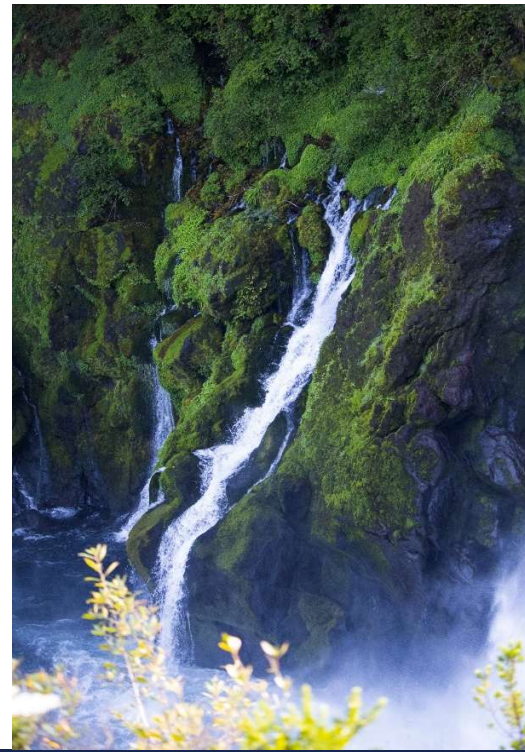




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Flood Risk Assessment and Surface Water Drainage Strategy AEG9910_SP4_HighPost_07

Site Address: High Post Business Park
Salisbury
Wiltshire
SP4 6AT

UK Experts in Flood Modelling, Flood Risk
Assessments, and Surface Water Drainage Strategies

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Document Issue Record

Project: Flood Risk Assessment and Surface Water Drainage Strategy

Prepared for: Spitfire Enterprises

Reference: AEG9910_SP4_HighPost_07

Site Location: High Post Business Park, Salisbury, Wiltshire, SP4 6AT

Issue	Date	Author	Check	Auth.	Comments
1	03/03/2026	Jack Allen	JC	OH	First issue
2	04/03/2026	Jack Allen	-		Minor amendments
3	10/07/2026	Jack Allen	-		LLFA Holding Objection

Issue 3

This report and the Surface Water Drainage Strategy Layout (Appendix B) have been updated in response to Wiltshire Council's Holding Objection dated 4 May 2026 (application ref: PL/2026/01867). The updates address each point raised as follows:

1. BRE365 infiltration testing is proposed at the locations and depths of the proposed infiltration features. It is proposed that this be secured by way of a pre-commencement planning condition, with the detailed drainage scheme updated where necessary to reflect the test results prior to commencement of development.
2. Exceedance flow routing has been added to the Surface Water Drainage Strategy Layout (Appendix B). Exceedance flows will initially be contained within the access road and car parking areas by 150mm upstand kerbs. Should this capacity be exceeded, flows will be routed southwards (in the western and central portions of the site) and eastwards (for the eastern portions of the site), consistent with the existing site topography, into the landscape buffer, which will promote further infiltration and attenuate flows prior to leaving the site.
3. Cross-sections for the proposed drainage features have been added as Appendix E.

4. Maintenance: The proposed surface water drainage features serving the site, including the geocellular soakaways, permeable paving, filter drains and bioretention areas, will remain in private ownership. Overall responsibility for the long-term operation and maintenance of these features will rest with the site owner, who will appoint a suitably qualified specialist contractor to undertake routine inspection, maintenance and repair in accordance with the maintenance schedule.

In addition to the matters raised in the Holding Objection, the Environment Agency's surface water flood maps have been reviewed against the updated dataset; there is no change in flood risk to the site as a result of this update.

Please Note:

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Summary

Development Description	Existing	Proposed
Development Type	Greenfield Land	Commercial development, including the erection of a building, a roundabout/ access, parking, and a solar farm.
EA Vulnerability Classification	No Vulnerability Classification	Less Vulnerable – commercial Essential Infrastructure – solar aspect of development.
Ground Floor Level	LiDAR data shows the ground elevation of the site varies between approximately 118.03m AOD and 135.17m AOD.	No change
Level of Sleeping Accommodation	N/A	None
Impermeable Surface Area	Greenfield	3.309ha
Surface Water Drainage	N/A ¹	See Section 6
Risk to Development	Summary	Comment
EA Flood Zone	Flood Zone 1	Low risk of fluvial flooding.
Flood Source	Pluvial	Low risk of pluvial flooding.
SFRA Available	Wiltshire Council Level 1 Strategic Flood Risk Assessment (Wiltshire Council, 2024)	
Management Measures	Summary	Comment
Ground floor level above extreme flood levels	Yes	Low risk of flooding from all analysed sources.
Safe Access/Egress Route	Yes	Low risk of flooding from all analysed sources.
Flood Resilient Design	Not required	Low risk of flooding from all analysed sources.

Site Drainage Plan	Yes	See Section 6 – all surface water on site managed up to the 1 in 100 year plus climate change event.
Flood Warning and Evacuation Plan	N/A ¹	Recommended that site manager monitor Met Office Weather Warnings for extreme weather events.
Offsite Impacts	Summary	Comment
Displacement of floodwater	No	Low risk of flooding from all analysed sources.
Increase in surface run-off generation	No	See Section 6 – all surface water on site managed up to the 1 in 100 year plus climate change event.
Impact on hydraulic performance of channels	No	No channels located within the site.

¹ not required for this assessment

² data not available.

1. Introduction

- 1.1. Aegaea were commissioned by Spitfire Enterprises to undertake a Flood Risk Assessment (FRA) and Surface Water Drainage Strategy (SWDS) to facilitate a planning application for the proposed development. This report has been prepared in accordance with the requirements set out in the National Planning Policy Framework (NPPF) and the associated Planning Practice Guidance.
- 1.2. This report is intended to support a full planning application and as such the level of detail included is commensurate and subject to the nature of the proposals.

Site Overview

- 1.3. The site of the proposed development is High Post Business Park, Salisbury, Wiltshire, SP4 6AT (Figure 1).

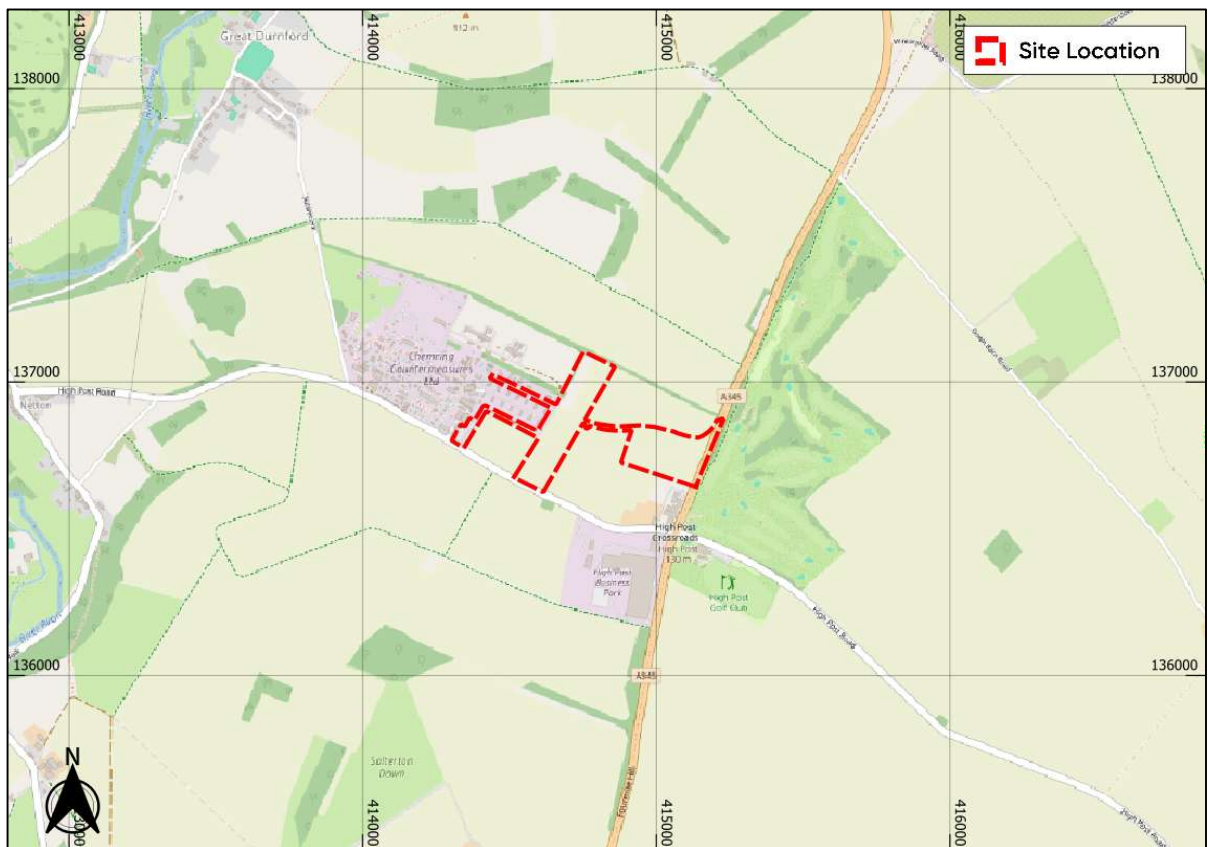


Figure 1: Site Location (Base map and data from OpenStreetMap and OpenStreetMap Foundation (CC-BY-SA). © <https://www.openstreetmap.org> and contributors)

- 1.4. The existing site is a greenfield parcel of land. The client is seeking planning permission for a commercial development, including the erection of a building, a roundabout/ access, parking, and a solar farm.
- 1.5. In the absence of a topographical survey, Environment Agency Light Detection and Ranging (LiDAR) data Digital Terrain Model has been used to review the topography of the site (Figure 2). LiDAR data shows the ground elevation of the site varies between approximately 118.03m AOD (metres Above Ordnance Datum) and 135.17m AOD. Site is shown to generally fall in an easterly direction.

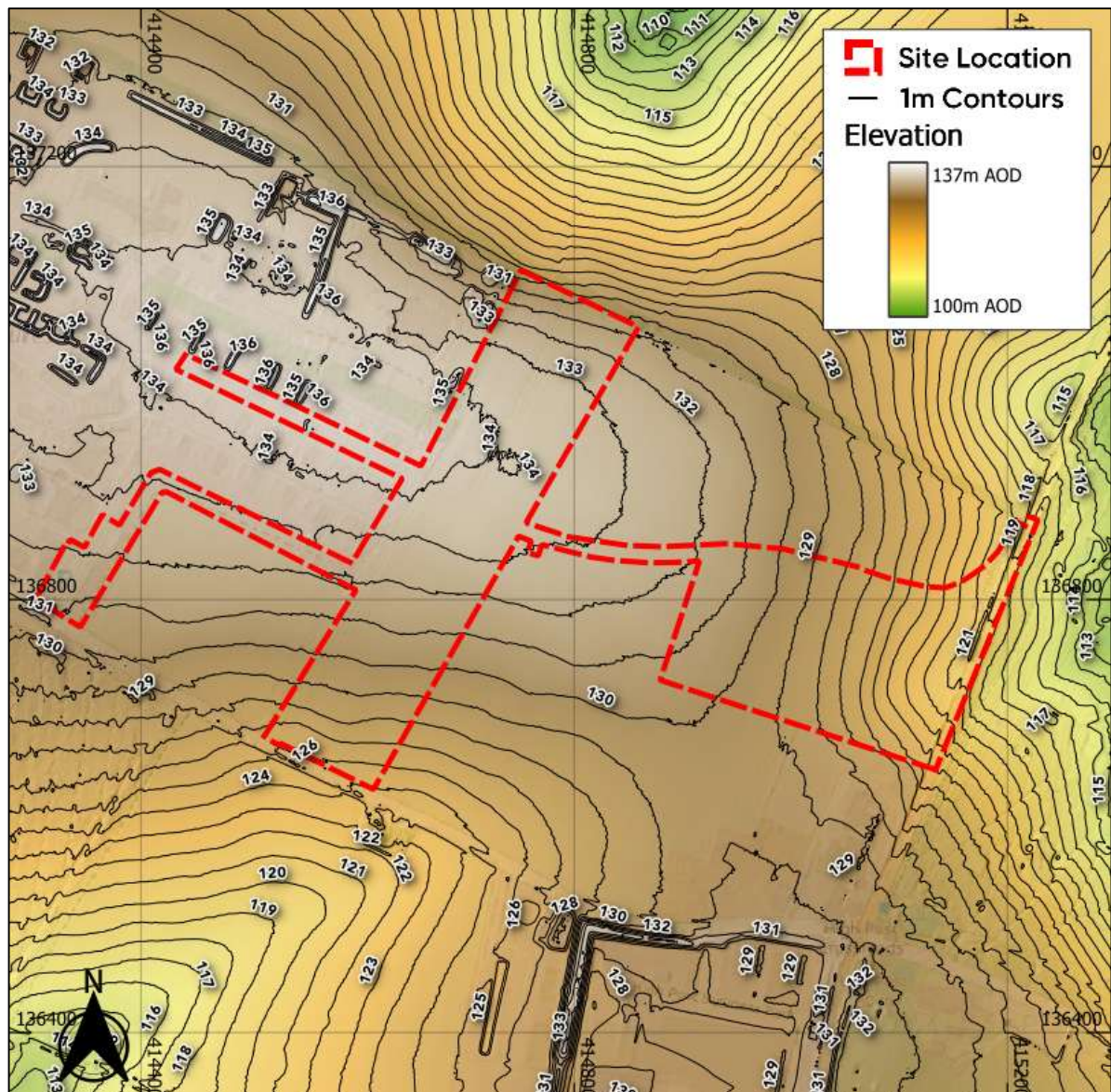


Figure 2: Site Topography (Base map and data from OpenStreetMap and OpenStreetMap Foundation (CC-BY-SA). © <https://www.openstreetmap.org> and contributors. Contains public sector information licensed under the Open Government Licence v3.0)

- 1.6. Wiltshire Council is the Local Planning Authority (LPA) for the site and also the designated Lead Local Flood Authority (LLFA). The site sits within the Environment Agency's Wessex region.

Planning Policy and Guidance

1.7. UK government planning guidance states¹ that an FRA is required for developments which are:

- *in flood zone 2 or 3 including minor development and change of use*
- *more than 1 hectare (ha) in flood zone 1*
- *less than 1 ha in flood zone 1, including a change of use in development type to a more vulnerable class (for example from commercial to residential), where they could be affected by sources of flooding other than rivers and the sea (for example surface water drains, reservoirs)*
- *in an area within flood zone 1 which has critical drainage problems as notified by the Environment Agency*

1.8. The site area is greater than 1ha therefore an FRA and SWDS are required in accordance with the NPPF.

1.9. The objective of this FRA is to demonstrate that the proposals are acceptable in terms of flood risk. This report summarises the findings of the study and specifically addresses the following issues in the context of the current legislative regime:

- Fluvial/tidal flood risk
- Surface water flood risk
- Risk of flooding from other sources

¹ <https://www.gov.uk/guidance/flood-risk-assessment-for-planning-applications#when-you-need-an-assessment>

2. Planning Policy

- 2.1. Inappropriate development in a flood risk area could pose significant risk in terms of personal safety and damage to property for the occupiers of the development or for people elsewhere. The approach taken in the assessment of flood risk at the planning stage is set out in national, regional, and local planning policy and associated guidance. This section summarises the key policies and guidance relevant to the proposed development.

National Planning Policy Framework (NPPF)

- 2.2. The National Planning Policy Framework² (NPPF) (MHCLG, 2024) which includes UK Government policy on development and flood risk states:

170. Inappropriate development in areas at risk of flooding should be avoided by directing development away from areas at highest risk (whether existing or future). Where development is necessary in such areas, the development should be made safe for its lifetime without increasing flood risk elsewhere.

181. When determining any planning applications, local planning authorities should ensure that flood risk is not increased elsewhere. Where appropriate, applications should be supported by a site-specific flood-risk assessment. Development should only be allowed in areas at risk of flooding where, in the light of this assessment (and the sequential and exception tests, as applicable) it can be demonstrated that:

- a) within the site, the most vulnerable development is located in areas of lowest flood risk, unless there are overriding reasons to prefer a different location;*
- b) the development is appropriately flood resistant and resilient such that, in the event of a flood, it could be quickly brought back into use without significant refurbishment;*
- c) it incorporates sustainable drainage systems, unless there is clear evidence that this would be inappropriate;*

² <https://www.gov.uk/guidance/national-planning-policy-framework>, last updated Dec 2024

- d) any residual risk can be safely managed; and*
- e) safe access and escape routes are included where appropriate, as part of an agreed emergency plan.*

176. Applications for some minor development and changes of use should also not be subject to the sequential test, nor the exception test [set out below], but should still meet the requirements for site-specific flood risk assessments set out in footnote 63.

2.3. Footnote 63 of the NPPF states:

A site-specific flood risk assessment should be provided for all development in Flood Zones 2 and 3. In Flood Zone 1, an assessment should accompany all proposals involving: sites of 1 hectare or more; land which has been identified by the Environment Agency as having critical drainage problems; land identified in a strategic flood risk assessment as being at increased flood risk in future; or land that may be subject to other sources of flooding, where its development would introduce a more vulnerable use.

2.4. Flood Zones in England are defined as follows:

Table 1: Flood Zone Definitions

Flood Zone	Definition
Zone 1 Low Probability	Land having less than 1 in 1,000 annual probability of river or sea flooding (all land outside Zones 2 and 3).
Zone 2 Medium Probability	Land having between a 1 in 100 and 1 in 1,000 annual probability of river flooding; or land having between a 1 in 200 and 1 in 1,000 annual probability of sea flooding.
Zone 3a High Probability	Land having a 1 in 100 or greater annual probability of river flooding; or Land having a 1 in 200 or greater annual probability of sea flooding.
Zone 3b The Functional Floodplain	This zone comprises land where water from rivers or the sea has to flow or be stored in times of flood. The identification of functional floodplain should take account of local circumstances and not be defined solely on rigid probability parameters. Functional floodplain will normally comprise: land having a 3.3% or greater annual probability of flooding, with any existing flood risk management infrastructure operating effectively; or land that is designed to flood (such as a flood attenuation scheme), even if it would only flood in more extreme events (such as 0.1% annual probability of flooding). Local planning authorities should identify in their Strategic Flood Risk Assessments areas of functional floodplain and its boundaries accordingly, in agreement with the Environment Agency. (Not separately distinguished from Zone 3a on the Flood Map)

- 2.5. An FRA should be appropriate to the scale, nature, and location of the development. It should identify and assess the risk from all sources of flooding to and from the development and demonstrate how any flood risks will be managed over the lifetime of the development.
- 2.6. An assessment of hydrological impacts should be undertaken, including to surface water runoff and impacts to drainage networks in order to demonstrate how flood risk to others will be managed following development and taking climate change into account.

National Standards for SuDS

2.7. Updates to the national planning policy have defined a framework for Sustainable Drainage Systems (SuDS). The new 'National Standards for SuDS'³ was published on June 19th, 2025, and is used to inform this drainage strategy. There are seven key standards as follows;

- **Standard 1 Runoff Destination:** The drainage hierarchy now prioritises water re-use followed by infiltration, before discharging to watercourses and sewers.
- **Standard 2 Management of Everyday Rainfall (interception):** The design must ensure that the first 5mm of rainfall from any event is managed within the site and does not leave the boundary.
- **Standard 3 Management of Extreme Rainfall and Flooding:** The SuDS must be designed to manage runoff from all extreme rainfall events, up to and including the 1 in 100-year return period (including an allowance for climate change).
- **Standard 4 Water Quality:** The system must be designed to prevent or mitigate the risk of surface water runoff pollution using SuDS management trains.
- **Standard 5 Amenity:** The SuDS should be designed to improve the aesthetic value of the development, demonstrating multifunctional solutions where feasible.
- **Standard 6 Biodiversity:** The system should be designed to promote biodiversity using above ground SuDS where feasible.
- **Standard 7 Maintenance:** A clear plan must be provided for the long-term operation and maintenance of the SuDS to ensure it remains effective for the lifetime of the development.

Local Plan

2.8. The Core Strategy (adopted in 2015) prepared by the Local Planning Authority, Wiltshire Council, sets out the policies for development in the local area.

2.9. Policy 67 Flood Risk outlines the requirements for new development within the area. It states:

³ <https://www.gov.uk/government/publications/national-standards-for-sustainable-drainage-systems/national-standards-for-sustainable-drainage-systems-suds>

Development proposed in Flood Zones 2 and 3 as identified within the Strategic Flood Risk Assessment will need to refer to the Strategic Housing Land Availability Assessment when providing evidence to the local planning authority in order to apply the sequential test in line with the requirements of national policy and established best practice. All new development will include measures to reduce the rate of rainwater run-off and improve rainwater infiltration to soil and ground (sustainable urban drainage) unless site or environmental conditions make these measures unsuitable.

Sequential and Exception Tests

- 2.10. The Sequential and Exception Tests are applied in specific cases defined by UK Government policy. Their purpose is to drive development to areas of low flood risk and to support developments which improve flood risk for developments in areas at risk of flooding.

Sequential Test

- 2.11. Paragraph 175 of the NPPF states:

The sequential test should be used in areas known to be at risk now or in the future from any form of flooding, except in situations where a site-specific flood risk assessment demonstrates that no built development within the site boundary, including access or escape routes, land raising or other potentially vulnerable elements, would be located on an area that would be at risk of flooding from any source, now and in the future (having regard to potential changes in flood risk).

- 2.12. The site is considered to be low risk from all analysed sources of flooding in this FRA. It is also considered that safe access / egress remains possible in all fluvial and pluvial scenarios.

Exception Test

- 2.13. The Exception Test is applied to sites based on the Flood Zone and the nature of the development. The proposed development is classed as 'Less Vulnerable' in line with government development use classes.

- 2.14. The Flood Risk Vulnerability Classification table⁴ provided below in Table 2 shows which vulnerabilities are appropriate in each Flood Zone.
- 2.15. The proposed development sits within Flood Zone 1 and the proposed development is 'Less Vulnerable' (commercial development) and 'Essential Infrastructure' (solar farm). Table 2 shows Flood Zone 1 is an appropriate location for 'Less Vulnerable' and 'Essential Infrastructure' uses without the need for an Exception Test.

Table 2: Flood risk vulnerability and flood zone 'incompatibility'

Flood Zones	Flood Risk Vulnerability Classification				
	Essential Infrastructure	Highly Vulnerable	More Vulnerable	Less Vulnerable	Water Compatible
Zone 1	✓	✓	✓	✓	✓
Zone 2	✓	Exception Test required	✓	✓	✓
Zone 3a	Exception Test required	x	Exception Test required	✓	✓
Zone 3b	Exception Test required	x	x	x	✓

Summary

- 2.16. This flood risk assessment has been prepared with due consideration to the above local and national policy.

⁴ <https://www.gov.uk/guidance/flood-risk-and-coastal-change#table2>

3. Document Review

Documents and Online Mapping

- 3.1. Local Governments and Lead Local Flood Authorities provide documents which contain data and policies on flood risk and new development in their areas. These documents are introduced and briefly summarised below. For the purposes of this FRA, these documents have been reviewed for relevant information and any relevant data is discussed within the appropriate sub heading of this report.
- 3.2. The following sources of information have been reviewed for this assessment:
- Flood Map for Planning on the Environment Agency website <https://flood-map-for-planning.service.gov.uk/>
 - Long Term Flood Risk Information on the Environment Agency website <https://www.gov.uk/check-long-term-flood-risk>
 - National Planning Policy Framework (NPPF) (Ministry of Housing, Communities and Local Government, 2024)
 - Planning Practice Guidance - Flood Risk and Coastal Change (Ministry of Housing, Communities and Local Government, 2022)
 - Geoindex Onshore (British Geological Survey, 2024)
 - Wiltshire Core Strategy⁵ (Wiltshire Council, 2015)
 - Preliminary Flood Risk Assessment⁶ (Wiltshire Council, 2011)
 - Wiltshire Council Level 1 Strategic Flood Risk Assessment⁷ (Wiltshire Council, 2024)
 - Wiltshire Local Flood Risk Management Strategy⁸ (Wiltshire Council, 2015)

⁵ <https://www.wiltshire.gov.uk/planning-policy-core-strategy>

⁶ https://www.wiltshire.gov.uk/media/203/Wiltshire-preliminary-flood-risk-assessment/pdf/Flood_Risk_Assessment_-_Appendix_1.pdf?m=1572622981843

⁷ <https://www.wiltshire.gov.uk/article/14991/Strategic-Flood-Risk-Assessments-SFRA>

⁸ <https://www.wiltshire.gov.uk/media/204/Wiltshire-local-flood-risk-management-strategy/pdf/Flood-risk-management-strategy-civil-emergencies.pdf?m=1572623137863>

Preliminary Flood Risk Assessment (PFRA)

- 3.3. The PFRA, published in 2011, is a high-level appraisal of flood risk across Lead Local Flood Authority Wiltshire Council. The flood risk from all sources, including fluvial, surface water, groundwater, and surcharged sewers is evaluated. It is the basis upon which the Local Flood Risk Management Strategy is produced.
- 3.4. The PFRA summarises historical flood incidents in Wiltshire Council. The site is not recorded as having been affected by any flood event.

Strategic Flood Risk Assessment (SFRA)

- 3.5. The SFRA, published in 2024, provides the evidence base for the Local Planning Authority Wiltshire Council Local Plan and guidance for consideration when determining planning applications. The SFRA seeks to place new development into areas of lower flood risk taking into account current flood risk, future flood risk, and the effect a proposed development would have on the risk of flooding.
- 3.6. The SFRA mapping provided by Wiltshire Council has been used throughout production of this report as a source of information, particularly pertaining to historical flood incidents.

Local Flood Risk Management Strategy (LFRMS)

- 3.7. The Local Flood Risk Management Strategy sets out roles and responsibilities for flood risk management, assesses the risk of flooding in the area, where funding can be found to manage flood risk, and the policies, objectives, and actions of the Lead Local Flood Authority.
- 3.8. The Wiltshire Council LFRMS is used within this report to identify any flood management infrastructure and historical incidences of flooding.

4. Sources of Flood Risk

Fluvial

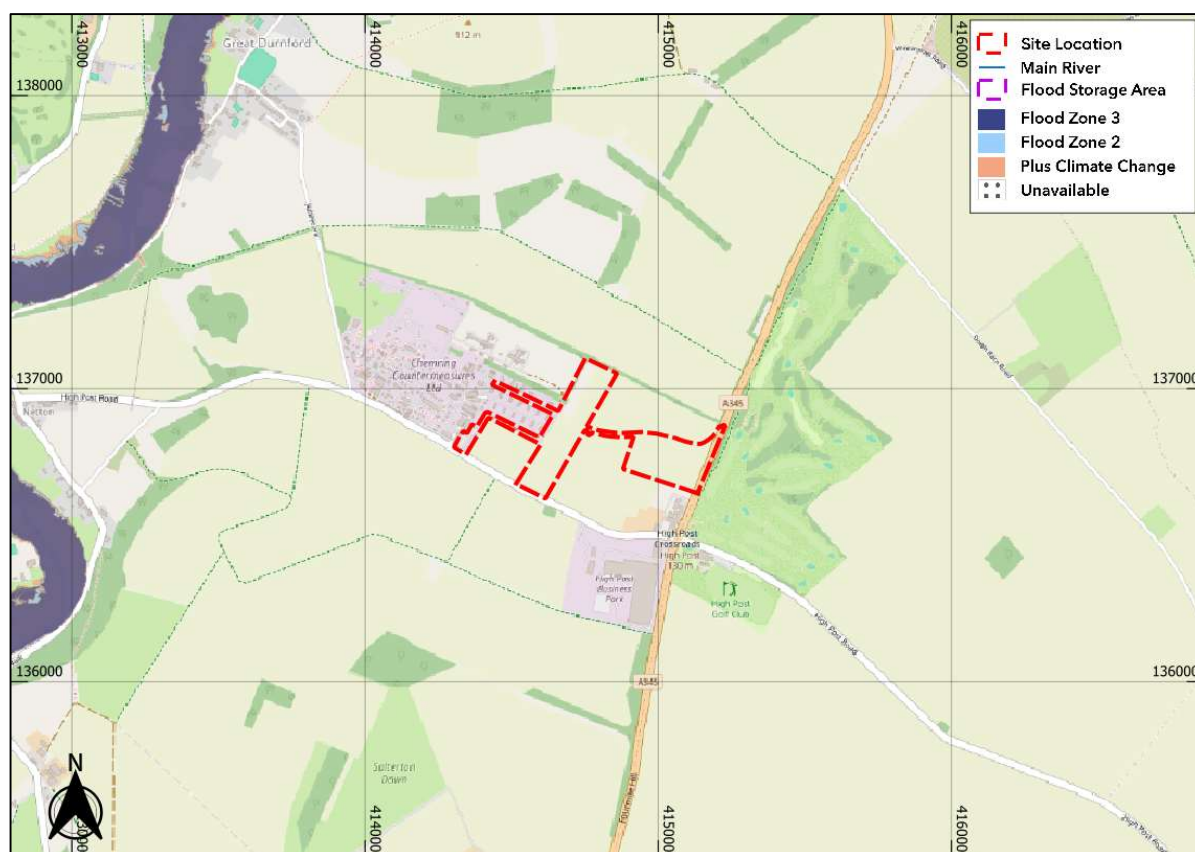
- 4.1. Flooding from watercourses arises when flows exceed the capacity of the channel, or where a restrictive structure is encountered, resulting in water overtopping the banks into the floodplain.

Main Rivers and Ordinary Watercourses

- 4.2. The nearest Main River to the site is the River Avon approximately 1.7km to the west of the site. According to OS mapping there are no ordinary watercourses in the vicinity of the site (within a 1km radius).

Environment Agency Flood Map for Planning

- 4.3. According to the EA's Flood Map for Planning the site is located within Flood Zone 1 (Figure 3). Flood Zone 1 is defined as land having less than 1 in 1,000 annual probability of river or sea flooding (all land outside Zones 2 and 3). The site is also located outside of the EA's Flood Zones plus climate change mapping.



Historical Fluvial Flooding

- 4.4. According to the EA Historic Flood Outline dataset, the site has not experienced flooding previously (Figure 4).

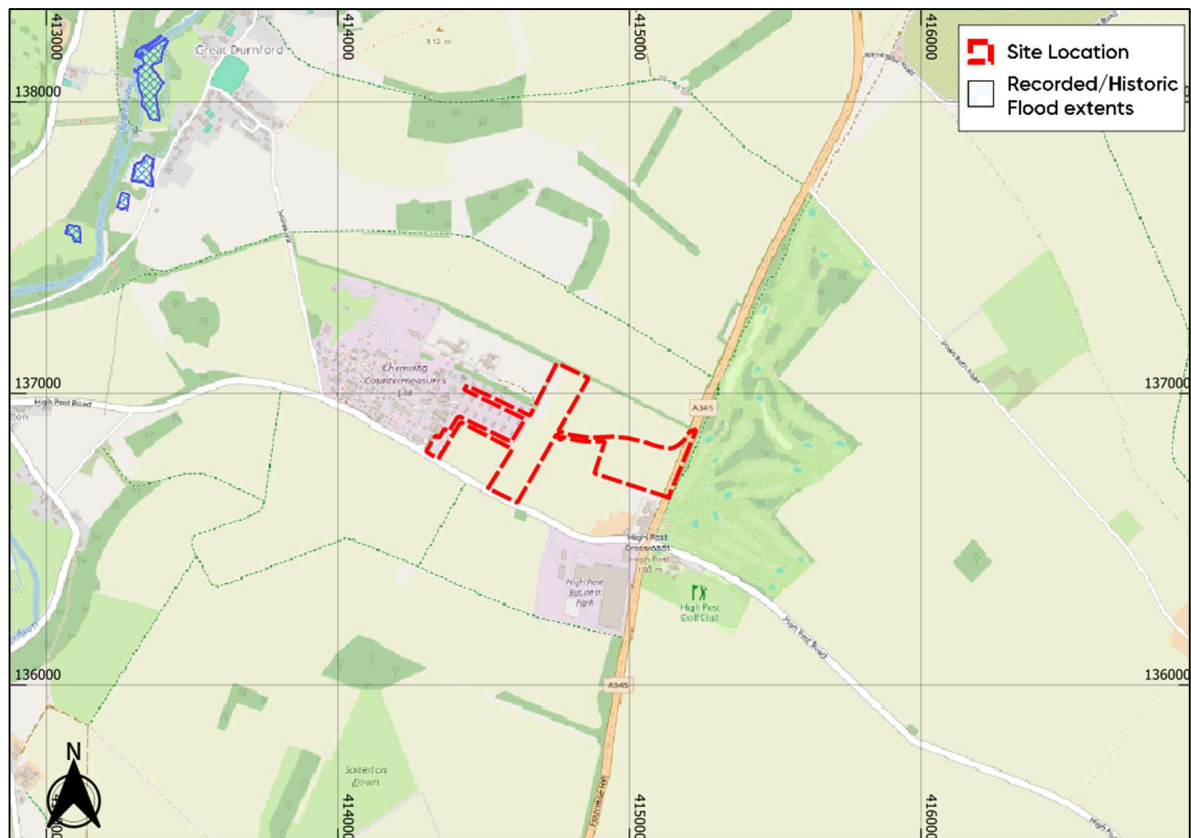


Figure 4: EA Historic Flood Mapping (Base map and data from OpenStreetMap and OpenStreetMap Foundation (CC-BY-SA). © <https://www.openstreetmap.org> and contributors. Contains public sector information licensed under the Open Government Licence v3.0)

Fluvial Flood Risk Summary

- 4.5. Based on the EA data above, the site is considered to be low risk from fluvial flooding.

Tidal

- 4.6. Tidal flooding occurs when a high tide and high winds combine to elevate sea levels. An area behind coastal flood defences can still flood if waves overtop the defences or break through them. Tidal flooding can also occur a long way from the coast by raising river levels. Water may overtop the river bank or river defences when tide levels are high.
- 4.7. The site is a significant distance from any tidal source and above the anticipated extreme tidal levels, even when considering the impacts of climate change.
- 4.8. Thus, the risk of flooding from tidal sources is considered to be low.

Canals

- 4.9. The Canal and River Trust (CRT) generally maintains canal levels using reservoirs, feeders, and boreholes and manages water levels by transferring it within the canal system.
- 4.10. According to the UK CRT Network mapping⁹, there are no CRT maintained canals within 1.0km of the site.
- 4.11. The risk of flooding to this site from canals is considered to be low.

Pluvial

- 4.12. Pluvial flooding can occur during prolonged or intense storm events when the infiltration potential of soils, or the capacity of drainage infrastructure is overwhelmed leading to the accumulation of surface water and the generation of overland flow routes.
- 4.13. The EA Flood Map for Planning Service in relation to Surface Water was published in May 2026. The Surface Water Spatial Planning Extents and Depths have been used within this FRA to assess the surface water flood risk to the site and proposed development.
- 4.14. These datasets show the extent of land at risk of flooding from surface water to a defined annual exceedance probability (AEP) or chance of flooding each year, taking into account the possible effects of climate change.
- 4.15. These datasets include the following scenarios:
- 0.1% AEP (1 in 1000);
 - 1% AEP (1 in 100) and
 - 3.3% AEP (1 in 30)
- 4.16. Alongside the following depth bands:
- Up to 150mm;
 - 150 – 300mm;
 - 300 – 600mm;
 - 600 – 900mm;

⁹ <https://canalrivertrust.org.uk/canals-and-rivers>

- 4.17. Alongside the present day scenarios, climate change scenarios have been produced to indicate the predicted impacts of climate change on future flood risk. The climate change allowances are based on the latest UK Climate Projections (UKCP18) from the Met Office, using the Representative Concentration Pathway (RCP) 8.5. The 'Upper End' for the 2070s epoch (2061-2125) has been used for the climate change scenarios. The long-term epochs are considered appropriate to support long-term planning and development decisions.
- 4.18. Review of present day and climate change extents are similar; thus the 'worst-case' scenario climate change extents and modelling have been assessed below.

Examination of the EA's Surface Water Spatial Planning dataset shows that the site is outside the modelled 100 and 1,000 year Climate Change extents (



Site Location

Surface Water Spatial Planning 1in100 1in1000 Ccp1

1 in 100

1 in 1000

North Arrow

Map labels include: Great Ouse, High Post Road, High Post Golf Club, High Post Crossroads, High Post 100m, A145, A146, A147, A148, A149, A150, A151, A152, A153, A154, A155, A156, A157, A158, A159, A160, A161, A162, A163, A164, A165, A166, A167, A168, A169, A170, A171, A172, A173, A174, A175, A176, A177, A178, A179, A180, A181, A182, A183, A184, A185, A186, A187, A188, A189, A190, A191, A192, A193, A194, A195, A196, A197, A198, A199, A200.

Pluvial Flood Risk Summary

Reservoirs

4.23. According to the EA's Flood Risk from Reservoirs mapping the site is outside modelled flood extents in the event of reservoir flooding (Figure 6).

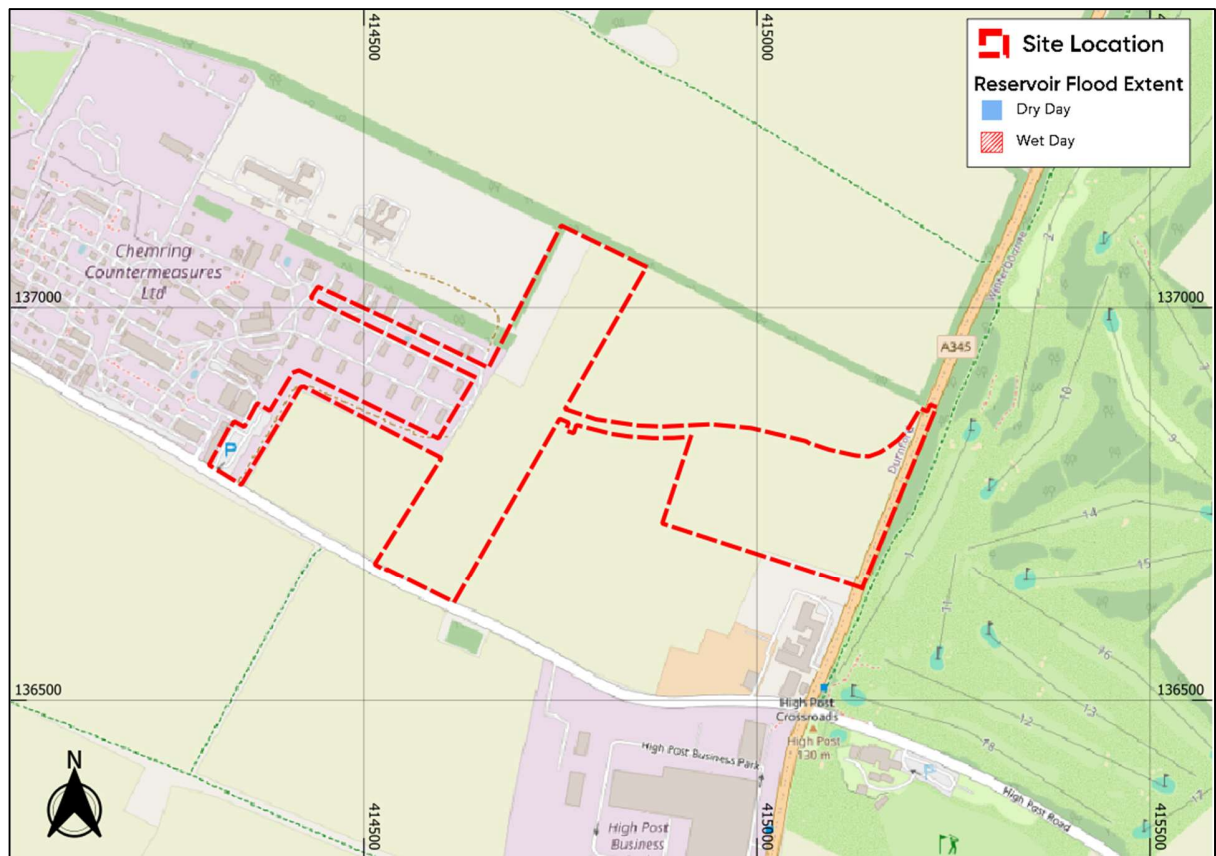


Figure 6: EA Reservoir Flood Risk Mapping (Base map and data from OpenStreetMap and OpenStreetMap Foundation (CC-BY-SA). ©<https://www.openstreetmap.org> and contributors. Contains public sector information licensed under the Open Government Licence v3.0)

4.24. Thus, the site is considered to be low risk from reservoir flooding.

Groundwater

- 4.25. Groundwater flooding occurs in areas where underlying geology is permeable and water can rise within the strata sufficiently to breach the surface.
- 4.26. The British Geological Survey's (BGS) mapping shows superficial deposits of Head comprising clay, silt, sand and gravel underlying the site. The bedrock underlying the site is Seaford Chalk Formation.
- 4.27. A historical borehole record (BGS Reference: SU13NE6) located approximately 70m to the south of the site recorded groundwater at 70m AOD. The minimum ground elevation on site is approximately 118.03mAOD. As such, the site is located approximately 48.03m above the

groundwater encountered in the borehole. Low groundwater levels are also recorded in BGS Borehole SU13NW45 with a rest level of groundwater: 63m AOD.

- 4.28. The SFRA presents areas at risk of groundwater emergence as reproduced in Figure 7 below. According to the SFRA the site is located in a zone deemed as having a negligible risk from groundwater flooding due to the nature of the local geological deposits.

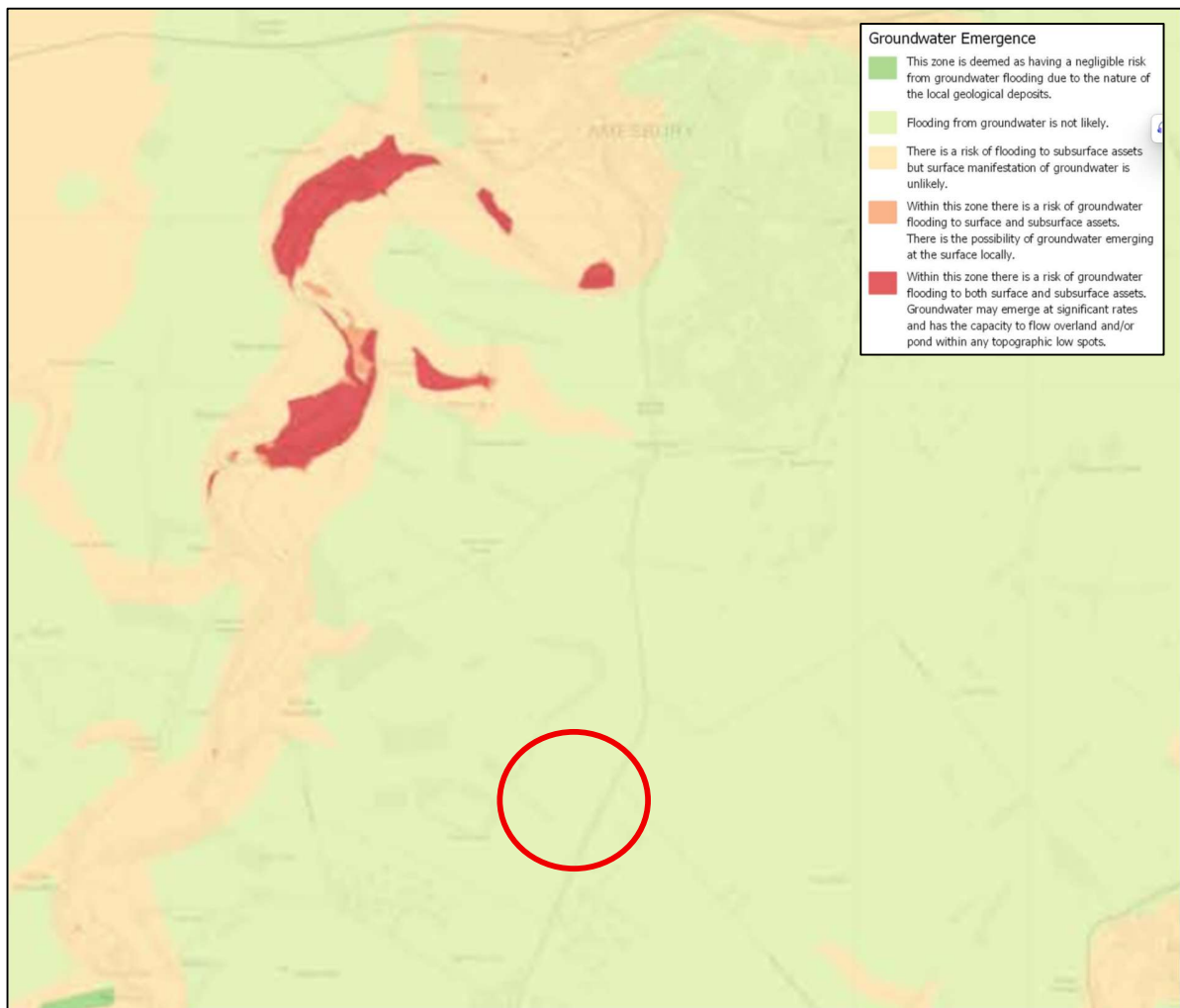


Figure 7: Groundwater Emergence Mapping (Site in Red) (Source: Wiltshire Level 1 SFRA)

- 4.29. Based on the information in this report the site is considered to be at a low risk of groundwater flooding.

Sewers

- 4.30. Foul or surface water sewers can be a cause of flooding if the drainage network becomes overwhelmed, either by blockage or due to local development beyond the designed capabilities of the drainage system.
- 4.31. The SFRA provides mapping of historical sewer flood incident records kept by the local authority (Figure 8). Based on the SFRA mapping there are no records of historical sewer flooding at the site location.

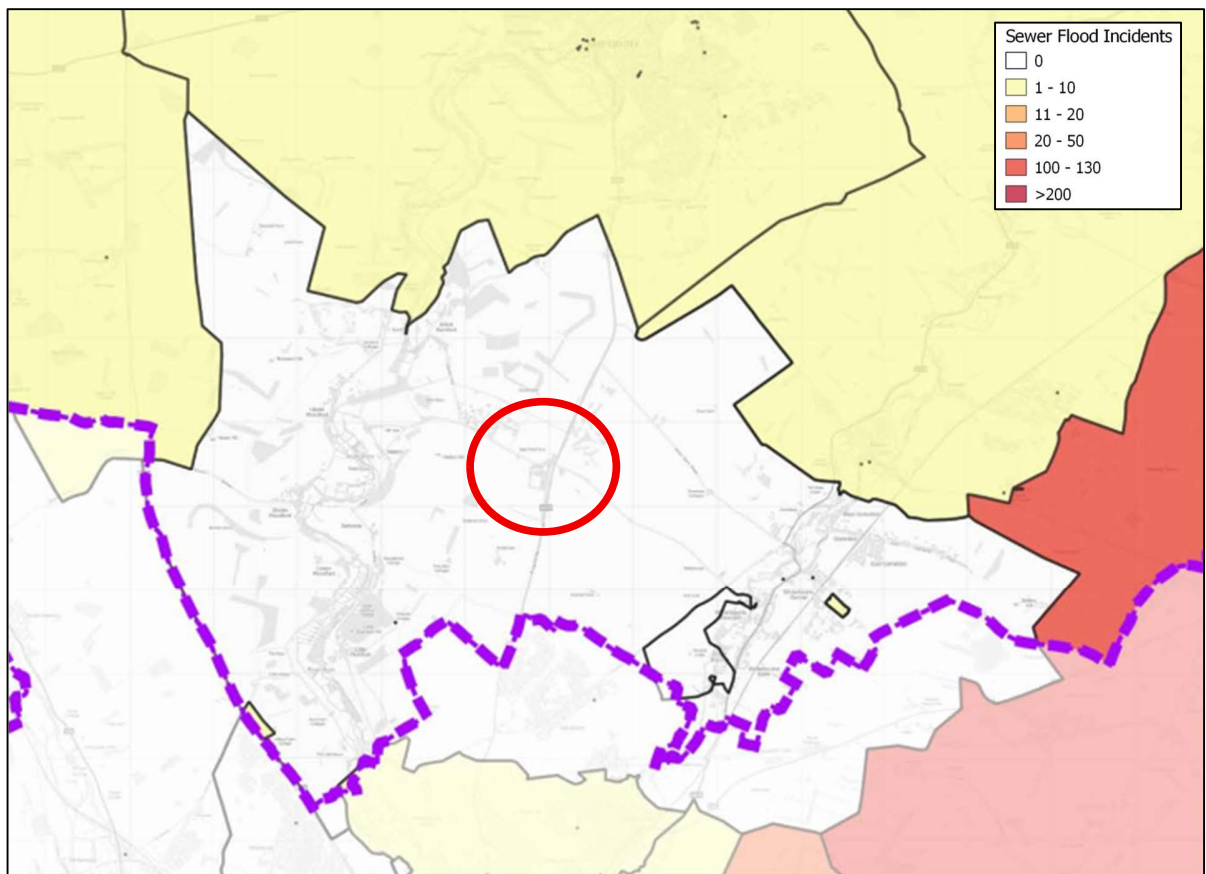


Figure 8: Sewer Flood Incidents (Source: SFRA) (Site in Red)

- 4.32. The development is therefore considered to be at low risk of flooding from sewers.

5. Flood Risk Mitigation

Fluvial, Pluvial, Canals, Reservoirs, Groundwater and Sewers

- 5.1. Flood risk from all analysed sources is deemed to be low, therefore mitigation is not required.

Increase to Flood Risk Elsewhere

- 5.2. As the site is considered to be at a low risk of flooding from all analysed sources no floodwater will be displaced as a result of the development.
- 5.3. Surface water will be managed on site for all storms up to and including the 1 in 100 year plus climate change event. See Section 6 for the SWDS.

Flood Warnings

- 5.4. The site is not in an area where the EA provide specific flood alerts and warnings. It is recommended that managers of the site should monitor Met Office Weather Warnings to be prepared for extreme weather events. In the event of an extreme event and red Met Office Weather Warning, the site should not be entered until the red Met Office Weather Warning is lifted. In addition, it is recommended that after a red Met Office Weather Warning is issued, checks by site managers / employees should be conducted to assess for any damages.
- 5.5. The Met Office issues weather warnings up to 5 days in advance, through the national Severe Weather Warning Service, when severe weather has the potential to bring impacts to the UK. It is also possible to stay up to date with weather warnings through the Met Office app (available on both Apple or Android devices), social media e.g. X and Facebook or email alerts.

6. Surface Water Drainage Strategy

Surface Water Drainage Hierarchy

- 6.1. In accordance with the SuDS management train approach, the use of various SuDS measures to reduce and control surface water flows have been considered in detail for the development in Table 3 overleaf.
- 6.2. The management of surface water has been considered in respect to the SuDS hierarchy below, as detailed in the Statutory Guidance National standards for sustainable drainage systems (SuDS) (2025).

Table 3: SuDS Drainage Hierarchy

SuDS Drainage Hierarchy				
			Suitability	Comment
	1.	Collection for non-potable use	✓	It is proposed that water butts should be installed beneath rainwater downpipes for non-potable uses on a plot level. Water butts should be included at the detailed design stage when the location of rainwater downpipes has been confirmed.
	2.	Infiltration	✓	Infiltration tests have been conducted on site in accordance with BRE365 Standards and the results are included in this report as Appendix C. The test results were varied, therefore, the results from Test Pit 2 (0.7164m/hr) and Test Pit 3 (0.2718m/hr) have been used for this strategy.
	3.	Discharge to surface waters	x	Infiltration is the preferred method of surface water discharge in the SuDS hierarchy.
	4.	Discharge to surface water sewer, highway drain or similar	x	
	5.	Discharge to combined sewer	x	

- 6.3. On review of the SuDS drainage hierarchy, and with reference to both national and local policy, it is proposed that the surface water runoff from the proposed development is discharged to ground via infiltration.
- 6.4. The potential for the use of various SuDS has been considered and optimised in the design, as per Table 4 overleaf.

Table 4: SuDS Components Suitability Summary Table

SuDS Component	Description	Constraints and Opportunities	Suitable?
1. Infiltrating SuDS	Infiltration can contribute to reducing runoff rates and volumes while supporting baseflow and groundwater recharge processes. The suitability and infiltration rate depends on the permeability of the surrounding soils.	Infiltration tests have been conducted on site in accordance with BRE365 Standards and the results are included in this report as Appendix C. The test results were varied, therefore, the results from Test Pit 2 (0.7164m/hr) and Test Pit 3 (0.2718m/hr) have been used for this strategy.	Yes
2. Filter Drains and Filter Strips	Filter drains are shallow trenches filled with stone gravel that create temporary subsurface storage for the attenuation, conveyance and filtration of surface water runoff. Filter strips are uniformly graded and gently sloping strips of grass or dense vegetation, designed to treat runoff from adjacent impermeable areas by promoting sedimentation, filtration and infiltration.	Filter drains have been used throughout this strategy.	Yes
3. Permeable Pavement	Pervious surfaces can be used in combination with aggregate sub-base and/or geocellular/modular storage to attenuate and/or infiltrate runoff from surrounding surfaces and roofs. Liners can be used where ground conditions are not suitable for infiltration.	All new hardstanding is to be constructed using permeable paving or similar permeable material. Permeable paving should be Type A.	Yes
4. Green & Blue Roofs	Green Roofs provide areas of visual benefit, ecological value, enhanced building performance and the reduction of surface water	There are no flat roofs included as part of the proposed development.	No

SuDS Component	Description	Constraints and Opportunities	Suitable?
	runoff. They are generally more costly to install and maintain than conventional roofs but can provide many long-term benefits and reduce the on-site storage volumes.		
5. Rainwater Harvesting (RWH)	Rainwater Harvesting is the collection of rainwater runoff for use. It can be collected from roofs or other impermeable areas, stored, treated (where required) and then used as a supply of water for domestic, commercial and industrial properties.	Rainwater harvesting could be utilised in water butts where feasible.	Yes
6. Swales	Swales are designed to convey, treat and attenuate surface water runoff and provide aesthetic and biodiversity benefits. They can replace conventional pipework as a means of conveying runoff; however, space constraints of some sites can make it difficult incorporating them into the design.	Filter drains are preferred over swales in this strategy. Open-water SuDS features cannot be used on this site. This is to prevent attracting birds, as the development is located approximately 2km from Boscombe Airfield.	No
7. Rills and Channels	Rills and Channels keep runoff on the surface and convey runoff along the surface to downstream SuDS components. They can be incorporated into the design to provide a visually appealing method of conveyance. They also provide effectiveness in pre-treatment removal of silts.	Not required for this strategy.	No

SuDS Component	Description	Constraints and Opportunities	Suitable?
8. Bioretention Systems	Bioretention systems can reduce runoff rates and volumes and treat pollution through the use of engineer soils and vegetation. They are particularly effective in delivering interception but can also be an attractive landscape feature whilst providing habitat and biodiversity.	Raised planters could be installed on rainwater downpipes to provide small attenuation and treatment benefits. Also bioretention areas have been proposed across the site in parking areas.	Yes
9. Retention Ponds and Wetlands	Ponds and Wetlands are features with a permanent pool of water that provide both attenuation and treatment of surface water runoff. They enhance treatment processes and have great amenity and biodiversity benefits. Often a flow control system at the outfall controls the rates of discharge for a range of water levels during storm events.	Open-water SuDS features cannot be used on this site. This is to prevent attracting birds, as the development is located approximately 2km from Boscombe Airfield.	No
10. Detention Basins	Detention Basins are landscaped depressions that are usually dry, except during and immediately following storm events, and can be used as a recreational or other amenity facility. They are generally appropriate to manage high volumes of surface water from larger sites, such as a neighbourhood.	Open-water SuDS features cannot be used on this site. This is to prevent attracting birds, as the development is located approximately 2km from Boscombe Airfield.	No
11. Geocellular Systems	Attenuation storage tanks are used to create a below-ground void space for the temporary storage of surface	Geocellular soakaways and tanks are proposed throughout this development.	Yes

SuDS Component	Description	Constraints and Opportunities	Suitable?
	water before infiltration, controlled release or use. The inherent flexibility in size and shape means they can be tailored to suit the specific characteristics and requirements of any site.		
12. Proprietary Treatment Systems	Proprietary treatment systems are manufactured products that remove specific pollutants from surface water runoff. They are especially useful where site constraints preclude the use of other methods and can be useful in reducing the maintenance requirements of downstream SuDS.	Site is generally considered a low level of pollutant risk. Additionally, permeable paving and filter drains should provide sufficient mitigation.	No

- 6.5. Following a review of the available SuDS options, and taking into account the site's constraints and opportunities, it is proposed that surface water runoff from the development will be managed via geocellular soakaways (unlined), geocellular storage tanks (lined), Type A (unlined) permeable paving, and filter drains, ultimately infiltrating into the ground.
- 6.6. Standard details including cross sections of each SuDS feature are included in this report as Appendix E.
- 6.7. As infiltration tests have only been conducted in a section of the site BRE365 Infiltration Tests are required at the detailed design stage at each location and depth of infiltrating SuDS features.
- 6.8. The proposed Surface Water Drainage Layout is included as Appendix B.

Infiltration Potential

- 6.9. The British Geological Survey's (BGS) mapping shows superficial deposits of Head comprising clay, silt, sand and gravel underlying the site. The bedrock underlying the site is Seaford Chalk Formation.

- 6.10. Infiltration tests have been conducted on site in accordance with BRE365 Standards and the results are included in this report as Appendix C. In summary, 4no. tests were conducted, however, only tests 2 and 3 were successful. Therefore, the results from Test Pit 2 (0.7164m/hr) and Test Pit 3 (0.2718m/hr) have been used for this strategy. The location of Test Pit 2 and 3 have been included in the Infiltration Test results (see 'Enclosure 1 – Exploratory Hole Location Plan' included in Appendix C of this report).
- 6.11. Where feasible, surface water should be conveyed as close to Test Pit 2 and 3 as possible. In areas of the site where this is not feasible a conservative infiltration rate of 0.0036m/hr has been applied. However, further infiltration tests should be conducted throughout the site prior to construction to confirm infiltration is feasible. This strategy will be updated in light of that information.

Nearby Watercourses

- 6.12. Infiltration is the preferred method of surface water discharge. Additionally, there are no Main Rivers or ordinary watercourses in the vicinity of the site.

Public Sewers

- 6.13. Infiltration is the preferred method of surface water discharge. Additionally, due to the rural nature of the site location it is assumed that there are no public sewers in the vicinity of the site.

InfoDrainage Modelling and Results

InfoDrainage Modelling

- 6.14. InfoDrainage Software (v2025.5) was used to calculate the indicative attenuation volumes from the development proposals area.
- 6.15. The total proposed impermeable area being managed was calculated to be 3.309 ha of impermeable areas.
- 6.16. The model comprises:
- 23no. contributing catchment areas to represent the proposed hardstanding areas.
 - 9 no. permeable paving units.

- Main Carpark Permeable Paving (1) - Modelled with 30% porosity, no infiltration, 1,680m² plan area, 0.31m deep clean crushed stone subbase, discharging into bioretention area 1 via a 75mm orifice. Total attenuation provided: 155.571m³.
- Main Carpark Permeable Paving (2) - Modelled with 30% porosity, no infiltration, 1,680m² plan area, 0.31m deep clean crushed stone subbase, discharging into bioretention area 2 via a 75mm orifice. Total attenuation provided: 155.571m³.
- Main Carpark Permeable Paving (3) - Modelled with 30% porosity, no infiltration, 1,680m² plan area, 0.31m deep clean crushed stone subbase, discharging into bioretention area 3 via a 75mm orifice. Total attenuation provided: 155.571m³.
- Carpark 2 Permeable Paving - Modelled with 30% porosity, 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests), 2,200m² plan area, 0.3m deep clean crushed stone subbase, discharging via infiltration only. Total attenuation provided: 203.085m³.
- Solar Field A Permeable Paving - Modelled with 30% porosity, 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests), 340m² plan area, 0.25m deep clean crushed stone subbase, discharging via infiltration only. Total attenuation provided: 25.777m³.
- Solar Field B Permeable Paving - Modelled with 30% porosity, 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests), 340m² plan area, 0.25m deep clean crushed stone subbase, discharging via infiltration only. Total attenuation provided: 25.777m³.
- Small Carpark Permeable Paving - Modelled with 30% porosity, 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests), 220m² plan area, 0.25m deep clean crushed stone subbase, discharging via infiltration only. Total attenuation provided: 16.371m³.
- Staff Carpark Permeable Paving - Modelled with 30% porosity, 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests), 3,810m² plan area, 0.30m deep clean crushed stone subbase, discharging via infiltration only. Total attenuation provided: 345.960m³.
- Staff Carpark Access Road Permeable Paving - Modelled with 30% porosity, 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via

further BRE365 infiltration tests), 3,550m² plan area, 0.3m deep clean crushed stone subbase, discharging via infiltration only. Total attenuation provided: 324.0m³.

- 4 no. Bioretention Areas;
 - Bioretention (1) – 30m² plan area, 0.3m ponding depth, 0.5m soil layer, 0.2m storage layer. Modelled with a 0.7164m/hr infiltration rate and a 2.0 Safety Factor. Infiltration rate based on Test Pit 2.
 - Bioretention (2) – 30m² plan area, 0.3m ponding depth, 0.5m soil layer, 0.2m storage layer. Modelled with a 0.7164m/hr infiltration rate and a 2.0 Safety Factor. Infiltration rate based on Test Pit 2.
 - Bioretention (3) – 17.5m² plan area, 0.3m ponding depth, 0.5m soil layer, 0.2m storage layer. Modelled with a 0.7164m/hr infiltration rate and a 2.0 Safety Factor. Infiltration rate based on Test Pit 2.
 - Bioretention (4) - 155m² plan area, 0.3m ponding depth, 0.5m soil layer, 0.2m storage layer. Modelled with an infiltration rate of 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests).
- 2 no. geocellular soakaway units.
 - Main Soakaway modelled with 95% porosity, an infiltration rate of 0.2718m/hr has been used with a Safety Factor of 2.0. Infiltration rate based on Test Pit 3. The total plan area of the tank is 504m² and the depth is 0.8m. The attenuation provided is 383.44m³.
 - Roundabout Soakaway modelled with 95% porosity, modelled with a 0.7164m/hr infiltration rate and a 2.0 Safety Factor. Infiltration rate based on Test Pit 2. The total plan area of the tank is 120m² and the depth is 0.8m. The attenuation provided is 91.964m³.
- 3 no. Filter Drains.
 - Access Road Filter Drain. 300m long, 1m deep, 1.5m width, 30% porosity, a 300mm under drain for conveyance, discharging into the drainage network, ultimately discharging into the Main Soakaway. Modelled with an infiltration rate of 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests).

- Roundabout Filter Drain. 50m long, 0.7m deep, 1.5m width, 30% porosity, a 300mm under drain for conveyance, discharging into the drainage network, ultimately discharging into the Roundabout Soakaway. Modelled with an infiltration rate of 0.2718m/hr has been used with a Safety Factor of 2.0. Infiltration rate based on Test Pit 3.
- Solar Field A Filter Drain. 250m long, 0.5m deep, 1.5m width, 30% porosity, a 300mm under drain for conveyance. Draining via infiltration only. Modelled with an infiltration rate of 0.0036m/hr infiltration rate and a 2.0 Safety Factor (actual infiltration rate TBC via further BRE365 infiltration tests).
- 1 no. geocellular storage tank. No infiltration. Plan area of 672m², depth of 0.8m, and a porosity of 95%. The attenuation provided is 511.176m³. Discharging into the underground drainage network ultimately discharging to the Main Soakaway.

6.17. Table 5 summarises the simulation criteria for the InfoDrainage model.

Table 5: Simulation Parameters

Catchment Area Simulation Parameters	
Rainfall Data	FEH22
Total Area	3.309Ha
Return Periods	2, 30, 30+40% for climate change, 100, 100 +45% for Climate Change. Summer and Winter
Storm Durations	15, 30, 60, 120, 240, 360, 480, 960, 1440, minute
Volumetric Runoff Coefficient	1.0 (summer and winter storms)
Percentage Impervious	100%
Time of Concentration	5 minutes

6.18. The Environment Agency Peak Rainfall Climate Change Allowance guidance (updated in May 2022) was reviewed and subsequently the DEFRA Peak Rainfall Allowances Map was assessed to determine appropriate climate change allowances to inform the surface water drainage strategy. In line with this guidance, the upper end allowances for the Avon Hampshire Management Catchment have been used for both the 1% and 3.3% annual exceedance probability events. An allowance of 40% has been used for the 3.3% AEP event and an allowance of 45% has been used for the 1% AEP event.

InfoDrainage Results

- 6.19. The full calculation outputs can be found in Appendix D of this report although the 1 in 100 year+45% climate change results SuDS features have been summarised below:
- The maximum depth within the 2no. 0.8m deep soakaways for a 1 in 100-year plus 45% climate change critical storm is 0.763m. The maximum half drain time is 64 minutes.
 - Across all 9 permeable paving units modelled there is no flooding observed. As such, all 9 permeable paving units are providing sufficient attenuation in their sub base storage layer listed previously. The maximum half drain time across all units is 1,337 minutes.
 - Across all 3 filter drains modelled there is no flooding observed. As such, all 3 filter drains are providing sufficient attenuation in the depths listed previously. The maximum half drain time across all filter drains is 1,375 minutes.
 - The maximum depth within the 4no. 1.0m deep bioretention areas for a 1 in 100-year plus 45% climate change critical storm is 0.813m. The maximum half drain time is 1,266 minutes.
 - The maximum depth within the 0.8m deep geocellular tank for a 1 in 100-year plus 45% climate change critical storm is 0.771m. The maximum half drain time is 550 minutes.
 - No flooding is observed in the critical storm event based on the InfoDrainage model.
- 6.20. As such, these results indicate that the runoff from the proposed development could be accommodated within a drainage system of the approximate size modelled.

Soil Erosion

- 6.21. Within each solar field in the site, filter drains are recommended periodically, to aid the flow of surface water and mitigate soil erosion. Surface water will be conveyed via the filter drains rather than through the soil, mitigating against soil erosion and promoting further infiltration where possible. These filter drains and potential infiltration are not accounted for in the attenuation calculations, adopting a conservative approach.
- 6.22. Within the filter drains on site, small check dams can be used in sloped areas. This is further reducing the rate of surface water runoff within the site.

Maintenance

- 6.23. Table 6, 7, 8, and 9 presents the details regarding the maintenance requirements for the proposed SuDS included as part of the development taken from the CIRIA C753 'The SuDS

Manual'. Each manufacturer will have bespoke requirements, however, the below should be used as a guide.

- 6.24. The proposed surface water drainage features serving the site, including the geocellular soakaways, permeable paving, filter drains and bioretention areas, will remain in private ownership. Overall responsibility for the long-term operation and maintenance of these features will rest with the site owner, who will appoint a suitably qualified specialist contractor to undertake routine inspection, maintenance and repair in accordance with the maintenance schedule.

Table 6: Maintenance Requirements for Geocellular Systems (soakaways and tanks)

Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Inspect and identify any area that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary.	Annually
	Remove sediment from pre-treatment structures and/ or internal forebays	Annually, or as required
Remedial Actions	Repair/ rehabilitate inlets, outlet, overflows and vents	As required
Monitoring	Inspect/ check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of tank for sediment build-up and remove if necessary	Every 5 years or as required

Table 7: Maintenance Requirements for Permeable Pavements

Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturers recommendations – pay particular attention to areas where water runs onto pervious surface from adjacent impermeable areas as this area is most likely to collect the most sediment
Occasional Maintenance	Stabilise and mow contributing and adjacent areas	As required
	Removal of weeds or management using glyphosate applied directly into the weeds by an applicator rather than spraying	As required - once per year on less frequently used pavements
Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50mm of the level of the paving	As required
	Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required
	Rehabilitation of surface and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)
Monitoring	Initial inspection	Monthly for three months after installation
	Inspect for evidence of poor operation and/or weed growth - if required, take remedial action	Three-monthly, 48hr after large storms in first six months
	Inspect silt accumulation rates and establish appropriate brushing frequencies	Annually
	Monitor inspection chambers	Annually

Table 8: Specific SuDS Maintenance Requirements for Filter Drains (Source: CIRIA SuDS Manual)

Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Litter and debris removal	Monthly
	Cut grass for spillways and access routes	Monthly
	Grass cutting - meadow grass	Half Yearly
	Manage bankside edge vegetation and remove nuisance plants	Monthly at start and then as required
	Inspect inlets, outlets and overflows for blockages, and clear if required.	Monthly
	Inspect banksides, structures, pipework etc for evidence of physical damage	Monthly
	Inspect inlets and facility surface for silt accumulation. Establish appropriate silt removal frequencies.	Monthly for the first year and then annually or as required
	Check any penstocks or other mechanical devices	Annually
	Tidy all dead growth before start of growing season	Annually
	Remove sediment from inlets, outlet and forebay	Annually (or as required)
	Manage wetland plants in outlet pool - where provided	Annually
Occasional Maintenance	Reseed areas of poor vegetation growth	As required
	Prune and trim any trees and remove cuttings	Every 2 years, or as required
	Remove sediment from inlets, outlets, forebay and main basin when required	Every 5 years, or as required
Remedial Actions	Repair of erosion or other damage by re-seeding or re-turfing	As required
	Re-alignment/repair of rip-rap	As required
	Repair/rehabilitation of inlets, outlets and overflows	As required

Table 9: Maintenance Requirements for Bioretention Systems

Maintenance Schedule	Required Action	Typical Frequency
Regular Inspections	Inspect infiltration surfaces for silting and ponding, record de-watering time of the facility and assess standing water levels in underdrain (if appropriate) to determine if maintenance is necessary	Quarterly
	Check operation of underdrains by inspection of flows after rain	Annually
	Assess plants for disease infection, poor growth, invasive species etc. and replace as necessary	Quarterly
	Inspect inlets and outlets for blockage	Quarterly
Regular Maintenance	Remove litter and surface debris and weeds	Quarterly (or as required)
	Replace any plants, to maintain planting density	As required
	Remove sediment, litter and debris build-up from around inlets or from forebays	Quarterly to biannually
Occasional Maintenance	Infill any holes or scour in the filter medium, improve erosion protection if required	As required
	Repair minor accumulations of silt by raking away surface mulch, scarifying surface of medium and replacing mulch	As required
Remedial Actions	Remove and replace filter medium and vegetation above	As required but likely to be > 20 years

Designing for Exceedance

- 6.25. Exceedance events are those greater than the design rainfall event, i.e. greater than the 100-year rainfall event plus 45% increase for climate change.
- 6.26. Periods of exceedance occur when the rate of surface water runoff exceeds the drainage system capacity. Conveyance beneath ground cannot, generally, be economically or sustainably constructed to the scale required for the most extreme rainfall events. This may result, on

occasion, in the surface water runoff exceeding the capacity of the drainage network, with excess water (exceedance flow) being conveyed above ground.

- 6.27. For situations where extreme rainfall intensity exceeds inlet capacities, or for extreme storm events exceeding the design flood event considered for drainage design, the surface water exceedance will be managed on site. Approximately 150mm high kerbs will also contain exceedance flows within the access road. Hardstanding areas should also be laid to fall away from internal thresholds.

Water Quality

- 6.28. In order to protect the downstream receiving water body, a key element of SuDS is that they have the potential to improve the quality of surface water discharged from a site. In order to assess this, the "Pollution hazard indices for different land use classifications", provided in table 26.2 of the CIRIA SuDS Manual (C753) has been reviewed. The indices use four different indicators for assessing pollution potential based on the hazard level, total suspended solids (TSS), Metals, and Hydrocarbons.
- 6.29. The Pollution Hazard Indices are summarised in Table 10 below (with reference to table 26.3 in the CIRIA SuDS manual). The pollution hazard indices for "non-residential low trafficked roads" have been used.

Table 10: Pollution Hazard Indices

Land Use	Pollution Hazard Level	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Non-residential low trafficked roads with infrequent change (<300 traffic movements/day)	Low	0.5	0.4	0.4

- 6.30. Table 11 shows permeable paving and bioretention areas will offer sufficient treatment for the development. However, any runoff treated by the filter drains should also include silt traps at their outlets as filter drains alone will not meet the required total suspended solids treatment requirement.

Table 11: Pollution Mitigation Indices

SuDS Feature		Total Suspended Solids (TSS)	Metals	Hydrocarbons
Permeable Paving		0.7	0.7	0.5
Filter Drains		0.4	0.4	0.4
Bioretention		0.8	0.8	0.8

6.31. As such, the proposed drainage strategy should offer sufficient treatment of runoff from the development.

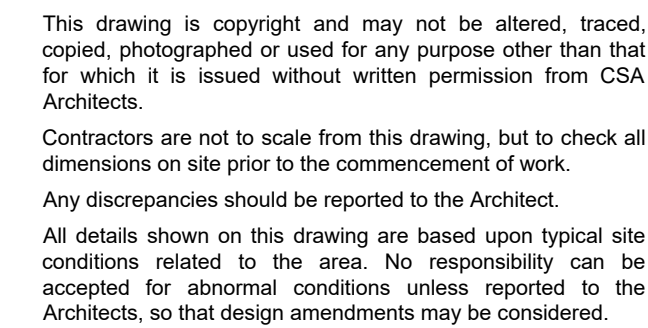
7. Conclusions

- 7.1. This FRA and SWDS has been undertaken with reference to the requirements of NPPF and Planning Practice Guidance with respect to the development at High Post Business Park, Salisbury, Wiltshire, SP4 6AT. It has been written to support a planning application and prepared with due consideration to the nature of the proposed development to provide the appropriate level of detail.
- 7.2. An assessment of the risk of flooding from all sources has been undertaken and is summarised in the table below:

Source of Flooding	Flood Risk Summary
Fluvial Pluvial Tidal Reservoirs Groundwater Sewers Canals	The site is considered to be at low risk from all analysed sources.

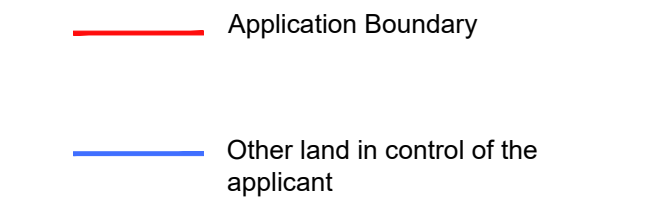
- 7.3. The FRA supports the planning application and demonstrates that there is an acceptable level of flood risk to the site if the mitigation strategies recommended are implemented in the scheme. The development does not increase flood risk off site or to the wider area.
- 7.4. Following a review of the available SuDS options, and taking into account the site’s constraints and opportunities, it is proposed that surface water runoff from the development will be managed via geocellular soakaways (unlined), geocellular storage tanks (lined), Type A (unlined) permeable paving, and filter drains, ultimately infiltrating into the ground.
- 7.5. This Flood Risk Assessment and Surface Water Drainage Strategy should be submitted as part of the planning application to satisfy the requirements under NPPF.

Appendix A - Development Proposals



ORIGINAL DRAWING SIZE : A1

Scale Reference



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HIGH POST, SALISBURY

Date FEB 26	Scale 1:2500	Drawn By SK	Checked
Job. No. 3553	Drawing No. 005	Revision G	

PLANNING

Appendix B - Surface Water Drainage Strategy

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DOCUMENTATION WHERE APPLICABLE.
IT IS ASSUMED THAT ALL WORKS ON THIS DRAWING WILL BE CARRIED OUT BY
A COMPETENT CONTRACTOR, WORKING WHERE APPROPRIATE TO AN
APPROVED METHOD STATEMENT.

DRAFT

LEGEND

- FILTER DRAIN
- SURFACE WATER DRAIN
- PAVING UNDER DRAIN
- SURFACE WATER MANHOLE
- GEOCELLULAR STORAGE TANK
- GEOCELLULAR SOAKAWAY
- PERMEABLE PAVING
- BIORETENTION AREAS
- CATCHMENT AREA
- _{RVP} RAINWATER DOWNPIPE
- ➔ EXCEEDANCE FLOW ROUTE

01 | PRO
SCALE

3.0	10/07/2026	THIRD ISSUE	JA
2.0	04/03/2026	SECOND ISSUE	JA
1.0	03/03/2026	FIRST ISSUE	JA
Rev	Date	Description	By

Client

SPITFIRE ENTERPRISES

Project

SURFACE WATER DRAINAGE LAYOUT

Title

HIGH POST BUSINESS PARK,
SALISBURY, WILTSHIRE, SP4 6AT.

Project No.		Drawing No.		Revision
AEG9910		DR01		3.0
Drawn	Checked	Approved	Date	Scale @ A1
JA	JC	OH	10/07/2026	SEE SCALE

Drawing Status
FOR PLANNING ONLY

aegaea
water, civils and environment

Appendix C – Infiltration Tests

Our Ref: 260201

12 February 2026

Nationwide Engineering Group Ltd,
Unit 19 The Bluestone Centre Sun Rise Way,
Amesbury,
Salisbury,
Wiltshire
SP4 7YR

BRE 365 Soakaway Assessment – High Post Business Park, Salisbury, Wiltshire, SP4 6AT

Scope and Objectives

Clear Environmental Limited (Clear) was commissioned by Nationwide Engineering Group Ltd (the 'Client') to undertake a BRE 365 Soakaway Assessment at High Post Business Park, Salisbury, Wiltshire, SP4 6AT (hereafter referred to as the 'site').

Purpose of Works

The purpose of this report is to provide information on the underlying ground conditions, infiltration rates and groundwater regime to assess the viability of infiltration and to inform future drainage design.

Scope of Works

- ≡ Progression of 5No. mechanically excavated trial pits to a maximum depth of 1.80m below ground level (bgl);
- ≡ Undertake 4No. Soakaway Tests in general accordance with BRE 365 guidance.
- ≡ Provision of a factual and interpretive report providing the calculated infiltration rates and records of ground conditions.

Limitations

This report presents Clear's observations, findings, and conclusions based on the information available and site conditions as understood at the time of reporting. These findings may be subject to change should material new information become available following the date of issue.

This report has been prepared specifically for the Client. It may not be relied upon by any other party without Clear's prior written agreement, which will typically require the execution of a reliance letter incorporating our standard terms and conditions. Where reliance has been granted to third parties, this will be noted in the report.

Site Constraints

Soakaway testing was undertaken in general accordance with BRE Digest 365. Trial pits were excavated and filled with water to assess the infiltration rates.

Results reflect conditions at the test locations only and may not represent the wider site. Variations in soil type, groundwater or compaction could affect soakaway performance and should be considered in design.

Due to the observed infiltration characteristics, TP04 and TP05 could not be refilled the additional two times recommended within the guidance.

Site Location

A site location plan is presented below.



Figure 1: Site Location Plan

Ground Conditions

Ground Conditions Encountered

The ground conditions encountered during the excavation of the trial pit are summarised below. An Exploratory Hole Location Plan is presented in Enclosure I.

Trial Pit Logs are presented in Enclosure II.

Strata Description	Top Depth (m bgl)	Bottom Depth (m bgl)	Thickness (m)
Topsoil	Ground Level	0.25 – 0.55	0.25 – 0.50
Brown sandy gravelly CLAY	0.50	1.10	0.60
Structureless CHALK	0.25 – 1.10	>1.80	>1.25 (Full depth not proven)

Topsoil

Topsoil was encountered comprising brown sandy slightly gravelly clay from ground level to 0.25 - 0.50m below ground level (bgl).

Residual Soils

A completely weathered residual soil was recorded beneath the Topsoil at TP01, comprising brown sandy gravelly clay with some sand and gravel of flint and chalk to 1.10m bgl.

Bedrock Deposits - Seaford Chalk Formation

The Seaford Chalk Formation was then recorded beneath the Topsoil and residual soil, extending beyond the maximum depth achieved of 1.80m bgl, beyond which the excavator was not able to progress excavations.

These deposits were encountered as a structureless white chalk, comprising gravel in a sandy silty matrix.

Groundwater

No groundwater was encountered within any of the exploratory hole locations.

Variations in groundwater conditions can occur in response to seasonal or climatic changes. However, when considering groundwater is recorded at circa 60m below ground level within BGS borehole SU13NE6, located approximately 100m south of the site, shallow groundwater is not considered to pose a constraint to infiltration drainage.

Solution Feature Risk

The exploratory works did not identify significant variance in rock head depth across the site. Therefore, there is no visual evidence of pronounced dissolution features, voiding or irregular rockhead profile within the exploratory locations progressed.

Two geotechnical bulk samples of the chalk have been submitted for laboratory testing to classify the chalk in accordance with CIRIA C574 Engineering in Chalk.

Concentrated ingress of water into chalk can initiate new dissolution features. On a precautionary basis at this stage, it is recommended that any proposed soakaways are sited a minimum of 10m from building foundations. This stand-off distance should be reviewed and confirmed once laboratory results are available.

Soakaway Testing Results

Soakaway testing was completed in general accordance with BRE 365.

The table below summarises the results, which are also contained as Enclosure III.

Location	Target Strata (Depth)	Infiltration Rate (ms ⁻¹)		Infiltration Rate for Design	Comments	
TP02	Structureless CHALK	Test 1	2.51 x 10 ⁻⁴	1.99 x 10 ⁻⁴	The deposits at TP02 and TP03 are considered suitable for infiltration drainage, however, the results should be passed to a drainage engineer to design a suitable scheme.	
		Test 2	2.33 x 10 ⁻⁴			
		Test 3	1.99 x 10 ⁻⁴			
TP03	Structureless CHALK	Test 1	1.20 x 10 ⁻⁴	7.55 x 10 ⁻⁵		
		Test 2	7.99 x 10 ⁻⁵			
		Test 3	7.55 x 10 ⁻⁵			
TP04	Structureless CHALK	Test 1	N/A	Fail	The water level did not fall between 75% and 25% of the effective depth over the 4 - 5 hour monitoring period at TP04 and TP05	
		Test 2	N/A			
		Test 3	N/A			
TP05	Structureless CHALK	Test 1	N/A	Fail		As such, an infiltration rate can not be calculated in accordance with BRE 365
		Test 2	N/A			
		Test 3	N/A			

Closing Remarks

The site is underlain by topsoil and (locally) residual sandy gravelly clay over shallow Seaford Chalk Formation. Chalk was encountered at shallow depth across all trial pit locations and extends beyond the maximum depth of excavation.

No groundwater was encountered during the works. Reference to nearby BGS borehole data indicates a regional groundwater table at significant depth (circa 60m bgl), and groundwater is therefore not considered to constrain infiltration drainage at the site.

Soakaway testing has demonstrated variability in infiltration characteristics across the site. In the northern portion (TP02 and TP03), calculated design infiltration rates of 1.99 x 10⁻⁴ m/s (TP02) and 7.55 x 10⁻⁵ m/s (TP03) were derived in accordance with BRE 365. These values confirm that infiltration is considered feasible in this area.

Testing within the central portion (TP04 and TP05) did not achieve sufficient drawdown within the monitoring period to allow calculation of infiltration rates in accordance with BRE 365. These locations are therefore not currently suitable for infiltration drainage without further investigation.

The observed variability may reflect localised differences in chalk weathering, fracture density and secondary constituent percentage (increased silt).

On the basis of the results obtained, a zoned approach to surface water management is recommended. Infiltration drainage is considered viable within the northern portion of the site (TP02 and TP03) where favourable design infiltration rates have been demonstrated.

Following completion of the detailed drainage design, should soakaway locations significantly differ from the location of TP02 and TP03, it would be prudent to undertake confirmatory BRE 365 testing at the precise proposed soakaway locations to verify infiltration performance under operational conditions.

Ben Lawry BSc(Hons) FGS

Director

For and on behalf of Clear Environmental Ltd

Mobile: 07769178244

Email: ben@clearenviro.co.uk

Enclosure I – Exploratory Hole Location Plan



Legend

- Tiral Pits 
- Site Boundary 

Figure Title	
Exploratory Hole Location Plan	
Site Location	
High Post Business Park, Salisbury, Wiltshire, SP4 6AT	
Job Number	Figure Number
260201	260201.01
Prepared By	Date
BJL	12/02/2026
Client	
Nationwide Engineering Group Ltd	

Enclosure II – Exploratory Hole Logs



Remarks 1) All recorded strengths and densities based on visual observations of ease of excavation. 2) No direct visual or olfactory evidence of mobile contamination. 3) Groundwater was not encountered. 4) Trial pit terminated at 1.8m within chalk. 5) On completion, the trial pit was backfilled with arisings and made safe at the surface.	Method, Plant, Stability, Dimensions 0.00 - 1.80m TP Stable $L = 1.50m$ $W = 0.40m$	Logger BL
	Checked By: BL Approved By: BL	



TP02
Sheet 1 of 1

Hole Type TP	Easting	Northing	Ground Level (m)	Scale 1:25
Project Name High Post Business Park, Salisbury		Project No. 260201	Start Date 2026-02-09	End Date 2026-02-09

Client	Consultant	Contractor
Nationwide Engineering Group Ltd	Clear Environmental Limited	

[illegible]

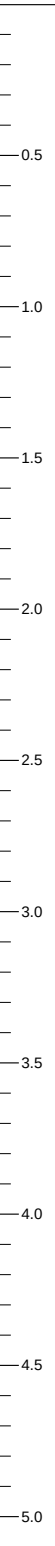
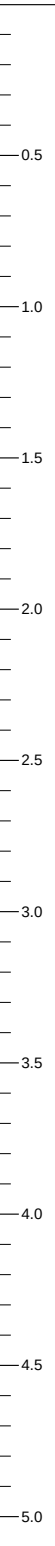
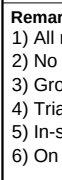
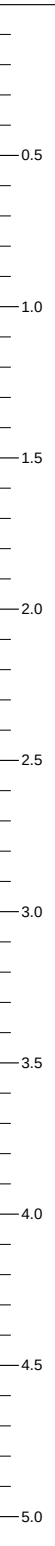
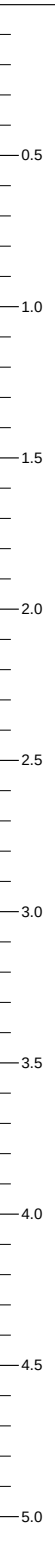
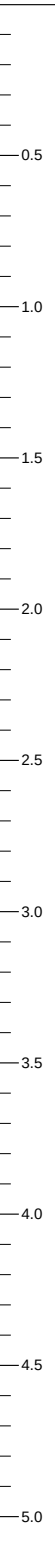
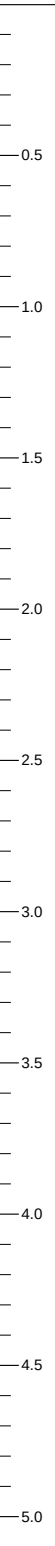
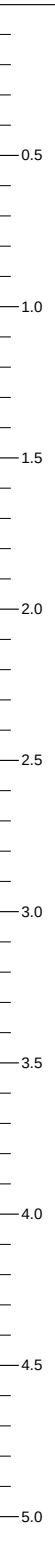
Remarks 1) All recorded strengths and densities based on visual observations of ease of excavation. 2) No direct visual or olfactory evidence of mobile contamination. 3) Groundwater was not encountered. 4) Trial pit terminated at 1.5m within chalk. 5) In-situ soakaway test undertaken in pit. 6) On completion, the trial pit was backfilled with arisings and made safe at the surface ...	Method, Plant, Stability, Dimensions 0.00 - 1.50m TP Stable $L = 1.40m$  $W = 0.35m$ Logger BL
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Checked By: BL Approved By: BL

Created using Pebble Geo

Hole Type TP	Easting	Northing	Ground Level (m)	Scale 1:25
Project Name High Post Business Park, Salisbury	Project No. 260201	Start Date 2026-02-09	End Date 2026-02-09	

Client Nationwide Engineering Group Ltd	Consultant Clear Environmental Limited	Contractor
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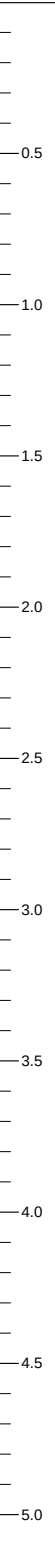
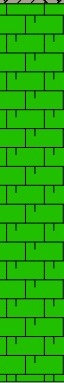
Inst/ Backfill	Water Levels	Samples and Tests			Level (m)	Depth (m) (thickness)	Strata		
		Depth (m)	Type/ Ref	Results			Legend	Description	
						(0.30)		Soft sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is angular to sub-rounded flint and chalk (TOPSOIL).	
						0.30		Structureless CHALK composed of medium dense slightly sandy very silty subangular and subrounded fine to coarse GRAVEL (SEAFORD CHALK FORMATION)	
						(1.20)			
						1.50		End of Trial Pit at 1.50m	
									
									
									
									
									

Remarks 1) All recorded strengths and densities based on visual observations of ease of excavation. 2) No direct visual or olfactory evidence of mobile contamination. 3) Groundwater was not encountered. 4) Trial pit terminated at 1.5m within chalk. 5) In-situ soakaway test undertaken in pit. 6) On completion, the trial pit was backfilled with arisings and made safe at the surface ...	Method, Plant, Stability, Dimensions 0.00 - 1.50m TP Stable L = 1.40m W = 0.38m	Logger BL

Checked By: BL Approved By: BL

Hole Type TP	Easting	Northing	Ground Level (m)	Scale 1:25
Project Name High Post Business Park, Salisbury	Project No. 260201	Start Date 2026-02-09	End Date 2026-02-09	

Client Nationwide Engineering Group Ltd	Consultant Clear Environmental Limited	Contractor
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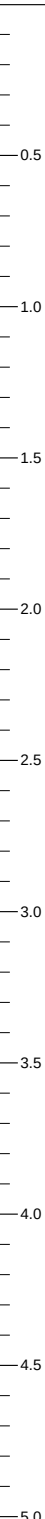
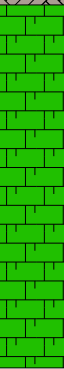
Inst/ Backfill	Water Levels	Samples and Tests			Level (m)	Depth (m) (thickness)	Strata		
		Depth (m)	Type/ Ref	Results			Legend	Description	
		0.50	B 1.5			(0.25)		Soft sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is angular to sub-rounded flint and chalk (TOPSOIL).	
						0.25		Structureless CHALK composed of medium dense slightly sandy very silty subangular and subrounded fine to coarse GRAVEL (SEAFORD CHALK FORMATION)	
						(1.25)			
						1.50	End of Trial Pit at 1.50m		

Remarks 1) All recorded strengths and densities based on visual observations of ease of excavation. 2) No direct visual or olfactory evidence of mobile contamination. 3) Groundwater was not encountered. 4) Trial pit terminated at 1.5m within chalk. 5) In-situ soakaway test undertaken in pit. 6) On completion, the trial pit was backfilled with arisings and made safe at the surface ...	Method, Plant, Stability, Dimensions 0.00 - 1.50m TP Stable L = 1.40m W = 0.35m	Logger BL

Checked By: BL Approved By: BL

Hole Type TP	Easting	Northing	Ground Level (m)	Scale 1:25
Project Name High Post Business Park, Salisbury	Project No. 260201	Start Date 2026-02-09	End Date 2026-02-09	

Client Nationwide Engineering Group Ltd	Consultant Clear Environmental Limited	Contractor
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Inst/ Backfill	Water Levels	Samples and Tests			Level (m)	Depth (thickness) (m)	Strata		
		Depth (m)	Type/ Ref	Results			Legend	Description	
						(0.30)		Soft sandy slightly gravelly CLAY. Sand is fine to coarse. Gravel is angular to sub-rounded flint and chalk (TOPSOIL).	
						0.30		Structureless CHALK composed of medium dense slightly sandy very silty subangular and subrounded fine to coarse GRAVEL (SEAFORD CHALK FORMATION)	
(1.20)									
1.50	End of Trial Pit at 1.50m								

Remarks 1) All recorded strengths and densities based on visual observations of ease of excavation. 2) No direct visual or olfactory evidence of mobile contamination. 3) Groundwater was not encountered. 4) Trial pit terminated at 1.5m within chalk. 5) In-situ soakaway test undertaken in pit. 6) On completion, the trial pit was backfilled with arisings and made safe at the surface ...	Method, Plant, Stability, Dimensions 0.00 - 1.50m TP Stable L = 1.40m W = 0.40m	Logger BL
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Checked By: BL Approved By: BL

Enclosure III – BRE 365 Soakaway Test Results



Project Number	260201	Location	TP02
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Calculated Infiltration Rate (ms⁻¹) = 1.99E-04

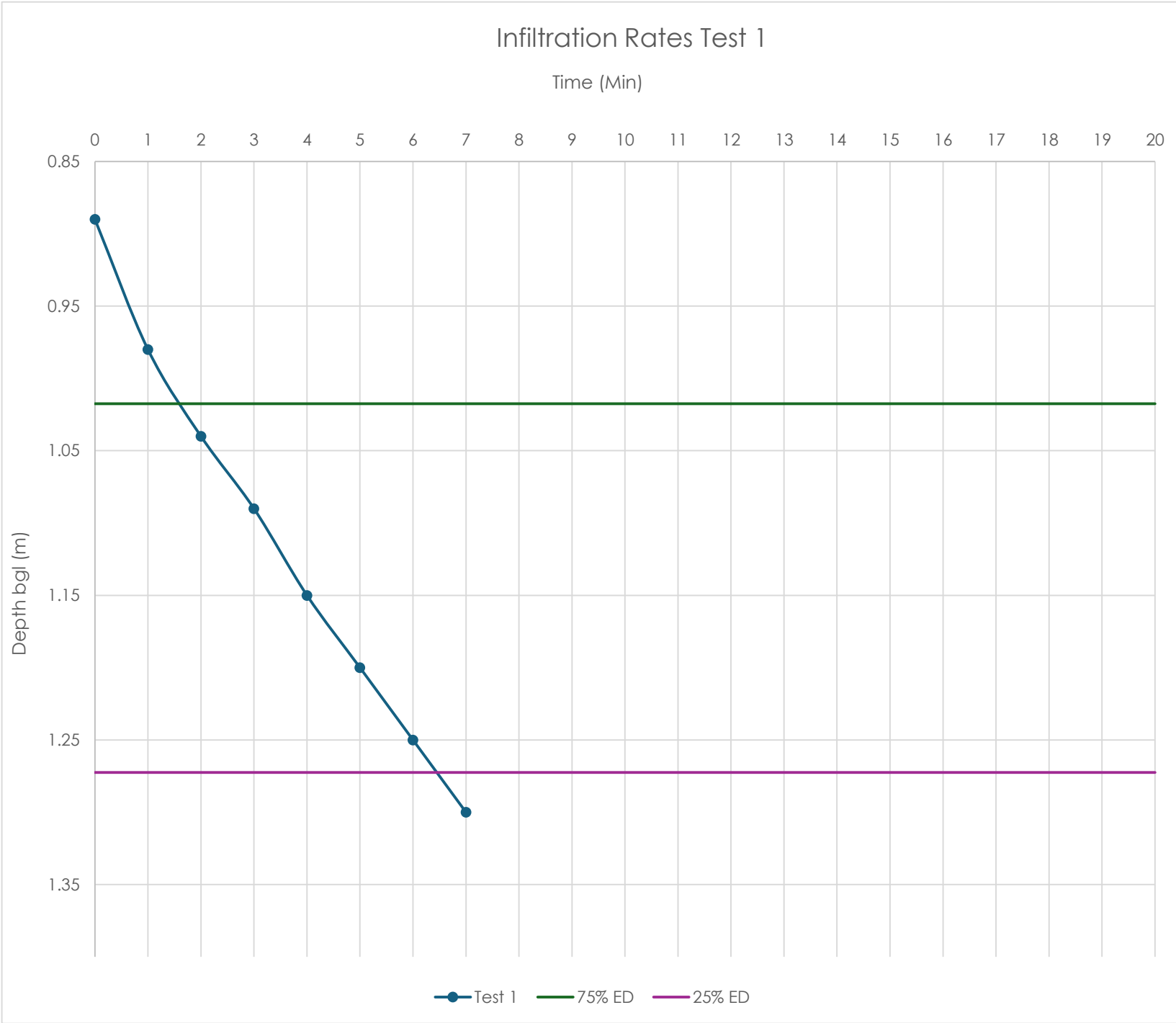
Time (min)	Water Level bgl (m)		
	Test No.		
	Test 1	Test 2	Test 3
0	0.89	0.80	0.80
1	0.98	0.90	0.89
2	1.04	0.97	0.95
3	1.09	1.04	1.00
4	1.15	1.10	1.06
5	1.20	1.13	1.10
6	1.25	1.16	1.14
7		1.20	1.18
8		1.23	1.21
9		1.26	1.25
10			
11			
12			
13			
14			
15			
16			
17			
18			
19			

Notes
Soakaway testing was undertaken in general accordance with BRE Digest 365. A trial pit was excavated and filled with water to assess the infiltration rates.

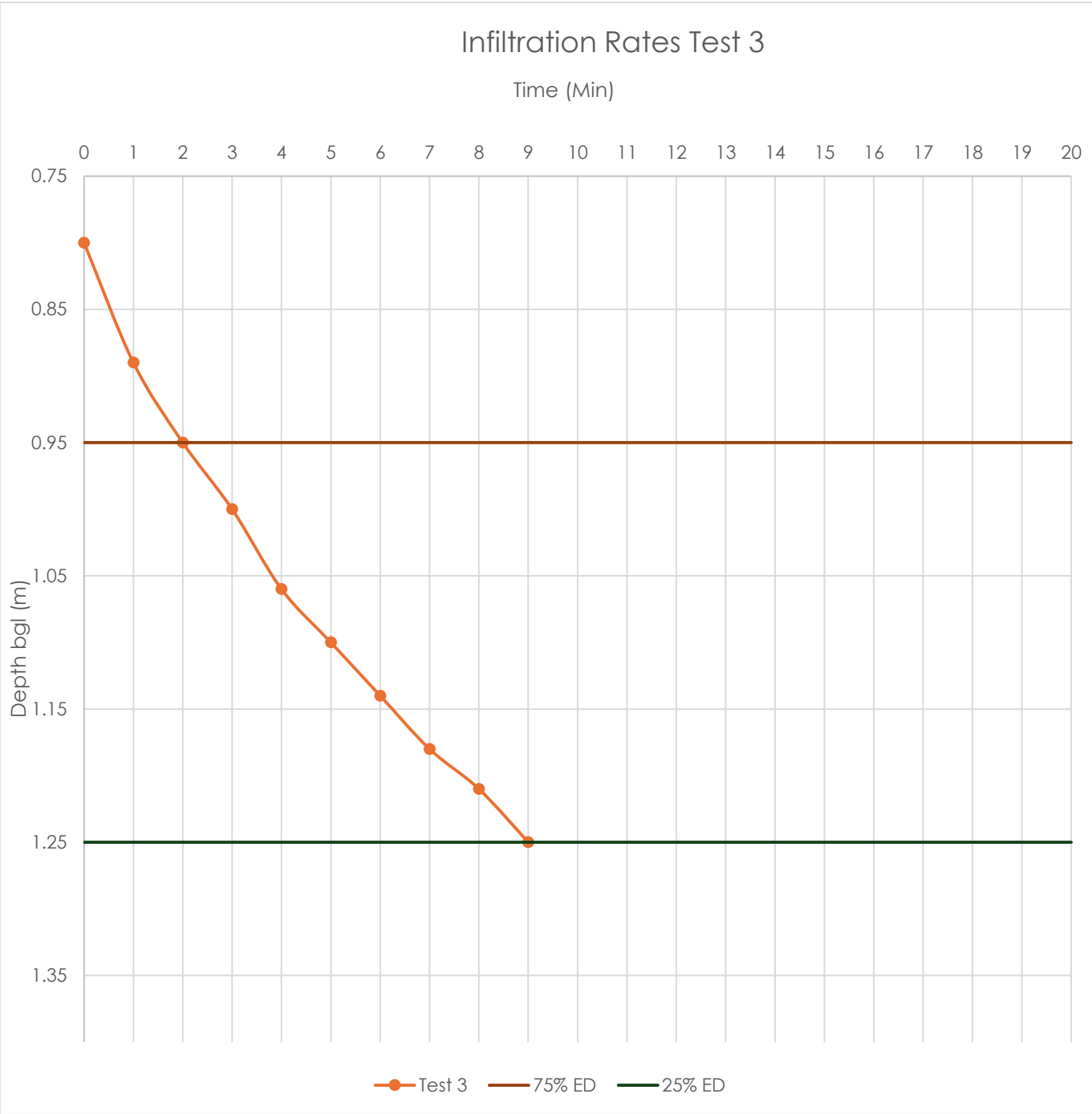
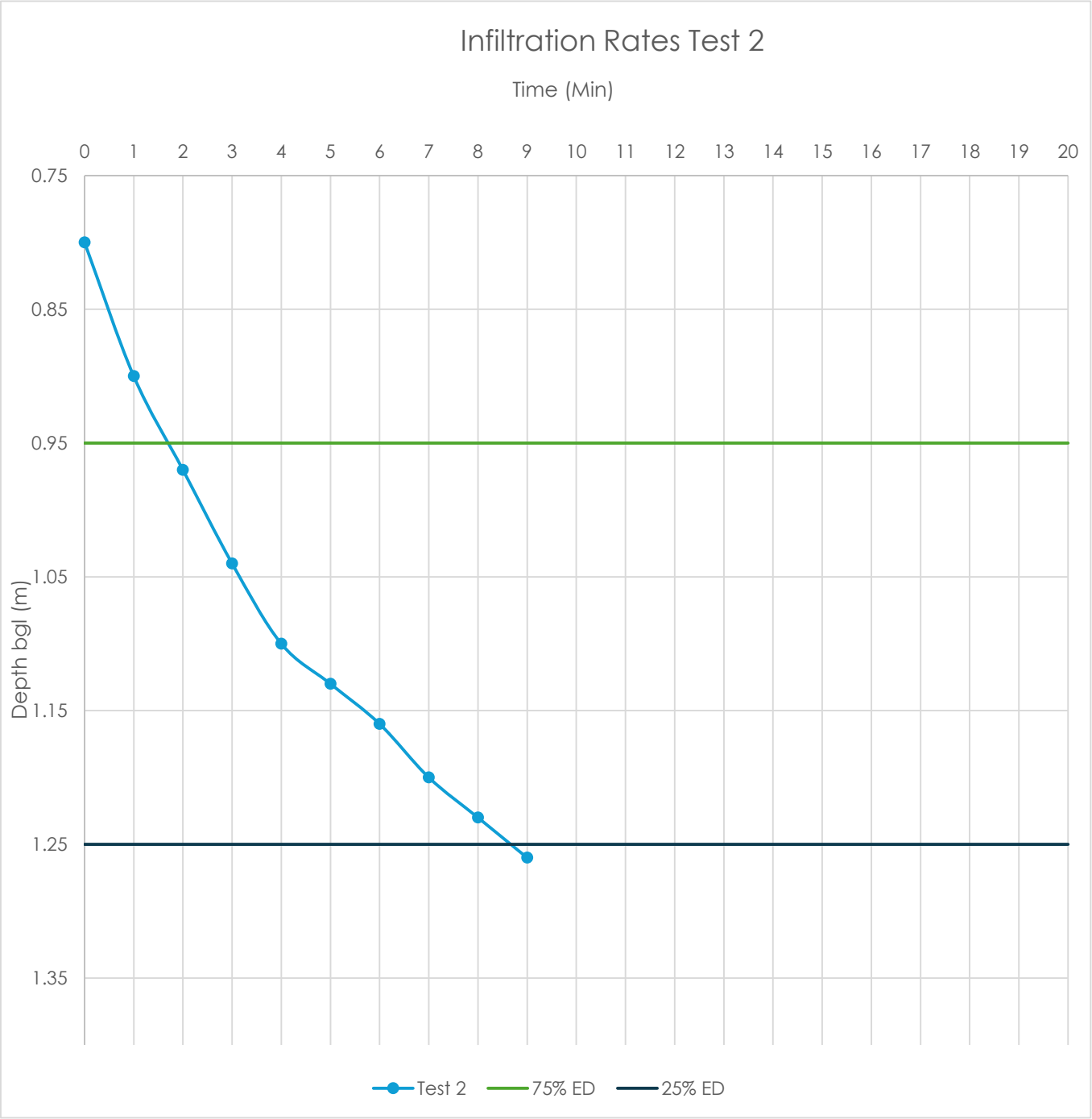
Results reflect conditions at the test location only and may not represent the wider site. Variations in soil type, groundwater or compaction could affect soakaway performance and should be considered in design.

Trial Pit excavated to 1.50m bgl, collapsed to 1.40m bgl on addition of water.

Test 1 Rate	Test 2 Rate	Test 3 Rate
2.51E-04	2.33E-04	1.99E-04



Pit Width (m)	0.35	Pit Length (m)	1.4	Pit Depth (m)	1.5
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Project Number	260201	Location	TP03
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[illegible]

Notes

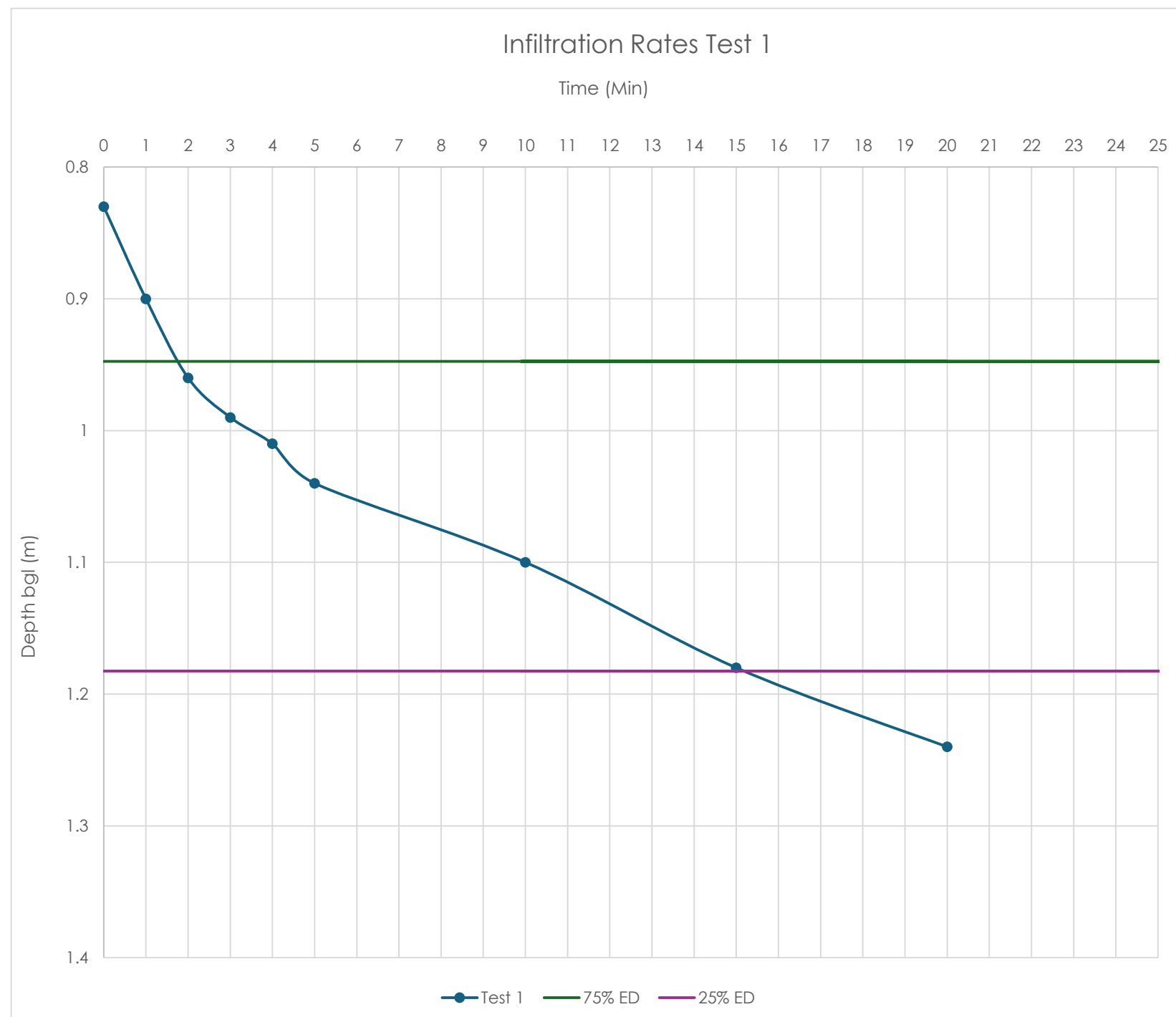
Soakaway testing was undertaken in general accordance with BRE Digest 365. A trial pit was excavated and filled with water to assess the infiltration rates.

Results reflect conditions at the test location only and may not represent the wider site. Variations in soil type, groundwater or compaction could affect soakaway performance and should be considered in design.

Trial Pit excavated to 1.50m bgl, collapsed to 1.30m bgl on addition of water.

Test 1 Rate	Test 2 Rate	Test 3 Rate
1.20E-04	7.99E-05	7.55E-05

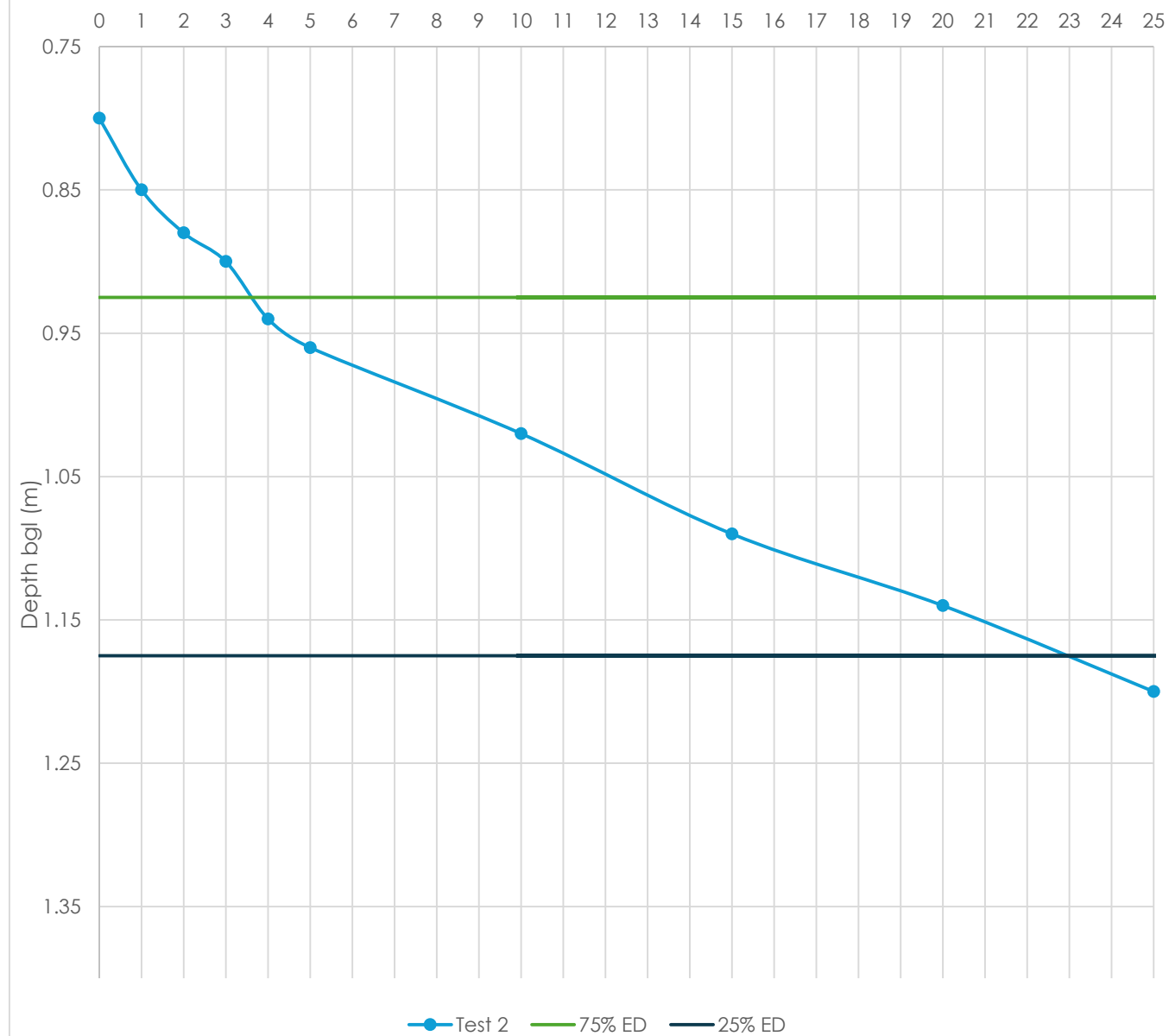
Calculated Infiltration Rate (ms^{-1}) = 7.55E-05



Pit Width (m)	0.38	Pit Length (m)	1.4	Pit Depth (m)	1.5
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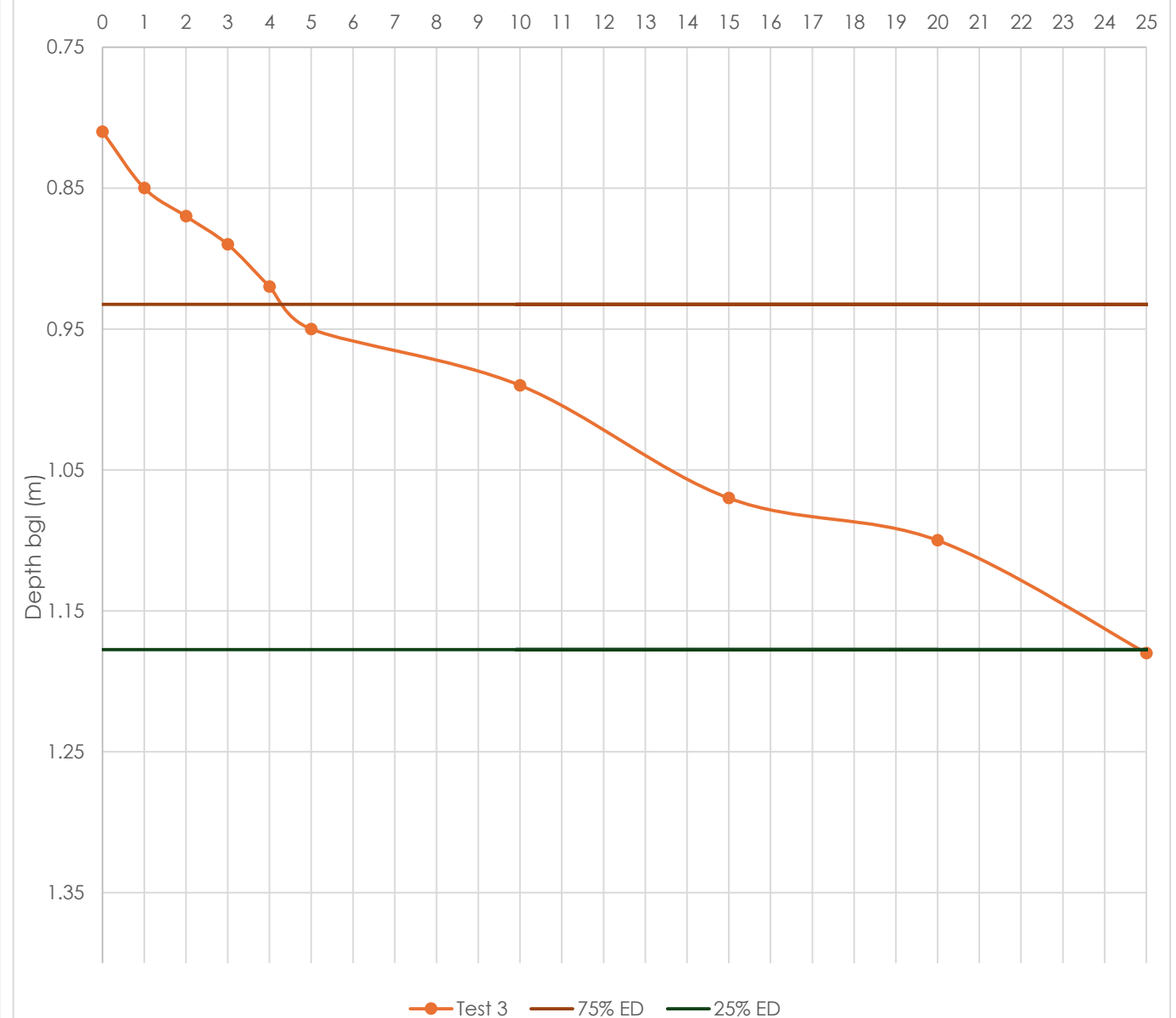
Infiltration Rates Test 2

Time (Min)



Infiltration Rates Test 3

Time (Min)





Project Number	260201	Location	TP04
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Calculated Infiltration Rate (ms⁻¹)

=

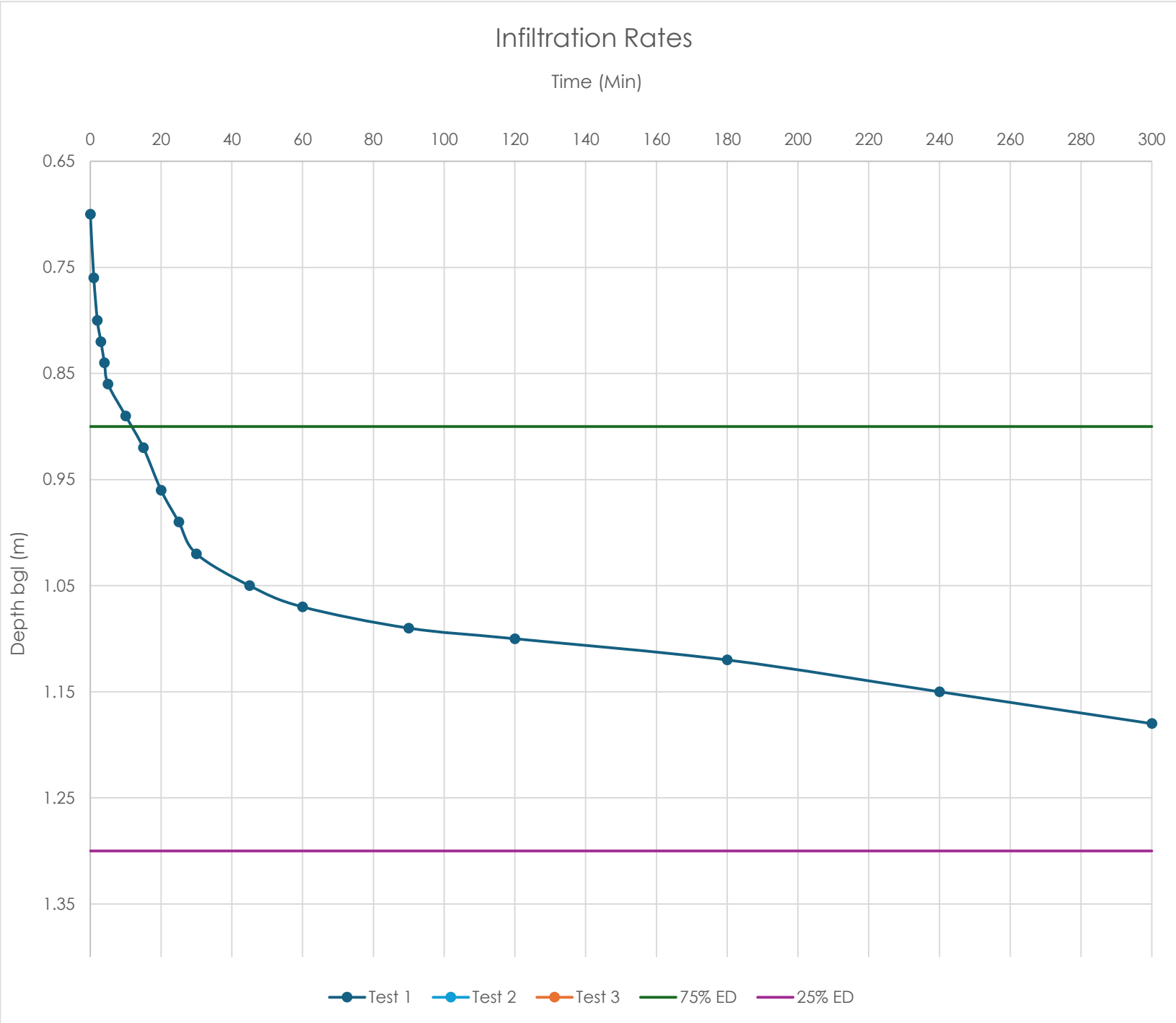
FAIL

Time (min)	Water Level bgl (m)		
	Test No.		
	Test 1	Test 2	Test 3
0	0.70		
1	0.76		
2	0.80		
3	0.82		
4	0.84		
5	0.86		
10	0.89		
15	0.92		
20	0.96		
25	0.99		
30	1.02		
45	1.05		
60	1.07		
90	1.09		
120	1.10		
180	1.12		
240	1.15		
300	1.18		

Notes
Soakaway testing was undertaken in general accordance with BRE Digest 365. A trial pit was excavated and filled with water once to assess the infiltration rates.

Results reflect conditions at the test location only and may not represent the wider site. Variations in soil type, groundwater or compaction could affect soakaway performance and should be considered in design.

Test 1 Rate	Test 2 Rate	Test 3 Rate
N/A	N/A	N/A



Pit Width (m)	0.35	Pit Length (m)	1.4	Pit Depth (m)	1.5
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Project Number	260201	Location	TP05
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Calculated Infiltration Rate (ms⁻¹)

=

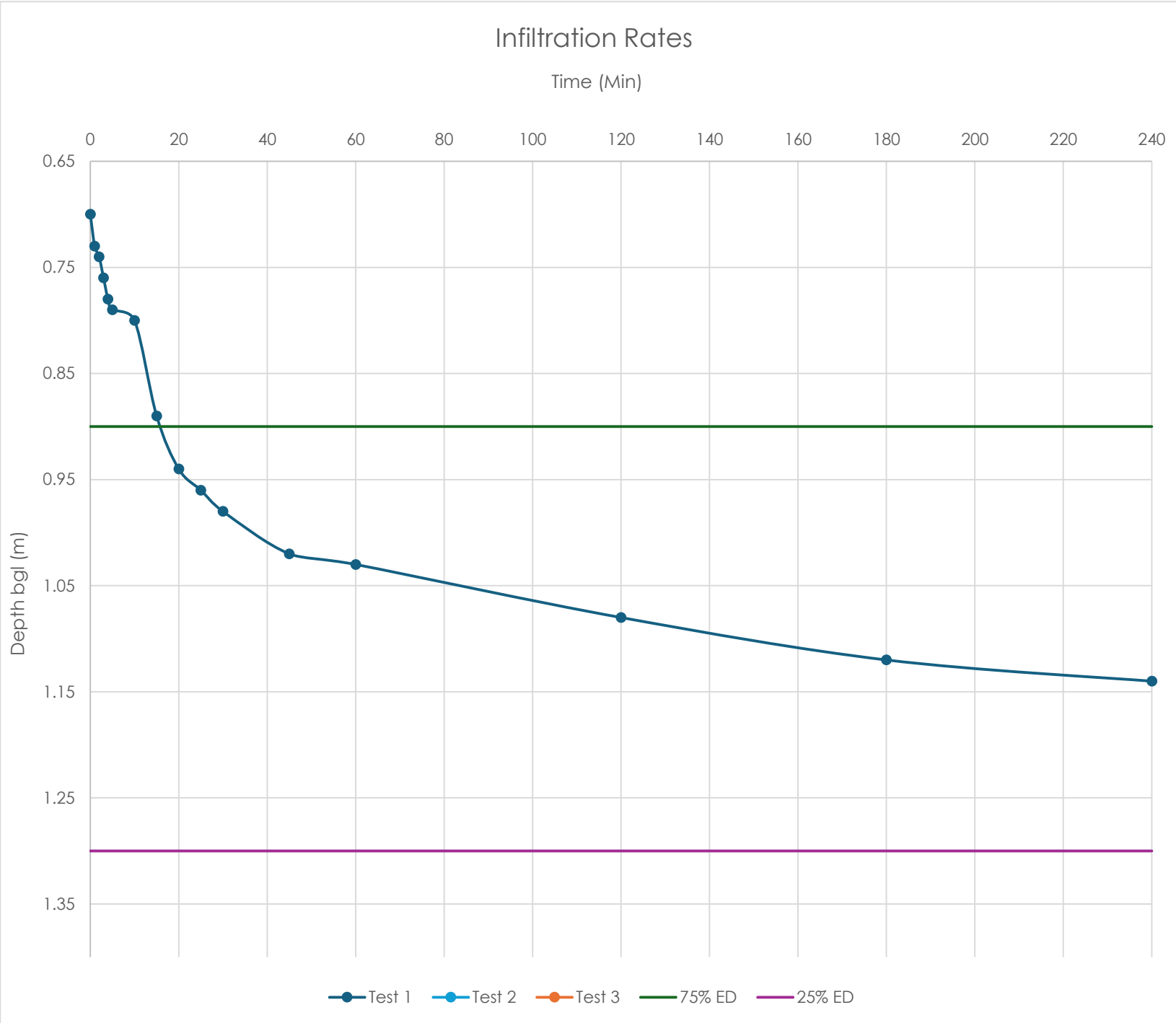
FAIL

Time (min)	Water Level bgl (m)		
	Test No.		
	Test 1	Test 2	Test 3
0	0.70		
1	0.73		
2	0.74		
3	0.76		
4	0.78		
5	0.79		
10	0.80		
15	0.89		
20	0.94		
25	0.96		
30	0.98		
45	1.02		
60	1.03		
90			
120	1.08		
180	1.12		
240	1.14		

Notes
Soakaway testing was undertaken in general accordance with BRE Digest 365. A trial pit was excavated and filled with water once to assess the infiltration rates.

Results reflect conditions at the test location only and may not represent the wider site. Variations in soil type, groundwater or compaction could affect soakaway performance and should be considered in design.

Test 1 Rate	Test 2 Rate	Test 3 Rate
N/A	N/A	N/A



Pit Width (m)	0.4	Pit Length (m)	1.4	Pit Depth (m)	1.5
---------------	-----	----------------	-----	---------------	-----

Appendix D – InfoDrainage Results

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflows Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Roof 2

Type : Catchment Area

Area (ha)	0.238
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Roof 1

Type : Catchment Area

Area (ha)	0.237
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Hardstanding 1

Type : Catchment Area

Area (ha)	0.052
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Commercial Access

Type : Catchment Area

Area (ha)	0.057
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflows Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Roof Area 3

Type : Catchment Area

Area (ha)	0.02
-----------	------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Staff Carpark

Type : Catchment Area

Area (ha)	0.381
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Solar Field A Access Road

Type : Catchment Area

Area (ha)	0.034
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.700
Winter Volumetric Runoff	0.700
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Roof 4

Type : Catchment Area

Area (ha)	0.011
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflows Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Small Carpark

Type : Catchment Area

Area (ha)	0.022
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.700
Winter Volumetric Runoff	0.700
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Catchment Area (25)

Type : Catchment Area

Area (ha)	0.074
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.700
Winter Volumetric Runoff	0.700
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Carpark Area 1

Type : Catchment Area

Area (ha)	0.168
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Carpark Area 2

Type : Catchment Area

Area (ha)	0.168
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflows Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Carpark Area 3

Type : Catchment Area

Area (ha)	0.168
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Carpark 2

Type : Catchment Area

Area (ha)	0.22
-----------	------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Roundabout and Part of the road

Type : Catchment Area

Area (ha)	0.246
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Access Road

Type : Catchment Area

Area (ha)	0.404
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflows Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Solar Field B Access Road

Type : Catchment Area

Area (ha)	0.034
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.700
Winter Volumetric Runoff	0.700
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Staff Carpark Access

Type : Catchment Area

Area (ha)	0.355
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Hardstanding 1 (1)

Type : Catchment Area

Area (ha)	0.047
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Catchment Area (1)

Type : Catchment Area

Area (ha)	0.093
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflows Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Catchment Area (1) (1)

Type : Catchment Area

Area (ha)	0.093
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Catchment Area (1) (1) (1)

Type : Catchment Area

Area (ha)	0.093
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Catchment Area (1) (2)

Type : Catchment Area

Area (ha)	0.093
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Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	1.000
Winter Volumetric Runoff	1.000
Time of Concentration (mins)	5
Percentage Impervious (%)	100

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Bioretention 1

Type : Bioretention

Ponding Area

Exceedance Level (m)	125.000
Depth (m)	0.300
Base Level (m)	124.700
Top Area (m²)	30.36
Side Slope (1:X)	3.00
Base Area (m²)	11.60
Freeboard (mm)	0
Porosity (%)	100
Length (m)	10.424
Long. Slope (1:X)	1000.00
Filtration Rate (m/hr)	3.0
Friction Scheme	Manning's n
n	0.015
Total Volume (m³)	8.728

Filter Area

Base Level (m)	124.000
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Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☒	Soil	500	30	200.0	Soil Type
	Storage	200	30	200.0	

Inlets

Inlet

Inlet Type	Point Inflow
Incoming Item(s)	Pipe
Bypass Destination	(None)
Inlet Destination	Ponding Area
Capacity Type	No Restriction

Advanced

Safety Factor	2.0
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Ponding Area

Side Infiltration Rate (m/hr)	0.7164
Base Perimeter (m)	23.073
Top Perimeter (m)	26.673

Filter Area

Base Infiltration Rate (m/hr)	0.7164
Side Infiltration Rate (m/hr)	0.7164

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Bioretention 2

Type : Bioretention

Ponding Area

Exceedance Level (m)	125.000
Depth (m)	0.300
Base Level (m)	124.700
Top Area (m²)	30.39
Side Slope (1:X)	3.00
Base Area (m²)	11.60
Freeboard (mm)	0
Porosity (%)	100
Length (m)	10.437
Long. Slope (1:X)	1000.00
Filtration Rate (m/hr)	3.0
Friction Scheme	Manning's n
n	0.015
Total Volume (m³)	8.734

Filter Area

Base Level (m)	124.000
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Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☒	Soil	500	30	200.0	Soil Type
	Storage	200	30	200.0	

Inlets

Inlet

Inlet Type	Point Inflow
Incoming Item(s)	Pipe (1)
Bypass Destination	(None)
Inlet Destination	Ponding Area
Capacity Type	No Restriction

Advanced

Safety Factor	2.0
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Ponding Area

Side Infiltration Rate (m/hr)	0.7164
Base Perimeter (m)	23.097
Top Perimeter (m)	26.697

Filter Area

Base Infiltration Rate (m/hr)	0.7164
Side Infiltration Rate (m/hr)	0.7164

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC	
		Approved By: OH		
		Company Address:		



Bioretention 3

Type : Bioretention

Ponding Area

Exceedance Level (m)	125.000
Depth (m)	0.300
Base Level (m)	124.700
Top Area (m²)	17.57
Side Slope (1:X)	2.00
Base Area (m²)	5.57
Freeboard (mm)	0
Porosity (%)	100
Length (m)	10.006
Long. Slope (1:X)	1000.00
Filtration Rate (m/hr)	3.0
Friction Scheme	Manning's n
n	0.015
Total Volume (m³)	4.807

Filter Area

Base Level (m)	123.900
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Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☒	Soil	600	30	200.0	Soil Type
	Storage	200	30	200.0	

Inlets

Inlet

Inlet Type	Point Inflow
Incoming Item(s)	Pipe (2)
Bypass Destination	(None)
Inlet Destination	Ponding Area
Capacity Type	No Restriction

Advanced

Safety Factor	2.0
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Ponding Area

Side Infiltration Rate (m/hr)	0.7164
Base Perimeter (m)	21.125
Top Perimeter (m)	23.525

Filter Area

Base Infiltration Rate (m/hr)	0.7164
Side Infiltration Rate (m/hr)	0.7164

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Bioretention (4)

Type : Bioretention

Ponding Area

Exceedance Level (m)	125.000
Depth (m)	0.300
Base Level (m)	124.700
Top Area (m²)	155.00
Side Slope (1:X)	3.00
Base Area (m²)	136.24
Freeboard (mm)	0
Porosity (%)	100
Length (m)	10.424
Long. Slope (1:X)	1000.00
Filtration Rate (m/hr)	3.0
Friction Scheme	Manning's n
n	0.015
Total Volume (m³)	72.295

Filter Area

Base Level (m)	124.000
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Filtration Layers

Use	Name	Filtration Layer Depth (mm)	Porosity (%)	Conductivity (m/hr)	Soil Type
☒	Soil	500	30	200.0	Soil Type
	Storage	200	30	200.0	

Inlets

Inlet (1)

Inlet Type	Point Inflow
Incoming Item(s)	Roof 4
Bypass Destination	(None)
Inlet Destination	Ponding Area
Capacity Type	No Restriction

Advanced

Safety Factor	2.0
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Ponding Area

Side Infiltration Rate (m/hr)	0.0036
Base Perimeter (m)	46.987
Top Perimeter (m)	50.587

Filter Area

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Main Carpark Permeable Paving 1

Type : Porous Paving

Dimensions

Exceedance Level (m)	125.000
Depth (m)	0.360
Base Level (m)	124.640
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	40.900
Long. Slope (1:X)	2000.00
Width (m)	40.900
Total Volume (m³)	155.571

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Carpark Area 1
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Pipe
Outlet Type	Orifice
Diameter (m)	0.075
Coefficient of Discharge	0.600
Invert Level (m)	124.640

Advanced

Conductivity (m/hr)	500.0
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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Main Carpark Permeable Paving 2

Type : Porous Paving

Dimensions

Exceedance Level (m)	125.000
Depth (m)	0.360
Base Level (m)	124.640
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	40.900
Long. Slope (1:X)	2000.00
Width (m)	40.900
Total Volume (m³)	155.571

Inlets

Inlet (5)

Inlet Type	Lateral Inflow
Incoming Item(s)	Carpark Area 2
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Pipe (1)
Outlet Type	Orifice
Diameter (m)	0.075
Coefficient of Discharge	0.600
Invert Level (m)	124.640

Advanced

Conductivity (m/hr)	500.0
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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Main Carpark Permeable Paving 3

Type : Porous Paving

Dimensions

Exceedance Level (m)	125.000
Depth (m)	0.350
Base Level (m)	124.650
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	40.900
Long. Slope (1:X)	2000.00
Width (m)	40.900
Total Volume (m³)	150.553

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Carpark Area 3
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Pipe (2)
Outlet Type	Orifice
Diameter (m)	0.075
Coefficient of Discharge	0.600
Invert Level (m)	124.650

Advanced

Conductivity (m/hr)	500.0
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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Staff Carpark Permeable Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	130.675
Depth (m)	0.350
Base Level (m)	130.325
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	62.000
Long. Slope (1:X)	2000.00
Width (m)	62.000
Total Volume (m³)	345.960

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Staff Carpark
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Carpark 2 Permeable Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	130.249
Depth (m)	0.350
Base Level (m)	129.899
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	89.462
Long. Slope (1:X)	2000.00
Width (m)	25.223
Total Volume (m³)	203.085

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Carpark 2
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Small Carpark Permeable Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	133.642
Depth (m)	0.300
Base Level (m)	133.342
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	21.308
Long. Slope (1:X)	2000.00
Width (m)	10.244
Total Volume (m³)	16.371

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Small Carpark
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Solar Field A Permeable Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	133.074
Depth (m)	0.300
Base Level (m)	132.774
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	30.515
Long. Slope (1:X)	2000.00
Width (m)	11.263
Total Volume (m³)	25.777

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Solar Field A Access Road
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Solar Field B Permeable Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	133.708
Depth (m)	0.300
Base Level (m)	133.408
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	30.515
Long. Slope (1:X)	2000.00
Width (m)	11.263
Total Volume (m³)	25.777

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Solar Field B Access Road
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Staff Carpark Access Permeable Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	133.024
Depth (m)	0.350
Base Level (m)	132.674
Paving Layer Depth (mm)	50
Membrane Percolation (m/hr)	3.0
Porosity (%)	30
Length (m)	60.000
Long. Slope (1:X)	2000.00
Width (m)	60.000
Total Volume (m³)	324.000

Inlets

Inlet (1)

Inlet Type	Lateral Inflow
Incoming Item(s)	Staff Carpark Access
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Filter Drain - Solar Field A

Type : Infiltration Trench

Dimensions

Exceedance Level (m)	126.661
Depth (m)	0.500
Base Level (m)	126.161
Freeboard (mm)	0
Porosity (%)	30
Length (m)	250.000
Long. Slope (1:X)	2000.00
Width (m)	1.500
Total Volume (m³)	68.620

Under Drain

Height Above Base (m)	0.000
Diameter (mm)	300
No. of Barrels	1
Release Height (m)	0.000
Friction Scheme	Manning's n
n	0.015

Inlets

Inlet (1)

Inlet Type	Lateral Inflow
Incoming Item(s)	Catchment Area (25)
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Filter Drain - Roundabout

Type : Infiltration Trench

Dimensions

Exceedance Level (m)	120.152
Depth (m)	0.700
Base Level (m)	119.452
Freeboard (mm)	0
Porosity (%)	30
Length (m)	50.000
Long. Slope (1:X)	2000.00
Width (m)	1.500
Total Volume (m³)	18.224

Under Drain

Height Above Base (m)	0.000
Diameter (mm)	300
No. of Barrels	1
Release Height (m)	0.000
Friction Scheme	Manning's n
n	0.015

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Roundabout and Part of the road
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Pipe (4)
Outlet Type	Free Discharge

Advanced

Base Infiltration Rate (m/hr)	0.2718
Side Infiltration Rate (m/hr)	0.2718
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Filter Drain - (Access Road)

Type : Infiltration Trench

Dimensions

Exceedance Level (m)	130.000
Depth (m)	1.000
Base Level (m)	129.000
Freeboard (mm)	0
Porosity (%)	30
Length (m)	300.000
Long. Slope (1:X)	2000.00
Width (m)	1.500
Total Volume (m³)	149.844

Under Drain

Height Above Base (m)	0.000
Diameter (mm)	300
No. of Barrels	1
Release Height (m)	0.000
Friction Scheme	Manning's n
n	0.015

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Access Road
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Pipe (9)
Outlet Type	Free Discharge

Advanced

Base Infiltration Rate (m/hr)	0.0036
Side Infiltration Rate (m/hr)	0.0036
Safety Factor	2.0
Conductivity (m/hr)	500.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Main Soakaway

Type : Cellular Storage

Dimensions

Exceedance Level (m)	126.000
Depth (m)	0.800
Base Level (m)	124.800
Number of Crates Long	90
Number of Crates Wide	35
Number of Crates High	2
Porosity (%)	95
Crate Length (m)	0.4
Crate Width (m)	0.4
Crate Height (m)	0.4
Total Volume (m³)	383.440

Inlets

Inlet (1)

Inlet Type	Point Inflow
Incoming Item(s)	Pipe (3) (1)
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Advanced

Base Infiltration Rate (m/hr)	0.2718
Side Infiltration Rate (m/hr)	0.2718
Safety Factor	2.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Roundabout Soakaway

Type : Cellular Storage

Dimensions	
Exceedance Level (m)	119.564
Depth (m)	0.800
Base Level (m)	118.000
Number of Crates Long	50
Number of Crates Wide	15
Number of Crates High	2
Porosity (%)	95
Crate Length (m)	0.4
Crate Width (m)	0.4
Crate Height (m)	0.4
Total Volume (m³)	91.964
Inlets	
Inlet	
Inlet Type	Point Inflow
Incoming Item(s)	Pipe (4)
Bypass Destination	(None)
Capacity Type	No Restriction
Outlets	
Advanced	
Base Infiltration Rate (m/hr)	0.7164
Side Infiltration Rate (m/hr)	0.7164
Safety Factor	2.0

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



Geocellular Storage Tank

Type : Cellular Storage

Dimensions

Exceedance Level (m)	128.256
Depth (m)	0.800
Base Level (m)	127.000
Number of Crates Long	100
Number of Crates Wide	42
Number of Crates High	2
Porosity (%)	95
Crate Length (m)	0.4
Crate Width (m)	0.4
Crate Height (m)	0.4
Total Volume (m³)	511.176

Inlets

Inlet (2)

Inlet Type	Point Inflow
Incoming Item(s)	Catchment Area (1) (2)
Bypass Destination	(None)
Capacity Type	No Restriction

Inlet (3)

Inlet Type	Point Inflow
Incoming Item(s)	Catchment Area (1) (1) (1)
Bypass Destination	(None)
Capacity Type	No Restriction

Inlet (4)

Inlet Type	Point Inflow
Incoming Item(s)	Catchment Area (1) (1)
Bypass Destination	(None)
Capacity Type	No Restriction

Inlet (5)

Inlet Type	Point Inflow
Incoming Item(s)	Catchment Area (1)
Bypass Destination	(None)
Capacity Type	No Restriction

Inlet (6)

Inlet Type	Point Inflow
Incoming Item(s)	Pipe (5)
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Pipe (12)
Outlet Type	Orifice
Diameter (m)	0.075
Coefficient of Discharge	0.600
Invert Level (m)	127.000

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Inflow Summary Storm Phase: Phase		Designed by: JA	Checked by: JC		
		Approved By: OH			
		Company Address:			

Inflow Label	Connected To	Flow (L/s)	Runoff Method	Area (ha)	Percentage Impervious (%)	Urban Creep (%)	Adjusted Percentage Impervious (%)	Area Analysed (ha)
Access Road	Filter Drain - (Access Road)		Time of Concentration	0.404	100	0	100	0.404
Carpark 2	Carpark 2 Permeable Paving		Time of Concentration	0.220	100	0	100	0.220
Carpark Area 1	Main Carpark Permeable Paving 1		Time of Concentration	0.168	100	0	100	0.168
Carpark Area 2	Main Carpark Permeable Paving 2		Time of Concentration	0.168	100	0	100	0.168
Carpark Area 3	Main Carpark Permeable Paving 3		Time of Concentration	0.168	100	0	100	0.168
Catchment Area (1)	Geocellular Storage Tank		Time of Concentration	0.093	100	0	100	0.093
Catchment Area (1) (1)	Geocellular Storage Tank		Time of Concentration	0.093	100	0	100	0.093
Catchment Area (1) (1) (1)	Geocellular Storage Tank		Time of Concentration	0.093	100	0	100	0.093
Catchment Area (1) (2)	Geocellular Storage Tank		Time of Concentration	0.093	100	0	100	0.093
Catchment Area (25)	Filter Drain - Solar Field A		Time of Concentration	0.074	100	0	100	0.074
Commercial Access	Manhole (4)		Time of Concentration	0.057	100	0	100	0.057
Hardstanding 1	Manhole		Time of Concentration	0.052	100	0	100	0.052
Hardstanding 1 (1)	Manhole (1)		Time of Concentration	0.047	100	0	100	0.047
Roof 1	Manhole (7)		Time of Concentration	0.237	100	0	100	0.237
Roof 2	Manhole		Time of Concentration	0.238	100	0	100	0.238
Roof 4	Bioretention (4)		Time of Concentration	0.011	100	0	100	0.011
Roof Area 3	Manhole (4)		Time of Concentration	0.020	100	0	100	0.020
Roundabout and Part of the road	Filter Drain - Roundabout		Time of Concentration	0.246	100	0	100	0.246
Small Carpark	Small Carpark Permeable Paving		Time of Concentration	0.022	100	0	100	0.022
Solar Field A Access Road	Solar Field A Permeable Paving		Time of Concentration	0.034	100	0	100	0.034
Solar Field B Access Road	Solar Field B Permeable Paving		Time of Concentration	0.034	100	0	100	0.034
Staff Carpark	Staff Carpark Permeable Paving		Time of Concentration	0.381	100	0	100	0.381
Staff Carpark Access	Staff Carpark Access Permeable Paving		Time of Concentration	0.355	100	0	100	0.355
TOTAL		0.0		3.309				3.309

AEG9910_SP4_HighPost_07:		Date:			
Atlas Planning		10/10/2026			
High Post Business Park, Salisbury, Wiltshire, SP4		Designed by:	Checked by:	Approved By:	
Surface Water Drainage Network		JA	JC	OH	
Report Details:		Company Address:			
Type: Network Design Criteria					
Storm Phase: Phase					



Flow Options

Peak Flow Calculation	(UK) Modified Rational Method
Min. Time of Entry (mins)	5
Max. Travel Time (mins)	30

Pipe Options

Lock Slope Options	None
Design Options	Minimise Excavation
Design Level	Level Soffits
Min. Cover Depth (m)	1.200
Min. Slope (1:X)	500.00
Max. Slope (1:X)	40.00
Min. Velocity (m/s)	1.0
Max. Velocity (m/s)	3.0
Use Flow Restriction	☐
Reduce Channel Depths	☐

Pipe Size Library

Default

Add. Increment (mm)	75
Max. Diameter (mm)	0

Diameter (mm)	Min. Slope (1:X)	Max. Slope (1:X)
100	0.00	0.00
150	0.00	0.00

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Network Design Criteria Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			

Manhole Options

Apply Offset	☐
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Manhole Size Library

Default

Diameter / Width

Connection (mm)	Diameter / Length (m)	Width (m)
0	1.200	0.000
375	1.350	0.000
500	1.500	0.000
750	1.800	0.000

Additional Sizing

Connection (mm)	900
Diameter / Length (m)	0.900
Width (m)	0.000

Depth

Depth (m)	Diameter / Length (m)	Width (m)
0.000	1.050	0.000
1.500	1.200	0.000

Access

Depth (m)	Ladder Protrusion (mm)
0.000	130
3.000	230

Benching Requirements

Landing Width (mm)	500
Benching Width (mm)	225

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment
Report Title: Rainfall Analysis Criteria		Designed by: JA	Checked by: JC	
		Approved By: OH		
		Company Address:		

Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Apply Global Value
Urban Creep Global Value (%)	0
Junction Flood Risk Margin (mm)	300
Perform No Discharge Analysis	☐

Rainfall		
FEH		Type: FEH
Site Location	GB 414895 136786 SU 14895 36786	
Rainfall Version	2022	
Summer	☒	
Winter	☒	

Return Period

Return Period (years)	Increase Rainfall (%)
2.0	0.000
30.0	0.000
100.0	0.000
30.0	40.000
100.0	45.000

Storm Durations

Duration (mins)	Run Time (mins)
15	30
30	60
60	120
120	240
240	480
360	720
480	960
960	1920
1440	2880

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Designed by: JA	Checked by: JC	
		Approved By: OH		
		Company Address:		



FEH: 2 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Resident Volume

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residual Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Lost Volume (m³)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Bioretention 1	FEH: 2 years: +0 %: 1440 mins: Winter	124.213	124.000	0.202	0.000	0.4	0.217	0.000	0.0	30.058	0.000	18	97.517
Bioretention 2	FEH: 2 years: +0 %: 1440 mins: Winter	124.213	124.000	0.202	0.000	0.4	0.224	0.000	0.0	30.309	0.000	19	97.433
Bioretention 3	FEH: 2 years: +0 %: 1440 mins: Summer	124.315	123.900	0.405	0.000	0.4	0.366	0.000	0.0	33.637	0.000	8	92.378
Main Carpark Permeable Paving 1	FEH: 2 years: +0 %: 1440 mins: Winter	124.770	124.732	0.109	0.092	1.9	52.487	0.000	0.4	0.000	30.425	2217	66.261
Main Carpark Permeable Paving 2	FEH: 2 years: +0 %: 1440 mins: Winter	124.770	124.732	0.109	0.092	1.9	52.328	0.000	0.4	0.000	30.556	2194	66.364
Main Carpark Permeable Paving 3	FEH: 2 years: +0 %: 1440 mins: Winter	124.778	124.736	0.108	0.086	1.9	50.943	0.000	0.4	0.000	33.821	1966	66.163
Filter Drain - Solar Field A Staff	FEH: 2 years: +0 %: 360 mins: Winter	126.313	126.313	0.027	0.152	1.7	10.073	0.000	0.0	8.336	0.000	417	85.321
Carpark Permeable Paving	FEH: 2 years: +0 %: 240 mins: Winter	130.411	130.383	0.055	0.059	16.1	65.804	0.000	0.0	54.702	0.000	303	80.979
Main Soakaway	FEH: 2 years: +0 %: 120 mins: Summer	124.898	124.898	0.098	0.098	56.1	46.742	0.000	0.0	175.306	0.000	26	87.810
Roundabout Soakaway	FEH: 2 years: +0 %: 30 mins: Summer	118.072	118.072	0.072	0.072	29.0	8.211	0.000	0.0	21.016	0.000	10	91.072
Filter Drain - Roundabout	FEH: 2 years: +0 %: 15 mins: Summer	119.660	119.541	0.183	0.089	49.0	3.597	0.000	39.4	4.475	16.888	2	80.262
Carpark 2 Permeable Paving	FEH: 2 years: +0 %: 240 mins: Winter	129.998	129.956	0.054	0.057	9.3	37.759	0.000	0.0	32.146	0.000	296	81.407
Filter Drain - (Access Road)	FEH: 2 years: +0 %: 60 mins: Summer	129.330	129.054	0.180	0.054	46.1	24.742	0.000	27.3	1.780	52.122	15	83.488
Small Carpark Permeable Paving	FEH: 2 years: +0 %: 240 mins: Winter	133.385	133.379	0.032	0.037	0.6	2.247	0.000	0.0	3.097	0.000	182	86.273

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
	Designed by: JA	Checked by: JC	Approved By: OH
	Company Address:		
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			
			 water, civils and environment

Status
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AEG9910_SP4_HighPost_07:		Date:		
Atlas Planning		10/10/2026		
High Post Business Park, Salisbury, Wiltshire, SP4		Designed by:	Checked by:	Approved By:
Surface Water Drainage Network		JA	JC	OH
Report Details:		Company Address:		
Type: Stormwater Controls Summary				
Storm Phase: Phase				



Solar Field A Permeable Paving	FEH: 2 years: +0 %: 240 mins: Winter	132.821	132.809	0.032	0.036	1.0	3.508	0.000	0.0	4.872	0.000	181	86.393
Solar Field B Permeable Paving	FEH: 2 years: +0 %: 240 mins: Winter	133.456	133.444	0.033	0.036	1.0	3.507	0.000	0.0	4.872	0.000	181	86.393
Geocellular Storage Tank	FEH: 2 years: +0 %: 480 mins: Summer	127.226	127.226	0.226	0.226	28.6	144.050	0.000	4.8	0.000	170.358	325	71.820
Bioretention (4)	FEH: 2 years: +0 %: 960 mins: Summer	124.211	124.016	0.200	0.016	0.3	4.128	0.000	0.0	3.049	0.000	996	94.290
Staff Carpark Access Permeable Paving	FEH: 2 years: +0 %: 240 mins: Winter	132.759	132.733	0.055	0.059	15.0	61.217	0.000	0.0	51.229	0.000	301	81.106

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
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	Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		
Company Address:			

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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



FEH: 30 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Resident Volume

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residual Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Lost Volume (m³)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Bioretention 1	FEH: 30 years: +0 %: 1440 mins: Summer	124.409	124.000	0.399	0.000	0.7	0.701	0.000	0.0	64.505	0.000	9	91.968
Bioretention 2	FEH: 30 years: +0 %: 1440 mins: Summer	124.426	124.000	0.415	0.000	0.7	0.716	0.000	0.0	65.136	0.000	9	91.799
Bioretention 3	FEH: 30 years: +0 %: 1440 mins: Summer	124.693	123.900	0.783	0.000	0.7	0.630	0.000	0.0	68.141	0.000	9	86.890
Main Carpark Permeable Paving 1	FEH: 30 years: +0 %: 1440 mins: Summer	124.832	124.783	0.171	0.143	4.9	81.468	0.000	0.7	0.000	64.942	1385	47.633
Main Carpark Permeable Paving 2	FEH: 30 years: +0 %: 1440 mins: Summer	124.831	124.782	0.171	0.142	4.9	81.179	0.000	0.7	0.000	65.453	1394	47.819
Main Carpark Permeable Paving 3	FEH: 30 years: +0 %: 1440 mins: Summer	124.840	124.788	0.170	0.138	4.9	80.091	0.000	0.7	0.000	68.387	1256	46.802
Filter Drain - Solar Field A Staff	FEH: 30 years: +0 %: 480 mins: Winter	126.406	126.406	0.120	0.245	2.4	20.503	0.000	0.0	12.465	0.000	800	70.121
Carpark Permeable Paving	FEH: 30 years: +0 %: 360 mins: Winter	130.478	130.457	0.122	0.132	21.9	146.858	0.000	0.0	82.682	0.000	659	57.551
Main Soakaway	FEH: 30 years: +0 %: 120 mins: Winter	125.139	125.139	0.339	0.339	87.4	162.125	0.000	0.0	261.273	0.000	84	57.718
Roundabout Soakaway	FEH: 30 years: +0 %: 60 mins: Summer	118.303	118.303	0.303	0.303	59.4	34.488	0.000	0.0	65.770	0.000	33	62.498
Filter Drain - Roundabout	FEH: 30 years: +0 %: 15 mins: Summer	119.783	119.574	0.306	0.122	107.6	5.718	0.000	92.5	4.976	41.742	1	68.622
Carpark 2 Permeable Paving	FEH: 30 years: +0 %: 360 mins: Winter	130.064	130.028	0.121	0.129	12.6	84.440	0.000	0.0	48.675	0.000	643	58.421
Filter Drain - (Access Road)	FEH: 30 years: +0 %: 60 mins: Summer	129.586	129.085	0.436	0.085	103.4	48.452	0.000	64.5	1.902	122.745	11	67.665
Small Carpark Permeable Paving	FEH: 30 years: +0 %: 240 mins: Winter	133.431	133.428	0.078	0.086	1.2	5.378	0.000	0.0	3.157	0.000	428	67.149

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
	Designed by: JA	Checked by: JC	Approved By: OH
	Company Address:		
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			

Status
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AEG9910_SP4_HighPost_07:		Date:		
Atlas Planning		10/10/2026		
High Post Business Park, Salisbury, Wiltshire, SP4		Designed by:	Checked by:	Approved By:
Surface Water Drainage Network		JA	JC	OH
Report Details:		Company Address:		
Type: Stormwater Controls Summary				
Storm Phase: Phase				



Solar Field A Permeable Paving	FEH: 30 years: +0 %: 240 mins: Winter	132.867	132.859	0.078	0.085	1.9	8.429	0.000	0.0	4.957	0.000	427	67.302
Solar Field B Permeable Paving	FEH: 30 years: +0 %: 240 mins: Winter	133.502	133.494	0.078	0.086	1.9	8.429	0.000	0.0	4.958	0.000	427	67.303
Geocellular Storage Tank	FEH: 30 years: +0 %: 360 mins: Winter	127.413	127.413	0.413	0.413	40.7	263.393	0.000	6.8	0.000	212.235	397	48.473
Bioretention (4)	FEH: 30 years: +0 %: 480 mins: Winter	124.211	124.084	0.200	0.084	0.5	6.226	0.000	0.0	3.237	0.000	910	91.389
Staff Carpark Access Permeable Paving	FEH: 30 years: +0 %: 360 mins: Winter	132.826	132.806	0.122	0.132	20.4	136.733	0.000	0.0	77.442	0.000	655	57.799

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



FEH: 100 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Resident Volume

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residual Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Lost Volume (m³)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Bioretention 1	FEH: 100 years: +0 %: 1440 mins: Summer	124.538	124.000	0.527	0.000	0.8	0.819	0.000	0.0	82.041	0.000	9	90.614
Bioretention 2	FEH: 100 years: +0 %: 1440 mins: Summer	124.550	124.000	0.539	0.000	0.9	0.831	0.000	0.0	82.958	0.000	9	90.488
Bioretention 3	FEH: 100 years: +0 %: 1440 mins: Summer	124.717	124.100	0.807	0.200	0.9	0.807	0.000	0.0	85.542	0.000	10	83.209
Main Carpark Permeable Paving 1	FEH: 100 years: +0 %: 960 mins: Winter	124.866	124.814	0.205	0.174	5.3	98.054	0.000	0.8	0.000	61.346	1354	36.971
Main Carpark Permeable Paving 2	FEH: 100 years: +0 %: 960 mins: Winter	124.865	124.813	0.205	0.173	5.3	97.656	0.000	0.9	0.000	62.200	1308	37.228
Main Carpark Permeable Paving 3	FEH: 100 years: +0 %: 960 mins: Winter	124.874	124.820	0.204	0.170	5.3	96.856	0.000	0.9	0.000	64.183	1247	35.666
Filter Drain - Solar Field A Staff	FEH: 100 years: +0 %: 480 mins: Winter	126.455	126.455	0.169	0.294	2.9	26.038	0.000	0.0	12.986	0.000	976	62.054
Carpark Permeable Paving	FEH: 100 years: +0 %: 360 mins: Winter	130.513	130.494	0.157	0.170	26.8	189.054	0.000	0.0	82.944	0.000	846	45.354
Main Soakaway	FEH: 100 years: +0 %: 120 mins: Winter	125.266	125.266	0.466	0.466	107.4	222.981	0.000	0.0	271.056	0.000	112	41.847
Roundabout Soakaway	FEH: 100 years: +0 %: 60 mins: Summer	118.440	118.440	0.440	0.440	76.9	50.198	0.000	0.0	86.084	0.000	45	45.416
Filter Drain - Roundabout	FEH: 100 years: +0 %: 15 mins: Summer	119.855	119.583	0.378	0.131	134.6	6.830	0.000	112.3	5.151	53.461	1	62.520
Carpark 2 Permeable Paving	FEH: 100 years: +0 %: 360 mins: Winter	130.099	130.065	0.156	0.166	15.5	108.778	0.000	0.0	48.878	0.000	825	46.437
Filter Drain - (Access Road)	FEH: 100 years: +0 %: 60 mins: Summer	129.731	129.096	0.581	0.096	132.3	61.055	0.000	80.9	1.969	158.319	11	59.254
Small Carpark Permeable Paving	FEH: 100 years: +0 %: 240 mins: Winter	133.455	133.453	0.102	0.111	1.5	6.968	0.000	0.0	3.179	0.000	551	57.439

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
	Designed by: JA	Checked by: JC	Approved By: OH
	Company Address:		
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			
			 water, civils and environment

Status
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AEG9910_SP4_HighPost_07:		Date:		
Atlas Planning		10/10/2026		
High Post Business Park, Salisbury, Wiltshire, SP4		Designed by:	Checked by:	Approved By:
Surface Water Drainage Network		JA	JC	OH
Report Details:		Company Address:		
Type: Stormwater Controls Summary				
Storm Phase: Phase				



Solar Field A Permeable Paving	FEH: 100 years: +0 %: 240 mins: Winter	132.890	132.883	0.101	0.110	2.3	10.908	0.000	0.0	4.988	0.000	550	57.683
Solar Field B Permeable Paving	FEH: 100 years: +0 %: 240 mins: Winter	133.525	133.519	0.102	0.110	2.3	10.908	0.000	0.0	4.988	0.000	550	57.683
Geocellular Storage Tank	FEH: 100 years: +0 %: 360 mins: Winter	127.515	127.515	0.515	0.515	49.9	328.755	0.000	7.7	0.000	243.826	445	35.687
Bioretention (4)	FEH: 100 years: +0 %: 480 mins: Winter	124.211	124.116	0.200	0.116	0.6	7.416	0.000	0.0	3.327	0.000	1079	89.742
Staff Carpark Access Permeable Paving	FEH: 100 years: +0 %: 360 mins: Winter	132.861	132.844	0.157	0.169	25.0	176.058	0.000	0.0	77.693	0.000	841	45.661

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
	Designed by: JA	Checked by: JC	Approved By: OH
	Company Address:		
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			



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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



FEH: 30 years: Increase Rainfall (%): +40: Critical Storm Per Item: Rank By: Max. Resident Volume

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residual Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Lost Volume (m³)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Bioretention 1	FEH: 30 years: +40 %: 1440 mins: Summer	124.617	124.000	0.606	0.000	1.0	0.893	0.000	0.0	97.262	0.000	9	89.769
Bioretention 2	FEH: 30 years: +40 %: 1440 mins: Summer	124.628	124.000	0.617	0.000	1.0	0.903	0.000	0.0	98.381	0.000	9	89.657
Bioretention 3	FEH: 30 years: +40 %: 1440 mins: Summer	124.721	124.101	0.811	0.201	1.0	0.879	0.000	0.0	100.528	0.000	9	81.711
Main Carpark Permeable Paving 1	FEH: 30 years: +40 %: 1440 mins: Summer	124.896	124.845	0.235	0.205	6.9	113.139	0.000	1.0	0.000	97.664	1317	27.275
Main Carpark Permeable Paving 2	FEH: 30 years: +40 %: 1440 mins: Summer	124.895	124.843	0.235	0.203	6.9	112.739	0.000	1.0	0.000	98.660	1279	27.532
Main Carpark Permeable Paving 3	FEH: 30 years: +40 %: 1440 mins: Summer	124.904	124.851	0.234	0.201	6.9	112.007	0.000	1.0	0.000	100.925	1255	25.603
Filter Drain - Solar Field A Staff	FEH: 30 years: +40 %: 480 mins: Winter	126.493	126.493	0.207	0.332	3.3	30.275	0.000	0.0	13.390	0.000	1097	55.880
Carpark Permeable Paving	FEH: 30 years: +40 %: 480 mins: Winter	130.541	130.527	0.185	0.202	24.4	223.501	0.000	0.0	110.725	0.000	989	35.397
Main Soakaway	FEH: 30 years: +40 %: 120 mins: Winter	125.341	125.341	0.541	0.541	119.3	259.151	0.000	0.0	276.257	0.000	127	32.414
Roundabout Soakaway	FEH: 30 years: +40 %: 60 mins: Summer	118.505	118.505	0.505	0.505	84.5	57.574	0.000	0.0	88.679	0.000	51	37.395
Filter Drain - Roundabout	FEH: 30 years: +40 %: 15 mins: Summer	119.902	119.587	0.425	0.135	150.3	7.632	0.000	120.9	5.251	60.167	1	58.123
Carpark 2 Permeable Paving	FEH: 30 years: +40 %: 480 mins: Winter	130.126	130.096	0.183	0.197	14.1	128.517	0.000	0.0	65.289	0.000	963	36.718
Filter Drain - (Access Road)	FEH: 30 years: +40 %: 60 mins: Summer	129.795	129.100	0.645	0.100	144.8	66.859	0.000	87.3	2.000	173.745	12	55.381
Small Carpark Permeable Paving	FEH: 30 years: +40 %: 360 mins: Winter	133.473	133.473	0.121	0.131	1.2	8.233	0.000	0.0	4.783	0.000	636	49.709

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
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Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			

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AEG9910_SP4_HighPost_07:		Date:		
Atlas Planning		10/10/2026		
High Post Business Park, Salisbury, Wiltshire, SP4		Designed by:	Checked by:	Approved By:
Surface Water Drainage Network		JA	JC	OH
Report Details:		Company Address:		
Type: Stormwater Controls Summary				
Storm Phase: Phase				



Solar Field A Permeable Paving	FEH: 30 years: +40 %: 360 mins: Winter	132.908	132.904	0.119	0.131	1.9	12.889	0.000	0.0	7.503	0.000	635	49.999
Solar Field B Permeable Paving	FEH: 30 years: +40 %: 360 mins: Winter	133.543	133.539	0.120	0.131	1.9	12.889	0.000	0.0	7.503	0.000	635	49.999
Geocellular Storage Tank	FEH: 30 years: +40 %: 360 mins: Winter	127.594	127.594	0.594	0.594	57.0	379.526	0.000	8.3	0.000	265.917	479	25.754
Bioretention (4)	FEH: 30 years: +40 %: 480 mins: Winter	124.211	124.141	0.200	0.141	0.7	8.376	0.000	0.0	3.390	0.000	1106	88.415
Staff Carpark Access Permeable Paving	FEH: 30 years: +40 %: 480 mins: Winter	132.889	132.876	0.184	0.201	22.7	208.122	0.000	0.0	103.719	0.000	983	35.765

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
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	Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		
Company Address:			

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AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network		Date: 10/10/2026		 water, civils and environment	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Designed by: JA	Checked by: JC		Approved By: OH
		Company Address:			



**FEH: 100 years: Increase Rainfall (%): +45: Critical Storm Per Item: Rank By:
Max. Resident Volume**

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Max. Outflow (L/s)	Total Lost Volume (m³)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Bioretention 1	FEH: 100 years: +45 %: 960 mins: Winter	124.714	124.000	0.703	0.000	1.2	1.177	0.000	0.0	88.805	0.000	11	86.516
Bioretention 2	FEH: 100 years: +45 %: 960 mins: Winter	124.714	124.000	0.704	0.000	1.2	1.190	0.000	0.0	90.114	0.000	11	86.377
Bioretention 3	FEH: 100 years: +45 %: 960 mins: Winter	124.723	124.102	0.813	0.202	1.2	1.089	0.000	0.0	90.862	0.000	10	77.354
Main Carpark Permeable Paving 1	FEH: 100 years: +45 %: 960 mins: Winter	124.954	124.904	0.293	0.264	7.8	142.494	0.000	1.2	0.000	89.688	1337	8.406
Main Carpark Permeable Paving 2	FEH: 100 years: +45 %: 960 mins: Winter	124.953	124.903	0.293	0.263	7.8	142.038	0.000	1.2	0.000	90.862	1306	8.699
Main Carpark Permeable Paving 3	FEH: 100 years: +45 %: 960 mins: Winter	124.962	124.911	0.292	0.261	7.8	141.382	0.000	1.2	0.000	91.688	1281	6.092
Filter Drain - Solar Field A Staff	FEH: 100 years: +45 %: 480 mins: Winter	126.580	126.580	0.294	0.419	4.2	40.041	0.000	0.0	14.318	0.000	1375	41.648
Carpark Permeable Paving	FEH: 100 years: +45 %: 480 mins: Winter	130.603	130.592	0.248	0.268	30.9	297.348	0.000	0.0	111.213	0.000	1311	14.051
Main Soakaway	FEH: 100 years: +45 %: 240 mins: Winter	125.563	125.563	0.763	0.763	101.3	365.239	0.000	0.0	569.623	0.000	144	4.747
Roundabout Soakaway	FEH: 100 years: +45 %: 60 mins: Winter	118.762	118.762	0.762	0.762	90.2	86.912	0.000	0.0	97.285	0.000	64	5.494
Filter Drain - Roundabout	FEH: 100 years: +45 %: 15 mins: Summer	120.063	119.599	0.586	0.147	195.6	10.195	0.000	150.4	5.600	79.392	1	44.058
Carpark 2 Permeable Paving	FEH: 100 years: +45 %: 480 mins: Winter	130.187	130.161	0.244	0.262	17.9	171.135	0.000	0.0	65.687	0.000	1276	15.732
Filter Drain - (Access Road)	FEH: 100 years: +45 %: 60 mins: Summer	130.052	129.110	0.902	0.110	191.8	91.717	0.000	104.1	2.140	231.544	13	38.792
Small Carpark Permeable Paving	FEH: 100 years: +45 %: 360 mins: Winter	133.516	133.517	0.164	0.174	1.6	11.067	0.000	0.0	4.837	0.000	846	32.399

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
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	Company Address:		
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase			

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AEG9910_SP4_HighPost_07:		Date:		
Atlas Planning		10/10/2026		
High Post Business Park, Salisbury, Wiltshire, SP4		Designed by:	Checked by:	Approved By:
Surface Water Drainage Network		JA	JC	OH
Report Details:		Company Address:		
Type: Stormwater Controls Summary				
Storm Phase: Phase				



Solar Field A Permeable Paving	FEH: 100 years: +45 %: 360 mins: Winter	132.950	132.948	0.161	0.174	2.5	17.341	0.000	0.0	7.573	0.000	847	32.728
Solar Field B Permeable Paving	FEH: 100 years: +45 %: 360 mins: Winter	133.585	133.583	0.162	0.175	2.5	17.341	0.000	0.0	7.574	0.000	847	32.728
Geocellular Storage Tank	FEH: 100 years: +45 %: 360 mins: Winter	127.771	127.771	0.771	0.771	71.4	492.480	0.000	9.5	0.000	309.491	550	3.657
Bioretention (4)	FEH: 100 years: +45 %: 480 mins: Winter	124.255	124.188	0.244	0.188	0.9	10.446	0.000	0.0	3.510	0.000	1266	85.551
Staff Carpark Access Permeable Paving	FEH: 100 years: +45 %: 480 mins: Winter	132.951	132.941	0.247	0.267	28.8	276.953	0.000	0.0	104.186	0.000	1303	14.521

AEG9910_SP4_HighPost_07: Atlas Planning High Post Business Park, Salisbury, Wiltshire, SP4 Surface Water Drainage Network	Date: 10/10/2026		
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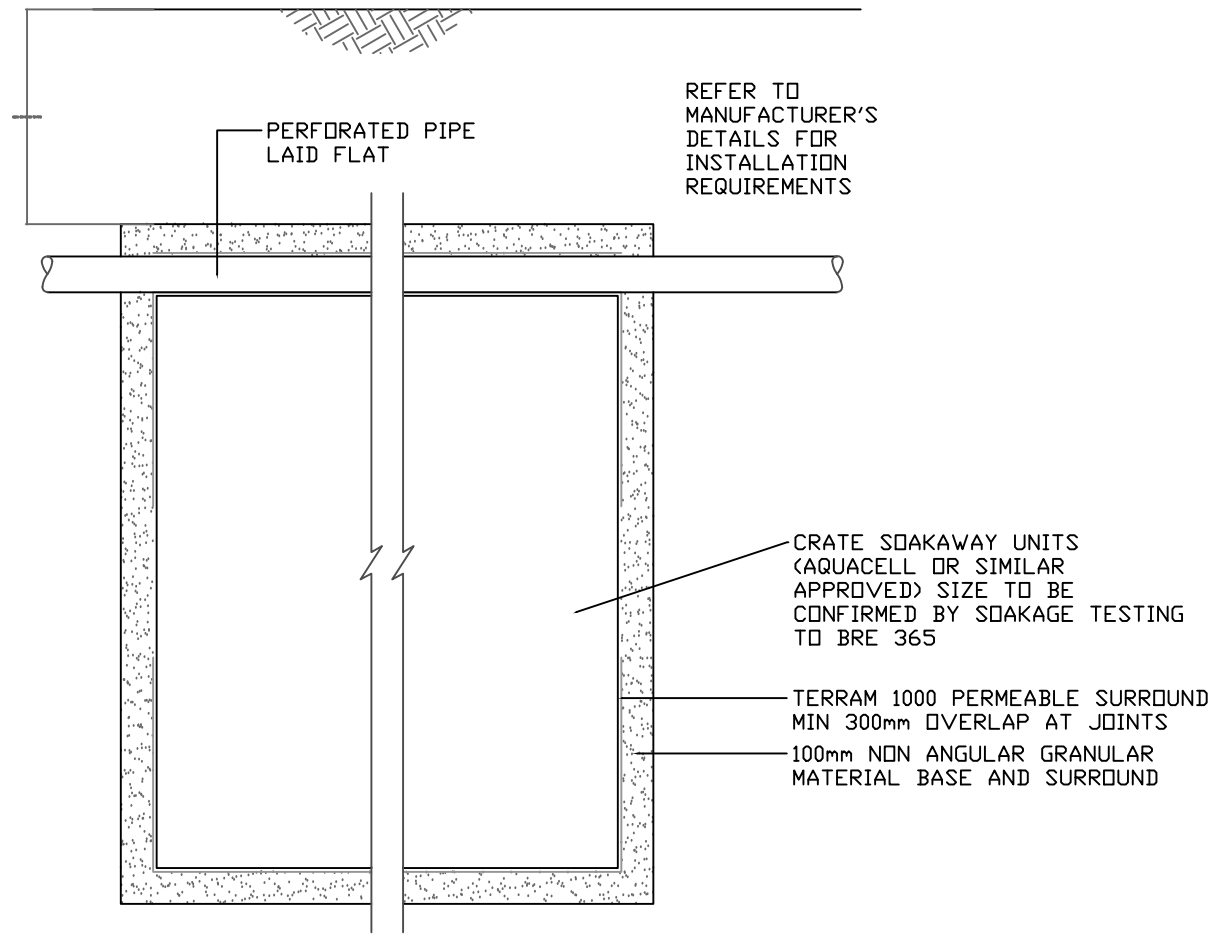
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Appendix E – Standard Details

DO NOT SCALE THIS DRAWING. USE FIGURED DIMENSIONS ONLY.
THE CONTRACTOR MUST CHECK & VERIFY ALL DIMENSIONS ON SITE.
ANY DISCREPANCIES MUST BE REPORTED IMMEDIATELY TO THE ENGINEER
FOR CLARIFICATION BEFORE PROCEEDING.
THIS DRAWING IS COPYRIGHT AND OWNED BY AEGAEA.

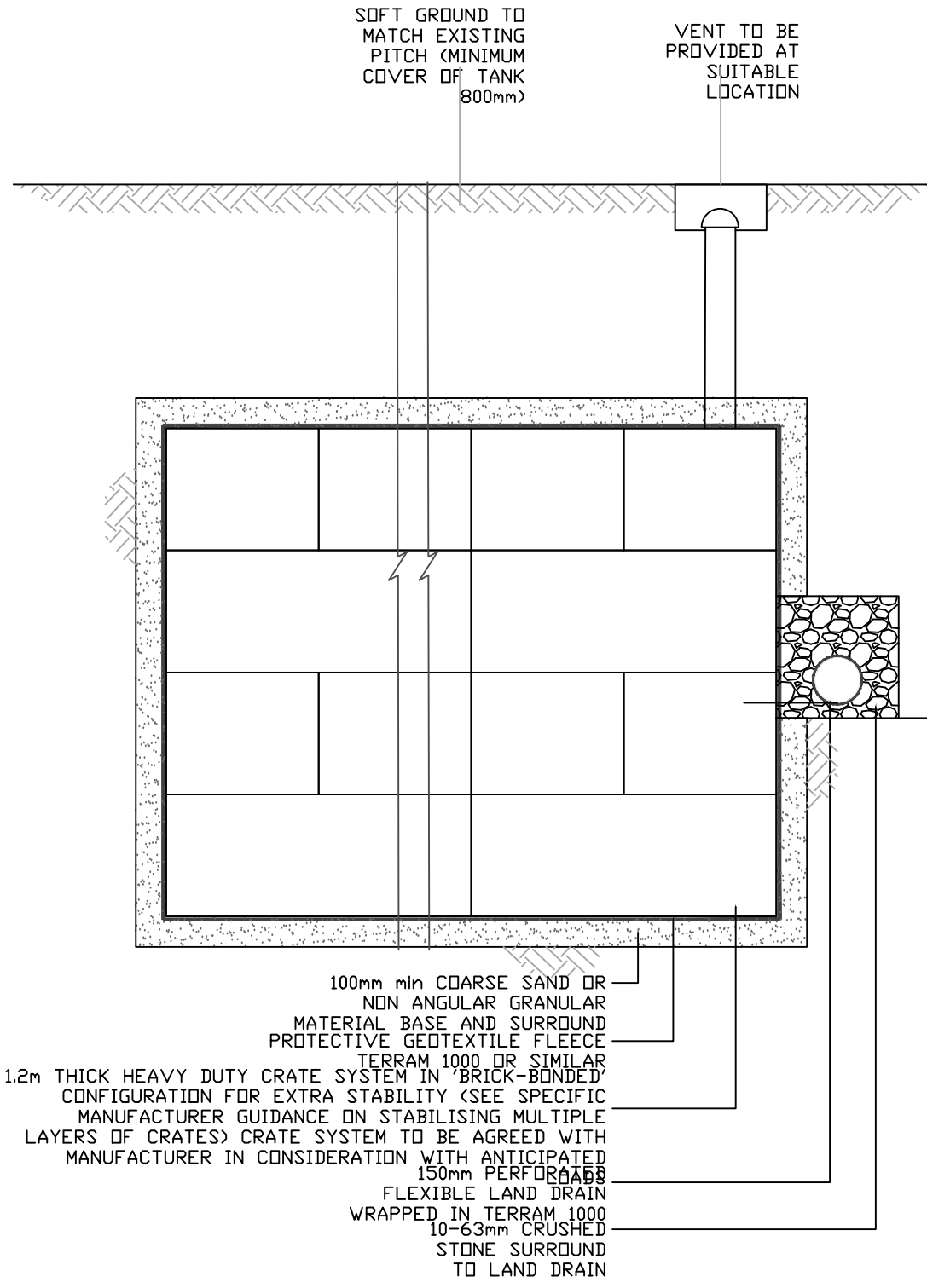
SAFETY, HEALTH AND ENVIRONMENTAL INFORMATION
REFER TO THE RELEVANT CONSTRUCTION (DESIGN AND MANAGEMENT)
DOCUMENTATION WHERE APPLICABLE.
IT IS ASSUMED THAT ALL WORKS ON THIS DRAWING WILL BE CARRIED OUT BY
A COMPETENT CONTRACTOR, WORKING WHERE APPROPRIATE TO AN
APPROVED METHOD STATEMENT.

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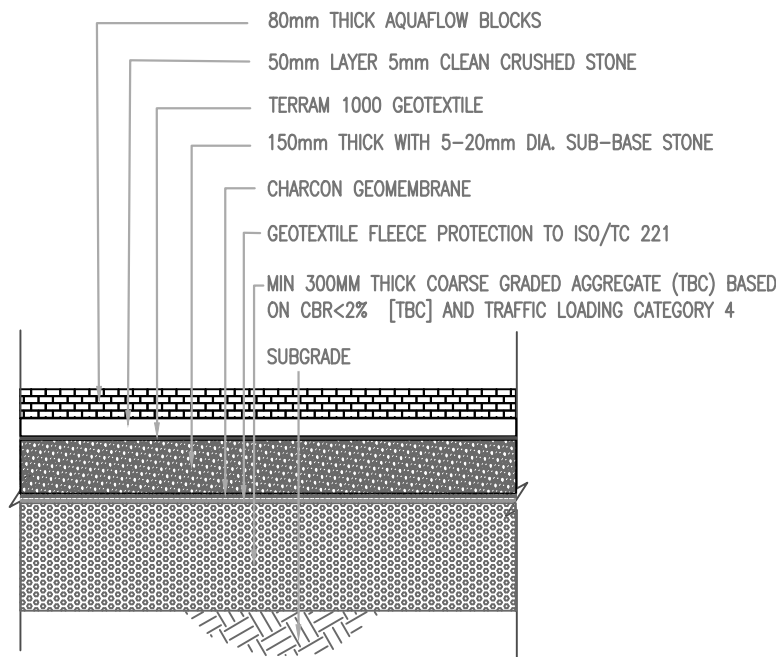
CRATE SOAKAWAY DETAIL.

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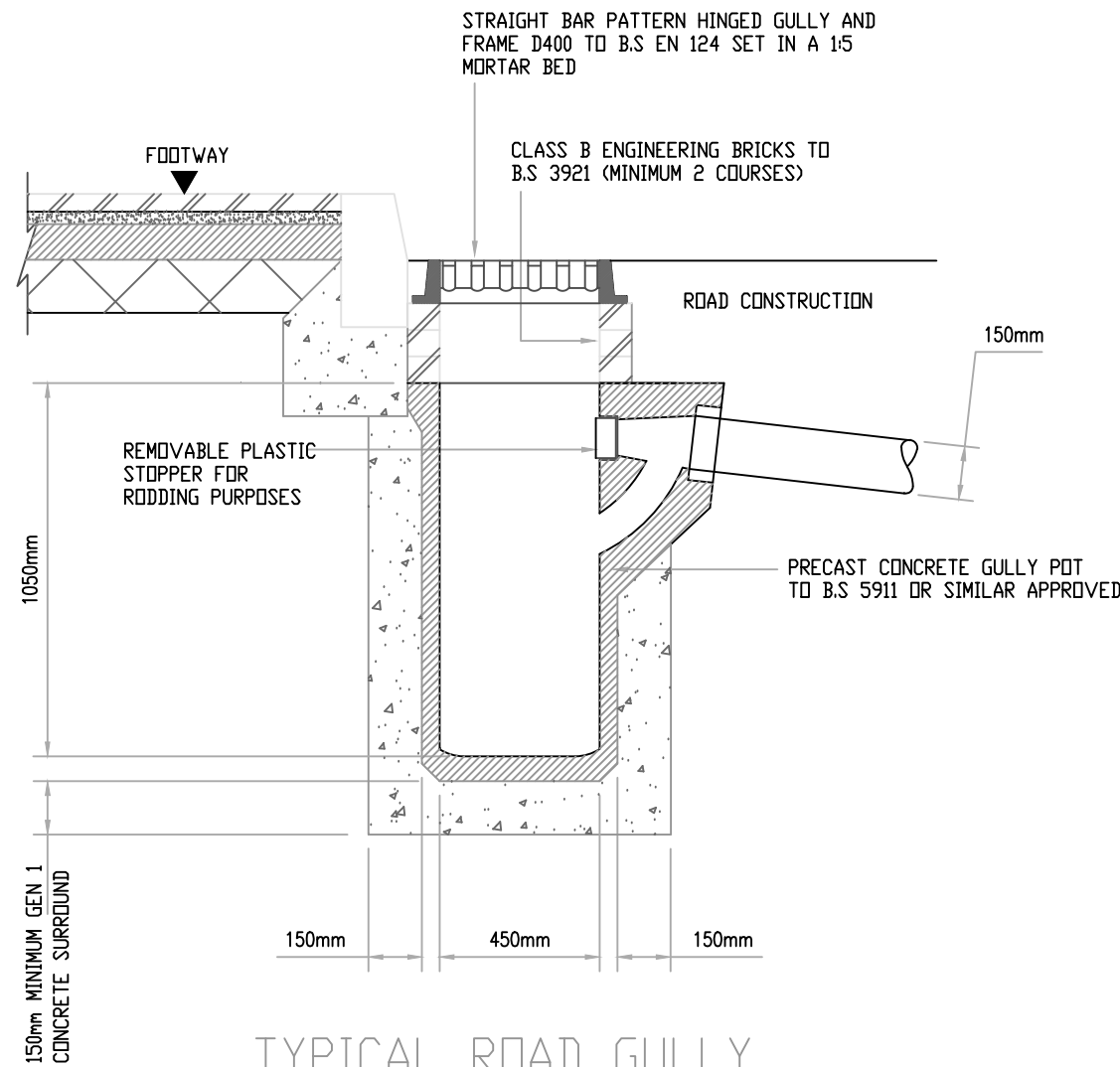
SECTION THROUGH
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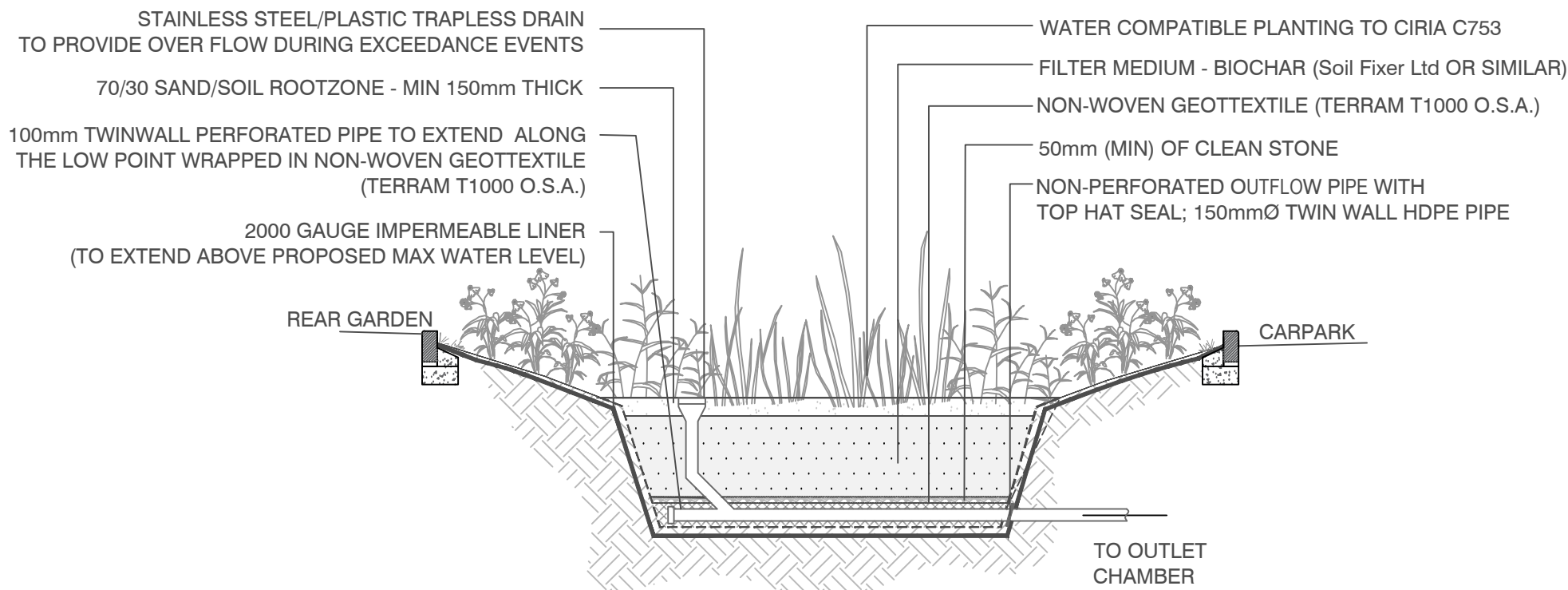


TYPICAL PERMEABLE PAVING
WITHOUT SUB-BASE STORAGE

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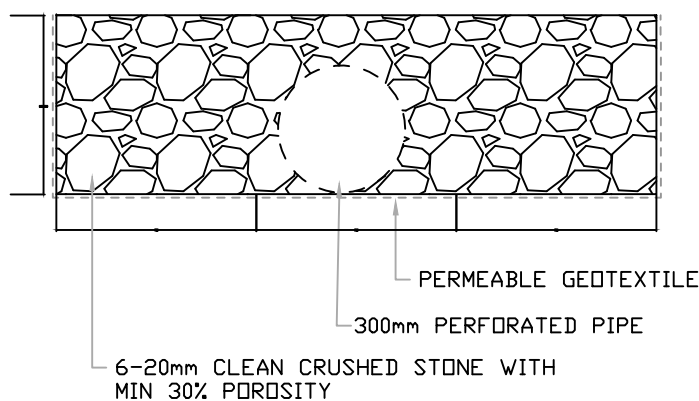


TYPICAL ROAD GULLY.



TYPICAL RAIN GARDEN / BIORETENTION AREA

(SCALE 1:50)



FILTER DRAIN

(1:20)

1.0	10/07/2026	FIRST ISSUE	JA
Rev	Date	Description	By

Client
SPITFIRE ENTERPRISES

Project
STANDARD DETAILS

Title
HIGH POST BUSINESS PARK, SALISBURY, WILTSHIRE, SP4 6AT.

Project No.		Drawing No.		Revision
AEG9910		DR02		1.0
Drawn	Checked	Approved	Date	Scale @ A1
JA	JC	OH	10/07/2026	SEE SCALE

Drawing Status
FOR PLANNING ONLY