0.066	2.5	1.899	0.000	5.341		74.684	OK
ICE Bays 2	146.290	146.147	0.060	0.067	1.9	1.423	0.000	4.008		74.706	OK
ICE Bays 3	146.081	145.636	0.111	0.211	4.7	4.840	0.000	9.055		56.155	OK
ICE Bays 4	146.028	145.447	0.093	0.205	4.3	4.245	0.000	8.394		61.545	OK
Elec bays 1	146.073	145.502	0.049	0.092	1.9	1.405	0.000	3.961		80.318	OK
Elec bays 2	145.485	144.913	0.049	0.093	1.9	1.421	0.000	3.985		80.136	OK
Elec bays 3	145.005	144.505	0.048	0.099	1.6	1.231	0.000	3.438		79.944	OK
Elec bays 4	144.545	143.854	0.051	0.086	2.6	1.954	0.000	5.503		80.276	OK
SPEL ESR Ph II	143.046	143.046	0.118	0.118	18.2	0.125	0.000	0.000		96.346	OK
Soakaway SA1	142.605	142.605	0.705	0.705	18.2	35.149	0.000	7.670		41.296	OK
Bio 1	146.409	146.290	0.802	0.850	1.4	1.946	0.001	0.827		-112.669	Flood
Bio 4	145.593	145.343	0.403	0.403	0.5	0.942	0.000	0.414		-3.000	OK
Bio 2	146.157	146.072	0.802	0.842	1.5	2.026	0.000	0.768		-121.421	Flood Risk
Bio 3	146.264	146.070	0.801	0.857	0.7	1.306	0.034	0.656		-42.954	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 120 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.372	146.195	0.052	0.059	1.8	1.662	0.000	5.329		77.846	OK
ICE Bays 2	146.283	146.140	0.052	0.060	1.4	1.256	0.000	4.015		77.665	OK
ICE Bays 3	146.072	145.635	0.103	0.210	3.4	4.748	0.000	9.077		56.989	OK
ICE Bays 4	146.019	145.444	0.084	0.202	3.1	4.131	0.000	8.443		62.582	OK
Elec bays 1	146.064	145.496	0.040	0.086	1.4	1.248	0.000	3.979		82.528	OK
Elec bays 2	145.475	144.905	0.040	0.085	1.4	1.245	0.000	3.979		82.601	OK
Elec bays 3	144.995	144.496	0.039	0.090	1.2	1.075	0.000	3.420		82.480	OK
Elec bays 4	144.536	143.847	0.042	0.079	1.9	1.720	0.000	5.509		82.632	OK
SPEL ESR Ph II	143.024	143.024	0.096	0.096	13.1	0.084	0.000	0.000		97.534	OK
Soakaway SA1	142.604	142.604	0.704	0.704	13.1	35.130	0.000	7.667		41.328	OK
Bio 1	146.408	146.289	0.801	0.849	1.0	2.000	0.000	0.833		-118.527	Flood Risk
Bio 4	145.593	145.343	0.403	0.403	0.3	0.871	0.000	0.347		4.771	OK
Bio 2	146.157	146.072	0.802	0.842	0.9	2.027	0.000	0.778		-121.492	Flood Risk
Bio 3	146.264	146.066	0.801	0.853	0.5	1.384	0.016	0.703		-51.470	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 240 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.362	146.183	0.042	0.047	1.7	1.329	0.000	6.516		82.276	OK
ICE Bays 2	146.272	146.128	0.041	0.048	1.3	0.996	0.000	4.872		82.299	OK
ICE Bays 3	146.059	145.621	0.090	0.196	3.1	4.269	0.000	12.073		61.336	OK
ICE Bays 4	146.007	145.429	0.072	0.187	2.9	3.647	0.000	11.083		66.966	OK
Elec bays 1	146.055	145.478	0.031	0.068	1.3	0.984	0.000	4.830		86.217	OK
Elec bays 2	145.467	144.888	0.031	0.068	1.3	0.987	0.000	4.842		86.201	OK
Elec bays 3	144.987	144.479	0.031	0.073	1.1	0.854	0.000	4.164		86.087	OK
Elec bays 4	144.527	143.832	0.033	0.064	1.7	1.373	0.000	6.709		86.140	OK
SPEL ESR Ph II	143.020	143.020	0.092	0.092	12.1	0.076	0.000	0.000		97.764	OK
Soakaway SA1	142.706	142.706	0.806	0.806	12.1	40.182	0.000	15.815		32.890	OK
Bio 1	146.408	146.289	0.801	0.849	0.9	2.053	0.000	1.674		-124.368	Flood Risk
Bio 4	145.593	145.343	0.403	0.403	0.2	0.840	0.000	0.426		8.223	OK
Bio 2	146.157	146.073	0.802	0.843	0.9	2.035	0.000	1.572		-122.382	Flood Risk
Bio 3	146.264	146.063	0.801	0.850	0.5	1.518	0.000	1.289		-66.100	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 240 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.344	146.164	0.024	0.028	1.1	0.771	0.000	6.498		89.725	OK
ICE Bays 2	146.255	146.109	0.024	0.029	0.9	0.589	0.000	4.884		89.530	OK
ICE Bays 3	146.040	145.604	0.070	0.179	2.1	3.722	0.000	12.085		66.286	OK
ICE Bays 4	145.988	145.408	0.053	0.166	1.9	3.062	0.000	11.101		72.265	OK
Elec bays 1	146.041	145.453	0.017	0.043	0.8	0.574	0.000	4.842		91.959	OK
Elec bays 2	145.452	144.863	0.017	0.043	0.8	0.577	0.000	4.848		91.940	OK
Elec bays 3	144.973	144.452	0.017	0.046	0.7	0.509	0.000	4.182		91.709	OK
Elec bays 4	144.512	143.807	0.018	0.039	1.2	0.794	0.000	6.696		91.983	OK
SPEL ESR Ph II	143.001	143.001	0.073	0.073	8.1	0.045	0.000	0.000		98.678	OK
Soakaway SA1	142.706	142.706	0.806	0.806	8.1	40.222	0.000	15.805		32.823	OK
Bio 1	146.408	146.288	0.801	0.848	0.6	2.056	0.000	1.685		-124.687	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.781	0.000	0.378		14.653	OK
Bio 2	146.157	146.073	0.802	0.843	0.7	2.033	0.000	1.598		-122.199	Flood Risk
Bio 3	146.263	146.029	0.800	0.816	0.3	1.506	0.000	1.309		-64.773	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 360 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.345	146.165	0.025	0.029	1.3	0.806	0.000	7.230		89.258	OK
ICE Bays 2	146.256	146.109	0.025	0.029	1.0	0.604	0.000	5.430		89.268	OK
ICE Bays 3	146.039	145.594	0.070	0.169	2.4	3.541	0.000	13.435		67.928	OK
ICE Bays 4	145.989	145.398	0.054	0.156	2.2	2.923	0.000	12.349		73.528	OK
Elec bays 1	146.043	145.452	0.019	0.042	0.9	0.598	0.000	5.388		91.619	OK
Elec bays 2	145.454	144.863	0.019	0.043	1.0	0.601	0.000	5.400		91.593	OK
Elec bays 3	144.975	144.451	0.018	0.045	0.8	0.522	0.000	4.644		91.491	OK
Elec bays 4	144.514	143.807	0.020	0.039	1.3	0.825	0.000	7.452		91.671	OK
SPEL ESR Ph II	143.006	143.006	0.078	0.078	9.2	0.054	0.000	0.000		98.428	OK
Soakaway SA1	142.737	142.737	0.837	0.837	9.2	41.744	0.000	23.795	335	30.281	OK
Bio 1	146.408	146.288	0.801	0.848	0.7	2.055	0.000	2.223		-124.581	Flood Risk
Bio 4	145.592	145.342	0.403	0.403	0.2	0.815	0.000	0.492		10.894	OK
Bio 2	146.157	146.073	0.802	0.843	0.7	2.034	0.000	2.042		-122.246	Flood Risk
Bio 3	146.264	146.034	0.801	0.821	0.4	1.530	0.000	1.566		-67.466	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 360 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.324	146.141	0.004	0.005	0.8	0.139	0.000	7.248		98.144	OK
ICE Bays 2	146.235	146.085	0.004	0.005	0.6	0.105	0.000	5.418		98.130	OK
ICE Bays 3	146.014	145.559	0.045	0.134	1.6	2.579	0.000	13.435		76.640	OK
ICE Bays 4	145.966	145.356	0.031	0.114	1.4	1.950	0.000	12.361		82.335	OK
Elec bays 1	146.028	145.417	0.004	0.007	0.6	0.110	0.000	5.388		98.459	OK
Elec bays 2	145.439	144.827	0.004	0.007	0.6	0.108	0.000	5.394		98.485	OK
Elec bays 3	144.960	144.413	0.003	0.007	0.5	0.089	0.000	4.656		98.549	OK
Elec bays 4	144.498	143.774	0.004	0.006	0.9	0.143	0.000	7.446		98.560	OK
SPEL ESR Ph II	142.990	142.990	0.062	0.062	6.1	0.030	0.000	0.000		99.135	OK
Soakaway SA1	142.740	142.740	0.840	0.840	6.1	41.896	0.000	23.776	345	30.027	OK
Bio 1	146.407	146.288	0.801	0.848	0.4	2.052	0.000	2.216		-124.247	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.763	0.000	0.504		16.633	OK
Bio 2	146.157	146.072	0.802	0.842	0.5	2.030	0.000	2.099		-121.872	Flood Risk
Bio 3	145.867	145.762	0.404	0.549	0.2	1.476	0.000	1.566		-61.464	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 480 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.333	146.151	0.013	0.015	1.1	0.415	0.000	7.788		94.473	OK
ICE Bays 2	146.243	146.095	0.013	0.015	0.8	0.309	0.000	5.844		94.509	OK
ICE Bays 3	146.023	145.564	0.053	0.139	2.0	2.814	0.000	14.455		74.508	OK
ICE Bays 4	145.974	145.365	0.040	0.123	1.8	2.226	0.000	13.279		79.840	OK
Elec bays 1	146.034	145.430	0.010	0.020	0.8	0.301	0.000	5.784		95.790	OK
Elec bays 2	145.446	144.841	0.010	0.021	0.8	0.310	0.000	5.802		95.671	OK
Elec bays 3	144.967	144.428	0.010	0.022	0.7	0.269	0.000	4.992		95.622	OK
Elec bays 4	144.504	143.787	0.010	0.019	1.1	0.417	0.000	8.004		95.793	OK
SPEL ESR Ph II	142.998	142.998	0.070	0.070	7.6	0.041	0.000	0.000		98.806	OK
Soakaway SA1	142.739	142.739	0.839	0.839	7.6	41.850	0.000	31.470	340	30.104	OK
Bio 1	146.407	146.288	0.801	0.848	0.6	2.053	0.000	2.552		-124.408	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.799	0.000	0.552		12.713	OK
Bio 2	146.157	146.072	0.802	0.842	0.6	2.030	0.000	2.215		-121.843	Flood Risk
Bio 3	146.263	146.015	0.800	0.802	0.3	1.502	0.000	1.722		-64.330	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 480 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.7	0.000	0.000	7.782		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	5.814		100.000	OK
ICE Bays 3	145.996	145.508	0.026	0.083	1.3	1.541	0.000	14.431		86.044	OK
ICE Bays 4	145.951	145.301	0.016	0.059	1.2	1.009	0.000	13.272		90.863	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	5.802		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	5.802		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	5.004		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.7	0.000	0.000	8.004		100.000	OK
SPEL ESR Ph II	142.983	142.983	0.055	0.055	4.9	0.022	0.000	0.000		99.354	OK
Soakaway SA1	142.746	142.746	0.846	0.846	4.9	42.218	0.000	31.467	360	29.490	OK
Bio 1	146.336	146.287	0.729	0.847	0.4	2.042	0.000	2.550		-123.192	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.725	0.000	0.540		20.792	OK
Bio 2	146.156	146.072	0.801	0.842	0.4	2.027	0.000	2.299		-121.539	Flood Risk
Bio 3	145.866	145.616	0.403	0.403	0.2	1.401	0.000	1.704		-53.321	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 960 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.6	0.000	0.000	9.216		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	6.906		100.000	OK
ICE Bays 3	145.984	145.460	0.015	0.035	1.2	0.735	0.000	17.142		93.339	OK
ICE Bays 4	145.943	145.261	0.008	0.019	1.1	0.412	0.000	15.738		96.267	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	6.864		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	6.870		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	5.910		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.6	0.000	0.000	9.498		100.000	OK
SPEL ESR Ph II	142.980	142.980	0.052	0.052	4.5	0.020	0.000	0.000		99.426	OK
Soakaway SA1	142.693	142.693	0.793	0.793	4.5	39.533	0.000	58.841	455	33.973	OK
Bio 1	146.013	146.287	0.406	0.847	0.3	1.995	0.000	3.572		-118.000	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.710	0.000	0.792		22.350	OK
Bio 2	146.156	146.071	0.801	0.841	0.3	2.012	0.000	2.813		-119.881	Flood Risk
Bio 3	145.866	145.616	0.403	0.403	0.2	1.241	0.000	2.124		-35.754	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 960 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	9.204		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	6.864		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.7	0.000	0.000	17.082		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.7	0.000	0.000	15.684		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	6.816		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	6.828		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	5.934		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	9.474		100.000	OK
SPEL ESR Ph II	142.969	142.969	0.041	0.041	2.9	0.011	0.000	0.000		99.678	OK
Soakaway SA1	142.695	142.695	0.795	0.795	2.9	39.642	0.000	58.650	435	33.791	OK
Bio 1	146.011	146.286	0.405	0.846	0.2	1.935	0.000	3.709		-111.500	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.762		23.981	OK
Bio 2	145.757	146.032	0.402	0.802	0.2	1.709	0.000	2.843		-86.778	Flood Risk
Bio 3	145.865	145.615	0.402	0.402	0.1	1.052	0.000	2.124		-15.124	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 1440 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.5	0.000	0.000	10.140		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.4	0.000	0.000	7.596		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.8	0.000	0.000	18.924		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.8	0.000	0.000	17.316		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	7.536		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	7.560		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	6.528		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.5	0.000	0.000	10.440		100.000	OK
SPEL ESR Ph II	142.972	142.972	0.044	0.044	3.3	0.013	0.000	0.000		99.617	OK
Soakaway SA1	142.648	142.648	0.748	0.748	3.3	37.309	0.000	73.301	495	37.688	OK
Bio 1	146.012	146.286	0.405	0.846	0.2	1.936	0.000	4.305		-111.620	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.696	0.000	0.960		23.979	OK
Bio 2	145.757	145.632	0.402	0.402	0.2	1.613	0.000	2.942		-76.322	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	1.020	0.000	2.292		-11.652	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 30 years: Increase Rainfall (%): +40: 1440 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.3	0.000	0.000	10.116		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	7.590		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.6	0.000	0.000	18.864		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.5	0.000	0.000	17.304		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	7.554		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	7.554		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	6.462		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.3	0.000	0.000	10.446		100.000	OK
SPEL ESR Ph II	142.963	142.963	0.035	0.035	2.1	0.007	0.000	0.000		99.785	OK
Soakaway SA1	142.638	142.638	0.738	0.738	2.1	36.789	0.000	73.691	435	38.557	OK
Bio 1	146.010	146.286	0.404	0.846	0.2	1.879	0.000	4.624		-105.405	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.942		23.982	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	1.034	0.000	2.611		-12.997	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.785	0.000	2.292		14.130	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 15 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.368	146.185	0.048	0.049	5.1	1.455	0.000	1.275		80.599	OK
ICE Bays 2	146.278	146.129	0.048	0.049	3.9	1.093	0.000	0.957		80.576	OK
ICE Bays 3	146.054	145.528	0.084	0.103	9.5	3.159	0.000	1.499		71.383	OK
ICE Bays 4	146.010	145.337	0.075	0.095	8.8	2.831	0.000	1.495		74.356	OK
Elec bays 1	146.068	145.465	0.044	0.055	3.8	1.083	0.000	0.949		84.834	OK
Elec bays 2	145.480	144.875	0.045	0.055	3.8	1.085	0.000	0.950		84.831	OK
Elec bays 3	145.001	144.463	0.044	0.057	3.3	0.936	0.000	0.815		84.750	OK
Elec bays 4	144.539	143.822	0.045	0.054	5.3	1.502	0.000	1.316		84.839	OK
SPEL ESR Ph II	143.106	143.106	0.178	0.178	33.9	0.259	0.000	0.000		92.424	OK
Soakaway SA1	142.214	142.214	0.314	0.314	32.6	15.641	0.000	0.803		73.878	OK
Bio 1	146.410	146.295	0.804	0.855	2.8	0.983	0.021	0.063		-7.442	Flood
Bio 4	145.990	145.741	0.800	0.801	0.6	0.584	0.000	0.000		36.164	Flood Risk
Bio 2	146.157	146.070	0.802	0.840	3.4	1.335	0.000	0.084		-45.895	Flood Risk
Bio 3	146.265	146.059	0.802	0.846	1.4	0.903	0.000	0.031		1.159	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 15 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.367	146.184	0.047	0.048	4.8	1.425	0.000	1.285		81.004	OK
ICE Bays 2	146.277	146.128	0.047	0.048	3.6	1.071	0.000	0.963		80.967	OK
ICE Bays 3	146.052	145.528	0.083	0.103	8.9	3.156	0.000	1.506		71.414	OK
ICE Bays 4	146.008	145.337	0.073	0.095	8.2	2.824	0.000	1.502		74.418	OK
Elec bays 1	146.068	145.465	0.044	0.055	3.6	1.061	0.000	0.956		85.142	OK
Elec bays 2	145.479	144.875	0.043	0.055	3.6	1.060	0.000	0.957		85.182	OK
Elec bays 3	145.000	144.462	0.043	0.056	3.1	0.916	0.000	0.822		85.074	OK
Elec bays 4	144.538	143.822	0.044	0.054	5.0	1.465	0.000	1.325		85.207	OK
SPEL ESR Ph II	143.099	143.099	0.171	0.171	32.2	0.241	0.000	0.000		92.938	OK
Soakaway SA1	142.214	142.214	0.314	0.314	30.9	15.644	0.000	0.803		73.873	OK
Bio 1	146.410	146.294	0.803	0.854	2.6	0.998	0.019	0.064		-9.108	Flood
Bio 4	145.990	145.740	0.800	0.800	0.6	0.584	0.000	0.000		36.165	Flood Risk
Bio 2	146.157	146.068	0.802	0.838	3.1	1.326	0.000	0.084		-44.871	Flood Risk
Bio 3	146.265	146.056	0.802	0.843	1.3	0.899	0.000	0.038		1.604	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 30 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.376	146.196	0.056	0.060	3.5	1.737	0.000	2.557		76.843	OK
ICE Bays 2	146.287	146.140	0.056	0.060	2.6	1.305	0.000	1.918		76.806	OK
ICE Bays 3	146.070	145.563	0.101	0.138	6.5	4.009	0.000	3.006		63.688	OK
ICE Bays 4	146.023	145.370	0.088	0.128	6.0	3.569	0.000	2.997		67.669	OK
Elec bays 1	146.075	145.481	0.051	0.071	2.6	1.293	0.000	1.848		81.890	OK
Elec bays 2	145.486	144.891	0.050	0.071	2.6	1.292	0.000	1.851		81.945	OK
Elec bays 3	145.007	144.479	0.050	0.073	2.3	1.114	0.000	1.576		81.857	OK
Elec bays 4	144.545	143.836	0.051	0.068	3.6	1.785	0.000	2.586		81.976	OK
SPEL ESR Ph II	143.074	143.074	0.146	0.146	25.2	0.185	0.000	0.000		94.594	OK
Soakaway SA1	142.322	142.322	0.422	0.422	25.2	21.022	0.000	1.703		64.890	OK
Bio 1	146.409	146.292	0.803	0.852	1.9	1.253	0.011	0.177		-36.887	Flood
Bio 4	145.194	144.940	0.004	0.000	0.4	0.677	0.000	0.020		26.004	OK
Bio 2	146.156	146.072	0.801	0.842	2.3	1.608	0.000	0.180		-75.772	Flood Risk
Bio 3	146.264	146.065	0.801	0.852	0.9	1.052	0.007	0.152		-15.084	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 30 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.376	146.196	0.056	0.060	3.3	1.737	0.000	2.559		76.841	OK
ICE Bays 2	146.286	146.140	0.056	0.060	2.5	1.306	0.000	1.919		76.784	OK
ICE Bays 3	146.070	145.563	0.100	0.138	6.1	4.011	0.000	3.005		63.664	OK
ICE Bays 4	146.022	145.370	0.087	0.128	5.6	3.578	0.000	2.996		67.590	OK
Elec bays 1	146.074	145.481	0.050	0.071	2.5	1.294	0.000	1.848		81.872	OK
Elec bays 2	145.485	144.891	0.049	0.071	2.5	1.293	0.000	1.851		81.928	OK
Elec bays 3	145.006	144.480	0.049	0.074	2.1	1.117	0.000	1.579		81.798	OK
Elec bays 4	144.544	143.836	0.050	0.068	3.4	1.785	0.000	2.587		81.975	OK
SPEL ESR Ph II	143.068	143.068	0.140	0.140	23.7	0.171	0.000	0.000		94.995	OK
Soakaway SA1	142.321	142.321	0.421	0.421	23.6	21.003	0.000	1.702		64.922	OK
Bio 1	146.409	146.292	0.803	0.852	1.8	1.268	0.009	0.179		-38.564	Flood
Bio 4	145.191	144.940	0.001	0.000	0.4	0.677	0.000	0.013		26.002	OK
Bio 2	146.156	146.064	0.801	0.834	2.1	1.595	0.000	0.180		-74.353	Flood Risk
Bio 3	146.264	146.063	0.801	0.850	0.9	1.057	0.000	0.155		-15.631	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 60 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.379	146.199	0.059	0.063	3.2	1.824	0.000	3.948		75.680	OK
ICE Bays 2	146.289	146.143	0.058	0.063	2.4	1.368	0.000	2.964		75.688	OK
ICE Bays 3	146.075	145.593	0.106	0.168	5.8	4.344	0.000	5.730		60.648	OK
ICE Bays 4	146.025	145.400	0.090	0.158	5.4	3.834	0.000	5.319		65.268	OK
Elec bays 1	146.074	145.492	0.050	0.082	2.3	1.356	0.000	2.848		81.006	OK
Elec bays 2	145.486	144.902	0.050	0.082	2.3	1.355	0.000	2.856		81.064	OK
Elec bays 3	145.007	144.492	0.050	0.086	2.0	1.174	0.000	2.426		80.872	OK
Elec bays 4	144.545	143.845	0.051	0.077	3.3	1.872	0.000	4.011		81.097	OK
SPEL ESR Ph II	143.064	143.064	0.136	0.136	22.7	0.162	0.000	0.000		95.243	OK
Soakaway SA1	142.435	142.435	0.535	0.535	22.7	26.705	0.000	3.589		55.398	OK
Bio 1	146.409	146.292	0.802	0.852	1.7	1.641	0.007	0.395		-79.307	Flood
Bio 4	145.592	145.342	0.402	0.402	0.4	0.783	0.000	0.174		14.418	OK
Bio 2	146.157	146.073	0.802	0.843	2.0	1.764	0.000	0.369		-92.772	Flood Risk
Bio 3	146.264	146.069	0.801	0.856	0.9	1.158	0.026	0.314		-26.733	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 60 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.376	146.197	0.056	0.061	2.5	1.745	0.000	3.949		76.736	OK
ICE Bays 2	146.286	146.141	0.056	0.061	1.9	1.307	0.000	2.952		76.758	OK
ICE Bays 3	146.073	145.593	0.104	0.168	4.7	4.385	0.000	5.709		60.279	OK
ICE Bays 4	146.023	145.400	0.088	0.158	4.3	3.835	0.000	5.310		65.263	OK
Elec bays 1	146.071	145.490	0.047	0.080	1.9	1.297	0.000	2.862		81.840	OK
Elec bays 2	145.482	144.900	0.047	0.080	1.9	1.297	0.000	2.869		81.878	OK
Elec bays 3	145.003	144.490	0.046	0.084	1.6	1.126	0.000	2.442		81.660	OK
Elec bays 4	144.542	143.844	0.048	0.076	2.6	1.795	0.000	4.033		81.877	OK
SPEL ESR Ph II	143.046	143.046	0.118	0.118	18.2	0.125	0.000	0.000		96.336	OK
Soakaway SA1	142.435	142.435	0.535	0.535	18.2	26.700	0.000	3.589		55.406	OK
Bio 1	146.409	146.290	0.802	0.850	1.4	1.695	0.001	0.399		-85.216	Flood
Bio 4	145.592	145.342	0.402	0.402	0.3	0.768	0.000	0.154		16.046	OK
Bio 2	146.156	146.054	0.801	0.824	1.5	1.736	0.000	0.373		-89.771	Flood Risk
Bio 3	146.264	146.065	0.801	0.852	0.7	1.188	0.011	0.329		-29.990	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 120 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.368	146.189	0.048	0.053	2.3	1.524	0.000	4.765		79.676	OK
ICE Bays 2	146.279	146.134	0.048	0.054	1.7	1.149	0.000	3.576		79.571	OK
ICE Bays 3	146.063	145.597	0.094	0.172	4.2	4.061	0.000	8.575		63.216	OK
ICE Bays 4	146.014	145.407	0.079	0.165	3.9	3.543	0.000	8.002		67.905	OK
Elec bays 1	146.064	145.480	0.040	0.070	1.7	1.132	0.000	3.534		84.145	OK
Elec bays 2	145.476	144.891	0.040	0.071	1.7	1.142	0.000	3.546		84.038	OK
Elec bays 3	144.996	144.481	0.039	0.075	1.4	0.990	0.000	3.066		83.874	OK
Elec bays 4	144.535	143.835	0.041	0.067	2.3	1.576	0.000	4.909		84.085	OK
SPEL ESR Ph II	143.038	143.038	0.110	0.110	16.3	0.109	0.000	0.000		96.808	OK
Soakaway SA1	142.518	142.518	0.618	0.618	16.3	30.834	0.000	7.420	30	48.503	OK
Bio 1	146.408	146.290	0.802	0.850	1.2	1.915	0.000	0.815		-109.307	Flood Risk
Bio 4	145.593	145.343	0.403	0.403	0.3	0.808	0.000	0.266		11.715	OK
Bio 2	146.155	146.049	0.800	0.819	1.3	1.873	0.000	0.760		-104.710	Flood Risk
Bio 3	146.264	146.065	0.801	0.852	0.6	1.264	0.011	0.625		-38.260	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 120 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.360	146.181	0.040	0.045	1.6	1.276	0.000	4.765		82.989	OK
ICE Bays 2	146.271	146.126	0.040	0.046	1.2	0.963	0.000	3.571		82.887	OK
ICE Bays 3	146.055	145.595	0.085	0.170	3.0	3.922	0.000	8.620		64.473	OK
ICE Bays 4	146.004	145.403	0.070	0.161	2.8	3.373	0.000	8.071		69.448	OK
Elec bays 1	146.055	145.473	0.031	0.063	1.2	0.951	0.000	3.546		86.687	OK
Elec bays 2	145.466	144.882	0.031	0.062	1.2	0.948	0.000	3.546		86.751	OK
Elec bays 3	144.987	144.472	0.030	0.066	1.0	0.823	0.000	3.054		86.594	OK
Elec bays 4	144.526	143.827	0.032	0.059	1.7	1.314	0.000	4.909		86.736	OK
SPEL ESR Ph II	143.018	143.018	0.090	0.090	11.7	0.073	0.000	0.000		97.855	OK
Soakaway SA1	142.519	142.519	0.619	0.619	11.7	30.867	0.000	7.420	30	48.448	OK
Bio 1	146.408	146.288	0.801	0.848	0.9	1.974	0.000	0.825		-115.760	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.2	0.744	0.000	0.198		18.654	OK
Bio 2	146.155	146.039	0.800	0.809	0.8	1.834	0.000	0.769		-100.415	Flood Risk
Bio 3	146.264	146.058	0.801	0.845	0.4	1.321	0.000	0.663		-44.571	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 240 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.350	146.170	0.030	0.034	1.5	0.951	0.000	5.760		87.318	OK
ICE Bays 2	146.260	146.114	0.030	0.034	1.1	0.710	0.000	4.314		87.379	OK
ICE Bays 3	146.041	145.574	0.072	0.149	2.8	3.381	0.000	10.669		69.372	OK
ICE Bays 4	145.992	145.380	0.057	0.138	2.5	2.846	0.000	9.799		74.223	OK
Elec bays 1	146.047	145.456	0.023	0.046	1.1	0.702	0.000	4.278		90.174	OK
Elec bays 2	145.459	144.866	0.023	0.046	1.1	0.699	0.000	4.278		90.231	OK
Elec bays 3	144.980	144.455	0.023	0.049	1.0	0.616	0.000	3.690		89.967	OK
Elec bays 4	144.518	143.812	0.024	0.044	1.5	0.977	0.000	5.947		90.132	OK
SPEL ESR Ph II	143.013	143.013	0.085	0.085	10.7	0.065	0.000	0.000		98.083	OK
Soakaway SA1	142.591	142.591	0.691	0.691	10.7	34.465	0.000	15.155	135	42.439	OK
Bio 1	146.408	146.288	0.801	0.848	0.8	2.030	0.000	1.645		-121.886	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.2	0.759	0.000	0.312		17.005	OK
Bio 2	146.156	146.071	0.801	0.841	0.6	2.012	0.000	1.555		-119.927	Flood Risk
Bio 3	146.264	146.044	0.801	0.831	0.4	1.379	0.000	1.176		-50.891	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 240 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.333	146.151	0.013	0.015	1.0	0.414	0.000	5.736		94.485	OK
ICE Bays 2	146.244	146.095	0.013	0.015	0.8	0.314	0.000	4.296		94.410	OK
ICE Bays 3	146.023	145.555	0.054	0.130	1.9	2.779	0.000	10.663		74.828	OK
ICE Bays 4	145.975	145.358	0.040	0.116	1.7	2.223	0.000	9.799		79.861	OK
Elec bays 1	146.034	145.431	0.010	0.021	0.8	0.318	0.000	4.272		95.552	OK
Elec bays 2	145.445	144.841	0.010	0.021	0.8	0.315	0.000	4.278		95.600	OK
Elec bays 3	144.966	144.428	0.010	0.022	0.6	0.271	0.000	3.666		95.579	OK
Elec bays 4	144.504	143.788	0.010	0.020	1.0	0.430	0.000	5.922		95.655	OK
SPEL ESR Ph II	142.996	142.996	0.068	0.068	7.2	0.038	0.000	0.000		98.894	OK
Soakaway SA1	142.593	142.593	0.693	0.693	7.2	34.556	0.000	15.152	140	42.286	OK
Bio 1	146.407	146.288	0.801	0.848	0.5	2.051	0.000	1.658		-124.104	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.741	0.000	0.306		18.972	OK
Bio 2	146.156	146.071	0.801	0.841	0.5	2.003	0.000	1.581		-118.938	Flood Risk
Bio 3	146.258	146.013	0.795	0.800	0.3	1.345	0.000	1.176		-47.204	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 360 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.335	146.153	0.015	0.017	1.1	0.484	0.000	6.408		93.543	OK
ICE Bays 2	146.246	146.097	0.015	0.017	0.9	0.365	0.000	4.824		93.505	OK
ICE Bays 3	146.023	145.548	0.054	0.123	2.1	2.679	0.000	11.905		75.729	OK
ICE Bays 4	145.976	145.352	0.041	0.110	1.9	2.167	0.000	10.939		80.369	OK
Elec bays 1	146.036	145.433	0.012	0.023	0.8	0.360	0.000	4.752		94.965	OK
Elec bays 2	145.448	144.844	0.012	0.023	0.9	0.363	0.000	4.776		94.924	OK
Elec bays 3	144.969	144.431	0.012	0.025	0.7	0.314	0.000	4.116		94.880	OK
Elec bays 4	144.506	143.790	0.012	0.022	1.2	0.498	0.000	6.612		94.974	OK
SPEL ESR Ph II	143.001	143.001	0.073	0.073	8.2	0.045	0.000	0.000		98.673	OK
Soakaway SA1	142.614	142.614	0.714	0.714	8.2	35.611	0.000	22.737	170	40.524	OK
Bio 1	146.407	146.288	0.801	0.848	0.6	2.049	0.000	2.171		-123.918	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.758	0.000	0.384		17.199	OK
Bio 2	146.156	146.071	0.801	0.841	0.5	2.017	0.000	1.985		-120.490	Flood Risk
Bio 3	146.263	146.019	0.800	0.806	0.3	1.368	0.000	1.350		-49.738	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 360 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.8	0.000	0.000	6.408		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.6	0.000	0.000	4.818		100.000	OK
ICE Bays 3	146.000	145.510	0.031	0.085	1.4	1.700	0.000	11.905		84.599	OK
ICE Bays 4	145.955	145.307	0.021	0.065	1.3	1.200	0.000	10.945		89.131	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.6	0.001	0.000	4.770		99.991	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.6	0.000	0.000	4.782		99.995	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.5	0.000	0.000	4.098		99.997	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.8	0.000	0.000	6.624		100.000	OK
SPEL ESR Ph II	142.986	142.986	0.058	0.058	5.4	0.025	0.000	0.000		99.266	OK
Soakaway SA1	142.618	142.618	0.718	0.718	5.4	35.804	0.000	22.730	180	40.202	OK
Bio 1	146.042	146.287	0.435	0.847	0.4	2.029	0.000	2.171		-121.739	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.726	0.000	0.414		20.683	OK
Bio 2	146.156	146.071	0.801	0.841	0.4	2.001	0.000	2.034		-118.708	Flood Risk
Bio 3	145.867	145.617	0.404	0.404	0.2	1.300	0.000	1.344		-42.196	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 480 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.326	146.143	0.006	0.007	0.9	0.191	0.000	6.954		97.459	OK
ICE Bays 2	146.237	146.087	0.006	0.007	0.7	0.145	0.000	5.202		97.420	OK
ICE Bays 3	146.009	145.522	0.040	0.097	1.7	2.037	0.000	12.883		81.551	OK
ICE Bays 4	145.964	145.322	0.029	0.080	1.6	1.545	0.000	11.827		86.001	OK
Elec bays 1	146.029	145.419	0.005	0.009	0.7	0.143	0.000	5.166		98.004	OK
Elec bays 2	145.441	144.828	0.005	0.008	0.7	0.141	0.000	5.166		98.032	OK
Elec bays 3	144.962	144.415	0.005	0.009	0.6	0.123	0.000	4.446		98.003	OK
Elec bays 4	144.499	143.776	0.005	0.008	1.0	0.194	0.000	7.128		98.037	OK
SPEL ESR Ph II	142.993	142.993	0.066	0.066	6.8	0.034	0.000	0.000		98.989	OK
Soakaway SA1	142.617	142.617	0.717	0.717	6.8	35.783	0.000	30.082	195	40.237	OK
Bio 1	146.374	146.287	0.767	0.847	0.5	2.044	0.000	2.486		-123.386	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.749	0.000	0.450		18.109	OK
Bio 2	146.156	146.071	0.801	0.841	0.4	1.998	0.000	2.131		-118.313	Flood Risk
Bio 3	145.941	145.691	0.478	0.478	0.2	1.336	0.000	1.476		-46.165	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 480 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.6	0.000	0.000	6.954		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	5.214		100.000	OK
ICE Bays 3	145.985	145.466	0.015	0.041	1.1	0.836	0.000	12.864		92.429	OK
ICE Bays 4	145.943	145.264	0.008	0.022	1.0	0.441	0.000	11.844		96.006	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.4	0.000	0.000	5.178		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.4	0.000	0.000	5.178		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	4.464		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.6	0.000	0.000	7.140		100.000	OK
SPEL ESR Ph II	142.979	142.979	0.051	0.051	4.4	0.019	0.000	0.000		99.448	OK
Soakaway SA1	142.623	142.623	0.723	0.723	4.4	36.046	0.000	30.053	190	39.798	OK
Bio 1	146.013	146.287	0.406	0.847	0.3	1.996	0.000	2.506		-118.092	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.698	0.000	0.432		23.676	OK
Bio 2	146.155	146.070	0.800	0.840	0.3	1.939	0.000	2.240		-111.909	Flood Risk
Bio 3	145.866	145.616	0.403	0.403	0.2	1.238	0.000	1.482		-35.477	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 960 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.6	0.000	0.000	8.406		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.4	0.000	0.000	6.342		100.000	OK
ICE Bays 3	145.977	145.441	0.008	0.016	1.1	0.379	0.000	15.606		96.571	OK
ICE Bays 4	145.938	145.248	0.004	0.006	1.0	0.157	0.000	14.346		98.576	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.4	0.000	0.000	6.288		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.4	0.000	0.000	6.288		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	5.370		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.6	0.000	0.000	8.664		100.000	OK
SPEL ESR Ph II	142.978	142.978	0.050	0.050	4.1	0.017	0.000	0.000		99.493	OK
Soakaway SA1	142.595	142.595	0.695	0.695	4.1	34.670	0.000	56.578	325	42.095	OK
Bio 1	146.013	146.287	0.406	0.847	0.3	1.958	0.000	3.467		-114.031	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.697	0.000	0.702		23.841	OK
Bio 2	145.758	145.633	0.403	0.403	0.3	1.666	0.000	2.436		-82.081	OK
Bio 3	145.866	145.616	0.403	0.403	0.2	1.131	0.000	1.860		-23.747	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 960 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	8.406		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	6.324		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.7	0.000	0.000	15.624		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.6	0.000	0.000	14.334		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	6.258		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	6.276		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	5.400		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	8.694		100.000	OK
SPEL ESR Ph II	142.967	142.967	0.039	0.039	2.6	0.010	0.000	0.000		99.718	OK
Soakaway SA1	142.600	142.600	0.700	0.700	2.6	34.891	0.000	56.414	305	41.727	OK
Bio 1	146.011	146.286	0.404	0.846	0.2	1.906	0.000	3.547		-108.294	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.654		23.982	OK
Bio 2	145.757	145.632	0.402	0.402	0.2	1.282	0.000	2.327		-40.119	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.937	0.000	1.926		-2.568	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 1440 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	9.420		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	7.056		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.8	0.000	0.000	17.400		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.7	0.000	0.000	15.984		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	6.978		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	7.002		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	6.006		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	9.672		100.000	OK
SPEL ESR Ph II	142.970	142.970	0.042	0.042	3.0	0.012	0.000	0.000		99.659	OK
Soakaway SA1	142.569	142.569	0.669	0.669	3.0	33.376	0.000	67.804	305	44.256	OK
Bio 1	146.011	146.286	0.405	0.846	0.2	1.906	0.000	4.162		-108.256	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.774		23.981	OK
Bio 2	145.757	145.632	0.402	0.402	0.2	1.256	0.000	2.479		-37.251	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.958	0.000	2.118		-4.802	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +0: 1440 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.3	0.000	0.000	9.366		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.2	0.000	0.000	7.092		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.5	0.000	0.000	17.412		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.5	0.000	0.000	16.068		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.2	0.000	0.000	7.026		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.2	0.000	0.000	7.026		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.2	0.000	0.000	6.006		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.3	0.000	0.000	9.642		100.000	OK
SPEL ESR Ph II	142.962	142.962	0.034	0.034	2.0	0.007	0.000	0.000		99.805	OK
Soakaway SA1	142.556	142.556	0.656	0.656	2.0	32.705	0.000	67.945	260	45.378	OK
Bio 1	146.010	146.286	0.403	0.846	0.2	1.851	0.000	4.518		-102.257	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	0.828		23.984	OK
Bio 2	145.757	145.632	0.402	0.402	0.1	0.765	0.000	2.136		16.373	OK
Bio 3	145.865	145.615	0.402	0.402	0.1	0.710	0.000	2.076		22.301	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 15 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.398	146.218	0.078	0.082	7.5	2.389	0.000	1.299		68.147	OK
ICE Bays 2	146.308	146.162	0.077	0.082	5.6	1.788	0.000	0.974		68.208	OK
ICE Bays 3	146.100	145.588	0.131	0.163	13.8	5.007	0.000	1.516		54.644	OK
ICE Bays 4	146.051	145.397	0.116	0.155	12.7	4.525	0.000	1.512		59.015	OK
Elec bays 1	146.096	145.504	0.072	0.094	5.6	1.775	0.000	0.966		75.136	OK
Elec bays 2	145.508	144.914	0.072	0.094	5.6	1.783	0.000	0.968		75.078	OK
Elec bays 3	145.028	144.502	0.072	0.096	4.8	1.534	0.000	0.830		75.007	OK
Elec bays 4	144.567	143.859	0.073	0.091	7.7	2.461	0.000	1.340		75.153	OK
SPEL ESR Ph II	143.162	143.162	0.234	0.234	45.8	0.404	0.000	0.000		88.171	OK
Soakaway SA1	142.369	142.369	0.469	0.469	43.0	23.375	0.000	0.852		60.961	OK
Bio 1	146.411	146.298	0.805	0.858	4.0	1.021	0.037	0.066		-11.565	Flood
Bio 4	145.991	145.752	0.801	0.812	0.9	0.865	0.000	0.075		5.495	Flood Risk
Bio 2	146.158	146.076	0.803	0.846	5.2	1.543	0.000	0.089		-68.621	Flood Risk
Bio 3	146.265	146.073	0.802	0.860	2.0	0.974	0.046	0.055		-6.552	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 15 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.397	146.218	0.077	0.082	7.0	2.383	0.000	1.305		68.231	OK
ICE Bays 2	146.308	146.162	0.077	0.082	5.3	1.789	0.000	0.978		68.189	OK
ICE Bays 3	146.098	145.588	0.129	0.163	13.0	4.997	0.000	1.520		54.739	OK
ICE Bays 4	146.049	145.396	0.115	0.154	11.9	4.517	0.000	1.517		59.086	OK
Elec bays 1	146.095	145.503	0.071	0.093	5.2	1.771	0.000	0.970		75.201	OK
Elec bays 2	145.506	144.913	0.071	0.093	5.2	1.778	0.000	0.972		75.142	OK
Elec bays 3	145.027	144.501	0.070	0.095	4.5	1.530	0.000	0.834		75.074	OK
Elec bays 4	144.566	143.859	0.072	0.091	7.2	2.458	0.000	1.346		75.189	OK
SPEL ESR Ph II	143.149	143.149	0.221	0.221	43.6	0.369	0.000	0.000		89.175	OK
Soakaway SA1	142.368	142.368	0.468	0.468	41.1	23.341	0.000	0.851		61.017	OK
Bio 1	146.411	146.298	0.804	0.858	3.7	1.031	0.034	0.066		-12.654	Flood
Bio 4	145.991	145.750	0.801	0.810	0.8	0.852	0.000	0.073		6.917	Flood Risk
Bio 2	146.158	146.076	0.803	0.846	4.9	1.555	0.000	0.090		-69.933	Flood Risk
Bio 3	146.265	146.073	0.802	0.860	1.9	0.987	0.046	0.061		-7.999	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 30 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.417	146.240	0.097	0.104	5.1	3.014	0.000	2.605		59.815	OK
ICE Bays 2	146.327	146.184	0.097	0.104	3.8	2.257	0.000	1.953		59.875	OK
ICE Bays 3	146.130	145.654	0.161	0.229	9.4	6.429	0.000	3.037	20	41.762	OK
ICE Bays 4	146.075	145.467	0.141	0.225	8.7	5.801	0.000	3.031	5	47.452	OK
Elec bays 1	146.109	145.538	0.085	0.128	3.8	2.243	0.000	1.937		68.593	OK
Elec bays 2	145.520	144.948	0.085	0.128	3.8	2.247	0.000	1.941		68.592	OK
Elec bays 3	145.040	144.539	0.084	0.133	3.3	1.937	0.000	1.665		68.438	OK
Elec bays 4	144.580	143.891	0.086	0.123	5.2	3.107	0.000	2.688		68.629	OK
SPEL ESR Ph II	143.130	143.130	0.202	0.202	38.5	0.319	0.000	0.000		90.664	OK
Soakaway SA1	142.534	142.534	0.634	0.634	38.0	31.621	0.000	1.849		47.188	OK
Bio 1	146.410	146.295	0.804	0.855	2.7	1.345	0.023	0.184		-46.945	Flood
Bio 4	145.991	145.748	0.801	0.808	1.3	1.126	0.000	0.169		-23.041	Flood Risk
Bio 2	146.162	146.082	0.807	0.852	3.5	1.721	0.011	0.188		-88.064	Flood
Bio 3	146.265	146.079	0.802	0.866	1.4	1.091	0.073	0.168		-19.401	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 30 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.417	146.240	0.097	0.104	4.8	3.017	0.000	2.604		59.777	OK
ICE Bays 2	146.327	146.185	0.097	0.105	3.6	2.266	0.000	1.952		59.720	OK
ICE Bays 3	146.130	145.654	0.161	0.229	8.8	6.436	0.000	3.036	20	41.706	OK
ICE Bays 4	146.074	145.466	0.139	0.224	8.1	5.802	0.000	3.030	5	47.448	OK
Elec bays 1	146.107	145.538	0.083	0.128	3.6	2.245	0.000	1.936		68.554	OK
Elec bays 2	145.519	144.948	0.083	0.128	3.6	2.250	0.000	1.940		68.554	OK
Elec bays 3	145.039	144.539	0.082	0.133	3.1	1.940	0.000	1.664		68.394	OK
Elec bays 4	144.579	143.891	0.085	0.123	4.9	3.107	0.000	2.686		68.635	OK
SPEL ESR Ph II	143.117	143.117	0.189	0.189	35.8	0.285	0.000	0.000		91.651	OK
Soakaway SA1	142.534	142.534	0.634	0.634	35.3	31.609	0.000	1.849		47.208	OK
Bio 1	146.410	146.295	0.803	0.855	2.6	1.343	0.021	0.184		-46.721	Flood
Bio 4	145.990	145.745	0.800	0.805	1.1	1.124	0.000	0.168		-22.830	Flood Risk
Bio 2	146.161	146.082	0.806	0.852	3.3	1.734	0.008	0.190		-89.520	Flood
Bio 3	146.265	146.077	0.802	0.864	1.3	1.098	0.063	0.172		-20.131	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 60 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.423	146.250	0.102	0.114	4.6	3.227	0.000	5.183		56.970	OK
ICE Bays 2	146.333	146.194	0.102	0.114	3.4	2.419	0.000	3.888		57.004	OK
ICE Bays 3	146.144	145.717	0.175	0.292	8.5	7.454	0.000	6.099	40	32.479	OK
ICE Bays 4	146.084	145.530	0.150	0.288	7.8	6.615	0.000	5.911	20	40.081	OK
Elec bays 1	146.109	145.569	0.085	0.159	3.4	2.398	0.000	3.358		66.416	OK
Elec bays 2	145.521	144.979	0.086	0.159	3.4	2.408	0.000	3.365		66.344	OK
Elec bays 3	145.041	144.572	0.084	0.166	2.9	2.077	0.000	2.857		66.163	OK
Elec bays 4	144.582	143.919	0.088	0.151	4.7	3.327	0.000	4.751		66.408	OK
SPEL ESR Ph II	143.113	143.113	0.185	0.185	34.9	0.277	0.000	0.000		91.880	OK
Soakaway SA1	142.708	142.708	0.808	0.808	34.6	40.275	0.000	3.979		32.734	OK
Bio 1	146.410	146.294	0.803	0.854	2.5	1.870	0.019	0.412		-104.352	Flood
Bio 4	145.990	145.743	0.800	0.803	1.4	1.351	0.000	0.359		-47.641	Flood Risk
Bio 2	146.161	146.081	0.806	0.851	3.1	1.926	0.006	0.381		-110.470	Flood
Bio 3	146.265	146.083	0.802	0.870	1.2	1.248	0.090	0.367		-36.611	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 60 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.422	146.250	0.102	0.114	3.7	3.222	0.000	5.181		57.040	OK
ICE Bays 2	146.333	146.195	0.102	0.115	2.8	2.422	0.000	3.888		56.937	OK
ICE Bays 3	146.141	145.719	0.172	0.294	6.8	7.516	0.000	6.070	40	31.917	OK
ICE Bays 4	146.082	145.532	0.147	0.290	6.3	6.680	0.000	5.882	20	39.489	OK
Elec bays 1	146.107	145.569	0.083	0.159	2.7	2.396	0.000	3.353		66.441	OK
Elec bays 2	145.519	144.979	0.083	0.159	2.7	2.400	0.000	3.359		66.459	OK
Elec bays 3	145.038	144.571	0.082	0.165	2.4	2.070	0.000	2.855		66.271	OK
Elec bays 4	144.580	143.918	0.086	0.150	3.8	3.316	0.000	4.749		66.521	OK
SPEL ESR Ph II	143.087	143.087	0.159	0.159	28.4	0.213	0.000	0.000		93.748	OK
Soakaway SA1	142.706	142.706	0.806	0.806	28.4	40.203	0.000	3.977		32.855	OK
Bio 1	146.409	146.293	0.803	0.853	2.0	1.882	0.012	0.413		-105.632	Flood
Bio 4	145.595	145.345	0.405	0.405	1.1	1.328	0.000	0.359		-45.083	OK
Bio 2	146.160	146.079	0.805	0.849	2.4	1.984	0.000	0.387		-116.855	Flood Risk
Bio 3	146.265	146.078	0.802	0.865	1.0	1.266	0.068	0.379		-38.517	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 120 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.414	146.241	0.094	0.105	3.3	2.972	0.000	6.913		60.378	OK
ICE Bays 2	146.324	146.186	0.093	0.106	2.5	2.227	0.000	5.184		60.414	OK
ICE Bays 3	146.130	145.744	0.161	0.319	6.1	7.154	0.000	10.421	50	35.200	OK
ICE Bays 4	146.069	145.555	0.134	0.313	5.6	6.302	0.000	9.469	35	42.917	OK
Elec bays 1	146.098	145.575	0.074	0.165	2.4	2.206	0.000	5.092		69.101	OK
Elec bays 2	145.510	144.985	0.074	0.165	2.4	2.216	0.000	5.101		69.032	OK
Elec bays 3	145.029	144.578	0.073	0.172	2.1	1.905	0.000	4.343		68.967	OK
Elec bays 4	144.571	143.923	0.077	0.155	3.4	3.058	0.000	7.129		69.131	OK
SPEL ESR Ph II	143.073	143.073	0.145	0.145	25.1	0.182	0.000	0.000		94.671	OK
Soakaway SA1	142.849	142.849	0.949	0.949	24.8	47.326	0.000	8.376		20.959	OK
Bio 1	146.409	146.292	0.803	0.852	1.8	2.011	0.008	0.852		-119.807	Flood
Bio 4	145.595	145.345	0.405	0.405	1.0	1.386	0.000	0.661		-51.517	OK
Bio 2	146.160	146.078	0.805	0.848	2.1	2.034	0.000	0.786		-122.295	Flood Risk
Bio 3	146.265	146.078	0.802	0.865	0.9	1.423	0.067	0.737		-55.720	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 120 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.407	146.237	0.087	0.101	2.4	2.812	0.000	6.907		62.508	OK
ICE Bays 2	146.317	146.182	0.087	0.101	1.8	2.108	0.000	5.178		62.530	OK
ICE Bays 3	146.121	145.744	0.152	0.319	4.4	7.150	0.000	10.430	45	35.233	OK
ICE Bays 4	146.059	145.555	0.124	0.313	4.0	6.279	0.000	9.477	35	43.125	OK
Elec bays 1	146.089	145.570	0.065	0.160	1.8	2.090	0.000	5.127		70.731	OK
Elec bays 2	145.500	144.981	0.065	0.161	1.8	2.107	0.000	5.146		70.548	OK
Elec bays 3	145.019	144.574	0.063	0.168	1.5	1.811	0.000	4.381		70.487	OK
Elec bays 4	144.562	143.918	0.068	0.150	2.4	2.892	0.000	7.123		70.802	OK
SPEL ESR Ph II	143.041	143.041	0.113	0.113	17.0	0.115	0.000	0.000		96.638	OK
Soakaway SA1	142.849	142.849	0.949	0.949	17.0	47.336	0.000	8.373		20.942	OK
Bio 1	146.409	146.290	0.802	0.850	1.3	2.051	0.000	0.854		-124.154	Flood Risk
Bio 4	145.594	145.344	0.404	0.404	0.6	1.301	0.000	0.630		-42.220	OK
Bio 2	146.158	146.075	0.803	0.845	1.4	2.049	0.000	0.798		-123.937	Flood Risk
Bio 3	146.264	146.072	0.801	0.859	0.6	1.488	0.043	0.778		-62.805	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 240 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.393	146.222	0.073	0.086	2.2	2.375	0.000	8.353		68.338	OK
ICE Bays 2	146.303	146.167	0.073	0.087	1.6	1.781	0.000	6.258		68.330	OK
ICE Bays 3	146.103	145.738	0.134	0.313	4.0	6.565	0.000	15.481	50	40.539	OK
ICE Bays 4	146.043	145.551	0.108	0.309	3.7	5.692	0.000	14.215	15	48.443	OK
Elec bays 1	146.077	145.548	0.053	0.138	1.6	1.763	0.000	6.204		75.309	OK
Elec bays 2	145.489	144.957	0.053	0.137	1.6	1.759	0.000	6.210		75.407	OK
Elec bays 3	145.009	144.552	0.052	0.146	1.4	1.530	0.000	5.358		75.073	OK
Elec bays 4	144.550	143.896	0.056	0.128	2.2	2.440	0.000	8.611		75.369	OK
SPEL ESR Ph II	143.035	143.035	0.107	0.107	15.5	0.103	0.000	0.000		96.976	OK
Soakaway SA1	142.982	142.982	1.082	1.082	15.5	53.980	0.000	17.424		9.846	OK
Bio 1	146.408	146.291	0.802	0.851	1.2	2.061	0.003	1.734		-125.277	Flood
Bio 4	145.594	145.344	0.404	0.404	0.5	1.243	0.000	0.902		-35.877	OK
Bio 2	146.159	146.075	0.804	0.845	1.5	2.051	0.000	1.608		-124.140	Flood Risk
Bio 3	146.264	146.071	0.801	0.858	0.6	1.679	0.035	1.428		-83.671	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 240 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.375	146.203	0.055	0.067	1.5	1.820	0.000	8.341		75.727	OK
ICE Bays 2	146.285	146.148	0.055	0.068	1.1	1.369	0.000	6.258		75.664	OK
ICE Bays 3	146.082	145.731	0.113	0.306	2.7	6.197	0.000	15.469	35	43.870	OK
ICE Bays 4	146.021	145.540	0.087	0.298	2.5	5.250	0.000	14.221		52.442	OK
Elec bays 1	146.060	145.524	0.036	0.114	1.1	1.356	0.000	6.204		81.003	OK
Elec bays 2	145.472	144.934	0.036	0.114	1.1	1.364	0.000	6.216		80.938	OK
Elec bays 3	144.992	144.528	0.035	0.122	0.9	1.184	0.000	5.364		80.705	OK
Elec bays 4	144.533	143.873	0.038	0.105	1.5	1.870	0.000	8.593		81.125	OK
SPEL ESR Ph II	143.012	143.012	0.084	0.084	10.4	0.063	0.000	0.000		98.148	OK
Soakaway SA1	142.985	142.985	1.085	1.085	10.4	54.094	0.000	17.419		9.656	OK
Bio 1	146.408	146.290	0.801	0.850	0.8	2.064	0.000	1.740		-125.531	Flood Risk
Bio 4	145.593	145.343	0.403	0.403	0.2	1.076	0.000	0.842		-17.610	OK
Bio 2	146.158	146.075	0.803	0.845	1.0	2.050	0.000	1.635		-124.040	Flood Risk
Bio 3	146.264	146.065	0.801	0.852	0.4	1.780	0.010	1.468		-94.801	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 360 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.374	146.201	0.054	0.065	1.7	1.776	0.000	9.294		76.322	OK
ICE Bays 2	146.284	146.145	0.053	0.065	1.2	1.323	0.000	6.966		76.482	OK
ICE Bays 3	146.080	145.714	0.111	0.289	3.0	5.859	0.000	17.239	25	46.932	OK
ICE Bays 4	146.021	145.523	0.086	0.281	2.8	4.984	0.000	15.841		54.857	OK
Elec bays 1	146.061	145.517	0.037	0.107	1.2	1.307	0.000	6.900		81.691	OK
Elec bays 2	145.473	144.927	0.038	0.107	1.2	1.315	0.000	6.936		81.617	OK
Elec bays 3	144.993	144.520	0.036	0.114	1.1	1.139	0.000	5.964		81.439	OK
Elec bays 4	144.534	143.867	0.039	0.099	1.7	1.815	0.000	9.565		81.677	OK
SPEL ESR Ph II	143.036	143.036	0.108	0.108	11.8	0.107	0.000	0.000		96.877	OK
Soakaway SA1	143.036	143.036	1.136	1.136	11.8	56.678	0.000	26.413		5.340	OK
Bio 1	146.408	146.290	0.801	0.850	0.9	2.065	0.000	2.315		-125.672	Flood Risk
Bio 4	145.593	145.343	0.403	0.403	0.3	1.123	0.000	0.985		-22.787	OK
Bio 2	146.158	146.076	0.803	0.846	1.1	2.052	0.000	2.171		-124.238	Flood Risk
Bio 3	146.264	146.066	0.801	0.853	0.4	1.784	0.014	1.895		-95.239	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 360 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.348	146.171	0.028	0.035	1.1	0.936	0.000	9.294		87.517	OK
ICE Bays 2	146.259	146.116	0.028	0.036	0.8	0.710	0.000	6.960		87.382	OK
ICE Bays 3	146.051	145.689	0.082	0.264	2.0	5.004	0.000	17.239		54.670	OK
ICE Bays 4	145.994	145.490	0.060	0.248	1.8	4.072	0.000	15.835		63.120	OK
Elec bays 1	146.042	145.472	0.018	0.062	0.8	0.703	0.000	6.906		90.156	OK
Elec bays 2	145.454	144.882	0.018	0.062	0.8	0.704	0.000	6.912		90.159	OK
Elec bays 3	144.974	144.472	0.017	0.066	0.7	0.612	0.000	5.940		90.035	OK
Elec bays 4	144.513	143.824	0.019	0.056	1.1	0.967	0.000	9.576		90.239	OK
SPEL ESR Ph II	143.041	143.041	0.113	0.113	7.8	0.115	0.000	0.000		96.627	OK
Soakaway SA1	143.041	143.041	1.141	1.141	7.8	56.897	0.000	26.401		4.974	OK
Bio 1	146.408	146.289	0.801	0.849	0.6	2.058	0.000	2.301		-124.959	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.994	0.000	0.913		-8.638	OK
Bio 2	146.157	146.074	0.802	0.844	0.8	2.044	0.000	2.204		-123.415	Flood Risk
Bio 3	146.263	146.065	0.800	0.852	0.3	1.828	0.007	1.943		-100.065	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 480 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.358	146.182	0.038	0.046	1.4	1.240	0.000	10.056		83.467	OK
ICE Bays 2	146.268	146.127	0.038	0.047	1.0	0.939	0.000	7.548		83.302	OK
ICE Bays 3	146.061	145.690	0.092	0.265	2.5	5.173	0.000	18.661		53.145	OK
ICE Bays 4	146.005	145.496	0.070	0.254	2.3	4.301	0.000	17.161		61.045	OK
Elec bays 1	146.050	145.488	0.026	0.078	1.0	0.927	0.000	7.476		87.014	OK
Elec bays 2	145.462	144.898	0.026	0.078	1.0	0.941	0.000	7.512		86.847	OK
Elec bays 3	144.982	144.490	0.025	0.084	0.9	0.813	0.000	6.468		86.752	OK
Elec bays 4	144.522	143.840	0.028	0.072	1.4	1.291	0.000	10.374		86.966	OK
SPEL ESR Ph II	143.061	143.061	0.133	0.133	9.8	0.156	0.000	0.000		95.430	OK
Soakaway SA1	143.061	143.061	1.161	1.161	9.8	57.895	0.000	35.212		3.307	OK
Bio 1	146.408	146.290	0.801	0.849	0.7	2.063	0.000	2.698		-125.460	Flood Risk
Bio 4	145.593	145.343	0.403	0.403	0.2	1.054	0.000	1.042		-15.185	OK
Bio 2	146.158	146.075	0.803	0.845	1.0	2.048	0.000	2.409		-123.826	Flood Risk
Bio 3	146.264	146.065	0.801	0.852	0.4	1.809	0.007	2.144		-97.985	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 480 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.329	146.147	0.009	0.011	0.9	0.305	0.000	10.092		95.928	OK
ICE Bays 2	146.240	146.091	0.009	0.011	0.7	0.224	0.000	7.548		96.010	OK
ICE Bays 3	146.028	145.646	0.059	0.221	1.6	3.895	0.000	18.661		64.716	OK
ICE Bays 4	145.976	145.440	0.041	0.198	1.5	3.006	0.000	17.149		72.769	OK
Elec bays 1	146.030	145.429	0.006	0.019	0.7	0.233	0.000	7.500		96.730	OK
Elec bays 2	145.442	144.839	0.007	0.019	0.7	0.238	0.000	7.524		96.673	OK
Elec bays 3	144.963	144.427	0.006	0.021	0.6	0.212	0.000	6.486		96.546	OK
Elec bays 4	144.501	143.785	0.007	0.017	0.9	0.321	0.000	10.350		96.762	OK
SPEL ESR Ph II	143.067	143.067	0.139	0.139	6.5	0.169	0.000	0.000		95.050	OK
Soakaway SA1	143.067	143.067	1.167	1.167	6.5	58.196	0.000	35.191		2.805	OK
Bio 1	146.407	146.288	0.801	0.848	0.5	2.055	0.000	2.669		-124.550	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.885	0.000	0.975		3.307	OK
Bio 2	146.157	146.074	0.802	0.844	0.6	2.040	0.000	2.467		-122.939	Flood Risk
Bio 3	145.907	146.064	0.444	0.851	0.2	1.801	0.003	2.187		-97.085	Flood

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 960 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.323	146.139	0.003	0.003	0.8	0.096	0.000	12.192		98.724	OK
ICE Bays 2	146.234	146.084	0.003	0.004	0.6	0.073	0.000	9.144		98.703	OK
ICE Bays 3	146.012	145.582	0.043	0.157	1.5	2.664	0.000	22.584		75.866	OK
ICE Bays 4	145.964	145.371	0.029	0.129	1.4	1.917	0.000	20.742		82.634	OK
Elec bays 1	146.026	145.414	0.002	0.004	0.6	0.070	0.000	9.078		99.014	OK
Elec bays 2	145.438	144.824	0.002	0.004	0.6	0.068	0.000	9.090		99.047	OK
Elec bays 3	144.959	144.411	0.003	0.005	0.5	0.067	0.000	7.842		98.907	OK
Elec bays 4	144.497	143.772	0.003	0.004	0.9	0.100	0.000	12.570		98.990	OK
SPEL ESR Ph II	143.067	143.067	0.139	0.139	6.0	0.168	0.000	0.000		95.069	OK
Soakaway SA1	143.067	143.067	1.167	1.167	6.0	58.181	0.000	67.471	915	2.830	OK
Bio 1	146.407	146.288	0.801	0.848	0.4	2.054	0.000	4.005		-124.429	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.787	0.000	1.098		13.944	OK
Bio 2	146.157	146.073	0.802	0.843	0.6	2.036	0.000	3.151		-122.494	Flood Risk
Bio 3	145.867	146.059	0.404	0.846	0.2	1.712	0.000	2.898		-87.332	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 960 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.5	0.000	0.000	12.186		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.4	0.000	0.000	9.144		100.000	OK
ICE Bays 3	145.978	145.458	0.009	0.033	1.0	0.558	0.000	22.602		94.945	OK
ICE Bays 4	145.937	145.248	0.002	0.006	0.9	0.125	0.000	20.760		98.866	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.4	0.000	0.000	9.078		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.4	0.000	0.000	9.084		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	7.812		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.6	0.000	0.000	12.600		100.000	OK
SPEL ESR Ph II	143.068	143.068	0.140	0.140	3.9	0.171	0.000	0.000		94.979	OK
Soakaway SA1	143.068	143.068	1.168	1.168	3.8	58.251	0.000	67.314	905	2.712	OK
Bio 1	146.012	146.287	0.406	0.847	0.3	2.022	0.000	4.033		-121.019	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.1	0.697	0.000	1.080		23.841	OK
Bio 2	146.156	146.072	0.801	0.842	0.3	2.029	0.000	3.307		-121.757	Flood Risk
Bio 3	145.866	146.019	0.403	0.806	0.1	1.509	0.000	2.934		-65.119	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 1440 mins: Summer

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.6	0.000	0.000	13.602		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.5	0.000	0.000	10.194		100.000	OK
ICE Bays 3	145.987	145.482	0.017	0.057	1.1	0.999	0.000	25.272		90.954	OK
ICE Bays 4	145.944	145.272	0.009	0.030	1.0	0.525	0.000	23.190		95.248	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.5	0.000	0.000	10.092		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.5	0.000	0.000	10.116		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.4	0.000	0.000	8.694		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.6	0.000	0.000	14.052		100.000	OK
SPEL ESR Ph II	143.033	143.033	0.105	0.105	4.4	0.101	0.000	0.000		97.046	OK
Soakaway SA1	143.033	143.033	1.133	1.133	4.4	56.519	0.000	94.491	995	5.605	OK
Bio 1	146.407	146.287	0.800	0.847	0.3	2.046	0.000	4.957		-123.625	Flood Risk
Bio 4	145.592	145.342	0.402	0.402	0.1	0.712	0.000	1.356		22.171	OK
Bio 2	146.156	146.072	0.801	0.842	0.4	2.029	0.000	3.703		-121.765	Flood Risk
Bio 3	145.866	146.019	0.403	0.806	0.2	1.512	0.000	3.318		-65.395	Flood Risk

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 1440 mins: Winter

Stormwater Control	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded	Total Lost Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)	Status
ICE Bays 1	146.320	146.136	0.000	0.000	0.4	0.000	0.000	13.620		100.000	OK
ICE Bays 2	146.231	146.080	0.000	0.000	0.3	0.000	0.000	10.134		100.000	OK
ICE Bays 3	145.969	145.425	0.000	0.000	0.7	0.000	0.000	25.224		100.000	OK
ICE Bays 4	145.935	145.242	0.000	0.000	0.7	0.000	0.000	23.190		100.000	OK
Elec bays 1	146.024	145.410	0.000	0.000	0.3	0.000	0.000	10.098		100.000	OK
Elec bays 2	145.436	144.820	0.000	0.000	0.3	0.000	0.000	10.110		100.000	OK
Elec bays 3	144.957	144.406	0.000	0.000	0.3	0.000	0.000	8.730		100.000	OK
Elec bays 4	144.494	143.768	0.000	0.000	0.4	0.000	0.000	13.974		100.000	OK
SPEL ESR Ph II	143.027	143.027	0.099	0.099	2.9	0.089	0.000	0.000		97.385	OK
Soakaway SA1	143.027	143.027	1.127	1.127	2.9	56.204	0.000	94.036	935	6.132	OK
Bio 1	146.011	146.287	0.404	0.847	0.2	1.956	0.000	5.153		-113.809	Flood Risk
Bio 4	145.591	145.341	0.401	0.401	0.0	0.696	0.000	1.314		23.981	OK
Bio 2	146.156	146.072	0.801	0.842	0.2	2.023	0.000	4.089		-121.067	Flood Risk
Bio 3	145.865	145.615	0.402	0.402	0.1	1.188	0.000	3.312		-30.047	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 15 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.050	1.009	0.9	0.08	2.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.040	0.586	0.7	0.06	1.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.063	1.012	0.5	0.18	2.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	2.827	1.3	0.1	6.0	OK
SW2.001	Pipe	RG3	SW3	0.063	1.151	1.1	0.06	2.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.050	0.583	0.3	0.07	1.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.093	6.180	0.8	0.33	12.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.088	6.182	0.8	0.24	11.9	OK
SW1.001	Pipe	SW1	SW2	0.043	1.608	0.8	0.07	3.6	OK
SW1.000	Pipe	RG1	SW1	0.037	1.609	1.1	0.13	3.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.080	6.180	0.9	0.24	11.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.079	4.427	1.0	0.33	9.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 15 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.048	1.006	0.9	0.08	2.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.038	0.583	0.7	0.05	1.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.061	1.018	0.5	0.17	2.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	2.833	1.3	0.1	5.7	OK
SW2.001	Pipe	RG3	SW3	0.063	1.148	1.1	0.06	2.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.048	0.580	0.3	0.06	1.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.090	6.179	0.8	0.31	11.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.085	6.181	0.8	0.23	11.2	OK
SW1.001	Pipe	SW1	SW2	0.042	1.605	0.8	0.07	3.4	OK
SW1.000	Pipe	RG1	SW1	0.036	1.606	1.0	0.12	3.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.078	6.179	0.9	0.23	11.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.076	4.430	1.0	0.31	8.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 30 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.040	1.331	0.8	0.06	1.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.032	0.778	0.6	0.04	0.9	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.052	1.345	0.4	0.13	1.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	3.700	1.2	0.07	4.1	OK
SW2.001	Pipe	RG3	SW3	0.063	1.473	1.0	0.04	1.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.040	0.777	0.3	0.05	0.9	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.079	8.096	0.7	0.25	9.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.076	8.099	0.8	0.19	9.0	OK
SW1.001	Pipe	SW1	SW2	0.036	2.131	0.8	0.05	2.5	OK
SW1.000	Pipe	RG1	SW1	0.030	2.131	1.0	0.09	2.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.070	8.123	0.9	0.19	9.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.066	5.824	0.9	0.23	6.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 30 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.038	1.325	0.8	0.05	1.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.031	0.773	0.6	0.04	0.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.050	1.351	0.4	0.12	1.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	3.701	1.2	0.07	3.9	OK
SW2.001	Pipe	RG3	SW3	0.063	1.467	1.0	0.04	1.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.038	0.771	0.3	0.05	0.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.076	8.072	0.7	0.23	8.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.073	8.075	0.8	0.17	8.4	OK
SW1.001	Pipe	SW1	SW2	0.035	2.119	0.7	0.05	2.3	OK
SW1.000	Pipe	RG1	SW1	0.029	2.119	0.9	0.08	2.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.068	8.102	0.8	0.18	8.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.064	5.812	0.9	0.22	6.1	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 60 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.037	1.697	0.8	0.05	1.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.030	0.989	0.6	0.04	0.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.049	1.725	0.4	0.11	1.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	4.693	1.1	0.06	3.6	OK
SW2.001	Pipe	RG3	SW3	0.063	1.839	1.0	0.04	1.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.037	0.987	0.3	0.04	0.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.074	10.246	0.7	0.22	7.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.070	10.254	0.7	0.16	7.9	OK
SW1.001	Pipe	SW1	SW2	0.034	2.713	0.7	0.04	2.2	OK
SW1.000	Pipe	RG1	SW1	0.029	2.713	0.9	0.08	2.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.066	10.280	0.8	0.17	7.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.062	7.399	0.8	0.2	5.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 2 years: Increase Rainfall (%): +0: 60 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.032	1.697	0.7	0.04	1.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.027	0.995	0.6	0.03	0.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.044	1.719	0.4	0.09	1.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	4.687	1.1	0.05	2.9	OK
SW2.001	Pipe	RG3	SW3	0.063	1.839	0.9	0.03	1.1	OK
SW2.002	Pipe	M100D30.2	SW3	0.031	0.993	0.3	0.04	0.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.066	10.239	0.7	0.17	6.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.062	10.248	0.7	0.13	6.4	OK
SW1.001	Pipe	SW1	SW2	0.030	2.707	0.7	0.04	1.7	OK
SW1.000	Pipe	RG1	SW1	0.026	2.707	0.9	0.06	1.7	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.059	10.270	0.8	0.14	6.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.055	7.387	0.8	0.16	4.7	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 120 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.033	2.369	0.7	0.04	1.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.027	1.385	0.6	0.03	0.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.044	2.404	0.4	0.09	1.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	6.476	1.1	0.05	3.0	OK
SW2.001	Pipe	RG3	SW3	0.063	2.511	0.9	0.03	1.1	OK
SW2.002	Pipe	M100D30.2	SW3	0.032	1.383	0.3	0.04	0.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.067	14.166	0.7	0.18	6.6	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.063	14.183	0.7	0.14	6.6	OK
SW1.001	Pipe	SW1	SW2	0.031	3.781	0.7	0.04	1.8	OK
SW1.000	Pipe	RG1	SW1	0.026	3.781	0.9	0.06	1.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.059	14.204	0.8	0.14	6.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.056	10.251	0.8	0.17	4.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 120 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.027	2.363	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	1.373	0.5	0.02	0.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.037	2.416	0.3	0.07	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	6.488	1.0	0.04	2.2	OK
SW2.001	Pipe	RG3	SW3	0.063	2.505	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.025	1.371	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.056	14.153	0.6	0.13	4.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.053	14.174	0.7	0.1	4.7	OK
SW1.001	Pipe	SW1	SW2	0.026	3.775	0.6	0.03	1.3	OK
SW1.000	Pipe	RG1	SW1	0.022	3.775	0.8	0.05	1.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.050	14.197	0.7	0.1	4.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.047	10.257	0.7	0.12	3.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 240 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.027	3.125	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	1.823	0.5	0.02	0.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.037	3.178	0.3	0.07	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	8.528	1.0	0.04	2.2	OK
SW2.001	Pipe	RG3	SW3	0.063	3.267	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.025	1.821	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.056	18.606	0.6	0.13	4.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.053	18.627	0.7	0.1	4.7	OK
SW1.001	Pipe	SW1	SW2	0.026	4.975	0.6	0.03	1.3	OK
SW1.000	Pipe	RG1	SW1	0.022	4.975	0.8	0.05	1.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.050	18.652	0.7	0.1	4.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.047	13.498	0.7	0.12	3.5	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 240 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.022	3.131	0.6	0.02	0.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.018	1.835	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.031	3.184	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	8.528	0.9	0.02	1.5	OK
SW2.001	Pipe	RG3	SW3	0.063	3.273	0.7	0.01	0.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.018	1.833	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.046	18.629	0.5	0.09	3.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.043	18.657	0.6	0.07	3.2	OK
SW1.001	Pipe	SW1	SW2	0.022	4.981	0.6	0.02	0.9	OK
SW1.000	Pipe	RG1	SW1	0.018	4.981	0.7	0.03	0.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.041	18.677	0.6	0.07	3.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.038	13.504	0.7	0.08	2.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 360 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.024	3.593	0.7	0.02	0.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.020	2.093	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.033	3.646	0.3	0.05	0.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	9.746	0.9	0.03	1.7	OK
SW2.001	Pipe	RG3	SW3	0.063	3.735	0.8	0.02	0.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.021	2.091	0.3	0.02	0.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.050	21.324	0.6	0.1	3.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.047	21.347	0.6	0.08	3.7	OK
SW1.001	Pipe	SW1	SW2	0.023	5.737	0.6	0.02	1.0	OK
SW1.000	Pipe	RG1	SW1	0.020	5.737	0.7	0.04	1.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.044	21.372	0.7	0.08	3.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.041	15.478	0.7	0.1	2.7	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 360 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.019	3.575	0.6	0.02	0.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.016	2.081	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.027	3.664	0.3	0.04	0.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	9.746	0.8	0.02	1.1	OK
SW2.001	Pipe	RG3	SW3	0.063	3.717	0.7	0.01	0.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.014	2.079	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.040	21.281	0.5	0.07	2.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.037	21.312	0.6	0.05	2.5	OK
SW1.001	Pipe	SW1	SW2	0.019	5.725	0.5	0.01	0.7	OK
SW1.000	Pipe	RG1	SW1	0.016	5.725	0.7	0.02	0.7	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.036	21.342	0.6	0.05	2.5	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.034	15.466	0.6	0.06	1.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 480 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.022	3.917	0.6	0.02	0.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.018	2.279	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.030	3.988	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	10.628	0.9	0.02	1.4	OK
SW2.001	Pipe	RG3	SW3	0.063	4.059	0.7	0.01	0.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.018	2.277	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.045	23.220	0.5	0.08	3.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.042	23.246	0.6	0.06	3.1	OK
SW1.001	Pipe	SW1	SW2	0.021	6.241	0.6	0.02	0.9	OK
SW1.000	Pipe	RG1	SW1	0.018	6.241	0.7	0.03	0.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.040	23.265	0.6	0.07	3.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.038	16.864	0.6	0.08	2.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 480 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.017	3.905	0.5	0.01	0.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.014	2.303	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.025	3.982	0.3	0.03	0.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	10.628	0.8	0.02	0.9	OK
SW2.001	Pipe	RG3	SW3	0.063	4.047	0.6	0.01	0.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.012	2.301	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.036	23.255	0.5	0.05	2.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.034	23.289	0.5	0.04	2.0	OK
SW1.001	Pipe	SW1	SW2	0.017	6.265	0.5	0.01	0.6	OK
SW1.000	Pipe	RG1	SW1	0.015	6.265	0.6	0.02	0.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.032	23.334	0.6	0.04	2.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.030	16.888	0.6	0.05	1.5	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 960 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.017	4.781	0.5	0.01	0.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.014	2.819	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.024	4.853	0.3	0.03	0.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	12.938	0.7	0.01	0.9	OK
SW2.001	Pipe	RG3	SW3	0.063	4.923	0.6	0.01	0.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.011	2.817	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.035	28.344	0.5	0.05	1.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.033	28.564	0.5	0.04	1.9	OK
SW1.001	Pipe	SW1	SW2	0.017	7.651	0.5	0.01	0.5	OK
SW1.000	Pipe	RG1	SW1	0.014	7.651	0.6	0.02	0.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.032	28.567	0.6	0.04	1.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.030	20.584	0.6	0.05	1.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 960 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.013	4.805	0.5	0.01	0.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.011	2.771	0.4	0.01	0.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.019	4.858	0.2	0.02	0.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	12.932	0.6	0.01	0.6	OK
SW2.001	Pipe	RG3	SW3	0.063	4.947	0.6	0.01	0.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.009	2.769	0.3	0.01	0.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.028	28.265	0.4	0.03	1.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.026	28.365	0.5	0.03	1.2	OK
SW1.001	Pipe	SW1	SW2	0.014	7.603	0.4	0.01	0.3	OK
SW1.000	Pipe	RG1	SW1	0.012	7.603	0.5	0.01	0.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.025	28.430	0.5	0.03	1.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.024	20.530	0.5	0.03	0.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 1440 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.014	5.303	0.5	0.01	0.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.012	3.059	0.4	0.01	0.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.021	5.411	0.2	0.02	0.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	14.432	0.7	0.01	0.6	OK
SW2.001	Pipe	RG3	SW3	0.063	5.445	0.6	0.01	0.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.010	3.058	0.3	0.01	0.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.031	31.632	0.4	0.04	1.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.028	32.298	0.5	0.03	1.4	OK
SW1.001	Pipe	SW1	SW2	0.015	8.683	0.4	0.01	0.4	OK
SW1.000	Pipe	RG1	SW1	0.012	8.683	0.6	0.01	0.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.027	32.258	0.5	0.03	1.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.026	23.110	0.5	0.04	1.0	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 2 years: Increase Rainfall (%): +0: 1440 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.012	5.363	0.4	0.01	0.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.010	3.167	0.3	0	0.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.017	5.470	0.2	0.01	0.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	14.492	0.6	0.01	0.4	OK
SW2.001	Pipe	RG3	SW3	0.063	5.505	0.5	0	0.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.008	3.165	0.3	0.01	0.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.025	31.799	0.4	0.03	0.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.023	32.277	0.4	0.02	0.9	OK
SW1.001	Pipe	SW1	SW2	0.012	8.623	0.4	0.01	0.3	OK
SW1.000	Pipe	RG1	SW1	0.010	8.623	0.5	0.01	0.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.022	32.324	0.5	0.02	0.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.021	23.110	0.5	0.02	0.7	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 15 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.069	1.675	1.0	0.14	3.7	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.055	0.975	0.7	0.1	2.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.082	1.693	0.6	0.31	3.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	4.621	1.5	0.17	10.1	OK
SW2.001	Pipe	RG3	SW3	0.063	1.816	1.2	0.1	3.7	OK
SW2.002	Pipe	M100D30.2	SW3	0.072	0.973	0.3	0.12	2.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.127	10.101	0.9	0.56	20.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.120	10.102	0.9	0.41	20.0	OK
SW1.001	Pipe	SW1	SW2	0.059	2.676	0.9	0.12	6.0	OK
SW1.000	Pipe	RG1	SW1	0.049	2.677	1.2	0.21	6.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.108	10.099	1.0	0.41	19.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.004	0.105	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.108	7.288	1.1	0.55	15.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 15 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.067	1.675	0.9	0.13	3.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.053	0.975	0.7	0.09	2.0	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.080	1.693	0.5	0.29	3.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	4.615	1.5	0.16	9.5	OK
SW2.001	Pipe	RG3	SW3	0.063	1.816	1.3	0.09	3.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.069	0.973	0.3	0.11	2.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.122	10.092	0.9	0.53	19.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.116	10.094	0.9	0.39	18.9	OK
SW1.001	Pipe	SW1	SW2	0.056	2.673	0.9	0.12	5.6	OK
SW1.000	Pipe	RG1	SW1	0.047	2.674	1.2	0.2	5.7	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.105	10.091	1.0	0.39	18.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.007	0.088	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.104	7.279	1.1	0.52	14.7	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 30 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.055	2.230	0.9	0.09	2.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.044	1.294	0.7	0.07	1.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.068	2.263	0.5	0.21	2.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	6.112	1.4	0.12	6.9	OK
SW2.001	Pipe	RG3	SW3	0.063	2.372	1.2	0.07	2.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.057	1.293	0.3	0.08	1.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.105	13.364	0.8	0.41	15.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.102	13.367	0.9	0.31	15.1	OK
SW1.001	Pipe	SW1	SW2	0.047	3.571	0.9	0.08	4.1	OK
SW1.000	Pipe	RG1	SW1	0.040	3.571	1.1	0.15	4.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.093	13.394	1.0	0.32	15.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.018	0.109	0.0		0.5	
SW1.002	Pipe	SW2	SW3	0.088	9.676	1.0	0.39	11.0	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 30 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.053	2.230	0.9	0.09	2.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.043	1.300	0.7	0.06	1.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.066	2.263	0.5	0.2	2.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	6.118	1.3	0.11	6.5	OK
SW2.001	Pipe	RG3	SW3	0.063	2.373	1.1	0.07	2.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.055	1.299	0.3	0.08	1.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.101	13.365	0.8	0.39	14.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.098	13.368	0.9	0.29	14.2	OK
SW1.001	Pipe	SW1	SW2	0.046	3.559	0.8	0.08	3.8	OK
SW1.000	Pipe	RG1	SW1	0.038	3.559	1.1	0.14	3.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.090	13.394	1.0	0.3	14.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.013	0.081	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.085	9.670	1.0	0.36	10.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 60 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.051	2.873	0.9	0.08	2.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.041	1.673	0.7	0.06	1.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.064	2.907	0.5	0.19	2.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	7.813	1.3	0.1	6.1	OK
SW2.001	Pipe	RG3	SW3	0.063	3.015	1.1	0.06	2.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.053	1.671	0.3	0.08	1.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.098	17.081	0.8	0.37	13.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.095	17.089	0.8	0.28	13.4	OK
SW1.001	Pipe	SW1	SW2	0.044	4.567	0.8	0.08	3.7	OK
SW1.000	Pipe	RG1	SW1	0.037	4.567	1.1	0.13	3.7	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.087	17.115	0.9	0.28	13.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.004	0.067	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.082	12.373	1.0	0.34	9.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 60 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.045	2.873	0.8	0.07	1.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.036	1.685	0.6	0.05	1.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.057	2.913	0.5	0.15	1.9	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	7.813	1.2	0.08	4.9	OK
SW2.001	Pipe	RG3	SW3	0.063	3.015	1.1	0.05	1.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.045	1.683	0.3	0.06	1.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.087	17.098	0.8	0.3	10.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.084	17.106	0.8	0.22	10.8	OK
SW1.001	Pipe	SW1	SW2	0.040	4.573	0.8	0.06	2.9	OK
SW1.000	Pipe	RG1	SW1	0.033	4.573	1.0	0.1	2.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.077	17.130	0.9	0.23	10.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.073	12.379	0.9	0.28	7.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 120 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.043	3.677	0.8	0.06	1.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.035	2.141	0.6	0.05	1.0	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.055	3.730	0.4	0.14	1.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	9.986	1.2	0.08	4.7	OK
SW2.001	Pipe	RG3	SW3	0.063	3.819	1.0	0.05	1.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.044	2.139	0.3	0.06	1.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.084	21.817	0.7	0.28	10.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.081	21.834	0.8	0.21	10.2	OK
SW1.001	Pipe	SW1	SW2	0.038	5.857	0.8	0.06	2.8	OK
SW1.000	Pipe	RG1	SW1	0.032	5.857	1.0	0.1	2.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.075	21.857	0.9	0.22	10.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.071	15.837	0.9	0.26	7.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 120 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.035	3.689	0.8	0.05	1.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.029	2.135	0.6	0.03	0.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.047	3.748	0.4	0.1	1.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	9.992	1.1	0.06	3.4	OK
SW2.001	Pipe	RG3	SW3	0.063	3.831	1.0	0.03	1.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.035	2.133	0.3	0.04	0.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.071	21.833	0.7	0.2	7.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.068	21.851	0.7	0.15	7.4	OK
SW1.001	Pipe	SW1	SW2	0.033	5.863	0.7	0.04	2.0	OK
SW1.000	Pipe	RG1	SW1	0.027	5.863	0.9	0.07	2.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.063	21.875	0.8	0.16	7.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.059	15.849	0.8	0.19	5.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 240 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.034	4.583	0.8	0.04	1.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.028	2.663	0.6	0.03	0.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.045	4.672	0.4	0.1	1.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	12.434	1.1	0.05	3.2	OK
SW2.001	Pipe	RG3	SW3	0.063	4.725	0.9	0.03	1.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.033	2.661	0.3	0.04	0.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.069	27.127	0.7	0.19	6.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.065	27.148	0.7	0.14	6.9	OK
SW1.001	Pipe	SW1	SW2	0.032	7.291	0.7	0.04	1.9	OK
SW1.000	Pipe	RG1	SW1	0.027	7.291	0.9	0.07	1.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.061	27.170	0.8	0.15	6.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.057	19.720	0.8	0.18	5.1	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 240 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.027	4.589	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	2.681	0.5	0.02	0.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.037	4.642	0.3	0.07	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	12.410	1.0	0.04	2.1	OK
SW2.001	Pipe	RG3	SW3	0.063	4.731	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.025	2.679	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.056	27.185	0.6	0.13	4.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.053	27.210	0.7	0.1	4.7	OK
SW1.001	Pipe	SW1	SW2	0.026	7.351	0.6	0.03	1.3	OK
SW1.000	Pipe	RG1	SW1	0.022	7.351	0.8	0.05	1.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.050	27.232	0.7	0.1	4.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.047	19.756	0.7	0.12	3.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 360 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.029	5.153	0.7	0.03	0.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.024	2.999	0.5	0.02	0.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.040	5.224	0.4	0.08	0.9	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	13.910	1.0	0.04	2.4	OK
SW2.001	Pipe	RG3	SW3	0.063	5.295	0.9	0.02	0.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.027	2.997	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.060	30.384	0.6	0.15	5.3	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.056	30.405	0.7	0.11	5.3	OK
SW1.001	Pipe	SW1	SW2	0.028	8.167	0.6	0.03	1.4	OK
SW1.000	Pipe	RG1	SW1	0.023	8.167	0.8	0.05	1.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.053	30.427	0.7	0.11	5.3	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.050	22.072	0.8	0.14	3.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 360 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.023	5.147	0.6	0.02	0.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.019	2.987	0.5	0.02	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.032	5.218	0.3	0.05	0.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	13.892	0.9	0.03	1.6	OK
SW2.001	Pipe	RG3	SW3	0.063	5.289	0.8	0.02	0.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.020	2.985	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.048	30.365	0.6	0.1	3.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.045	30.394	0.6	0.07	3.5	OK
SW1.001	Pipe	SW1	SW2	0.023	8.185	0.6	0.02	0.9	OK
SW1.000	Pipe	RG1	SW1	0.019	8.185	0.7	0.03	0.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.043	30.418	0.7	0.07	3.5	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.040	22.072	0.7	0.09	2.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 480 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.026	5.525	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	3.233	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.036	5.626	0.3	0.06	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	14.990	1.0	0.03	2.0	OK
SW2.001	Pipe	RG3	SW3	0.063	5.667	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.024	3.231	0.3	0.02	0.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.054	32.742	0.6	0.12	4.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.051	32.765	0.7	0.09	4.4	OK
SW1.001	Pipe	SW1	SW2	0.025	8.839	0.6	0.02	1.2	OK
SW1.000	Pipe	RG1	SW1	0.021	8.839	0.8	0.04	1.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.048	32.785	0.7	0.09	4.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.045	23.824	0.7	0.11	3.2	OK

Project: Chicklade Services, Car Park Extension 12718	Date: 21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 10 years: Increase Rainfall (%): +0: 480 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.021	5.555	0.6	0.02	0.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.017	3.227	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.029	5.638	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	14.966	0.8	0.02	1.3	OK
SW2.001	Pipe	RG3	SW3	0.063	5.697	0.7	0.01	0.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.017	3.225	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.043	32.711	0.5	0.08	2.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.040	32.741	0.6	0.06	2.9	OK
SW1.001	Pipe	SW1	SW2	0.020	8.809	0.5	0.02	0.8	OK
SW1.000	Pipe	RG1	SW1	0.017	8.809	0.7	0.03	0.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.039	32.778	0.6	0.06	2.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.036	23.770	0.6	0.07	2.1	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 960 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.020	6.557	0.6	0.02	0.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.016	3.809	0.4	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.028	6.677	0.3	0.04	0.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	17.726	0.8	0.02	1.2	OK
SW2.001	Pipe	RG3	SW3	0.063	6.699	0.7	0.01	0.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.015	3.807	0.3	0.01	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.042	38.718	0.5	0.07	2.6	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.039	38.745	0.6	0.05	2.6	OK
SW1.001	Pipe	SW1	SW2	0.020	10.471	0.5	0.01	0.7	OK
SW1.000	Pipe	RG1	SW1	0.017	10.471	0.7	0.03	0.7	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.037	38.766	0.6	0.06	2.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.035	28.192	0.6	0.07	1.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 960 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.016	6.551	0.5	0.01	0.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.013	3.839	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.022	6.670	0.3	0.02	0.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	17.684	0.7	0.01	0.8	OK
SW2.001	Pipe	RG3	SW3	0.063	6.693	0.6	0.01	0.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.010	3.837	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.033	38.681	0.5	0.05	1.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.031	38.741	0.5	0.03	1.7	OK
SW1.001	Pipe	SW1	SW2	0.016	10.453	0.5	0.01	0.5	OK
SW1.000	Pipe	RG1	SW1	0.013	10.453	0.6	0.02	0.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.030	38.824	0.5	0.04	1.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.028	28.132	0.5	0.04	1.2	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 1440 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.017	7.271	0.5	0.01	0.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.014	4.265	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.024	7.379	0.3	0.03	0.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	19.592	0.7	0.01	0.9	OK
SW2.001	Pipe	RG3	SW3	0.063	7.413	0.6	0.01	0.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.011	4.263	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.036	42.918	0.5	0.05	1.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.033	43.231	0.5	0.04	1.9	OK
SW1.001	Pipe	SW1	SW2	0.017	11.635	0.5	0.01	0.5	OK
SW1.000	Pipe	RG1	SW1	0.014	11.635	0.6	0.02	0.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.032	43.222	0.6	0.04	1.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.030	31.222	0.6	0.05	1.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 10 years: Increase Rainfall (%): +0: 1440 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.013	7.295	0.5	0.01	0.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.011	4.181	0.4	0.01	0.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.020	7.426	0.2	0.02	0.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	19.562	0.7	0.01	0.6	OK
SW2.001	Pipe	RG3	SW3	0.063	7.437	0.6	0.01	0.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.009	4.179	0.3	0.01	0.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.029	42.701	0.4	0.03	1.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.026	42.842	0.5	0.03	1.2	OK
SW1.001	Pipe	SW1	SW2	0.014	11.509	0.4	0.01	0.3	OK
SW1.000	Pipe	RG1	SW1	0.012	11.509	0.5	0.01	0.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.026	42.906	0.5	0.03	1.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.000	0.000	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.024	31.066	0.5	0.03	0.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 15 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.102	3.060	1.1	0.25	6.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.081	1.785	0.8	0.18	3.9	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.113	3.108	0.7	0.57	7.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.119	8.340	1.2	0.31	18.6	OK
SW2.001	Pipe	RG3	SW3	0.068	3.202	1.1	0.18	6.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.115	1.782	0.3	0.2	3.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.175	18.287	1.1	0.98	35.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.177	18.289	1.0	0.73	35.1	OK
SW1.001	Pipe	SW1	SW2	0.114	4.942	0.8	0.23	11.1	OK
SW1.000	Pipe	RG1	SW1	0.068	4.944	1.4	0.39	11.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.057	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.036	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.154	18.284	1.2	0.73	34.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.034	0.625	0.0		2.4	
SW1.002	Pipe	SW2	SW3	0.150	13.273	1.5	0.92	26.2	Surcharged

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 15 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.098	3.063	1.1	0.24	6.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.078	1.782	0.8	0.17	3.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.109	3.109	0.7	0.53	6.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.097	8.337	1.4	0.29	17.5	OK
SW2.001	Pipe	RG3	SW3	0.065	3.205	1.1	0.17	6.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.110	1.779	0.3	0.2	3.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.172	18.259	1.1	0.95	34.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.173	18.261	1.0	0.71	34.2	OK
SW1.001	Pipe	SW1	SW2	0.093	4.917	0.9	0.21	10.3	OK
SW1.000	Pipe	RG1	SW1	0.066	4.918	1.4	0.37	10.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.001	0.037	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.007	0.020	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.152	18.256	1.2	0.72	33.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.033	0.613	0.0		2.2	
SW1.002	Pipe	SW2	SW3	0.150	13.244	1.4	0.88	25.1	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 30 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.081	4.114	1.0	0.17	4.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.065	2.392	0.8	0.12	2.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.093	4.170	0.6	0.39	4.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.073	11.140	1.5	0.21	12.7	OK
SW2.001	Pipe	RG3	SW3	0.063	4.256	1.0	0.13	4.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.088	2.391	0.3	0.16	2.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.150	24.552	1.0	0.76	27.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.150	24.555	1.0	0.57	27.8	OK
SW1.001	Pipe	SW1	SW2	0.070	6.746	0.9	0.16	7.6	OK
SW1.000	Pipe	RG1	SW1	0.055	6.746	1.3	0.27	7.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.004	0.200	0.0		0.3	
Overflow2	No Delay	Bio 3	Bio 4	0.011	0.085	0.0		0.2	
SW1.005	Pipe	SW4	Soakaway SA1	0.133	24.579	1.1	0.59	27.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.029	0.748	0.0		1.6	
SW1.002	Pipe	SW2	SW3	0.130	17.878	1.2	0.71	20.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 30 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.078	4.108	1.0	0.16	4.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.062	2.398	0.8	0.12	2.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.090	4.177	0.6	0.37	4.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.070	11.146	1.5	0.2	11.9	OK
SW2.001	Pipe	RG3	SW3	0.063	4.250	1.1	0.12	4.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.084	2.397	0.3	0.15	2.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.145	24.528	1.0	0.71	26.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.143	24.531	1.0	0.54	26.1	OK
SW1.001	Pipe	SW1	SW2	0.067	6.716	0.9	0.15	7.1	OK
SW1.000	Pipe	RG1	SW1	0.053	6.716	1.3	0.25	7.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	0.163	0.0		0.4	
Overflow2	No Delay	Bio 3	Bio 4	0.011	0.082	0.0		0.2	
SW1.005	Pipe	SW4	Soakaway SA1	0.128	24.557	1.1	0.55	26.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.028	0.734	0.0		1.4	
SW1.002	Pipe	SW2	SW3	0.125	17.854	1.2	0.67	19.0	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 60 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.075	5.296	1.0	0.15	4.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.060	3.082	0.8	0.11	2.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.088	5.385	0.6	0.35	4.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.068	14.322	1.5	0.19	11.4	OK
SW2.001	Pipe	RG3	SW3	0.063	5.438	1.1	0.11	4.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.081	3.081	0.3	0.14	2.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.141	31.529	0.9	0.68	24.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.139	31.537	1.0	0.51	24.8	OK
SW1.001	Pipe	SW1	SW2	0.064	8.670	0.9	0.14	6.8	OK
SW1.000	Pipe	RG1	SW1	0.052	8.670	1.3	0.24	6.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.006	0.227	0.0		0.5	
Overflow2	No Delay	Bio 3	Bio 4	0.013	0.153	0.0		0.3	
SW1.005	Pipe	SW4	Soakaway SA1	0.124	31.560	1.1	0.52	24.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.027	0.760	0.0		1.3	
SW1.002	Pipe	SW2	SW3	0.121	22.985	1.2	0.63	18.1	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 60 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.066	5.297	0.9	0.12	3.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.053	3.088	0.7	0.09	2.0	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.079	5.385	0.5	0.28	3.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	14.323	1.4	0.15	9.1	OK
SW2.001	Pipe	RG3	SW3	0.063	5.439	1.3	0.09	3.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.070	3.087	0.3	0.11	2.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.124	31.431	0.9	0.55	20.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.121	31.440	0.9	0.41	20.0	OK
SW1.001	Pipe	SW1	SW2	0.056	8.566	0.9	0.11	5.4	OK
SW1.000	Pipe	RG1	SW1	0.046	8.566	1.2	0.19	5.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.007	0.123	0.0		0.5	
Overflow2	No Delay	Bio 3	Bio 4	0.012	0.121	0.0		0.3	
SW1.005	Pipe	SW4	Soakaway SA1	0.109	31.463	1.0	0.42	20.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.024	0.706	0.0		1.0	
SW1.002	Pipe	SW2	SW3	0.105	22.882	1.1	0.51	14.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 120 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.062	6.551	0.9	0.11	3.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.050	3.815	0.7	0.08	1.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.075	6.658	0.5	0.26	3.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	17.690	1.4	0.14	8.3	OK
SW2.001	Pipe	RG3	SW3	0.063	6.693	1.2	0.08	3.1	OK
SW2.002	Pipe	M100D30.2	SW3	0.065	3.813	0.3	0.1	1.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.118	38.983	0.9	0.5	18.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.114	39.002	0.9	0.38	18.2	OK
SW1.001	Pipe	SW1	SW2	0.052	10.770	0.9	0.1	4.9	OK
SW1.000	Pipe	RG1	SW1	0.044	10.770	1.2	0.18	4.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	0.317	0.0		0.2	
Overflow2	No Delay	Bio 3	Bio 4	0.012	0.157	0.0		0.3	
SW1.005	Pipe	SW4	Soakaway SA1	0.103	39.034	1.0	0.38	18.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.023	0.917	0.0		0.9	
SW1.002	Pipe	SW2	SW3	0.098	28.453	1.1	0.47	13.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 120 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.050	6.563	0.9	0.08	2.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.041	3.821	0.7	0.06	1.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.063	6.658	0.5	0.19	2.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	17.684	1.3	0.1	6.0	OK
SW2.001	Pipe	RG3	SW3	0.063	6.705	1.1	0.06	2.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.052	3.819	0.3	0.07	1.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.097	38.932	0.8	0.36	13.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.094	38.950	0.8	0.27	13.1	OK
SW1.001	Pipe	SW1	SW2	0.044	10.708	0.8	0.07	3.6	OK
SW1.000	Pipe	RG1	SW1	0.037	10.709	1.1	0.13	3.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	0.268	0.0		0.2	
Overflow2	No Delay	Bio 3	Bio 4	0.009	0.083	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.086	38.984	0.9	0.28	13.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.018	0.889	0.0		0.5	
SW1.002	Pipe	SW2	SW3	0.081	28.386	1.0	0.34	9.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 240 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.048	7.997	0.9	0.08	2.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.039	4.673	0.7	0.05	1.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.060	8.104	0.5	0.17	2.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	21.518	1.3	0.09	5.5	OK
SW2.001	Pipe	RG3	SW3	0.063	8.139	1.1	0.06	2.1	OK
SW2.002	Pipe	M100D30.2	SW3	0.049	4.671	0.3	0.07	1.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.093	47.724	0.8	0.33	12.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.089	47.744	0.8	0.25	12.1	OK
SW1.001	Pipe	SW1	SW2	0.042	13.379	0.8	0.07	3.3	OK
SW1.000	Pipe	RG1	SW1	0.035	13.379	1.0	0.12	3.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.006	0.664	0.0		0.4	
Overflow2	No Delay	Bio 3	Bio 4	0.005	0.018	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.082	47.765	0.9	0.26	12.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.019	1.208	0.0		0.5	
SW1.002	Pipe	SW2	SW3	0.078	34.892	1.0	0.31	8.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 240 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.038	7.991	0.8	0.05	1.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.030	4.643	0.6	0.04	0.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.049	8.122	0.4	0.12	1.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	21.524	1.1	0.06	3.7	OK
SW2.001	Pipe	RG3	SW3	0.063	8.133	1.0	0.04	1.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.037	4.641	0.3	0.05	0.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.075	47.641	0.7	0.22	8.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.071	47.665	0.8	0.17	8.1	OK
SW1.001	Pipe	SW1	SW2	0.034	13.328	0.7	0.05	2.2	OK
SW1.000	Pipe	RG1	SW1	0.029	13.329	0.9	0.08	2.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.006	0.638	0.0		0.3	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.066	47.689	0.8	0.17	8.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.018	1.220	0.0		0.4	
SW1.002	Pipe	SW2	SW3	0.062	34.847	0.9	0.21	5.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 360 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.040	8.897	0.8	0.06	1.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.033	5.195	0.6	0.04	0.9	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.053	9.046	0.4	0.13	1.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	23.936	1.2	0.07	4.2	OK
SW2.001	Pipe	RG3	SW3	0.063	9.039	1.0	0.04	1.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.041	5.193	0.3	0.05	0.9	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.080	53.043	0.7	0.25	9.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.077	53.063	0.8	0.19	9.2	OK
SW1.001	Pipe	SW1	SW2	0.036	14.857	0.8	0.05	2.5	OK
SW1.000	Pipe	RG1	SW1	0.031	14.858	1.0	0.09	2.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.006	0.709	0.0		0.4	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.071	53.083	0.9	0.19	9.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.018	1.281	0.0		0.4	
SW1.002	Pipe	SW2	SW3	0.067	38.788	0.9	0.24	6.7	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 360 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.032	8.873	0.7	0.04	1.0	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.026	5.165	0.6	0.03	0.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.042	9.034	0.4	0.09	1.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	23.930	1.0	0.05	2.8	OK
SW2.001	Pipe	RG3	SW3	0.063	9.015	0.9	0.03	1.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.030	5.163	0.3	0.03	0.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.064	52.942	0.7	0.17	6.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.061	52.968	0.7	0.13	6.1	OK
SW1.001	Pipe	SW1	SW2	0.030	14.819	0.7	0.03	1.6	OK
SW1.000	Pipe	RG1	SW1	0.025	14.820	0.9	0.06	1.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.006	0.665	0.0		0.3	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.057	52.987	0.8	0.13	6.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.016	1.270	0.0		0.3	
SW1.002	Pipe	SW2	SW3	0.053	38.744	0.8	0.16	4.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 480 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.036	9.557	0.8	0.05	1.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.029	5.561	0.6	0.03	0.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.048	9.718	0.4	0.11	1.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	25.742	1.1	0.06	3.5	OK
SW2.001	Pipe	RG3	SW3	0.063	9.699	1.0	0.04	1.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.035	5.559	0.3	0.04	0.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.072	56.848	0.7	0.21	7.6	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.069	56.868	0.7	0.16	7.6	OK
SW1.001	Pipe	SW1	SW2	0.033	15.831	0.7	0.04	2.1	OK
SW1.000	Pipe	RG1	SW1	0.028	15.831	0.9	0.07	2.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.006	0.620	0.0		0.3	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.064	56.889	0.8	0.16	7.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.017	1.228	0.0		0.4	
SW1.002	Pipe	SW2	SW3	0.060	41.568	0.8	0.19	5.5	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 480 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.028	9.563	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.023	5.603	0.5	0.02	0.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.038	9.712	0.4	0.07	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	25.730	1.0	0.04	2.2	OK
SW2.001	Pipe	RG3	SW3	0.063	9.705	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.026	5.601	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.057	56.836	0.6	0.13	4.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.054	56.863	0.7	0.1	4.9	OK
SW1.001	Pipe	SW1	SW2	0.027	15.785	0.6	0.03	1.3	OK
SW1.000	Pipe	RG1	SW1	0.022	15.786	0.8	0.05	1.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	0.527	0.0		0.2	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.051	56.888	0.7	0.1	4.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.014	1.218	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.048	41.510	0.7	0.13	3.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 960 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.027	11.303	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	6.587	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.036	11.524	0.3	0.06	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	30.446	1.0	0.03	2.1	OK
SW2.001	Pipe	RG3	SW3	0.063	11.445	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.024	6.585	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.055	66.718	0.6	0.12	4.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.052	66.741	0.7	0.09	4.5	OK
SW1.001	Pipe	SW1	SW2	0.026	18.227	0.6	0.03	1.2	OK
SW1.000	Pipe	RG1	SW1	0.022	18.227	0.8	0.04	1.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.003	0.142	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.049	66.762	0.7	0.1	4.5	OK
Overflow1	No Delay	Bio 1	Bio 2	0.013	0.982	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.046	48.668	0.7	0.12	3.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 960 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.021	11.339	0.6	0.02	0.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.017	6.596	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.030	11.530	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	30.488	0.8	0.02	1.3	OK
SW2.001	Pipe	RG3	SW3	0.063	11.481	0.7	0.01	0.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.017	6.594	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.044	66.614	0.5	0.08	2.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.041	66.652	0.6	0.06	2.9	OK
SW1.001	Pipe	SW1	SW2	0.021	18.037	0.5	0.02	0.8	OK
SW1.000	Pipe	RG1	SW1	0.017	18.037	0.7	0.03	0.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.039	66.712	0.6	0.06	2.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.011	0.857	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.037	48.520	0.6	0.07	2.1	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 1440 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.022	12.467	0.6	0.02	0.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.019	7.289	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.031	12.665	0.3	0.05	0.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	33.536	0.9	0.03	1.5	OK
SW2.001	Pipe	RG3	SW3	0.063	12.609	0.8	0.02	0.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.019	7.287	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.047	73.254	0.6	0.09	3.3	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.044	73.278	0.6	0.07	3.3	OK
SW1.001	Pipe	SW1	SW2	0.022	19.807	0.6	0.02	0.9	OK
SW1.000	Pipe	RG1	SW1	0.019	19.807	0.7	0.03	0.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.042	73.299	0.7	0.07	3.3	OK
Overflow1	No Delay	Bio 1	Bio 2	0.011	0.735	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.039	53.338	0.7	0.09	2.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 30 years: Increase Rainfall (%): +40: 1440 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.018	12.419	0.6	0.01	0.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.015	7.301	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.025	12.730	0.3	0.03	0.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	33.698	0.8	0.02	1.0	OK
SW2.001	Pipe	RG3	SW3	0.063	12.561	0.7	0.01	0.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.013	7.299	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.038	73.535	0.5	0.06	2.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.035	73.594	0.5	0.04	2.1	OK
SW1.001	Pipe	SW1	SW2	0.018	19.963	0.5	0.01	0.6	OK
SW1.000	Pipe	RG1	SW1	0.015	19.963	0.6	0.02	0.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.033	73.688	0.6	0.05	2.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.009	0.463	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.031	53.656	0.6	0.05	1.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 15 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.095	2.736	1.0	0.22	6.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.075	1.596	0.8	0.16	3.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.107	2.779	0.6	0.51	6.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.087	7.470	1.6	0.28	16.6	OK
SW2.001	Pipe	RG3	SW3	0.063	2.878	1.1	0.16	6.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.108	1.593	0.3	0.18	3.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.169	16.328	1.1	0.92	33.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.169	16.329	1.0	0.68	33.1	OK
SW1.001	Pipe	SW1	SW2	0.083	4.368	1.0	0.2	9.8	OK
SW1.000	Pipe	RG1	SW1	0.064	4.369	1.4	0.35	9.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.004	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.148	16.325	1.2	0.69	32.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.032	0.500	0.0		2.1	
SW1.002	Pipe	SW2	SW3	0.150	11.827	1.4	0.87	24.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 15 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.091	2.739	1.0	0.21	5.7	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.072	1.593	0.8	0.15	3.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.103	2.782	0.6	0.48	5.9	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.082	7.476	1.6	0.26	15.6	OK
SW2.001	Pipe	RG3	SW3	0.063	2.881	1.1	0.15	5.7	OK
SW2.002	Pipe	M100D30.2	SW3	0.101	1.591	0.3	0.17	3.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.164	16.331	1.0	0.88	32.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.162	16.334	1.0	0.65	31.4	OK
SW1.001	Pipe	SW1	SW2	0.078	4.365	1.0	0.19	9.2	OK
SW1.000	Pipe	RG1	SW1	0.062	4.365	1.4	0.33	9.3	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.143	16.328	1.2	0.65	30.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.031	0.487	0.0		1.9	
SW1.002	Pipe	SW2	SW3	0.146	11.830	1.4	0.86	24.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 30 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.076	3.730	1.0	0.16	4.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.061	2.170	0.8	0.11	2.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.089	3.793	0.6	0.36	4.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.068	10.126	1.5	0.19	11.5	OK
SW2.001	Pipe	RG3	SW3	0.063	3.872	1.1	0.12	4.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.082	2.169	0.3	0.14	2.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.142	22.142	1.0	0.69	25.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.140	22.144	1.0	0.52	25.2	OK
SW1.001	Pipe	SW1	SW2	0.065	5.956	0.9	0.14	6.8	OK
SW1.000	Pipe	RG1	SW1	0.052	5.956	1.3	0.24	6.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.004	0.015	0.0		0.2	
Overflow2	No Delay	Bio 3	Bio 4	0.007	0.020	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.125	22.171	1.1	0.53	25.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.027	0.610	0.0		1.4	
SW1.002	Pipe	SW2	SW3	0.122	16.073	1.2	0.64	18.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 30 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.073	3.724	1.0	0.15	4.0	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.058	2.176	0.8	0.11	2.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.086	3.787	0.6	0.34	4.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.065	10.120	1.5	0.18	10.8	OK
SW2.001	Pipe	RG3	SW3	0.063	3.866	1.1	0.11	4.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.078	2.175	0.3	0.13	2.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.137	22.121	0.9	0.65	23.6	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.134	22.123	1.0	0.49	23.6	OK
SW1.001	Pipe	SW1	SW2	0.062	5.940	0.9	0.13	6.4	OK
SW1.000	Pipe	RG1	SW1	0.050	5.941	1.2	0.23	6.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.005	0.013	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.120	22.147	1.1	0.5	23.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.026	0.588	0.0		1.3	
SW1.002	Pipe	SW2	SW3	0.117	16.053	1.2	0.6	17.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 60 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.071	4.846	1.0	0.14	3.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.057	2.824	0.7	0.1	2.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.084	4.917	0.6	0.32	3.9	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.064	13.093	1.5	0.18	10.4	OK
SW2.001	Pipe	RG3	SW3	0.063	4.989	1.2	0.11	3.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.076	2.823	0.3	0.13	2.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.134	28.668	0.9	0.62	22.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.131	28.676	0.9	0.47	22.7	OK
SW1.001	Pipe	SW1	SW2	0.060	7.747	0.9	0.13	6.2	OK
SW1.000	Pipe	RG1	SW1	0.049	7.747	1.2	0.22	6.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	0.042	0.0		0.3	
Overflow2	No Delay	Bio 3	Bio 4	0.011	0.078	0.0		0.2	
SW1.005	Pipe	SW4	Soakaway SA1	0.117	28.702	1.1	0.48	22.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.026	0.619	0.0		1.2	
SW1.002	Pipe	SW2	SW3	0.114	20.832	1.2	0.58	16.5	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 60 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.062	4.847	0.9	0.11	3.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.050	2.825	0.7	0.08	1.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.075	4.923	0.5	0.26	3.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	13.111	1.4	0.14	8.3	OK
SW2.001	Pipe	RG3	SW3	0.063	4.989	1.2	0.08	3.1	OK
SW2.002	Pipe	M100D30.2	SW3	0.065	2.823	0.3	0.1	1.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.118	28.649	0.9	0.5	18.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.114	28.658	0.9	0.38	18.2	OK
SW1.001	Pipe	SW1	SW2	0.052	7.711	0.9	0.1	5.0	OK
SW1.000	Pipe	RG1	SW1	0.044	7.711	1.2	0.18	5.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.052	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.103	28.680	1.0	0.38	18.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.023	0.555	0.0		0.9	
SW1.002	Pipe	SW2	SW3	0.099	20.815	1.1	0.47	13.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 120 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.058	5.861	0.9	0.1	2.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.046	3.413	0.7	0.07	1.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.070	5.950	0.5	0.23	2.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	15.806	1.4	0.13	7.4	OK
SW2.001	Pipe	RG3	SW3	0.063	6.003	1.2	0.08	2.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.060	3.411	0.3	0.09	1.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.110	34.544	0.8	0.44	16.3	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.106	34.561	0.9	0.34	16.3	OK
SW1.001	Pipe	SW1	SW2	0.049	9.307	0.9	0.09	4.4	OK
SW1.000	Pipe	RG1	SW1	0.041	9.307	1.1	0.16	4.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.062	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.097	34.585	1.0	0.34	16.3	OK
Overflow1	No Delay	Bio 1	Bio 2	0.021	0.632	0.0		0.7	
SW1.002	Pipe	SW2	SW3	0.092	25.107	1.0	0.42	11.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 120 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.047	5.861	0.8	0.07	2.0	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.038	3.413	0.7	0.05	1.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.059	5.950	0.5	0.17	2.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	15.794	1.3	0.09	5.4	OK
SW2.001	Pipe	RG3	SW3	0.063	6.003	1.1	0.05	2.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.048	3.411	0.3	0.07	1.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.091	34.548	0.8	0.32	11.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.088	34.565	0.8	0.24	11.7	OK
SW1.001	Pipe	SW1	SW2	0.041	9.325	0.8	0.07	3.2	OK
SW1.000	Pipe	RG1	SW1	0.035	9.325	1.0	0.11	3.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.081	34.585	0.9	0.25	11.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.016	0.606	0.0		0.4	
SW1.002	Pipe	SW2	SW3	0.076	25.113	0.9	0.3	8.5	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 240 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.044	7.049	0.8	0.07	1.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.036	4.115	0.6	0.05	1.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.057	7.174	0.5	0.15	1.9	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	19.046	1.2	0.08	4.9	OK
SW2.001	Pipe	RG3	SW3	0.063	7.191	1.1	0.05	1.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.045	4.113	0.3	0.06	1.1	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.087	41.770	0.8	0.29	10.7	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.083	41.789	0.8	0.22	10.7	OK
SW1.001	Pipe	SW1	SW2	0.039	11.403	0.8	0.06	2.9	OK
SW1.000	Pipe	RG1	SW1	0.033	11.403	1.0	0.1	2.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.003	0.140	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.077	41.814	0.9	0.23	10.7	OK
Overflow1	No Delay	Bio 1	Bio 2	0.016	0.842	0.0		0.3	
SW1.002	Pipe	SW2	SW3	0.072	30.444	0.9	0.27	7.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 240 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.035	7.061	0.8	0.04	1.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.028	4.127	0.6	0.03	0.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.046	7.180	0.4	0.1	1.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	19.040	1.1	0.06	3.3	OK
SW2.001	Pipe	RG3	SW3	0.063	7.203	0.9	0.03	1.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.034	4.125	0.3	0.04	0.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.070	41.740	0.7	0.2	7.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.067	41.764	0.7	0.15	7.2	OK
SW1.001	Pipe	SW1	SW2	0.032	11.357	0.7	0.04	2.0	OK
SW1.000	Pipe	RG1	SW1	0.027	11.357	0.9	0.07	2.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.003	0.094	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.062	41.785	0.8	0.15	7.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.016	0.828	0.0		0.3	
SW1.002	Pipe	SW2	SW3	0.058	30.392	0.8	0.18	5.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 360 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.038	7.877	0.8	0.05	1.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.030	4.595	0.6	0.04	0.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.049	7.990	0.4	0.12	1.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	21.194	1.1	0.06	3.7	OK
SW2.001	Pipe	RG3	SW3	0.063	8.019	1.0	0.04	1.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.037	4.593	0.3	0.05	0.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.075	46.533	0.7	0.22	8.2	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.072	46.553	0.8	0.17	8.2	OK
SW1.001	Pipe	SW1	SW2	0.034	12.711	0.7	0.05	2.2	OK
SW1.000	Pipe	RG1	SW1	0.029	12.711	0.9	0.08	2.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.003	0.164	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.066	46.574	0.8	0.17	8.2	OK
Overflow1	No Delay	Bio 1	Bio 2	0.015	0.877	0.0		0.3	
SW1.002	Pipe	SW2	SW3	0.062	33.900	0.9	0.21	6.0	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 360 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.029	7.871	0.7	0.03	0.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.024	4.577	0.5	0.02	0.5	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.040	7.996	0.4	0.08	0.9	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	21.212	1.0	0.04	2.5	OK
SW2.001	Pipe	RG3	SW3	0.063	8.013	0.9	0.02	0.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.028	4.575	0.3	0.03	0.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.060	46.482	0.6	0.15	5.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.057	46.508	0.7	0.11	5.4	OK
SW1.001	Pipe	SW1	SW2	0.028	12.668	0.6	0.03	1.5	OK
SW1.000	Pipe	RG1	SW1	0.024	12.668	0.8	0.05	1.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.003	0.103	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.054	46.528	0.7	0.11	5.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.014	0.877	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.050	33.874	0.8	0.14	3.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 480 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.034	8.519	0.8	0.04	1.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.027	4.961	0.6	0.03	0.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.045	8.668	0.4	0.1	1.2	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	22.946	1.1	0.05	3.1	OK
SW2.001	Pipe	RG3	SW3	0.063	8.661	0.9	0.03	1.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.032	4.959	0.3	0.04	0.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.068	50.265	0.7	0.18	6.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.064	50.287	0.7	0.14	6.8	OK
SW1.001	Pipe	SW1	SW2	0.031	13.683	0.7	0.04	1.8	OK
SW1.000	Pipe	RG1	SW1	0.026	13.684	0.9	0.07	1.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.003	0.099	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.060	50.311	0.8	0.14	6.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.014	0.850	0.0		0.3	
SW1.002	Pipe	SW2	SW3	0.056	36.624	0.8	0.17	4.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 480 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.026	8.507	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	4.973	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.036	8.632	0.3	0.06	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	22.928	1.0	0.03	2.0	OK
SW2.001	Pipe	RG3	SW3	0.063	8.649	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.024	4.971	0.3	0.02	0.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.054	50.147	0.6	0.12	4.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.051	50.175	0.6	0.09	4.4	OK
SW1.001	Pipe	SW1	SW2	0.025	13.585	0.6	0.02	1.2	OK
SW1.000	Pipe	RG1	SW1	0.021	13.585	0.8	0.04	1.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.002	0.018	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.048	50.204	0.7	0.09	4.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.013	0.848	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.045	36.508	0.7	0.11	3.2	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 960 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.025	10.319	0.7	0.03	0.7	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.021	6.017	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.035	10.505	0.3	0.06	0.7	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	27.788	0.9	0.03	1.9	OK
SW2.001	Pipe	RG3	SW3	0.063	10.461	0.8	0.02	0.7	OK
SW2.002	Pipe	M100D30.2	SW3	0.022	6.015	0.3	0.02	0.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.052	60.792	0.6	0.11	4.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.049	60.814	0.6	0.08	4.1	OK
SW1.001	Pipe	SW1	SW2	0.024	16.513	0.6	0.02	1.1	OK
SW1.000	Pipe	RG1	SW1	0.021	16.513	0.8	0.04	1.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.047	60.839	0.7	0.09	4.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.012	0.691	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.044	44.296	0.7	0.11	3.0	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 960 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.020	10.349	0.6	0.02	0.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.016	6.047	0.4	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.028	10.498	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	27.776	0.8	0.02	1.2	OK
SW2.001	Pipe	RG3	SW3	0.063	10.491	0.7	0.01	0.4	OK
SW2.002	Pipe	M100D30.2	SW3	0.015	6.045	0.3	0.01	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.042	60.797	0.5	0.07	2.6	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.039	60.839	0.6	0.05	2.6	OK
SW1.001	Pipe	SW1	SW2	0.020	16.471	0.5	0.01	0.7	OK
SW1.000	Pipe	RG1	SW1	0.017	16.471	0.7	0.03	0.7	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.037	60.900	0.6	0.06	2.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.010	0.527	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.035	44.242	0.6	0.07	1.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 1440 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.021	11.513	0.6	0.02	0.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.018	6.761	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.030	11.735	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	31.034	0.9	0.02	1.4	OK
SW2.001	Pipe	RG3	SW3	0.063	11.655	0.7	0.01	0.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.018	6.759	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.045	67.752	0.5	0.08	3.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.042	67.778	0.6	0.06	3.0	OK
SW1.001	Pipe	SW1	SW2	0.021	18.289	0.5	0.02	0.8	OK
SW1.000	Pipe	RG1	SW1	0.018	18.289	0.7	0.03	0.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.040	67.802	0.6	0.06	3.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.010	0.470	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.037	49.318	0.6	0.08	2.2	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +0: 1440 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.017	11.543	0.5	0.01	0.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.014	6.671	0.4	0.01	0.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.024	11.692	0.3	0.03	0.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	31.010	0.7	0.02	0.9	OK
SW2.001	Pipe	RG3	SW3	0.063	11.685	0.6	0.01	0.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.012	6.669	0.3	0.01	0.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.036	67.787	0.5	0.05	2.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.033	67.849	0.5	0.04	2.0	OK
SW1.001	Pipe	SW1	SW2	0.017	18.409	0.5	0.01	0.5	OK
SW1.000	Pipe	RG1	SW1	0.014	18.409	0.6	0.02	0.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.000	0.000	0.0		0.0	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.032	67.942	0.6	0.04	2.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.007	0.174	0.0		0.0	
SW1.002	Pipe	SW2	SW3	0.030	49.414	0.6	0.05	1.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 15 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.120	3.975	1.1	0.33	8.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.095	2.316	0.9	0.23	5.1	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.130	4.032	0.7	0.74	9.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.150	10.772	1.4	0.41	24.2	OK
SW2.001	Pipe	RG3	SW3	0.096	4.116	1.2	0.24	8.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.150	2.313	0.3	0.24	4.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.213	24.098	1.2	1.25	45.8	Surcharged
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.218	24.100	1.1	0.9	43.7	Surcharged
SW1.001	Pipe	SW1	SW2	0.150	6.880	0.9	0.3	14.3	OK
SW1.000	Pipe	RG1	SW1	0.080	6.882	1.5	0.51	14.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.007	0.543	0.0		1.0	
Overflow2	No Delay	Bio 3	Bio 4	0.012	0.194	0.0		0.4	
SW1.005	Pipe	SW4	Soakaway SA1	0.184	24.095	1.2	0.91	43.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.036	0.999	0.0		3.3	
SW1.002	Pipe	SW2	SW3	0.150	17.642	1.9	1.19	34.0	Surcharged

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 15 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.116	3.972	1.1	0.31	8.4	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.092	2.313	0.9	0.22	4.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.126	4.032	0.7	0.7	8.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.150	10.772	1.3	0.38	22.7	OK
SW2.001	Pipe	RG3	SW3	0.088	4.113	1.2	0.23	8.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.149	2.310	0.3	0.22	4.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.201	24.066	1.2	1.19	43.6	Surcharged
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.207	24.067	1.1	0.86	41.8	OK
SW1.001	Pipe	SW1	SW2	0.150	6.852	1.0	0.28	13.5	OK
SW1.000	Pipe	RG1	SW1	0.077	6.854	1.5	0.48	13.5	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.007	0.521	0.0		1.1	
Overflow2	No Delay	Bio 3	Bio 4	0.012	0.180	0.0		0.4	
SW1.005	Pipe	SW4	Soakaway SA1	0.177	24.062	1.2	0.87	41.1	OK
Overflow1	No Delay	Bio 1	Bio 2	0.036	0.986	0.0		3.1	
SW1.002	Pipe	SW2	SW3	0.150	17.615	1.8	1.13	32.2	Surcharged

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 30 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.096	5.404	1.1	0.23	6.2	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.077	3.154	0.8	0.16	3.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.107	5.490	0.7	0.52	6.3	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.150	14.613	1.6	0.28	16.7	OK
SW2.001	Pipe	RG3	SW3	0.072	5.546	1.1	0.17	6.2	OK
SW2.002	Pipe	M100D30.2	SW3	0.124	3.152	0.3	0.2	3.5	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.184	32.826	1.1	1.05	38.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.190	32.828	1.1	0.79	38.2	OK
SW1.001	Pipe	SW1	SW2	0.150	9.491	1.0	0.27	13.0	OK
SW1.000	Pipe	RG1	SW1	0.075	9.491	1.5	0.46	13.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.013	0.875	0.0		3.1	
Overflow2	No Delay	Bio 3	Bio 4	0.016	0.340	0.0		0.7	
SW1.005	Pipe	SW4	Soakaway SA1	0.165	32.854	1.2	0.8	38.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.032	1.243	0.0		2.2	
SW1.002	Pipe	SW2	SW3	0.150	24.096	1.6	1.01	28.9	Surcharged

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 30 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.092	5.410	1.0	0.21	5.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.074	3.154	0.8	0.15	3.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.104	5.484	0.6	0.49	6.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.123	14.608	1.6	0.27	15.7	OK
SW2.001	Pipe	RG3	SW3	0.066	5.552	1.1	0.16	5.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.114	3.153	0.3	0.18	3.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.175	32.813	1.1	0.98	35.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.178	32.816	1.0	0.73	35.5	OK
SW1.001	Pipe	SW1	SW2	0.122	9.478	1.0	0.25	12.1	OK
SW1.000	Pipe	RG1	SW1	0.072	9.479	1.4	0.43	12.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.013	0.857	0.0		2.8	
Overflow2	No Delay	Bio 3	Bio 4	0.015	0.337	0.0		0.6	
SW1.005	Pipe	SW4	Soakaway SA1	0.156	32.839	1.2	0.74	35.3	OK
Overflow1	No Delay	Bio 1	Bio 2	0.032	1.246	0.0		2.0	
SW1.002	Pipe	SW2	SW3	0.150	24.078	1.5	0.95	27.0	Surcharged

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 60 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.090	7.018	1.0	0.21	5.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.072	4.096	0.8	0.15	3.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.102	7.136	0.6	0.47	5.7	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.112	18.930	1.5	0.25	15.0	OK
SW2.001	Pipe	RG3	SW3	0.064	7.160	1.2	0.15	5.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.110	4.095	0.3	0.18	3.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.173	42.405	1.1	0.95	34.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.176	42.413	1.0	0.72	34.7	OK
SW1.001	Pipe	SW1	SW2	0.111	12.199	1.0	0.24	11.6	OK
SW1.000	Pipe	RG1	SW1	0.070	12.199	1.4	0.41	11.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.013	1.009	0.0		2.7	
Overflow2	No Delay	Bio 3	Bio 4	0.019	0.460	0.0		0.9	
SW1.005	Pipe	SW4	Soakaway SA1	0.154	42.436	1.2	0.73	34.6	OK
Overflow1	No Delay	Bio 1	Bio 2	0.031	1.315	0.0		1.9	
SW1.002	Pipe	SW2	SW3	0.150	31.122	1.5	0.92	26.2	Surcharged

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 60 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.078	7.018	1.0	0.17	4.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.063	4.090	0.8	0.12	2.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.091	7.137	0.6	0.37	4.6	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.074	18.930	1.5	0.2	12.1	OK
SW2.001	Pipe	RG3	SW3	0.063	7.160	1.2	0.12	4.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.088	4.089	0.3	0.15	2.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.152	42.336	1.0	0.78	28.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.152	42.345	1.0	0.59	28.4	OK
SW1.001	Pipe	SW1	SW2	0.073	12.139	1.1	0.19	9.1	OK
SW1.000	Pipe	RG1	SW1	0.061	12.139	1.4	0.32	9.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.011	0.948	0.0		1.9	
Overflow2	No Delay	Bio 3	Bio 4	0.017	0.434	0.0		0.6	
SW1.005	Pipe	SW4	Soakaway SA1	0.135	42.376	1.1	0.6	28.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.028	1.304	0.0		1.5	
SW1.002	Pipe	SW2	SW3	0.133	31.062	1.3	0.74	21.2	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 120 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.073	8.483	1.0	0.15	4.0	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.058	4.948	0.8	0.1	2.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.086	8.626	0.6	0.33	4.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.068	22.874	1.5	0.18	10.8	OK
SW2.001	Pipe	RG3	SW3	0.063	8.625	1.2	0.11	4.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.081	4.947	0.3	0.13	2.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.142	51.429	1.0	0.68	25.1	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.139	51.448	1.0	0.51	24.9	OK
SW1.001	Pipe	SW1	SW2	0.067	14.965	1.0	0.17	8.0	OK
SW1.000	Pipe	RG1	SW1	0.057	14.965	1.3	0.28	8.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.011	1.465	0.0		1.6	
Overflow2	No Delay	Bio 3	Bio 4	0.017	0.450	0.0		0.6	
SW1.005	Pipe	SW4	Soakaway SA1	0.124	51.485	1.1	0.52	24.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.027	1.689	0.0		1.2	
SW1.002	Pipe	SW2	SW3	0.123	37.833	1.2	0.66	18.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 120 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.059	8.495	0.9	0.11	2.9	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.047	4.943	0.7	0.08	1.7	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.072	8.620	0.5	0.24	3.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	22.856	1.4	0.13	7.8	OK
SW2.001	Pipe	RG3	SW3	0.063	8.637	1.2	0.08	2.9	OK
SW2.002	Pipe	M100D30.2	SW3	0.062	4.941	0.3	0.09	1.7	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.113	51.402	0.9	0.46	17.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.109	51.422	0.9	0.35	17.0	OK
SW1.001	Pipe	SW1	SW2	0.050	14.953	0.9	0.1	4.9	OK
SW1.000	Pipe	RG1	SW1	0.043	14.953	1.2	0.17	4.9	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.008	1.434	0.0		0.8	
Overflow2	No Delay	Bio 3	Bio 4	0.014	0.370	0.0		0.3	
SW1.005	Pipe	SW4	Soakaway SA1	0.099	51.457	1.0	0.36	17.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.022	1.684	0.0		0.8	
SW1.002	Pipe	SW2	SW3	0.094	37.803	1.1	0.43	12.4	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 240 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.056	10.247	0.9	0.1	2.6	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.045	5.975	0.7	0.07	1.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.069	10.408	0.5	0.22	2.7	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	27.554	1.4	0.12	7.1	OK
SW2.001	Pipe	RG3	SW3	0.063	10.389	1.2	0.07	2.6	OK
SW2.002	Pipe	M100D30.2	SW3	0.058	5.973	0.3	0.09	1.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.107	62.207	0.8	0.42	15.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.104	62.210	0.9	0.32	15.5	OK
SW1.001	Pipe	SW1	SW2	0.048	18.272	0.9	0.09	4.4	OK
SW1.000	Pipe	RG1	SW1	0.041	18.272	1.1	0.16	4.4	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.009	1.963	0.0		0.9	
Overflow2	No Delay	Bio 3	Bio 4	0.012	0.272	0.0		0.3	
SW1.005	Pipe	SW4	Soakaway SA1	0.094	62.208	1.0	0.33	15.5	OK
Overflow1	No Delay	Bio 1	Bio 2	0.024	2.130	0.0		0.9	
SW1.002	Pipe	SW2	SW3	0.089	45.821	1.0	0.4	11.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 240 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.044	10.235	0.8	0.07	1.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.035	5.969	0.6	0.05	1.0	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.056	10.414	0.4	0.15	1.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	27.560	1.2	0.08	4.8	OK
SW2.001	Pipe	RG3	SW3	0.063	10.377	1.1	0.05	1.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.044	5.967	0.3	0.06	1.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.085	62.199	0.8	0.28	10.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.082	62.203	0.8	0.22	10.4	OK
SW1.001	Pipe	SW1	SW2	0.039	18.279	0.9	0.07	3.2	OK
SW1.000	Pipe	RG1	SW1	0.035	18.279	1.0	0.11	3.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.009	1.976	0.0		0.8	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.206	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.096	62.206	0.9	0.22	10.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.022	2.142	0.0		0.7	
SW1.002	Pipe	SW2	SW3	0.071	45.834	0.9	0.27	7.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 360 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.047	11.399	0.8	0.07	2.0	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.038	6.647	0.7	0.05	1.2	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.060	11.608	0.5	0.17	2.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	30.710	1.3	0.09	5.4	OK
SW2.001	Pipe	RG3	SW3	0.063	11.541	1.1	0.05	2.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.048	6.645	0.3	0.07	1.2	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.092	69.203	0.8	0.32	11.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.121	69.179	0.8	0.24	11.8	OK
SW1.001	Pipe	SW1	SW2	0.042	20.293	0.9	0.07	3.6	OK
SW1.000	Pipe	RG1	SW1	0.037	20.293	1.1	0.13	3.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.009	2.106	0.0		0.9	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.229	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.147	69.200	0.9	0.25	11.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.022	2.274	0.0		0.7	
SW1.002	Pipe	SW2	SW3	0.077	50.998	1.0	0.3	8.6	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 360 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.037	11.423	0.8	0.05	1.3	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.030	6.653	0.6	0.03	0.8	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.048	11.596	0.4	0.11	1.4	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	30.698	1.1	0.06	3.6	OK
SW2.001	Pipe	RG3	SW3	0.063	11.565	1.0	0.04	1.3	OK
SW2.002	Pipe	M100D30.2	SW3	0.036	6.651	0.3	0.04	0.8	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.073	69.181	0.7	0.21	7.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.125	69.178	0.7	0.16	7.8	OK
SW1.001	Pipe	SW1	SW2	0.035	20.256	0.8	0.05	2.6	OK
SW1.000	Pipe	RG1	SW1	0.031	20.256	1.0	0.09	2.6	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.008	2.087	0.0		0.6	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.181	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.152	69.168	0.8	0.16	7.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.020	2.288	0.0		0.5	
SW1.002	Pipe	SW2	SW3	0.061	50.950	0.9	0.2	5.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 480 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.042	12.359	0.8	0.06	1.7	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.034	7.199	0.6	0.04	1.0	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.054	12.556	0.4	0.14	1.7	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	33.212	1.2	0.08	4.5	OK
SW2.001	Pipe	RG3	SW3	0.063	12.501	1.0	0.05	1.7	OK
SW2.002	Pipe	M100D30.2	SW3	0.042	7.197	0.3	0.05	1.0	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.083	74.671	0.7	0.27	9.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.145	74.631	0.8	0.2	9.8	OK
SW1.001	Pipe	SW1	SW2	0.038	21.748	0.9	0.06	3.0	OK
SW1.000	Pipe	RG1	SW1	0.034	21.748	1.0	0.11	3.0	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.009	2.079	0.0		0.8	
Overflow2	No Delay	Bio 3	Bio 4	0.008	0.196	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.172	74.636	0.9	0.21	9.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.022	2.300	0.0		0.7	
SW1.002	Pipe	SW2	SW3	0.069	54.957	0.9	0.25	7.2	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		

FEH2022: 100 years: Increase Rainfall (%): +45: 480 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.032	12.353	0.7	0.04	1.1	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.026	7.181	0.6	0.03	0.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.043	12.532	0.4	0.09	1.1	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	33.194	1.1	0.05	2.9	OK
SW2.001	Pipe	RG3	SW3	0.063	12.495	0.9	0.03	1.1	OK
SW2.002	Pipe	M100D30.2	SW3	0.031	7.179	0.3	0.04	0.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.078	74.605	0.7	0.18	6.5	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.151	74.556	0.7	0.13	6.5	OK
SW1.001	Pipe	SW1	SW2	0.032	21.731	0.8	0.04	2.1	OK
SW1.000	Pipe	RG1	SW1	0.028	21.731	0.9	0.08	2.1	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.007	2.045	0.0		0.5	
Overflow2	No Delay	Bio 3	Bio 4	0.007	0.123	0.0		0.1	
SW1.005	Pipe	SW4	Soakaway SA1	0.178	74.568	0.8	0.14	6.5	OK
Overflow1	No Delay	Bio 1	Bio 2	0.018	2.316	0.0		0.4	
SW1.002	Pipe	SW2	SW3	0.056	54.922	0.8	0.17	4.9	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 960 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.031	14.993	0.7	0.04	1.0	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.025	8.741	0.6	0.03	0.6	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.042	15.226	0.4	0.08	1.0	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	40.214	1.0	0.05	2.7	OK
SW2.001	Pipe	RG3	SW3	0.063	15.135	0.9	0.03	1.0	OK
SW2.002	Pipe	M100D30.2	SW3	0.030	8.739	0.3	0.03	0.6	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.078	89.612	0.6	0.16	6.0	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.151	89.581	0.7	0.12	6.0	OK
SW1.001	Pipe	SW1	SW2	0.029	25.513	0.7	0.04	1.7	OK
SW1.000	Pipe	RG1	SW1	0.026	25.513	0.9	0.06	1.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.007	1.704	0.0		0.4	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.178	89.616	0.8	0.13	6.0	OK
Overflow1	No Delay	Bio 1	Bio 2	0.017	2.138	0.0		0.4	
SW1.002	Pipe	SW2	SW3	0.053	65.723	0.8	0.15	4.3	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 960 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.024	15.035	0.7	0.02	0.7	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.020	8.741	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.034	15.232	0.3	0.05	0.7	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	40.232	0.9	0.03	1.8	OK
SW2.001	Pipe	RG3	SW3	0.063	15.177	0.8	0.02	0.7	OK
SW2.002	Pipe	M100D30.2	SW3	0.025	8.739	0.3	0.02	0.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.079	89.476	0.6	0.11	3.8	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.152	89.463	0.6	0.08	3.8	OK
SW1.001	Pipe	SW1	SW2	0.024	25.318	0.7	0.02	1.2	OK
SW1.000	Pipe	RG1	SW1	0.021	25.318	0.8	0.04	1.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	1.497	0.0		0.2	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.179	89.510	0.7	0.08	3.8	OK
Overflow1	No Delay	Bio 1	Bio 2	0.014	2.093	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.042	65.547	0.7	0.1	2.8	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 1440 mins: Summer

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.026	16.661	0.7	0.03	0.8	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.022	9.737	0.5	0.02	0.4	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.036	16.949	0.3	0.06	0.8	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	44.846	1.0	0.03	2.0	OK
SW2.001	Pipe	RG3	SW3	0.063	16.803	0.8	0.02	0.8	OK
SW2.002	Pipe	M100D30.2	SW3	0.024	9.735	0.3	0.02	0.4	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.054	99.378	0.6	0.12	4.4	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.118	99.402	0.7	0.09	4.4	OK
SW1.001	Pipe	SW1	SW2	0.025	27.982	0.6	0.02	1.2	OK
SW1.000	Pipe	RG1	SW1	0.021	27.982	0.8	0.04	1.2	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.005	1.305	0.0		0.2	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.144	99.407	0.7	0.09	4.4	OK
Overflow1	No Delay	Bio 1	Bio 2	0.014	1.949	0.0		0.2	
SW1.002	Pipe	SW2	SW3	0.045	72.823	0.7	0.11	3.2	OK

Project:	Date:		
Chicklade Services, Car Park Extension 12718	21/07/2026		
	Designed by: CJB	Checked by: SW	Approved By:
Report Details: Type: Connections Summary Storm Phase: Phase 2 Car Park	Company Address: AWA Consulting Engineers The Old Brewery House Portersbridge Street, Romsey		



FEH2022: 100 years: Increase Rainfall (%): +45: 1440 mins: Winter

Connection	Connection Type	From	To	Max. Flow Depth (m)	Discharge Volume (m³)	Max. Velocity (m/s)	Flow / Capacity	Max. Flow (L/s)	Status
Aco M100D30	Custom	M100D30	RG3	0.021	16.745	0.6	0.02	0.5	Flood Risk
Aco M100D30.1	Custom	M100D30.1	M100D30.2	0.017	9.740	0.5	0.01	0.3	Flood Risk
Aco M150D20	Custom	M150D20	RG2 ACO	0.029	17.050	0.3	0.04	0.5	Flood Risk
SW2.000	Pipe	RG2 ACO	SW2	0.063	44.882	0.8	0.02	1.3	OK
SW2.001	Pipe	RG3	SW3	0.063	16.887	0.7	0.01	0.5	OK
SW2.002	Pipe	M100D30.2	SW3	0.017	9.738	0.3	0.02	0.3	OK
SW1.003	Pipe	SW3	SPEL ESR Ph II	0.044	98.859	0.5	0.08	2.9	OK
SW1.004	Pipe	SPEL ESR Ph II	SW4	0.111	98.890	0.6	0.06	2.9	OK
SW1.001	Pipe	SW1	SW2	0.020	27.340	0.6	0.02	0.8	OK
SW1.000	Pipe	RG1	SW1	0.017	27.340	0.7	0.03	0.8	OK
Overflow Bio4	Lagged Flow	Bio 4	M150D20	0.000	0.000	0.0		0.0	
Overflow Bio 2	Lagged Flow	Bio 2	RG1	0.004	0.807	0.0		0.1	
Overflow2	No Delay	Bio 3	Bio 4	0.000	0.000	0.0		0.0	
SW1.005	Pipe	SW4	Soakaway SA1	0.138	98.952	0.6	0.06	2.9	OK
Overflow1	No Delay	Bio 1	Bio 2	0.012	1.765	0.0		0.1	
SW1.002	Pipe	SW2	SW3	0.036	72.216	0.6	0.07	2.1	OK

Soakaway SA10
Aquacell Core-R
15.0m x 3.5m x 1.2m deep
No. Crates L: 15 W: 7 H: 3

GL: 144.725m
TL: 143.100m
IL: 142.875m
BL: 141.900m
Infiltration rate: 0.135m/hr
Effective Volume: 59.875m³ (95% porosity)
Storage Req'd: 58.251m³ (1% ARP + 45% cc, 960mins winter)

Sub-base baffle

150mm perforated pipe in 1.0 x 0.6m deep trench filled with permeable backfill, top dressed with shingle

Final profile and level of new embankments to be constructed to architect's details

110mm vent pipe with cap, 300mm min termination above ground

2 No. Marsh Industries 90,000 litre static water storage tank for fire fighting reservoir, total 180,000 litre capacity, Ø160 link pipes between tank bases. 600mm access covers, C250 rating
Tank 1 - CL: 145.860 & 145.425
Tank 1 - OL: 144.220
Tank 2 - CL: 145.825 & 145.330

FIRE TANK 1
FIRE TANK 2

Bio 3 - rain garden
EL: 146.063m
Depth: 0.050m
BL: 146.013m
Sub-base slope: 20.00 (1:X)

Overflow: 145.960
Link pipe from overflow into Bio 4 storage layer

Bio 4 - rain garden
EL: 145.790m
Depth: 0.050m
BL: 145.740m
Sub-base slope: 20.00 (1:X)
Flows up to the 10yr event contained within the rain garden; flows beyond this are designed to crest the kerb and overflow to the channels and gullies downstream.

Bio 1 - rain garden
EL: 146.230m
Surface depth: 0.050m
Filter layer BL: 145.840m
Storage BL: 145.440
Sub-base slope: 30.00 (1:X)

Overflow: 146.220m
Link pipe from overflow into Bio 2 storage layer

Bio 2 - rain garden
EL: 146.100m
Surface depth: 0.050m
Filter layer BL: 145.630m
Storage BL: 145.230
Sub-base slope: 40.00 (1:X)
Flows up to the 10yr event contained within the rain garden; flows beyond this are designed to crest the kerb and overflow to the channels and gullies downstream.

Note: the fire reservoirs are a static volume and not topped up. The overflow is a safeguard against nominal water ingress via joints and covers, and there is no quantifiable flow/volume regularly discharged to the surface water network

Exceedance flows carry onto the existing site entrance road, where the gullies and wider drainage network discharge to a swale. All flows are directed away from the building and site infrastructure

Land drain to base of slope set below top of kerb, to prevent washout onto permeable bays

Land drain to base of slope set below top of kerb, to prevent washout onto road

Concrete slab under sub-station draining to car park surface

Asphalt Concrete aisle (impermeable)

SW4
CL: 144.603m
Depth: 1.700m
IL: 142.903m

SW1.005
120.16 (1:X)
225mm

SW1.004
115.14 (1:X)
225mm

SW1.003
200.0 (1:X)
225mm

SW1.002
42.05 (1:X)
150mm

SW2.001
33.93 (1:X)
150mm

SW2.002
98.98 (1:X)
150mm

SW3
CL: 144.036m
Depth: 1.511m
IL: 142.525m
Sump Depth: 0.500m

SW2.000
9.16 (1:X)
150mm

SW3.000 overflow
12.57 (1:X)
150mm

SW2
CL: 144.721m
Depth: 1.357m
IL: 143.363m

SW1.001
13.61 (1:X)
150mm

SW1
CL: 145.432m
Depth: 1.200m
IL: 144.232m

SW1.000
39.86 (1:X)
150mm

RG2 ACO
CL: 144.978m
Depth: 1.320m
OL: 144.098
BL: 143.658m

Aco M150D20
88.14 (1:X)
150mm

RG3 ACO
CL: 144.217m
Depth: 1.320m
OL: 143.337m
BL: 142.897m

Aco M100D30
38.29 (1:X)
100mm

M100D30.2
Aco Sump unit
CL: 143.700m
Depth: 0.710m
IL: 142.990m

Aco M100D30.1
57.24 (1:X)
100mm

RG1
CL: 145.637m
Depth: 1.215m
OL: 144.980m
BL: 144.422m

Elec bays 1
EL: 145.960m
Depth: 0.450m
BL: 145.410m
Paving Depth: 130mm
Sub-base slope: 22.00 (1:X)

Elec bays 2
EL: 145.270m
Depth: 0.450m
BL: 144.820m
Paving Depth: 130mm
Sub-base slope: 22.00 (1:X)

Elec bays 3
EL: 144.856m
Depth: 0.450m
BL: 144.406m
Paving Depth: 130mm
Sub-base slope: 22.00 (1:X)

Elec bays 4
EL: 144.218m
Depth: 0.450m
BL: 143.768m
Paving Depth: 130mm
Sub-base slope: 22.00 (1:X)

ICE Bays 1
EL: 146.516m
Depth: 0.380m
BL: 146.136m
Paving Depth: 130mm

ICE Bays 2
EL: 146.460m
Depth: 0.380m
BL: 146.080m
Paving Depth: 130mm

ICE Bays 3
EL: 145.875m
Depth: 0.450m
BL: 145.425m
Paving Depth: 130mm

ICE Bays 4
EL: 145.692m
Depth: 0.450m
BL: 145.242m
Paving Depth: 130mm

100dia vent pipe laid with a backfall to the tank, terminating 2.4m above ground level with aspiromatic cowl

SPEL 3501 x 2501 x 900h mild steel kiosk on 150mm concrete plinth.
Alarm system - IdOil-30 with 3No. probes - 110mm PVC-u duct from kiosk to tank to suit alarm connection
Indicative position - final location to suit client requirements

SPEL 210CI/ESR - 2.92m x 1.22m dia. tank
CL: 144.320
IL: 142.978
OL: 142.928
Certified Mitigation Indices
TSS 0.8
Metals 0.6
Hydrocarbons 0.8

LV d
enc
-1.2m

Key Plan

Drainage Legend

- New storm water
- Land drainage
- Base fall of paving

ACO	channel drainage with ductile iron grating
RG	road gully
RG ACO	Aco Universal Gully outlet
OF	Overflow (pipe)
OFK	Overflow (kerb)
+30.12	Existing levels (topographical survey)
145.600	Proposed levels
+145.500	Surface level (rain garden)
1:20	Fall of surfacing
	Soakaway - geocellular crates wrapped in geotextile
	Bio Infiltration Rain garden - 400mm sandy loam, 400mm granular storage
E1	Permeable paving Parking bays - 250mm sub-base Type A Infiltration
E2	Permeable paving Parking bays - 320mm sub-base, Type A Infiltration

THIS DRAWING TO BE VIEWED AND PRINTED IN COLOUR

M 1:100
0 1 2 3 4 5 6 7 8 9 10 [m]

KERB ABBREVIATIONS	
HB2	125 x 255 PC Half battered kerb
DHB	PC dropping kerb
CS1	125 x 225 PC flat channel flush kerb
CS2	125 x 150 PC flat channel flush kerb
BN2	125 x 255 PC bull nosed kerb
BN3	125 x 150 PC bull nosed kerb deliniation
TN	PC bull nosed to half battered transition
EF1	50 x 150 type EF PC edging kerb flat top
EF2	50 x 200 type EF PC edging kerb flat top

REV	DATE	REVISIONS	REV BY	CHK BY
P2	22/7/26	Planning Issue	CJB	SW
P1	7/7/26	First Issue	CJB	SW

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awa ANDREW WARING ASSOCIATES
Consulting Engineers
STRUCTURAL - CIVIL - MARINE
Romsey, Hampshire - Bath, Somerset
Romsey: 01794 526427
Bath: 01225 259088
mail@awaengineers.com
www.awaengineers.com

ARCHITECT: Studio Four Architects
CLIENT: InstaVolt
JOB TITLE: CHICKLADE SERVICES
DRG TITLE: CAR PARK EXTENSION: DRAINAGE PLAN
STATUS: PLANNING
DRG NO: 12718-D-116
REV: P2
SCALE: 1:100 @ A1 uno
DATE: JULY 2026
DRAWN BY: CJB
CHECKED BY: SW

STORM WATER MANHOLES

Node	Cover Level (m)	Invert Level (m)	Outlet Level (m)	Base Level inc. sump (m)	Depth (m)	Sump Depth (m)	Diameter (m)	Junction Type	Cover size (mm)	Cover Type	Cover Class
RG1	145.637		144.980	144.422	1.215	0.565	0.450	Road gully	420 x 420	Gully grating	C250
RG2 ACO	144.978		144.098	143.658	1.320	0.565	0.450	Road gully	420 x 420	Gully grating	C250
RG3	144.217		143.337	142.897	1.320	0.565	0.450	Road gully	420 x 420	Gully grating	C250
SW1	145.432	144.232	144.232		1.200		0.900	900Ø conc	600 x 600	Cast iron solid	C250
SW2	144.721	143.363	143.363		1.357		0.900	900Ø conc	600 x 600	Cast iron solid	C250
SW3	144.036		143.000	142.500	1.536	0.500	0.900	900Ø conc	600 x 600	Cast iron solid	C250
SW4	144.603	142.903	142.903		1.700		0.900	900Ø conc	600 x 600	Cast iron solid	C250
M100D30.2	143.700		143.140	142.990	0.710	0.150	ACO	Sump Unit	500 x 100	Cast iron grate	C250

All surface water pipes to be 100 dia. laid min 1 : 100 unless noted otherwise
All cover levels are approximate. Cover classes to be in accordance with BS EN 124.
Cover types assumed: Recessed in paved landscape, solid in soft, solid in gravel drives
(all subject to final design by landscape architect)
Poly = Polyproprylene manhole Conc = Precast concrete box/ring as appropriate

SURFACE WATER PIPES

Name	Incoming Item(s)	Outgoing Item	Length (m)	Diameter (mm)	Slope (1:X)	Upstream Cover Level (m)	Upstream Invert Level (m)	Upstream Pipe cover (m)	Zone Upstream	Upstream Bedding Class	Downstream Cover Level (m)	Downstream Invert Level (m)	Downstream Pipe cover (m)	Zone Downstream	Downstream Bedding Class
SW1.000	RG1	SW1	6.804	150.000	39.86	145.637	144.980	0.507	Road	Class Z	145.432	144.809	0.473	Road	Class Z
SW1.001	SW1	SW2	11.828	150.000	13.61	145.432	144.232	1.050	Road	Class Z	144.721	143.363	1.207	Road	Class S
SW1.002	SW2	SW3	14.223	150.000	39.14	144.721	143.363	1.207	Road	Class S	144.036	143.000	0.886	Pedestrian	Class S
SW1.003	SW3	SPEL ESR Ph II	4.318	225.000	200.00	144.036	143.000	0.811	Road	Class Z	144.289	142.978	1.085	Road	Class Z
SW1.004	SPEL ESR Ph II	SW4	2.872	225.000	115.14	144.446	142.928	1.293	Road	Class S	144.603	142.903	1.475	Road	Class S
SW1.005	SW4	Soakaway SA1	3.365	225.000	120.16	144.603	142.903	1.475	Road	Class S	144.760	142.875	1.660	Road	Class S
SW2.000	RG2 ACO	SW2	6.730	150.000	9.16	144.978	144.098	0.730	Road	Class Z	144.721	143.363	1.207	Road	Class S
SW2.001	RG3	SW3	5.559	150.000	23.46	144.217	143.337	0.730	Road	Class Z	144.036	143.100	0.786	Pedestrian	Class S
SW2.002	M100D30.2	SW3	11.352	150.000	98.98	143.700	143.140	0.410	Road	Class Z	144.036	143.025	0.861	Pedestrian	Class S
Aco M100D30	M100D30	RG3	5.821	100.000	38.29	144.370	144.110	0.160	Road	Class Z	144.217	143.958	0.159	Road	Class Z
Aco M100D30.1	M100D30.1	M100D30.2	11.449	100.000	57.24	143.900	143.640	0.160	Road	Class Z	143.700	143.440	0.160	Road	Class Z
Aco M150D20	M150D20	RG2 ACO	6.189	150.000	884.14	144.986	144.721	0.115	Road	Class Z	144.978	144.714	0.114	Road	Class Z

GENERAL NOTES:

1. This drawing is to be read in conjunction with all relevant Architect's, Engineer's, Specialist Sub-Contractor's drawings and the specification.

2. All setting out to be in accordance with the Architect's drawings and any discrepancies between the Engineer's and Architect's drawings to be referred to the Architect before proceeding. Drawings should not be scaled for construction and fabrication purposes.

3. All work is to comply with current requirements of the Building Regulations & allied legislation whether referred to or not.

4. All materials should be used in compliance with the appropriate manufacturers recommendations.

5. The Contractor & Sub-Contractors must ensure that they have the latest issued drawings and details before commencing the relevant work.

6. Main Contractor to provide and fit suitable bracing and propping for all elements in the temporary condition during the construction stage, such as to ensure structural stability at all times.

7. In case of any discrepancies between drawings or a lack of dimensions required AWA should be contacted before work proceeds.

8. All dimensions indicated are in millimetres and all levels are in metres (Unless noted otherwise).

9. Electronic drawing information (2D or 3D) is for information only. Whilst it may be used for expediency during the design process it shall be deemed to be unchecked even where check boxes show it as checked. Electronic drawing information does not form part of the contract drawings or documentation set. PDF drawing information always takes precedence. Electronic drawing or PDF drawing information should not be scaled: only written dimensions should be used. Electronic drawing & PDF files may contain third party information and whilst every reasonable effort is made to avoid errors AWA do not accept any liability for errors in third party information. AWA do not accept liability for setting out. All setting out must be confirmed with the architect and correct setting out is the contractor's responsibility

DO NOT SCALE THIS DRAWING IF IN ANY DOUBT ASK

DRAINAGE NOTES:

1. All work to be in accordance with BS EN 752, BS 4660, BS EN 1401, BS 65 BS EN 124 & to be to the Local Authority approval.

2. All private drainage to comply with current Building Regulations, BS EN 752, and other relevant Eurocodes, British Standards and Codes of Practice.

3. Private inspection chambers and manholes to be constructed in accordance with Tables 11 & 12 of the Building Regulations Approved Document H.

4. Manhole covers and frames to BS EN 124 to be as follows:
D400 in Carriageway
B125/C250 in Other Trafficked Areas
A15 in Landscaped Areas
Covers lockable where noted.

5. Private drainage to be constructed using Osma Plastic Drainage System or similar approved, laid to manufacturer's instructions.
All private drainage 100mm dia. unless shown otherwise on table.

6. AWA are responsible for below ground drainage terminating at slab level. All above ground drainage is the responsibility of the architect/M&E designer, inclusive of stand alone pump units (e.g. Saniflo).

7. Private drains located adjacent to dwellings are to be constructed in accordance with Diagram 8 of the Building Regulations Approved Document H.

8. Foul popup positions are to be checked against the Architect's drawings to ensure compatibility. The venting requirements are the responsibility of others and any notation of 'SVP' on AWA drawings does not constitute design input of vent locations.

9. Rainwater downpipes are to be connected direct to drain via suitable adaptor removable to permit access for rodding.

10. Rainwater downpipe positions are to be checked against Architect's drawings to ensure compatibility (any discrepancies to be referred to the Architect).

11. Any external linear channels shall have their grating style and material choice agreed by the Architect/Landscape Architect to suit the client's aesthetic preference. Loading categories to be in accordance with Note 4. In hygiene sensitive environments such as commercial kitchens, hospitals etc, the material, finish and grating performance requirements are to be specified by the Architect/M&E designer. AWA input is strictly limited to the hydraulic performance of channels.

12. The Main Contractor is to provide all necessary sleeves or lintels where drainage pipes pass through walls or foundations.

13. Drainage runs adjacent to or through foundations may result in deepening the foundations locally. Where pipelines pass within 1m of buildings or walls the foundations are to be taken down below the bottom of the trench.

14. The back filling of drainage trenches which are in the proximity of foundations must be in accordance with any special requirements as set out or requested by the Structural Engineer and the Building Regulations.

15. Use of this drawing does not absolve the Client or Contractor from their responsibilities under Health and Safety: The Construction (Design & Management) Regulations 2015.

16. Unless noted otherwise all levels are in metres.

17. Unless noted otherwise, this drawing including the existing site levels, foul and surface water invert levels, have been produced with reference to survey information compiled by others and supplied to AWA. No responsibility is taken for any inaccuracies and/or omissions in the information as supplied

18. Where new connections are to be made to existing manholes/sewers, the invert levels should be checked prior to commencement of works and any variance reported to the Engineer immediately.

19. This drawing is to be read in conjunction with all relevant Architect's, Engineer's and Specialist Sub-contractor's drawings and The Specification.

20. All setting-out is to be in accordance with the Architects drawings. Any discrepancies between the Engineer's and Architect's drawings to be referred to the Architect before proceeding.
Dimensions must not be scaled.

21. Where drains pass through foundations or connect to manholes, flexible pipe joints are to be provided within 150mm of the of the face of the structure and within a further 600mm to form a rocker pipe.

22. Where depth to invert does not exceed 600mm and the pipe size does not exceed 100mm diameter, 300mm dia. polypropylene mini access chambers (MACs) may be used. Elsewhere, proprietary polypropylene or precast concrete manholes are to be used. Refer to manhole schedule for full details.

23. Where pipelines cross, each is to be surrounded with grade C16/20 mass concrete for a distance not less than 1m centred on the crossing point. Length of surround to be extended as necessary to within150mm of the next nearest flexible joint.

24. All pipework with less than 600mm of effective cover is to be surrounded with grade C16/20 mass concrete as follows:
100mm dia. = 100mm thick surround
150mm dia. = 150mm thick surround
Flexible joints are to be provided at all pipe joints.

25. Effective cover' is the minimum depth of cover over the pipe crown at any time during the construction process.

26. The Contractor is to ensure that protective measurers are taken to ensure that drainage pipework and fittings are not damaged by site traffic prior to oversite filling operations being
completed around buildings.

27. The top run of each foul private network is to be laid no slacker than 1:60.

28. All surface water drainage to be 100Ø and all foul water drainage to be 100Ø U.N.O

29. Pipeline arrangements shown are indicative only, Contractor is to ensure all pipework connections are arranged to ensure flows are directed down or into the main channel in the direction of the main flow. Where necessary3#4bends are to be used on oblique connections inside the manhole benching where sufficient room exists or the pipelines on oblique connections are arranged outside the chamber to be able to divert flows down the main channel. Connection bought in perpendicular to the main channel are not acceptable. Where possible the main channel flow shall be from any connections with WCs to ensure a flush through the main channel is achieved.

30. The Contractor's attention is drawn to the need to ensure any trenches excavated through previously compacted filled areas in particular under the building footprint and immediately around the outside are re-compacted to ensure localised differential settlement does not occur.

31. Unless noted otherwise, the services indicated are taken from information supplied by the relevant statutory authority or groundscan survey. No guarantee is given as to their accuracy and the Contractor is responsible for locating all existing services prior to the commencement of the works.

REV	DATE	REVISIONS	REV BY	CHK BY
P1	22/7/26	Issued for Planning	CJB	SW

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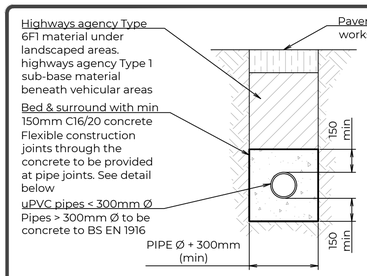
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awa ANDREW WARING ASSOCIATES
Consulting Engineers
STRUCTURAL - CIVIL - MARINE
Romsey, Hampshire - Bath, Somerset
Romsey: 01794 524647
Bath: 01225 251068
mail@awaengineers.com
www.awaengineers.com

ARCHITECT: Studio Four Architects
CLIENT: InstaVolt
JOB TITLE: CHICKLADE SERVICES
DRG TITLE: CAR PARK EXTENSION: DRAINAGE TABLES AND NOTES
STATUS: CONSTRUCTION

DRG NO	REV
12718-D-117	P1

SCALE	DATE	DRAWN BY	CHECKED BY
N/A	JULY 2026	CJB	SW



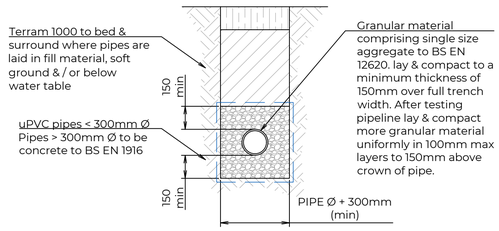
CLASS Z PIPE BEDDING DETAIL

1 : 20

For pipes with cover from finished surface to crown less than 1.2m in vehicular areas or less than 0.6m cover in landscaped areas or beneath buildings

uPVC / THERMOPLASTIC PIPE NOTES

1. Pipes less than or equal to 500mm Ø to have a nominal short-term ring stiffness of not less than 8 kN/m² (SN8).
2. Pipes greater than 500mm Ø to have nominal short-term ring stiffness of not less than 2 kN/m² (SN2), where accompanied by manufacturer's design load calculations for the proposed use.
3. uPVC pipework to BS EN 13476 parts 1, 2 & 3 & WIS 4-35-01.
4. uPVC pipework to be BSI kitemarked or have equivalent third party certification.
5. Pipes must be capable of withstanding 4000 psi jetting pressure, with evidence supplied to the engineer prior to procurement.



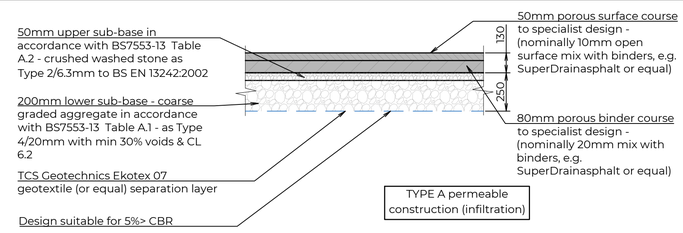
CLASS S PIPE BEDDING DETAIL

1 : 20

For pipes with cover from finished surface to crown greater than 1.2m in vehicular areas, or greater than 0.6m cover in landscaped areas or beneath buildings

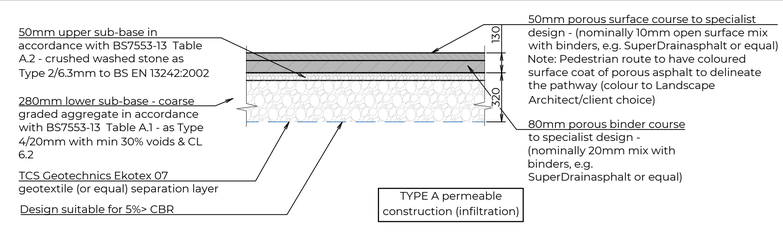
Notation as Class Z unless otherwise noted

GRANULAR MATERIAL	
PIPE SIZE	AGGREGATE
100mm	10mm Single size
150mm	10 OR 14mm Single size of 14-15mm graded
150-500mm	10,14,20mm Single size or 20-25mm graded
600mm	10,14,20 OR 40mm Single crushed rock or 40-50mm graded



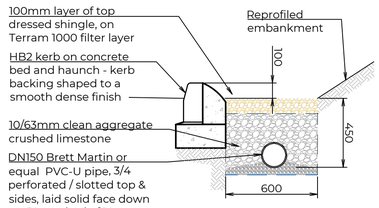
POROUS ASPHALT PERMEABLE PAVING PARKING BAYS - E1

1 : 20



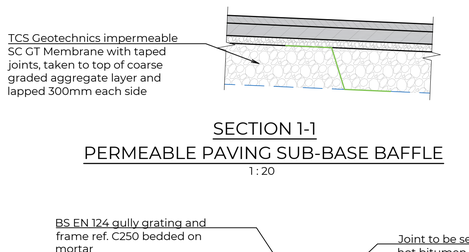
POROUS ASPHALT PERMEABLE PAVING PARKING BAYS - E2

1 : 20



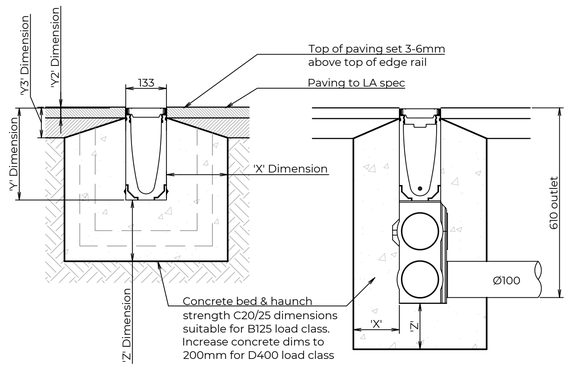
LAND DRAIN AT BASE OF EMBANKMENT

1:20



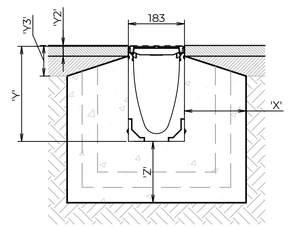
SECTION 1-1 PERMEABLE PAVING SUB-BASE BAFFLE

1 : 20



ACO MULTIDRAIN M100D UNIVERSAL SUMP

1:10



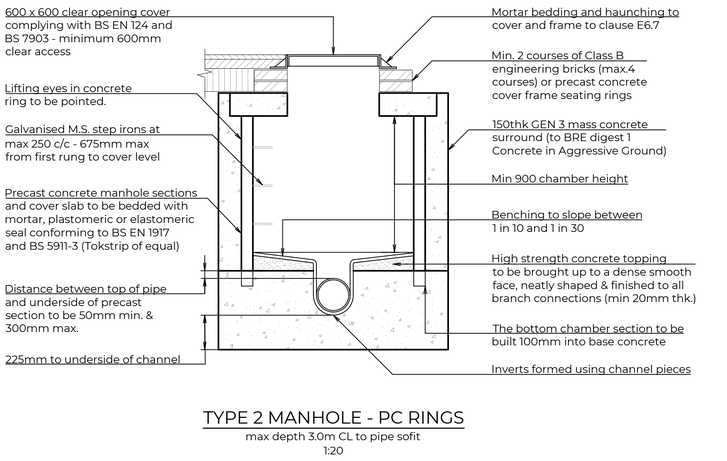
ACO MULTIDRAIN M150D20

1:10

ACO MULTIDRAIN M100D30

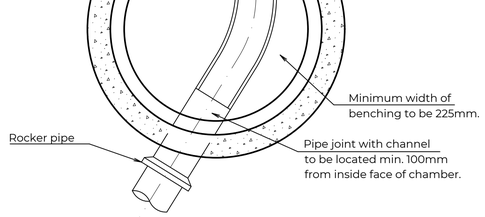
1:10

ACO MULTIDRAIN CHANNEL CONCRETE DEPTHS					
	Load Class	A15	B125	C250	D400
Minimum Dimensions (mm)	X	100	150	150	200
	Y	Full channel height (less Y2 where applicable)			
	Z	100	150	150	200
Maximum Dimensions (mm)	Y2	35	35	35	35
	Y3	100	60	60	60



TYPE 2 MANHOLE - PC RINGS

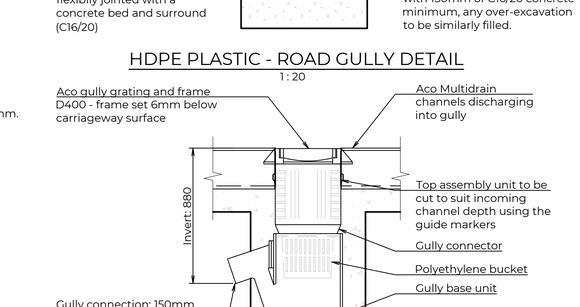
max depth 3.0m CL to pipe sofit
1:20



Rocker pipes	
Nominal diameter (mm)	Effective length (m)
150 to 600	0.6

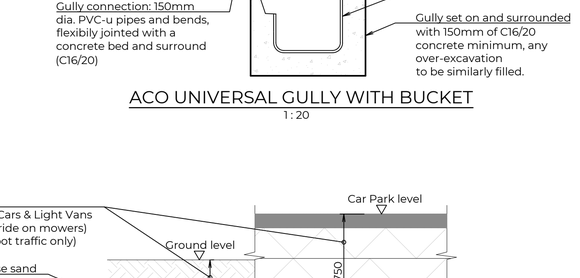
PLAN ON CONCRETE MANHOLE BASE

1 : 20



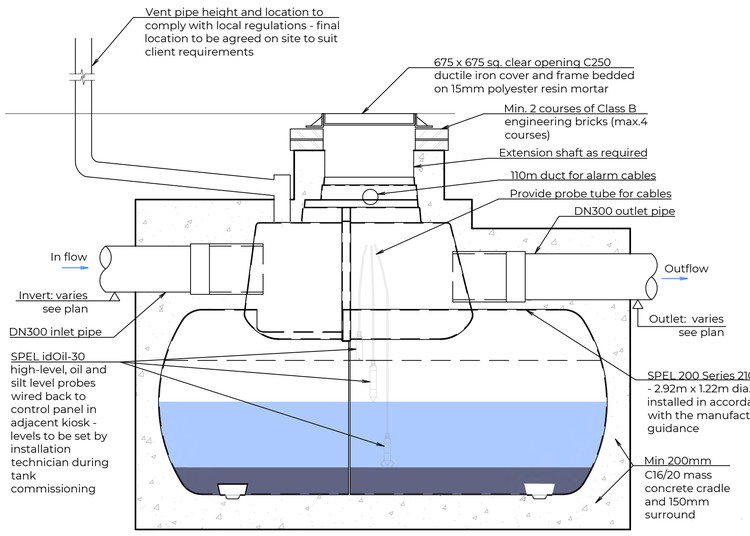
HDPE PLASTIC - ROAD GULLY DETAIL

1 : 20



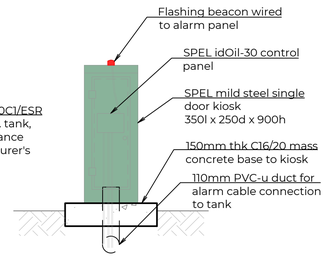
ACO UNIVERSAL GULLY WITH BUCKET

1 : 20



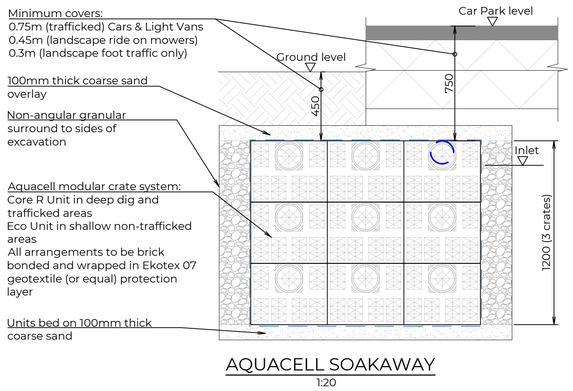
INDICATIVE SECTION THROUGH SPEL ESR TREATMENT BY-PASS SEPARATOR

1 : 20



SPEL KIOSK

1 : 20



AQUACELL SOAKAWAY

1:20

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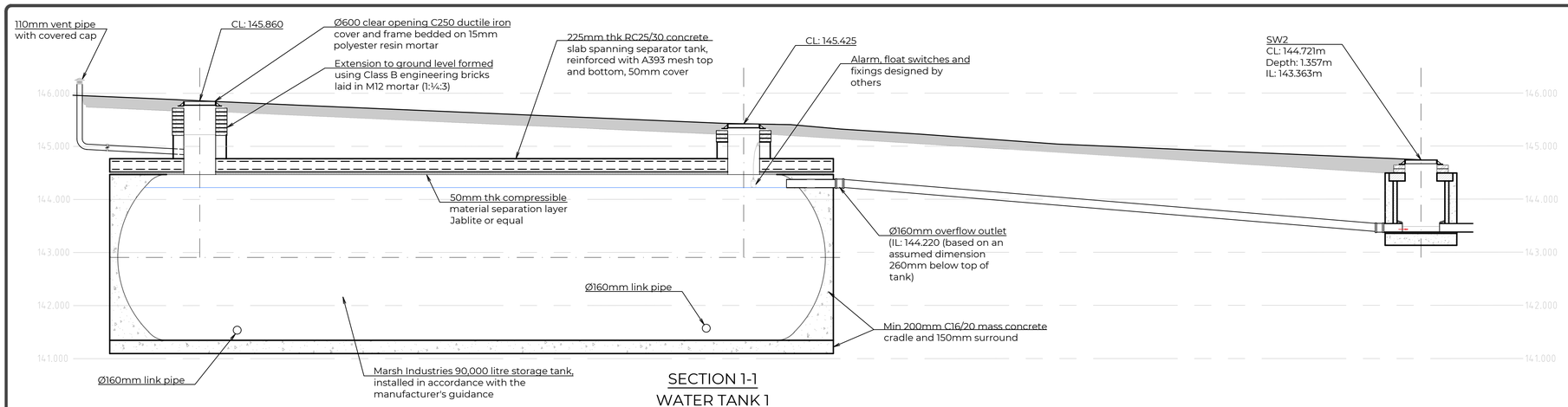
M 1:10
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REV	DATE	REVISIONS	REV BY	CHK BY
P1	22/7/26	Issued for Planning	CJB	SW

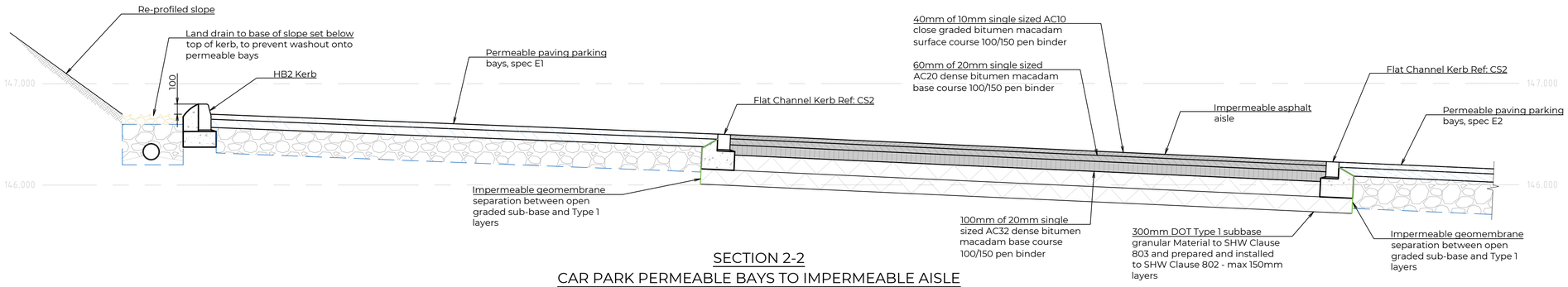
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ANDREW WARING ASSOCIATES
Consulting Engineers
STRUCTURAL - CIVIL - MARINE
Romsey, Hampshire - Bath, Somerset
Romsey: 01794 526427
Bath: 01225 259098
mail@awengineers.com
www.awengineers.com

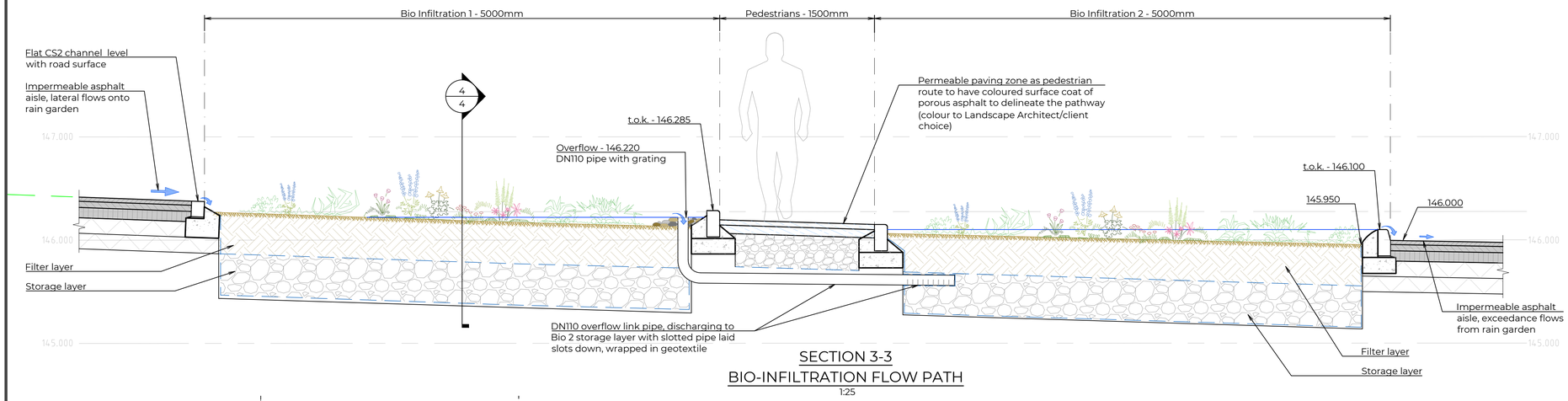
ARCHITECT: Studio Four Architects
CLIENT: InstaVolt
JOB TITLE: CHICKLADE SERVICES
DRG TITLE: CAR PARK EXTENSION: DRAINAGE DETAILS 1
STATUS: PLANNING
DRG NO: 12718-D-129
REV: P1
SCALE: 1:20 & 1:10 @ A1
DATE: JULY 2026
DRAWN BY: CJB
CHECKED BY: SW



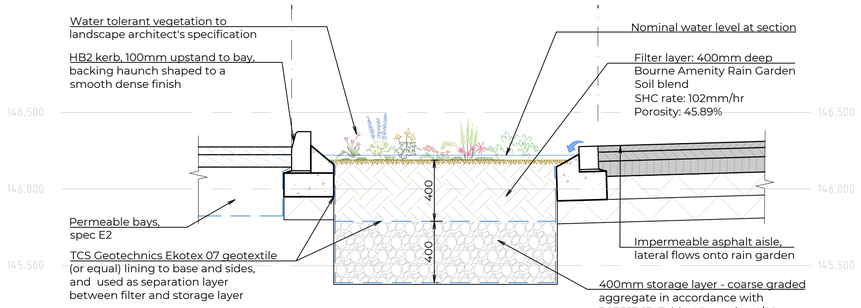
SECTION 1-1
WATER TANK 1



SECTION 2-2
CAR PARK PERMEABLE BAYS TO IMPERMEABLE AISLE
1:25



SECTION 3-3
BIO-INFILTRATION FLOW PATH
1:25



SECTION 4-4 - BIO INFILTRATION (RAIN GARDEN)
1 : 20

THIS DRAWING TO BE VIEWED AND PRINTED IN COLOUR

M 1:50
0 0.5 1.0 1.5 2.0 3.0 4.0 5.0[m]

M 1:25
0 0.25 0.50 0.75 1.0 1.5 2.0 2.5 [m]

M 1:20
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REV	DATE	REVISIONS	REV BY	CHK BY
P1	22/7/26	Issued for Planning	CJB	SW

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ANDREW WARING ASSOCIATES
Consulting Engineers
STRUCTURAL - CIVIL - MARINE
Romsey, Hampshire - Bath, Somerset
Romsey: 01794 526447
Bath: 01225 259028
mail@awaengineers.com
www.awaengineers.com

ARCHITECT	Studio Four Architects
CLIENT	InstaVolt
JOB TITLE	CHICKLADE SERVICES
DRG TITLE	CAR PARK EXTENSION: DRAINAGE DETAILS 2
STATUS	PLANNING
DRG NO	12718-D-130
REV	P1
SCALE	1:50, 1:25 & 1:20 @ A1
DATE	JULY 2026
DRAWN BY	CJB
CHECKED BY	SW