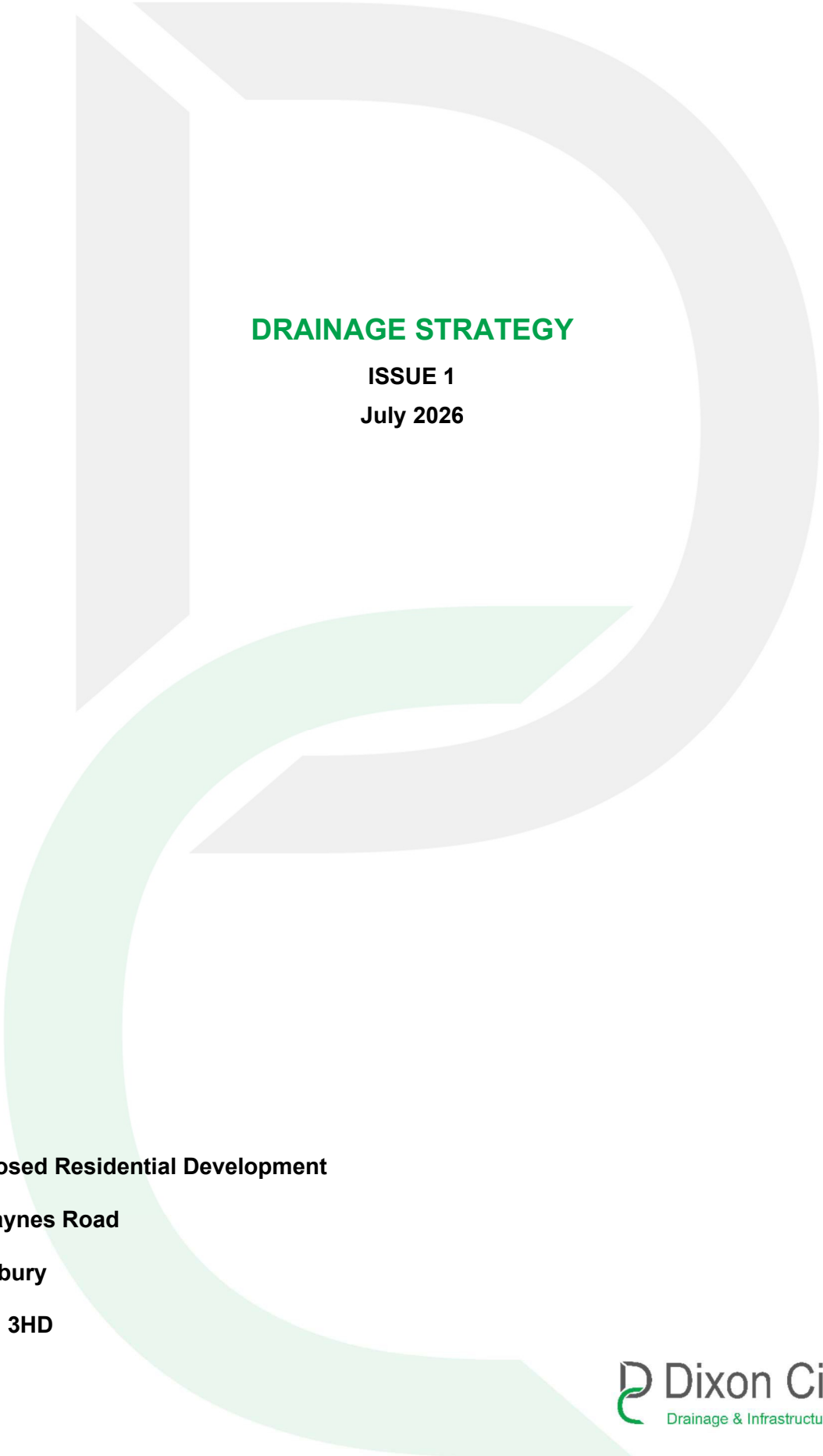




DRAINAGE STRATEGY

ISSUE 1
July 2026

Proposed Residential Development

51 Haynes Road

Westbury

BA13 3HD

Document Control

Client	Duncan Wilkins		
Report title	Drainage Strategy		
Report reference	DC1265-REP01		
Report status	For Approval		
Prepared by	A.Dixon BSc(Hons) CEng MICE		
Document check	ZW		
Issue Number/status/Date	1	Draft	02-07-2026

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APPENDIX	TITLE	PRODUCED BY
Appendix A – Drawings by DC	SW Drainage Strategy	Dixon Civils
Appendix B – Drawings by others	Proposed Site Plan	Atelier 17
Appendix C – Sewer Records		
Appendix D – Brownfield Runoff Calculations		
Appendix E – Infodrainage Design Calculations		

1.0 Introduction

This drainage strategy has been prepared on behalf of Mr Duncan Wilkins in support of a planning application for a proposed 4 plot residential development at 51 Haynes Road, Westbury.

This report will provide flood risk information and demonstrate how the appropriate means of sustainable surface water drainage will be achieved for the development without worsening flood risk in the area or putting itself at risk of flooding in line with national guidance.

The development proposal comprises the demolition of a commercial building and replacement with four terraced dwellings with associated gardens. The existing parking area for the commercial site will be retained for dwelling parking.

Access to the site is off Haynes Road as existing.

Refer to the Proposed Block Plan (A17 26 09 P01) enclosed within Appendix B of this report.

2.0 The Existing Site

The application site covers 320m² (0.032ha) of land occupied by a commercial building, garage and hardstanding.

The site is centred on Ordnance Survey grid reference ST 87264 51144 and co-ordinates: E387264, N151144. The post code for the site is BA13 3HD.

The site is located within flood zone 1 and is bounded by a mix of commercial, retail and residential properties.

2.1 Existing ground conditions

British Geological Survey (BGS) mapping indicates that the site is located over the Gault Formation - Mudstone.

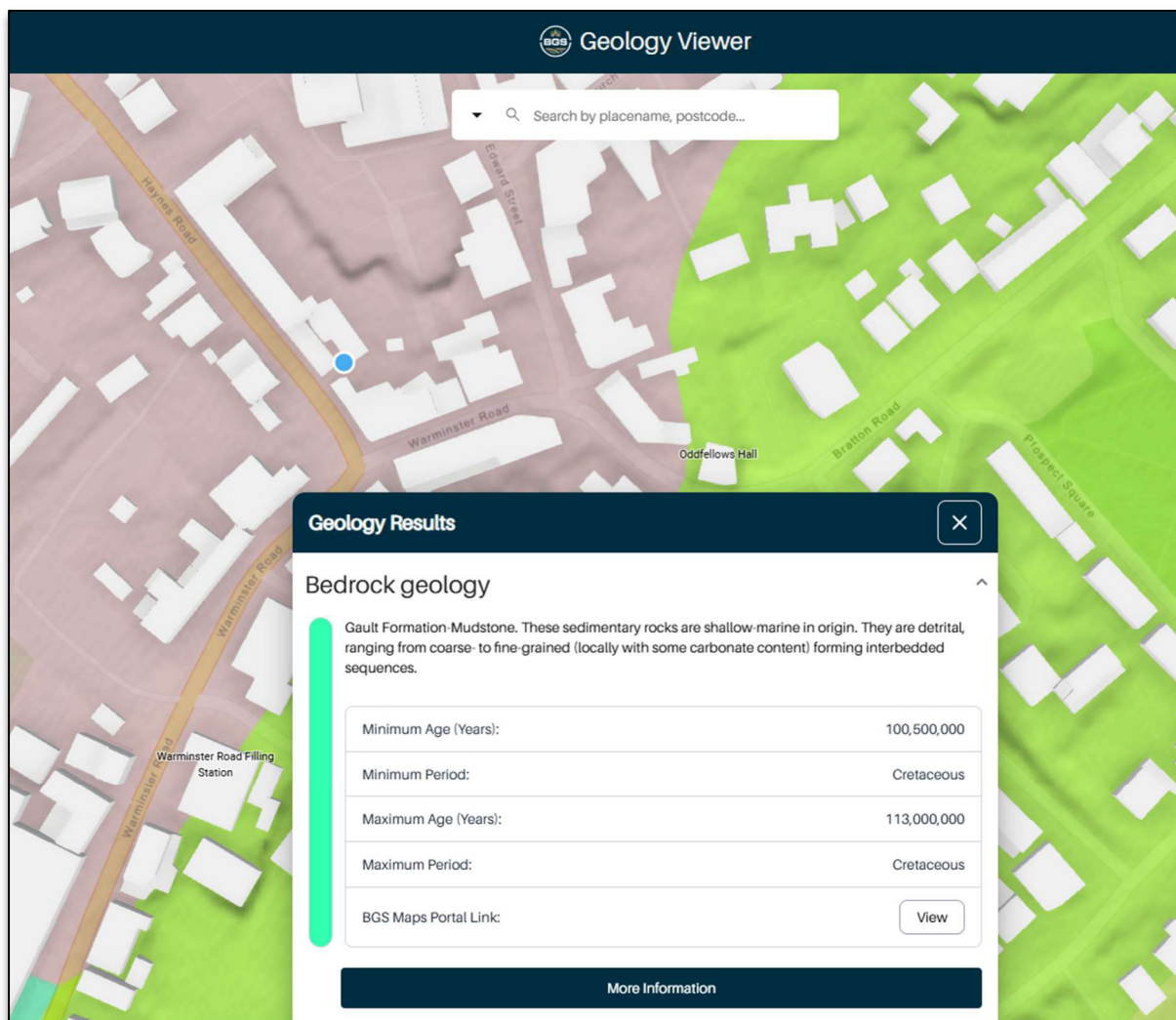


Figure 1: Bedrock Geology - Extract from British Geological Survey Geology of Britain Viewer
Source: <https://www.bgs.ac.uk/map-viewers/bgs-geology-viewer>

2.2 Existing Watercourses and Drainage Features

There are no watercourses within the vicinity of the development. The nearest watercourse to the site is the Bliss Brook, approximately 1km to the west.

Asset records obtained from Wessex Water indicate the presence of public combined water sewers within Haynes Road fronting the site. A public foul water sewer passes through the shared access to the site, into which the existing site drains. This foul water sewer joins the combined sewer within Edward Street to the north. There are no surface water sewers within the vicinity of the site. Refer to Wessex Water sewer records within Appendix C of this report.

2.3 Existing surface water drainage arrangement

The existing building, garage, and hardstanding areas are partially drained, with surface water currently discharging freely to the surrounding land, the foul sewer, and the adjacent highway.

The front section of the existing building has no gutters, resulting in roof runoff discharging directly onto the highway footpath. The rear section of the building drains into the foul sewer. Surface water from the external hardstanding is collected by gullies and also discharged into the foul sewer. The garage roof is assumed to drain onto the adjacent hardstanding, where runoff enters a yard gully before discharging into the foul drainage network.

The existing drainage system does not incorporate any Sustainable Drainage Systems (SuDS) features.

The current brownfield runoff rates for the existing site are provided in Appendix D and section 4.0 of this report.

3.0 Flood Risk Summary

As the site lies within Flood Zone 1 and covers an area of less than 1 hectare, a full Flood Risk Assessment is not required. Nevertheless, potential flood risks affecting the site have been considered and are summarised in the table below.

3.1 Summary of Flood Risk

Source of Potential Flooding to the Development Site	Flood Risk	Mitigation/Comments
Fluvial	Low	EA mapping indicates that the site is located within Flood Zone 1 (Low Risk).
Public Sewers / Drains	Low	Wessex Water records indicate the present of sewers within Haynes Road and adjacent shared access, any sewer flooding would be contained within the kerbed highway falling away from the site and not result in flooding of the building.
Overland Flow	Low	EA mapping indicates the site is at Low risk of flooding from this source.
Groundwater	Low	Wiltshire Council's SFRA Data Explorer indicates the site to be in a zone at negligible risk from groundwater flooding due to the nature of the local geological deposits. Flooding from groundwater emergence is therefore considered low.
Artificial Sources	Low	Wiltshire Council's SFRA Data Explorer indicates that the site is not located within an area at risk of flooding from this source.

4.0 Surface Water Drainage Proposals

Refer to DC Drawing 1265-01-01 [*Proposed Drainage Strategy*] in Appendix A.

This section of the report addresses how flood risk generated by redeveloping the site will be managed using sustainable control measures to ensure that flood risks are not increased on and off the site and how surface water is managed post development.

Wiltshire Council requirements for brownfield sites, where disposal of surface water via infiltration is not viable, requires post development discharges to provide a 30% betterment over existing discharges for all storm events between the 1 in 1year and 1 in 100year return period events.

Existing run off from the current impermeable 185m² (0.0185Ha) catchment are summarised in the table below. The 30% reduction in flow rates for the 1:1, 1:30 and 1:100 year events are included within this table.

Storm Return Period	Average Rainfall Intensity (i) 15 Minute Winter Profile	Flow Rate (l/s)	30% reduction target (l/s)
1 year	31.2	1.88	1.32
30 year	76.6	4.61	3.23
100 year	99.5	5.99	4.2

4.1 Drainage Hierarchy

The surface water drainage strategy for the site has been prepared according to the drainage discharge hierarchy from CIRIA C753 The SuDS Manual, where the first available option will be utilised as follows.

Technique		Summary
1	Infiltration to the maximum extent that is practical	Mudstone soils and unavailable land greater than 5m from buildings prevent use of infiltration.
2	To surface water body	Not available
3	To a surface water sewer, highway drain or another drainage system	Not available
4	To a combined sewer	Proposed solution – Flows attenuated prior to flow restricted discharge to the existing sewer connection.

4.2 Infiltration Design

Poor infiltration properties of the clay mudstone soils below the site precludes the use of soakaways and infiltration devices. The compact nature of the terraced town centre site also does not offer the opportunity to locate soakaways 5m away from property or highways.

4.3 Attenuation Design

Drainage calculations demonstrate that a minimum attenuation storage volume of 5.67 m³ is required to manage surface water runoff from the proposed dwelling roof for all rainfall events up to and including the 1 in 100-year event, incorporating a 45% allowance for climate change, with discharge restricted to a maximum rate of 1.0 l/s.

Wiltshire Council's guidance requires post-development runoff rates from brownfield sites to achieve a minimum 30% betterment over the existing runoff rate. However, as the site currently discharges surface water to a combined foul and surface water sewer, the proposed development provides an opportunity to deliver a greater level of betterment by reducing the volume and rate of surface water entering the public sewer network.

Accordingly, the proposed drainage strategy limits the post-development discharge rate to 1.0 l/s for all events up to and including the 1 in 100-year plus 45% climate change event. This represents an approximate 87% reduction in peak discharge compared with the existing site, significantly exceeding Wiltshire Council's minimum betterment requirement and supporting the wider objective of reducing surface water contributions to the combined sewer system.

Refer to the proposed drainage calculations enclosed within Appendix E of this report.

Storm Return Period	Existing Flow Rate (l/s)	30% reduction target (l/s)	Proposed outflow rate (l/s)	Betterment Provided (%)
1 year	1.88	1.32	0.4	78.7
30 year	4.61	3.23	0.7	84.8
100 year	5.99	4.2	0.8	86.6
100 year +45% climate change increase	5.99+	4.2	1.0	>83

4.4 Design Exceedance

Should the drainage system and SuDS fail under extreme events or blockages, floodwaters will follow elevated design levels directing surface water flow away from the building toward Haynes Road where it will follow the road topography to the south and enter the highway network, mirroring the existing development arrangement.

Overland design exceedance arrows are indicated on the proposed drainage strategy layout drawing 1265-01-01 in appendix A.

4.5 Water Quality

Roof water is considered fairly uncontaminated and as such no additional pollutant mitigation systems beyond the cellular attenuation are proposed, before discharging to the combined sewer network.

4.6 Adoption

The onsite drainage components will remain private and be the maintenance responsibility of the property owners.

4.7 Maintenance

Regular inspection and maintenance are important for the effective operation of the SuDS systems as designed. The regular maintenance of SuDS should be carried out in line with C753 The SUDS Manual (CIRIA, November 2015) or as per manufacturer's specification.

Pipework and Catchpits

It is not envisaged that silt build up within the pipework systems will require rigorous maintenance. Notwithstanding this, a suitable maintenance regime for the systems will comprise of routine inspection (every six months) and silt removal as necessary.

Cellular Attenuation Tank

Silt build up within the cellular crate systems should not require a rigorous maintenance regime so long as silt is removed from upstream catch pits and inspection chambers on a regular basis. Camera access can be gained via inspection chambers and inlet pipework to the tank. Silt removal can be achieved by jetting the inspection tunnels, where a standard jet head with rear facing nozzles should be used. The head should be fed to the far end of the crate tunnel via the nearest inspection chamber, activated and retracted. As the nozzle is removed, debris will be swept back into the inspection chamber where it can then be removed with the use of a standard gully sucker.

Drainage Element	Schedule	Maintenance Requirement	Frequency
Pipework and Catchpits	Regular	Inspect for accumulation of silt	Every six months
		Inspect for debris and litter	
		Inspect inlets and outlets for blockages	
	Occasional	Remove debris and litter	As required
Cellular Attenuation Storage	Regular	Remove silt	Monthly for 3 months, then annually
		Inspect and identify any areas that are not operating correctly	
		Remove debris from the catchment surface	
	Remedial	Remove sediment from pre-treatment structures and internal forebays	Monthly
		Repair and rehabilitate inlets, outlets, overflows and vents	Annually, or as required
	Monitoring	Inspect inlets, outlets, overflows and vents to ensure they are operating as designed	As required
		Survey inside of tank for sediment build-up and remove as necessary	Annually
			Every 5 years, or as required

5.0 Conclusions

The proposed development will reduce the extent of impermeable surfaces across the site by replacing existing hardstanding with landscaped garden areas, resulting in an overall improvement to surface permeability.

Surface water runoff from the proposed dwellings will be managed using sustainable drainage systems (SuDS). All runoff will be conveyed to a cellular attenuation tank designed to accommodate the 1 in 100-year rainfall event, including a 45% allowance for climate change, in accordance with local planning policy.

As a result of the proposed redevelopment, surface water discharge to the combined sewer network will be reduced by up to 80% compared with the existing site.

The proposed SuDS features and all associated surface water drainage infrastructure will remain under private ownership and will be the responsibility of the property owner to inspect and maintain.

Subject to the implementation and ongoing maintenance of the drainage measures described within this report, it is concluded that the proposed development can proceed without increasing flood risk either within the site or to the surrounding area.

Dixon Civils Ltd

Issue 1 - July 2026

APPENDIX A

Drawings by Dixon Civils

Drawing ref. 1284-01-01

Surface Water Drainage Strategy



KEY:

- Existing Foul Water Sewer
- Existing Private Foul Water Drain to be removed
- Existing Private Surface Water Drain to be removed
- Proposed Private Foul Water Drain
- Proposed Private Surface Water Drain
- Rodding Eye
- Surface Water Catchpit Chamber
- Flood Flow Route (Over 1:100yr+45% storm event)
- Site Boundary

- GENERAL NOTES:**
- DO NOT SCALE FROM THIS DRAWING. WORK ONLY FROM STATED DIMENSIONS OR SETTING OUT COORDINATES.
 - ALL DIMENSIONS AND LEVELS ARE IN METRES AND RELATE TO ORDNANCE SURVEY LEVELS.
 - THIS DRAWING SHALL BE READ IN CONJUNCTION WITH ALL RELEVANT SCHEME DRAWINGS AND SPECIFICATIONS.
 - ANY WORKS CONSTRUCTED PRIOR TO THE ISSUE OF LOCAL AUTHORITY APPROVAL IS ENTIRELY AT THE RISK OF THE CONTRACTOR/CLIENT.
 - THIS DRAWING IS FOR PLANNING APPROVAL PURPOSES ONLY.

Rev	Description	Dm	Date
1	FIRST ISSUE	AD	01.06.26

REVISIONS

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Mr D Wilkins

51 Haynes Road
Westbury

Proposed Drainage Strategy

DATE: JULY 2026 DRAWN: AD
DRAWING No: 1265-01-01 REV: - SCALE: 1:100 @ A1

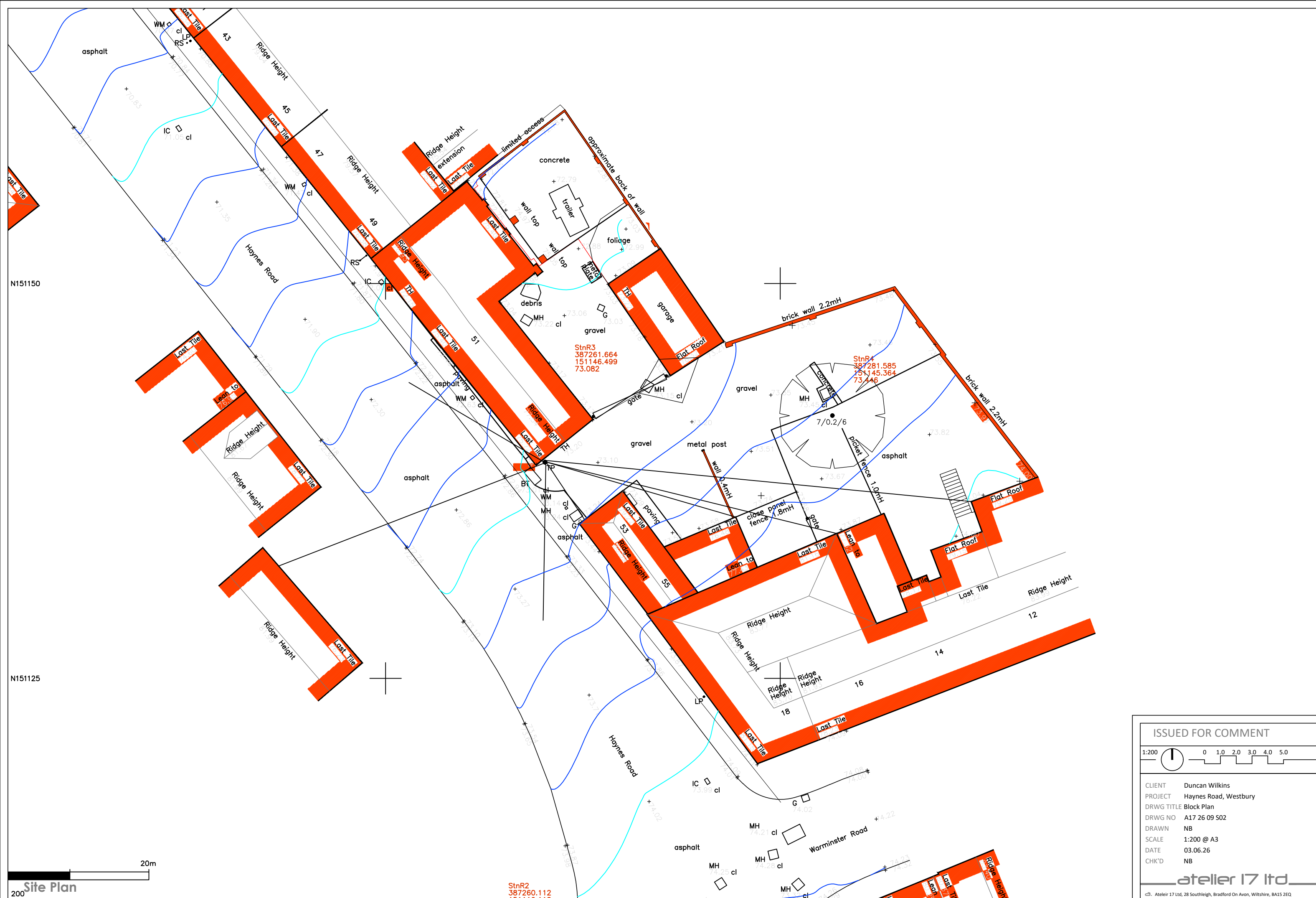
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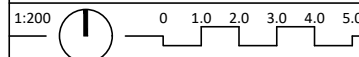
APPENDIX B

Drawings by others

A17 26 09 S02	Block Plan (Atelier 17)
A17 26 09 P01	Proposed Block Plan (Atelier 17)



ISSUED FOR COMMENT



CLIENT Duncan Wilkins
PROJECT Haynes Road, Westbury
DRWG TITLE Block Plan
DRWG NO A17 26 09 S02
DRAWN NB
SCALE 1:200 @ A3
DATE 03.06.26
CHK'D NB

atelier 17 ltd

Atelier 17 Ltd, 28 Southleigh, Bradford On Avon, Wiltshire, BA15 2EQ
nigel.a.w.bedford@gmail.com www.atelier17.co.uk F.(01225) 862900

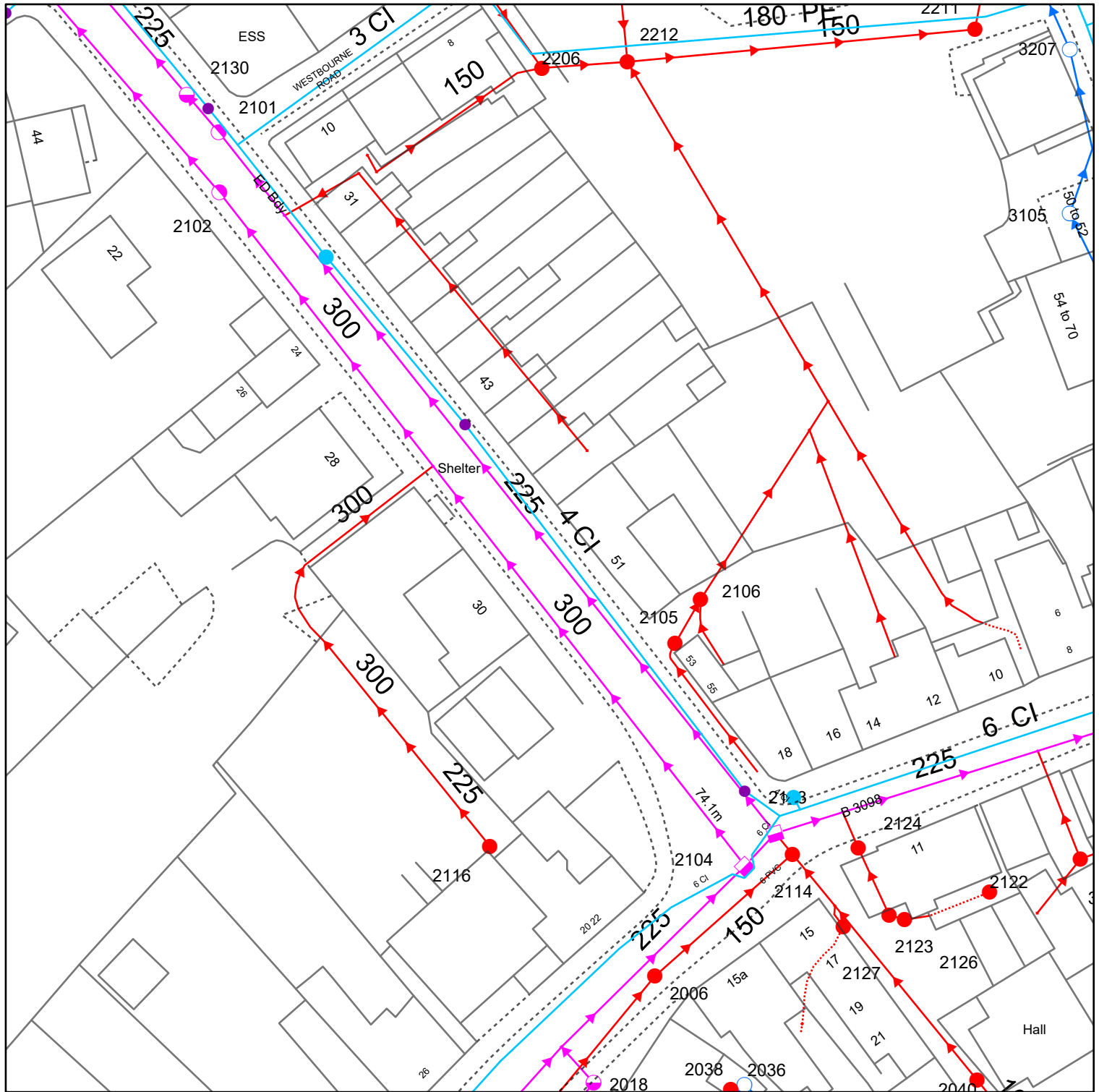
This drawing is copyright. Responsibility is not accepted for errors made by others in scaling from this drawing. All dimensions to be checked on site prior to commencement of work. All construction information should be taken from figured dimensions only. Only to be used on the site for which designed. Any discrepancies are to be reported to the architect.



APPENDIX C

Wessex Water Sewer Records

Wessex Water Network Map

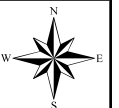


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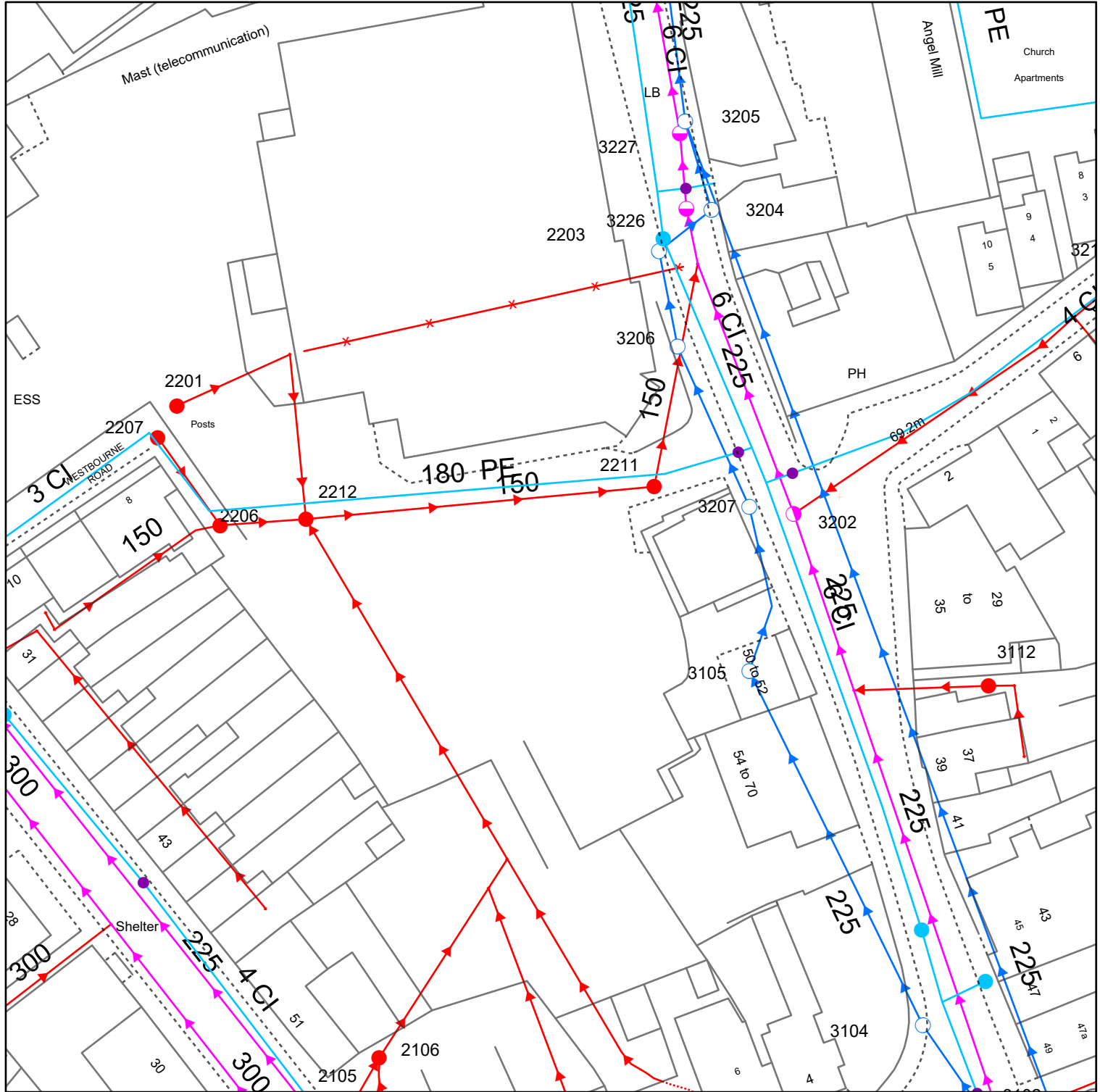
WATER MAINS		SEWERS		STRATEGIC		PUBLIC		PRIVATE		SECTION 104		OTHER WESSEX PIPES		NON-WESSEX / UNKNOWN	
	Distribution		Foul		Surface								Rising Mains		Private Rising Mains
	Washout		Surface		Combined								Standby Rising Mains		Culverted Watercourse
	Raw Water		Abandoned										EDM Effluent Disposal		Highway Drain
	Abandoned												Overflow		Use Unknown
	Private												Syphon		Status Unknown
Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow). Some styles of line and symbol are shown on the key in sample/typical colours.															
FITTINGS		STRUCTURES													
	Hydrant		Manhole - Foul		Pumping Station - Surface										
	Other		Manhole - Surface		Pumping Stn - Foul/Combined										
			Manhole - Combined		Gully										
			Inlet		Outfall										
			Lamphole		Vent Column										
			Bifurcation - Foul		Rodding Eye										
			Bifurcation - Surface		Catchpit										
			Bifurcation - Combined		Flushing Chamber										
			Combined Sewage Overflow		Soakaway										
					Non Return Valve										
					Air Valve		Hatch Box		Washout						
OTHER STRUCTURES Attenuation Tank Chamber Storage Tank Tunnel Interceptor															
Wessex Water YTL GROUP															

Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526633 or e-mail: developer.enquiries@wessexwater.co.uk to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.

Date: 13/04/2026
Centre: 387248E, 151147N
Scale: 1:625
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Wessex Water Network Map



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WATER MAINS	SEWERS	STRATEGIC PUBLIC	PRIVATE	SECTION 104	OTHER WESSEX PIPES	NON-WESSEX / UNKNOWN
- Distribution - Washout - Raw Water - Abandoned - Private	- Foul - Surface - Combined - Abandoned	- Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow). - Some styles of line and symbol are shown on the key in sample/typical colours.	- Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow). - Some styles of line and symbol are shown on the key in sample/typical colours.	- Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow). - Some styles of line and symbol are shown on the key in sample/typical colours.	- Rising Mains - Standby Rising Mains - EDM - Effluent Disposal - Overflow - Syphon	- Private Rising Mains - Culverted Watercourse - Highway Drain - Use Unknown - Status Unknown
FITTINGS	STRUCTURES	STRUCTURES	STRUCTURES	STRUCTURES	OTHER STRUCTURES	OTHER STRUCTURES
- Hydrant - Other	- Manhole - Foul - Manhole - Surface - Manhole - Combined - Inlet - Outfall - Lamphole - Bifurcation - Foul - Bifurcation - Surface - Bifurcation - Combined - Combined Sewage Overflow	- Manhole - Foul - Manhole - Surface - Manhole - Combined - Inlet - Outfall - Lamphole - Bifurcation - Foul - Bifurcation - Surface - Bifurcation - Combined - Combined Sewage Overflow	- Manhole - Foul - Manhole - Surface - Manhole - Combined - Inlet - Outfall - Lamphole - Bifurcation - Foul - Bifurcation - Surface - Bifurcation - Combined - Combined Sewage Overflow	- Manhole - Foul - Manhole - Surface - Manhole - Combined - Inlet - Outfall - Lamphole - Bifurcation - Foul - Bifurcation - Surface - Bifurcation - Combined - Combined Sewage Overflow	- Attenuation Tank - Storage Tank - Chamber - Tunnel - Interceptor	- Attenuation Tank - Storage Tank - Chamber - Tunnel - Interceptor
Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526333 or e-mail: developer.enquiries@wessexwater.co.uk to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.						
Date: 01/07/2026 Centre: 387285E, 151199N Scale: 1:625 (when printed at A4 size)						

APPENDIX D

Calculations

Existing Brownfield Runoff

Runoff from Existing Impermeable Areas

The industry-standard Modified Rational Method has been used in the calculation of runoff rates from existing impermeable areas which are to be provided with a new drainage system to serve the increased impermeable catchment area.

It uses the following equation to calculate the peak rate of runoff generated by each catchment area of the development.

$$Q_p = 2.78 C_v C_R i A$$

Where:

2.78 = Coefficient which accounts for the differences in units used for the inputs and outputs of the equation.

C_v = 0.9 Volumetric Runoff

C_R = 1.3 (Routing Coefficient)

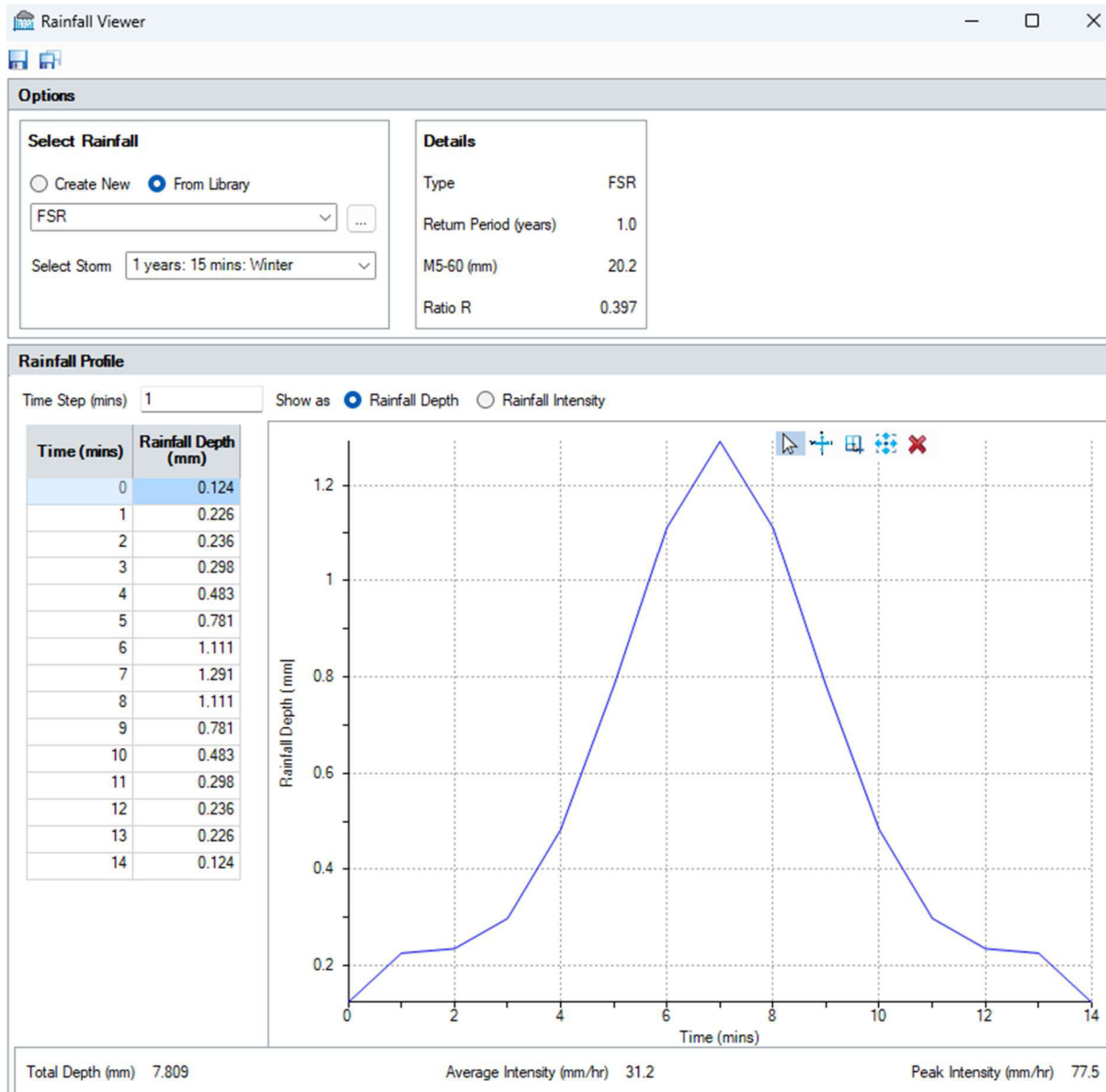
i = Rainfall Intensity (mm/hr)

A = Area (0.0185ha)

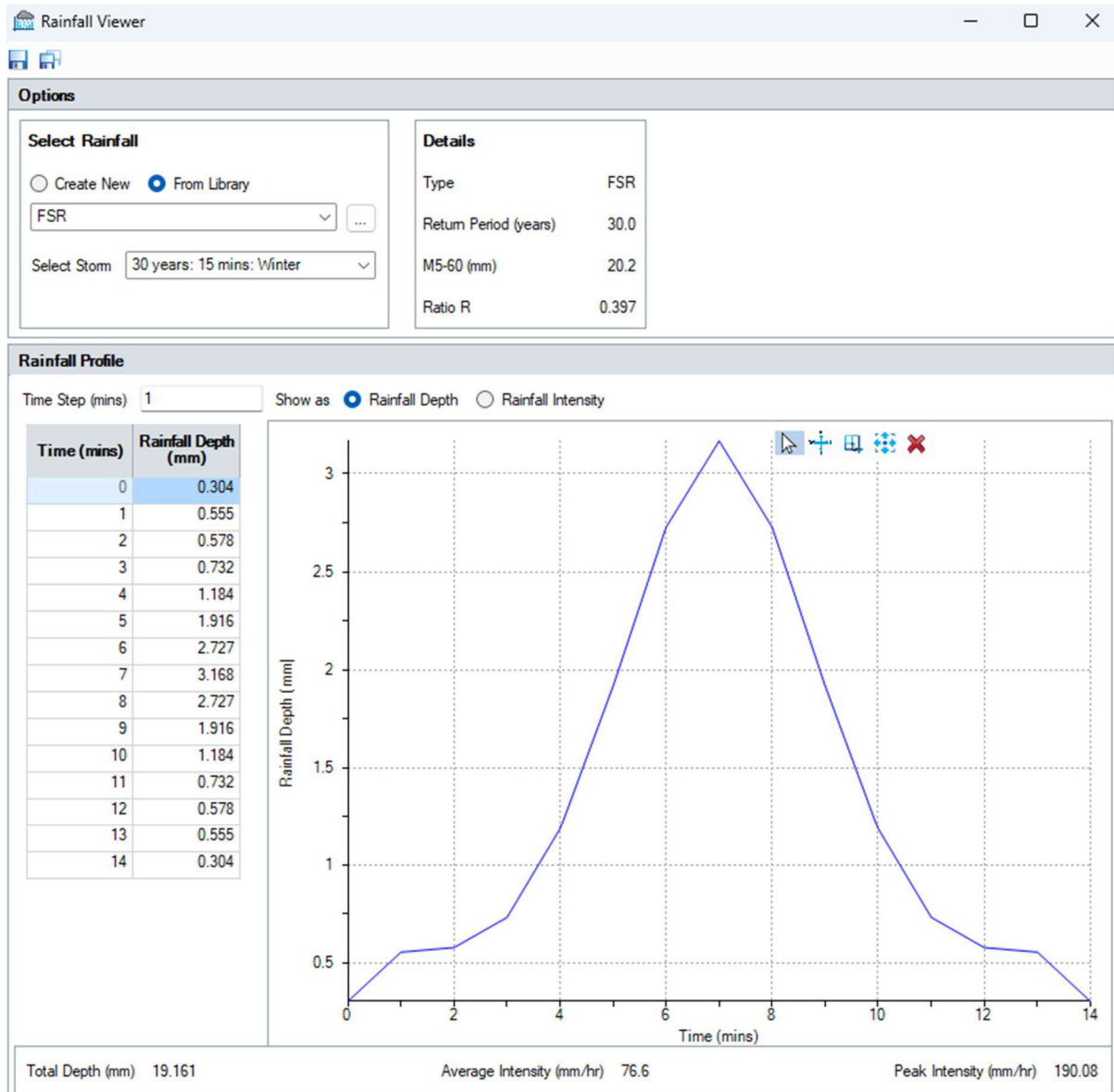
Rainfall intensity data has been obtained from the Infodrainage design software by Autodesk, peak rainfall intensities have been generated for a range of storm return periods for a 15-minute duration winter storm. Output from the software is shown below along with a table summarising the average rainfall intensity.

Storm Return Period	Average Rainfall Intensity (i) 15 Minute Winter Profile	Flow Rate (Qp)
1 year	31.2	1.88
30 year	76.6	4.61
100 year	99.5	5.99

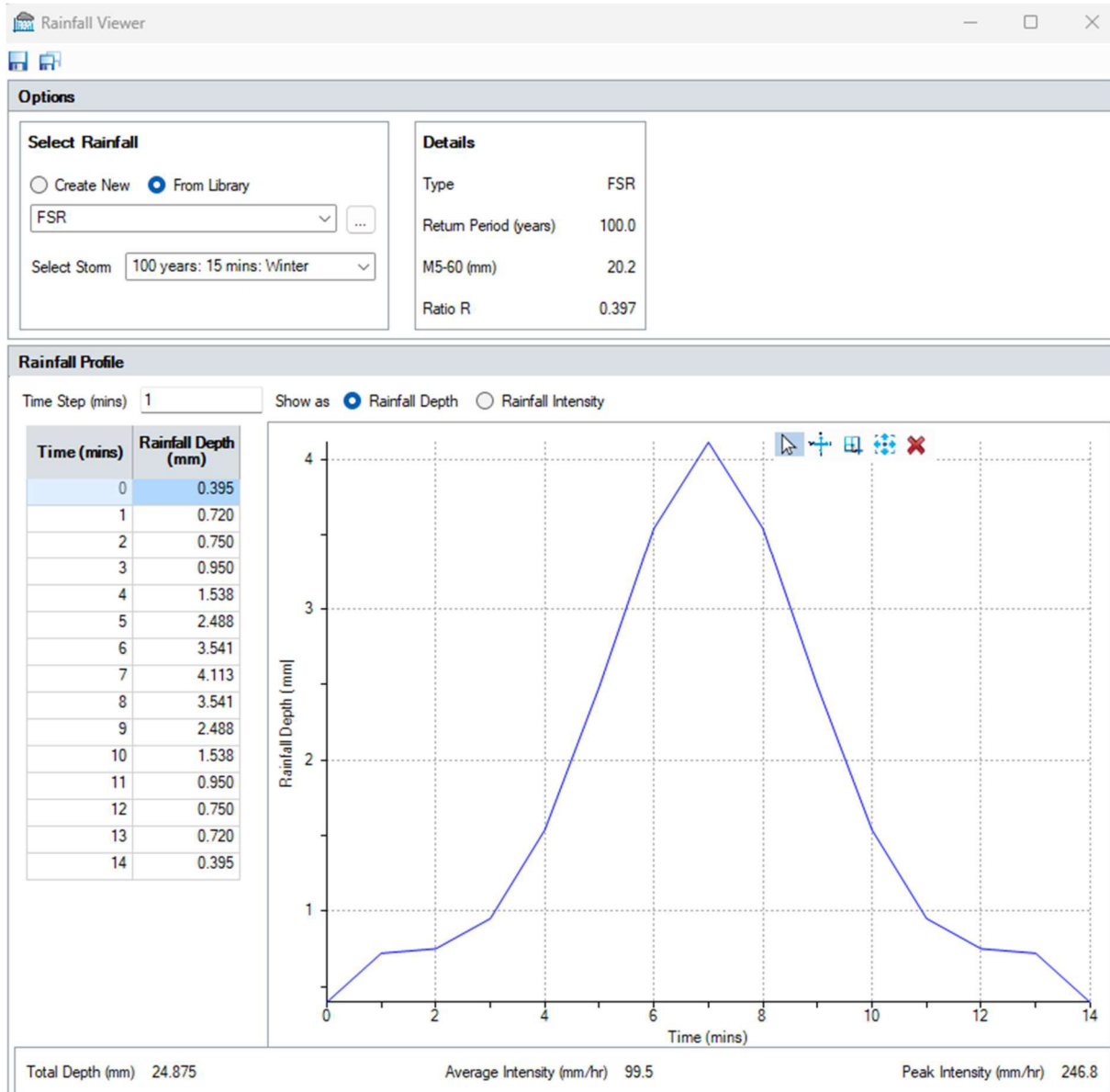
1 Year, 15 minute winter



30 Year, 15 minute winter



100 Year, 15 minute winter



1st July 2026

APPENDIX E

Calculations

Infodrainage Attenuation Calculations

Project: Haynes Road Westbury	Date: 19/06/2026			
	Designed by: AD	Checked by:	Approved By:	
Report Details: Type: Inflows Storm Phase: Phase	Design: Attenuation Tank			



Catchment Area

Type : Catchment Area

Area (ha)	0.016
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.750
Winter Volumetric Runoff	0.840
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Project: Haynes Road Westbury	Date: 19/06/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase	Design: Attenuation Tank		





Cellular Storage

Type : Cellular Storage

Dimensions

Exceedance Level (m)	73.000
Depth (m)	0.800
Base Level (m)	71.500
Number of Crates Long	5
Number of Crates Wide	3
Number of Crates High	2
Porosity (%)	95
Crate Length (m)	1
Crate Width (m)	0.5
Crate Height (m)	0.4
Total Volume (m³)	6.400

Inlets

Inlet

Inlet Type	Point Inflow
Incoming Item(s)	Catchment Area
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	(None)
Outlet Type	Orifice
Diameter (m)	0.023
Coefficient of Discharge	0.600
Invert Level (m)	71.500

Project: Haynes Road Westbury	Date: 19/06/2026			
	Designed by: AD	Checked by:	Approved By:	
Report Title: Rainfall Analysis Criteria	Design: Attenuation Tank			

Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Apply Global Value
Urban Creep Global Value (%)	0
Junction Flood Risk Margin (mm)	300
Perform No Discharge Analysis	☐

Rainfall

FSR Type: FSR

Region	England And Wales
M5-60 (mm)	20.2
Ratio R	0.397
Summer	☒
Winter	☒

Return Period

Return Period (years)	Increase Rainfall (%)
1.0	0.000
30.0	0.000
100.0	0.000
100.0	45.000

Storm Durations

Duration (mins)	Run Time (mins)
15	30
30	60
60	120
120	240
180	360
240	480
360	720
480	960
600	1200
720	1440
960	1920
1440	2880
2160	4320
2880	5760
4320	8640
5760	11520
7200	14400
8640	17280
10080	20160

Project: Haynes Road Westbury		Date: 19/06/2026			
		Designed by: AD	Checked by:		Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Design: Attenuation Tank			



FSR: 1 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residual Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Cellular Storage	FSR: 1 years: +0 %: 60 mins: Winter	71.638	71.638	0.138	0.138	1.1	0.984	0.000	0.000	0.4	1.604		84.626

Project: Haynes Road Westbury	Date: 19/06/2026			
	Designed by: AD	Checked by:	Approved By:	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Design: Attenuation Tank			

Status
OK

Project: Haynes Road Westbury		Date: 19/06/2026			
		Designed by: AD	Checked by:		Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Design: Attenuation Tank			



FSR: 30 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residant Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Cellular Storage	FSR: 30 years: +0 %: 60 mins: Winter	71.881	71.881	0.381	0.381	2.6	2.718	0.000	0.000	0.7	3.297		57.538

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Status
OK

Project: Haynes Road Westbury	Date: 19/06/2026		
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FSR: 100 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residant Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Cellular Storage	FSR: 100 years: +0 %: 60 mins: Winter	72.023	72.023	0.523	0.523	3.5	3.724	0.000	0.000	0.8	4.005	20	41.817

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**FSR: 100 years: Increase Rainfall (%): +45: Critical Storm Per Item: Rank By:
Max. Avg. Depth**

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Residual Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Cellular Storage	FSR: 100 years: +45 %: 60 mins: Winter	72.296	72.296	0.796	0.796	5.0	5.670	0.000	0.000	1.0	5.124	50	11.405

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FOR MORE INFORMATION CONTACT US AT:

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