

Report Title

Drainage Technical Note

Project

CLFA142-Court Farm, Trowbridge

Report Reference

CLFA142-CLFA-00-XX-DR-0661

Client

Wainhomes Severn Valley

Date

June 2026

REPORT CONTROL SHEET

Client: Wainhomes Severn Valley

Project: Court Farm, Trowbridge

Job Number: CLFA142

Report Title: Drainage Technical Note

Report Reference: CLFA142-CLFA-00-XX-DR-0661

Prepared By: C LEDOUX

Reviewed and Authorised By: F ALGUACIL MEng (Hons)

Revision	Date	Description	Prepared By	Authorised By
P01	10/11/25	FIRST ISSUE	C LEDOUX	F ALGUACIL
P02	27/04/25	REVISED LAYOUT	C LEDOUX	F ALGUACIL
P03	12/05/25	LAYOUT UPDATES	C LEDOUX	F ALGUACIL
P04	27/05/25	CULV. FREEBOARD	C LEDOUX	F ALGUACIL
P05	17/06/25	INTRO. WORDING	C LEDOUX	F ALGUACIL

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1. INTRODUCTION

Wain Homes (Severn Valley) has commissioned CLFA Design to prepare this Drainage Technical Note in support of a Reserved Matters (RM) application for the proposed residential development at Court Farm, Trowbridge.

The drainage strategy has been developed with reference to the approved Flood Risk Assessment (FRA) prepared by Teignconsult (Ref: 447/FRA2 V4) and its associated Addendum documents (Versions 1–4), which together form the approved technical baseline supporting the outline planning permission.

Outline planning permission was granted at appeal (Ref: APP/Y3940/W/23/3321957, dated 20 March 2024) on the basis of all matters reserved.

This Drainage Technical Note should be read alongside the Rappor Hydraulic Modelling Note (V01), which accompanies the RM submission and accompanies the updated drainage strategy prepared by CLFA Design.

Where any elements of the updated drainage strategy are identified as not fully complying with the requirements of Condition 11 attached to the outline consent, it is the applicant's intention to submit an application under Section 73 of the Town and Country Planning Act 1990 to vary Condition 11. This will enable the approved condition to align with the Rappor modelling outputs and the detailed drainage design prepared by CLFA Design.

This approach ensures that the Local Planning Authority is able to consider the Reserved Matters application and the detailed drainage and flood risk strategy without introducing procedural risk. In particular, it allows the drainage strategy to be regularised, where necessary, through a Section 73 application, thereby avoiding any potential risk of legal challenge to the decision-making process.

We would be happy to discuss the proposed approach with the Lead Local Flood Authority, the Environment Agency, or the Council as required.

2. PROPOSED DRAINAGE STRATEGY

1.1. FLOOD ALLEVIATION STRATEGY

Teignconsult Drawing FRA22 showed three flood alleviation (cut off) swales to the north-east of the site with a total capacity for 348l/s. These discharged to a piped system under the roads and eventually to the attenuation ponds to the south of the site. It is the Client's intention to offer the drainage network for adoption, however the water incumbent would not accept flood flows into their system, so these flows need to be separated.

In addition to this, it is not best practice to lay private apparatus in main roads, so this flood alleviation system will be constructed in private areas.

The total volume of water in these swales is also unknown (only flows -348l/s- are known), so it is recommended these flows don't discharge to the proposed basins.

It is therefore proposed these swales run along the site's northern development edge and discharge directly into the stream. This would mimic the existing situation. The swale details are as follows:

- Bottom width: 1.1m
- Top width: 6.5m
- Depth: 0.9m
- Side slopes 1:3
- Min. gradient: 1:500

The alignment of the cut-off swale can be seen on the Drainage Strategy drawings attached to Appendix A.

The previous scheme also included flood compensation works, and it is believed these are no longer required. The Rappor Hydraulic Modelling Note should be referred to for additional information regarding the span of the culverts under the access road and the use of the floodplain storage.

1.2. SURFACE WATER PROPOSAL

The proposed Drainage Strategy is as per the approved Outline drawings, with a slight deviation in discharge rates to comply with new Wiltshire Council Drainage and Betterment Strategy (June 2024) and with a new attenuation basin to the north-west of the site to drain the access road, which was not considered at outline Stage.

As it can be seen from the contours shown on the Teignconcult drawings, the access road from approximately Chainage +138m felt back towards the site access with no attenuation at this location.

Flows from this section of road were discharged (unattenuated) onto the main road at the front of the site, which is not considered acceptable and it is likely to increase flood risk on the main road. It is therefore proposed to construct a new attenuation basin to deal with these flows.

Wiltshire's Council requirements regarding discharge rates and volumes are as follows:

- 1) Post development discharges from greenfield sites to provide 30% betterment over pre-development discharges for all storm events between the 1 in 1year and 1 in 100year return period.
- 2) The runoff volume from the development to any highway drain, sewer or surface water body in the 1 in 100 year, 6 hour rainfall event should never exceed the greenfield runoff volume for the same event.
- 3) Attenuation features to either have 50% capacity available 24 hours after a 1 in 30-year + climate change storm event (i.e., half drain time < 1440mins) or have the capacity to store a subsequent 1 in 10-year storm event after a 1 in 30-year+ climate change storm event.

A comparison of the approved flows against the RM proposal is shown below. As it can be seen, the betterments are significant.

Table 1. Approved discharge rates comparison

Storage feature	Previous rate 1 in 1 year event	Previous rate 1 in 100 year event	New rate 1 in 1 year event	New rate 1 in 100 year event
Basin 1	N/A	N/A	0.5 l/s	0.5 l/s
Basin 2	N/A	1.8 l/s	0.5 l/s	0.5 l/s
Basins 3-4 and Pond 1	N/A	13.8 l/s	5.4 l/s	7 l/s
Total		15.6 l/s		8 l/s

A copy of the 1 in 1 year, 1 in 100 year and 1 in 100cc calculations can be found in Appendix B.

Compliance with Wiltshire Council Guidance has been checked below:

Table 2. Condition 1 – 30% betterment over greenfield rates

Storage feature	Greenfield rate 1 in 1 year event	Greenfield rate 1 in 100 year event	Discharge rate 1 in 1 year event	Discharge rate 1 in 100 year event	Betterment provided 1 in 1 year event	Betterment provided 1 in 100 year event
Basin 1	0.2 l/s	0.7 l/s	0.5 l/s	0.5 l/s	-150%	28%
Basin 2	0.7 l/s	2.2 l/s	0.5 l/s	0.5 l/s	28%	77%
Basins 3-4 and Pond 1	14.1 l/s	44.1 l/s	5.4 l/s	7 l/s	61%	84%
Total	15 l/s	47 l/s	6.4 l/s	8 l/s	57%	83%

As it can be seen, the discharge from Basin 1 during the 1 in 1 year event would be in excess of the greenfield rate for that same event, however the discharge cannot be reduced further due to potential blockages issues. It should be noted that the overall betterment provided is well in excess of the required 30%.

A copy of the greenfield runoff calculations can be seen in Appendix C.

Table 3. Condition 2 – Greenfield Volume

Storage feature	Greenfield Volume (m³)	Discharged volume 1 in 100 year, 6 hour rainfall event (m³)	Condition met?
Basin 1	4.259	13.898	X
Basin 2	12.778	14.699	X
Basins 3-4 and Pond 1	259.229	217.229	✓
Total	276.266	245.826	✓

A copy of the greenfield volume calculations can be seen in Appendix E and a copy of the 1 in 100 year, 6 hour rainfall event calculations are shown in Appendix F.

Table 4. Condition 3 – Half Drain Times

Storage feature	Half Drain Time < 1440mins for a 1 in 30cc event (cc-Climate change 45%)
Basin 1	✓
Basin 2	✓
Basin 3	✓
Basin 4	✓
Pond 1	✓

A copy of the 1 in 30cc year calculations can be found in Appendix G.

It is recommended that the drainage system is maintained following the recommendations in the table below.

Maintenance Schedule	Required Action	Typical Frequency	Maintenance Responsibilities
Attenuation Basins and Ponds	Remove litter and debris	Monthly	By Management Company (TBC)
	Cut grass – for spillways and access routes	Monthly (during growing season), or as required	
	Cut grass- meadow grass in and around basin	Half yearly (spring – before nesting season and autumn)	
	Manage other vegetation and remove nuisance plants	Monthly (at the start, then as required)	
	Inspect inlets, outlets and overflows for blockages and clear if required	Monthly	
	Inspect banksides, structures, pipework etc for evidence of physical damage	Monthly	
	Inspect inlets and facility surface for silt accumulation. Establish appropriate silt removal frequencies	Monthly (for first year), then annually or as required	
	Check any mechanical devices	Annually	
	Tidy all dead growth before start of growing season	Annually	
	Remove sediments from inlets and outlet	Annually (or as required)	
	Reseed areas of poor vegetation growth	As required	
	Prune and trim any trees and remove cuttings	Every 2 years, or as required	
	Remove sediments from inlets, outlets and main basin when required	Every 5 years, or as required (likely to be minimal requirements where effective upstream sources control is provided)	
	Repair erosion or other damage by reseeded re-turfing	As required	
	Repair/rehabilitation of inlets, outlets and overflows	As required	
	Relevel uneven surfaces and reinstate design levels	As required	

Maintenance Schedule	Required Action	Typical Frequency	Maintenance Responsibilities
Swales and rain gardens	Remove litter and debris	Monthly, or as required	Management Company (TBC)
	Cut grass – for spillways and access routes	Monthly (during growing season), or as required	
	Manage other vegetation and remove nuisance plants	Monthly (at the start, then as required)	
	Inspect inlets, outlets and overflows for blockages and clear if required	Monthly	
	Inspect infiltration surfaces for ponding, compaction, silt accumulation, record areas where water is ponding for > 48 hours	Monthly, or as required	
	Inspect vegetation coverage	Monthly for six months, quarterly for 2 years, then half yearly	
	Inspect inlets and facility surface for silt accumulation. Establish appropriate silt removal frequencies	Half yearly	
	Reseed areas of poor vegetation growth	As required or if bare soil is exposed over 10% or more of the swale treatment area	
	Repair erosion or other damage by reseeded re-turfing	As required	
	Relevel uneven surfaces and reinstate design levels	As required	
	Scarify and spike topsoil to improve infiltration performance, break up silt deposits and prevent compaction of the soil surface	As required	
	Remove and dispose of oils or petrol residues using safe standard practices	As required	
Drainage runs, chambers and flow control structures	Remove weeds and clear obstructions	Once a year or as required	Water Authority or Management Company (TBC)
	Monitor inspection chambers. Inspect silt accumulation rates and determine cleaning frequencies	Once a year or as required	

1.3. WATER QUALITY

According to the CIRIA SuDS Manual (C753 Report, 2015) and to deliver adequate treatment, the selected SuDS component should have a total pollution index (for each contaminant type) that equals or exceeds the pollution hazard index (for each contaminant type). The Table below shows that the proposed SuDS features will provide the required treatment for the new development.

Land use	Pollution hazard level	Total suspended solids	Metals	Hydrocarbons
Residential roofs	Very low	0.2	0.2	0.05
Residential car parking and access roads	Low	0.5	0.4	0.4
Total pollution level		0.7	0.6	0.45
SuDS mitigation – rain garden/filter drain		0.4	0.4	0.4
SuDS mitigation - basin		0.5	0.5	0.6
SuDS mitigation - swale		0.5	0.6	0.6
Total mitigation (> Total pollution level)		1.4	1.5	1.6

1.4. FIRST FLUSH

According to CIRIA, the initial runoff from a site will collect or dissolve pollutants and the ‘first flush’ portion of the flow may be the most contaminated as a result. As ground investigation works have shown there is no infiltration, the calculations below show the amount of water that the planting in the proposed basins, rain gardens and swales will need to absorb.

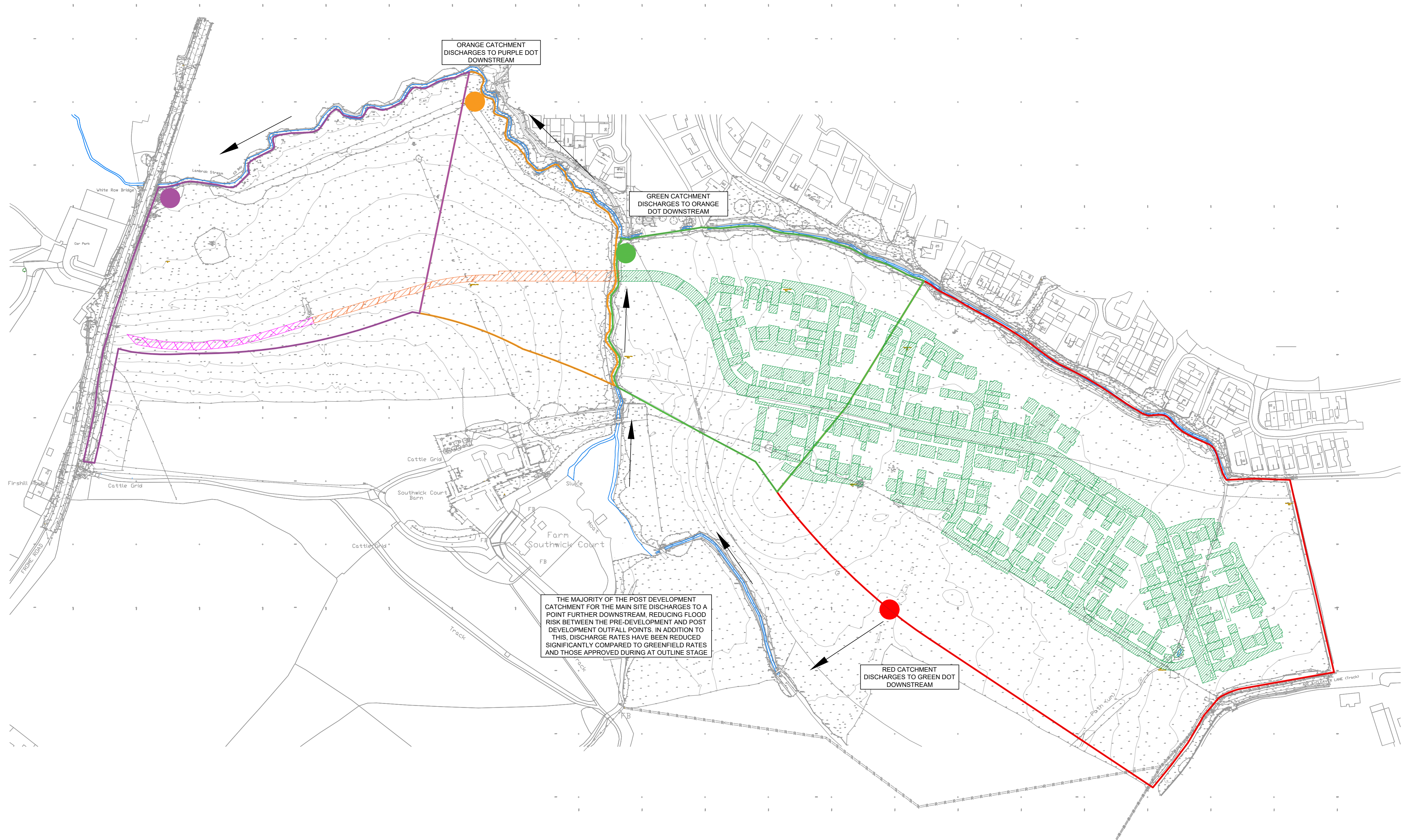
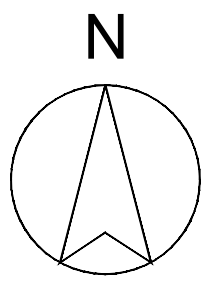
Catchment 1 – Access road			
Impermeable Area	Volume produced by the first flush (5mm of rainfall)	SuDS Area (Swale and basin)	Ponding depth to be absorbed by proposed planting
789 m ²	3.945 m ³	600 m ²	6.5mm
Catchment 2 – Road and bridge			
Impermeable Area	Volume produced by the first flush (5mm of rainfall)	SuDS Area (Swale and basin)	Ponding depth to be absorbed by proposed planting
1500 m ²	7.5 m ³	710 m ²	10.5 mm
Catchment 3 – Main Site			
Impermeable Area	Volume produced by the first flush (5mm of rainfall)	SuDS Area (Swale , rain gardens and basins, pond excluded)	Ponding depth to be absorbed by proposed planting
35236 m ²	176.18 m ³	2763 m ²	63.7 mm

1.1. FOUL WATER PROPOSAL

A new pumping station will be required and it will be located to the south of the site. A rising main will discharge via a break chamber to the existing Wessex Water network near the site entrance.

A copy of the Wessex Water Maps can be seen in Appendix H.

APPENDIX A – PROPOSED DRAINAGE STRATEGY



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- KEY:
- SOLID OUTLINES INDICATE PRE-DEVELOPMENT CATCHMENTS
 - HATCHED AREAS INDICATE POST-DEVELOPMENT CATCHMENTS

P01	FIRST ISSUE	MR	FA	27/04/26
REV	DESCRIPTION	BY	APP	DATE

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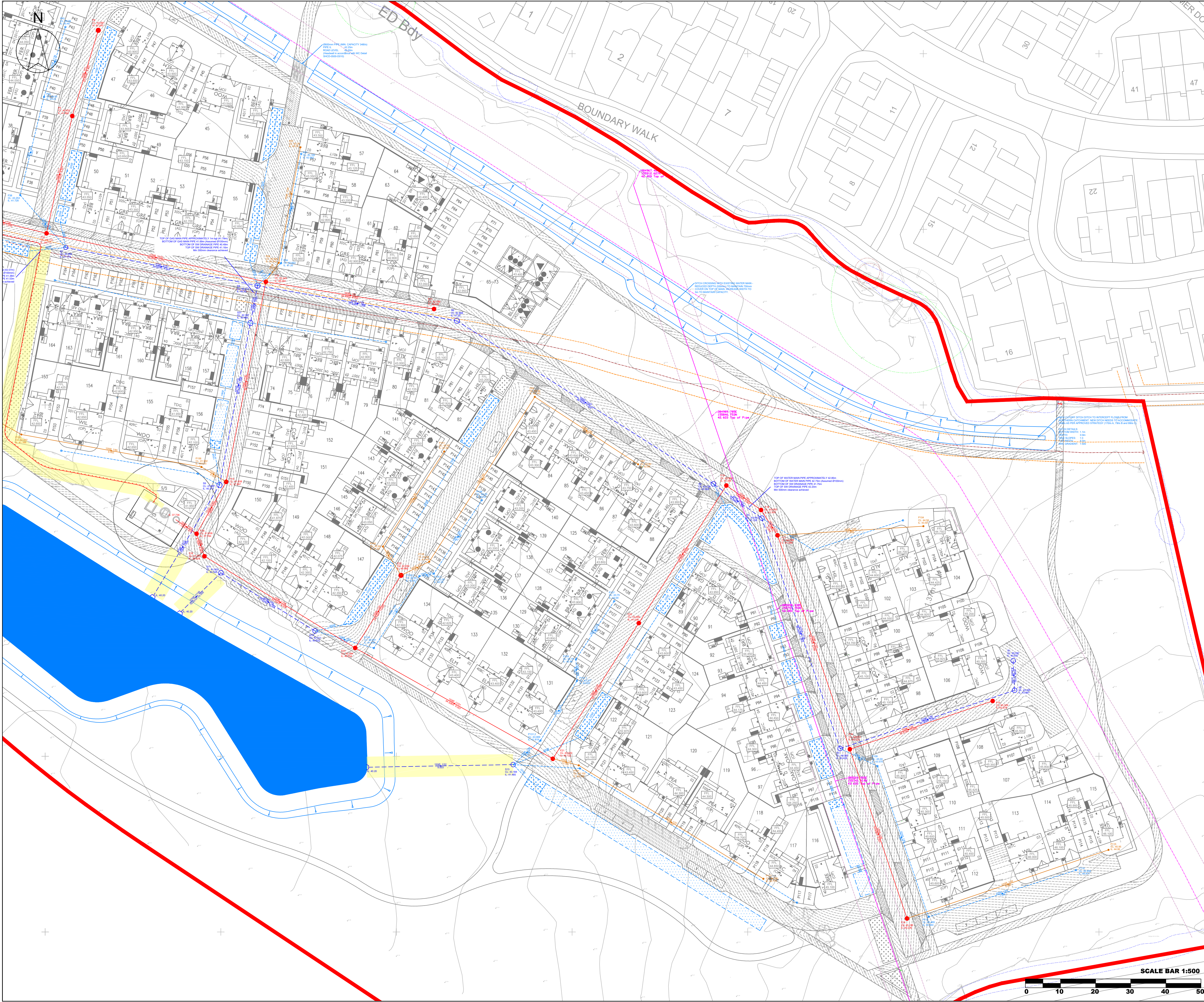
CLIENT:

Wain HOMES

PROJECT:
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:
SW CATCHMENT PLAN

PROJECT NUMBER: CLFA142	DRAWING NUMBER: CLFA142-CLFA-00-XX-DR-C-0501
STATUS: S2	SCALE @ A1 NOT TO SCALE
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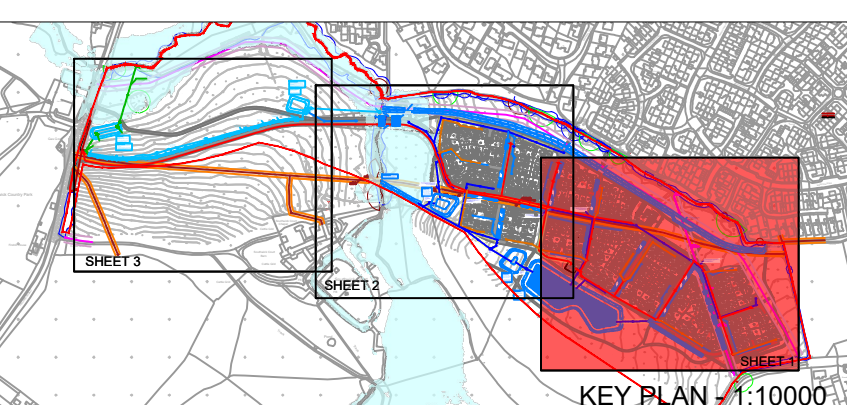
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- LEGEND:
-  PROPOSED ADOPTABLE S104 FOUL DRAINAGE
 -  PROPOSED PRIVATE FOUL DRAINAGE
 -  PROPOSED ADOPTABLE S104 FOUL RISING MAIN
 -  PROPOSED ADOPTABLE S104 SURFACE WATER DRAINAGE
 -  PROPOSED PRIVATE SURFACE WATER DRAINAGE
 -  PROPOSED HIGHWAYS SURFACE WATER DRAINAGE
 -  EXISTING GAS MAIN AND EASEMENT
 -  EXISTING WATER MAIN AND EASEMENT
 -  EXISTING FW DRAINAGE
 -  POST DEVELOPMENT 1 IN 100CC FLOOD OUTLINES (Refer to Rappor April 2026 Hydraulic Modelling Note for details)
 -  SITE BOUNDARY
 -  ADOPTABLE SEWER EASEMENT (3m min unless otherwise shown)
 -  RAIN GARDEN (For road drainage only)
 -  RAIN GARDEN (With filter drain at base for plot and road drainage)
 -  EXISTING TREE RPA (CAT A)
 -  EXISTING TREE RPA (CAT B)
 -  EXISTING TREE RPA (CAT U)

P04	MINOR UPDATES TO REAR FENCES	CL	FA	27/05/26
P03	MINOR SITE LAYOUT UPDATES	CL	FA	12/05/26
P02	SITE LAYOUT UPDATED	CL	FA	27/04/26
P01	WIP FOR COMMENTS 06/10/25	CL	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE

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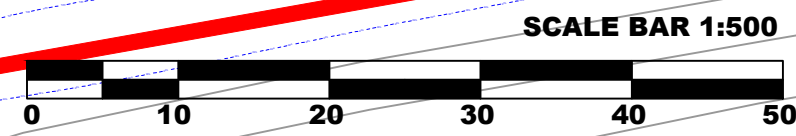
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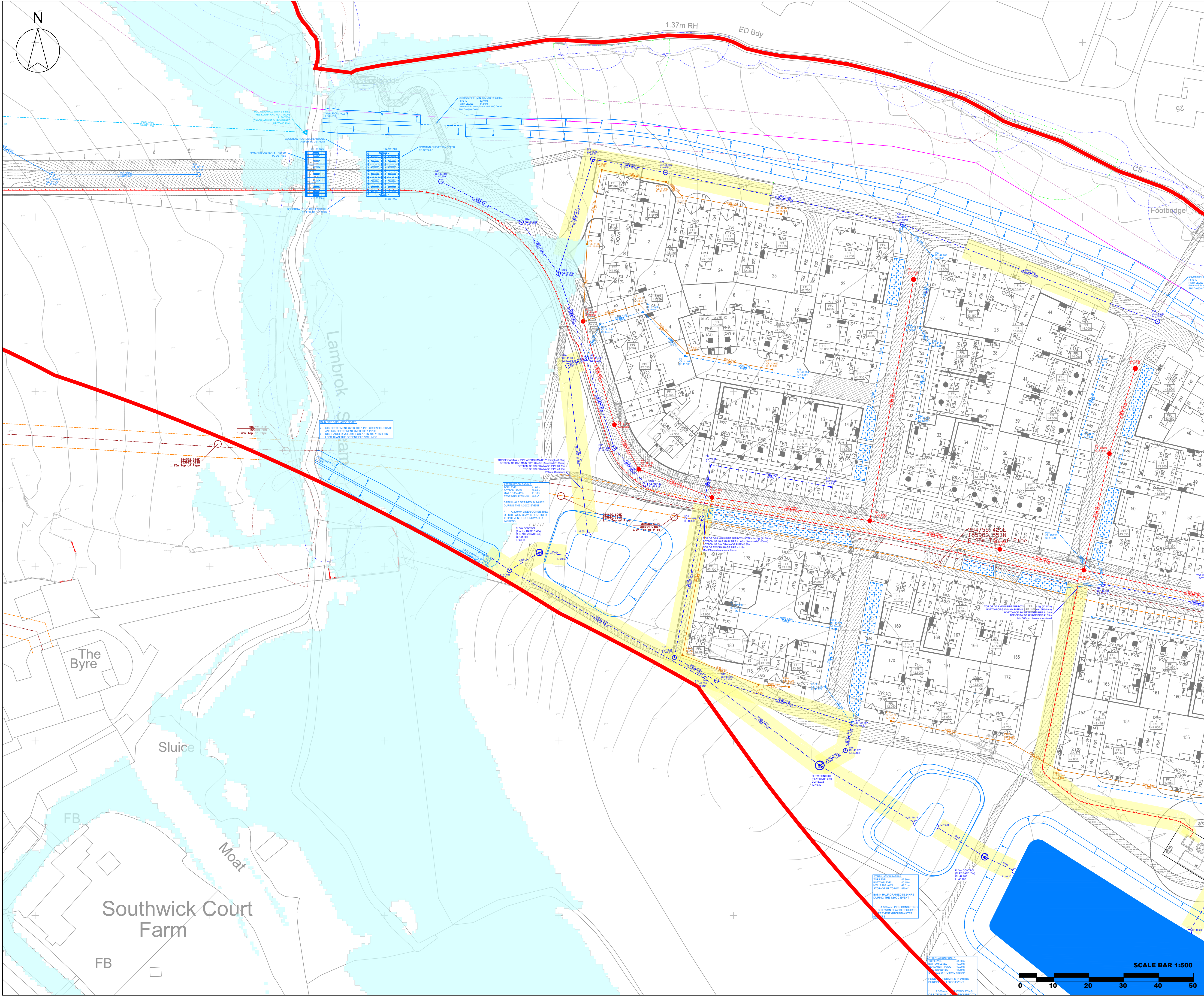
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:

DRAINAGE STRATEGY FOR PLANNING
SHEET 1

PROJECT NUMBER:	CLFA142	DRAWING NUMBER:	CLFA142-CLFA-00-XX-DR-C-0512.1
STATUS:	S2	SCALE @ A1	1:500
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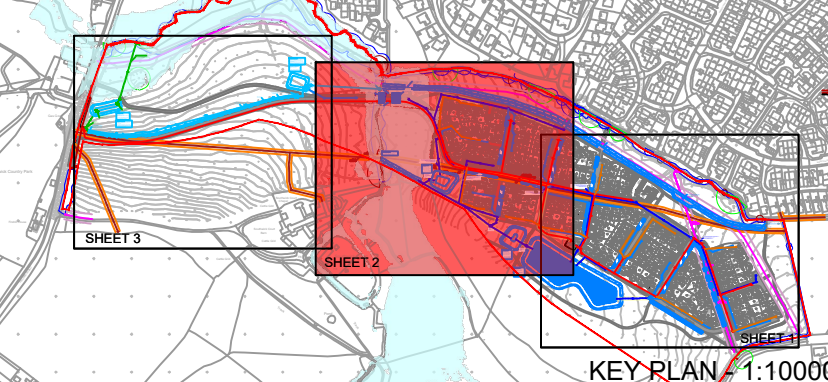
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-  PROPOSED ADOPTABLE S104 FOUL RISING MAIN
-  PROPOSED ADOPTABLE S104 SURFACE WATER DRAINAGE
-  PROPOSED PRIVATE SURFACE WATER DRAINAGE
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-  EXISTING WATER MAIN AND EASEMENT
-  EXISTING FW DRAINAGE
-  POST DEVELOPMENT 1 IN 100CC FLOOD OUTLINES (Refer to Rappor April 2026 Hydraulic Modelling Note for details)
-  SITE BOUNDARY
-  ADOPTABLE SEWER EASEMENT (3m min unless otherwise shown)
-  RAIN GARDEN (For road drainage only)
-  RAIN GARDEN (With filter drain at base for plot and road drainage)
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-  EXISTING TREE RPA (CAT B)
-  EXISTING TREE RPA (CAT U)

REV	DESCRIPTION	BY	APP	DATE
P04	ACCESS ROAD LEVELS UPDATED TO PROVIDE 600mm FREEBOARD ON CULVERTS. DRAINAGE COVER LEVELS ADJUSTED TO SUIT	CL	FA	27/05/26
P03	MINOR SITE LAYOUT UPDATES	CL	FA	12/05/26
P02	SITE LAYOUT UPDATED	CL	FA	27/04/26
P01	WIP FOR COMMENTS 06/10/25	CL	FA	10/11/25

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CLIENT:

Wain HOMES

PROJECT:

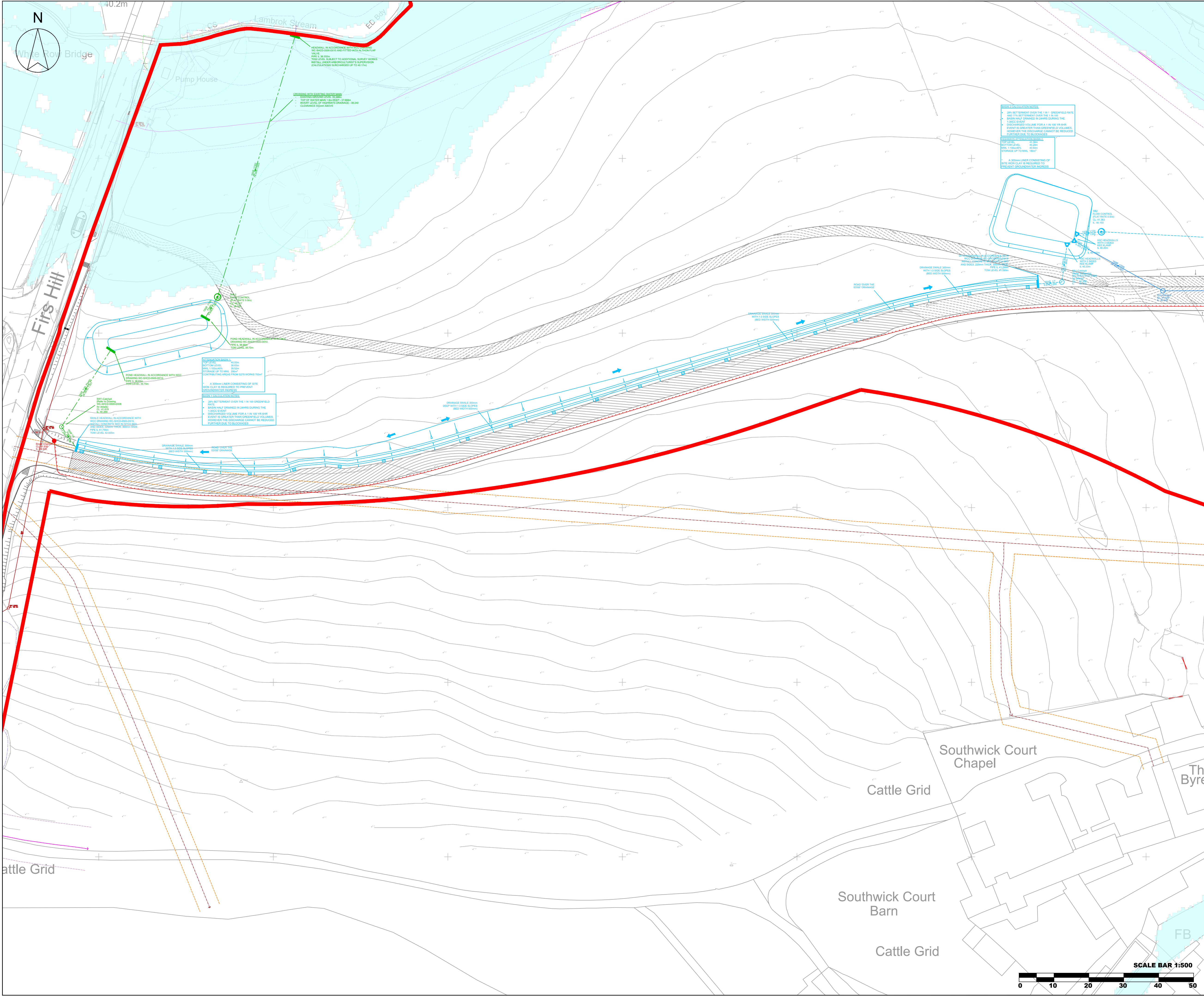
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:

DRAINAGE STRATEGY FOR PLANNING
SHEET 2

PROJECT NUMBER:	DRAWING NUMBER:
CLFA142	CLFA142-CLFA-00-XX-DR-C-0512.2

STATUS:	SCALE @ A1	REV:
S2	1:500	P04



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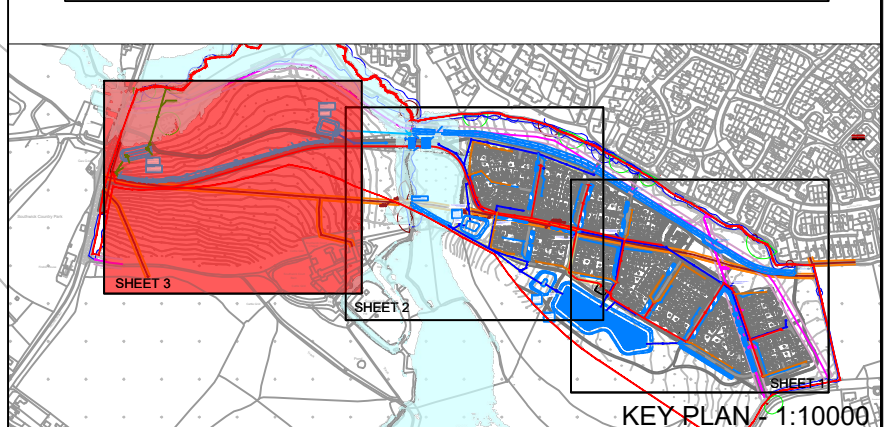
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P04	ACCESS ROAD LEVELS UPDATED TO PROVIDE 600mm FREEBOARD ON CULVERTS. DRAINAGE COVER LEVELS ADJUSTED TO SUIT	CL	FA	27/05/26
P03	MINOR SITE LAYOUT UPDATES	CL	FA	12/05/26
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P01	WIP FOR COMMENTS 06/10/25	CL	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE



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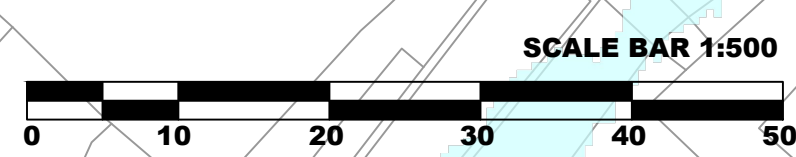
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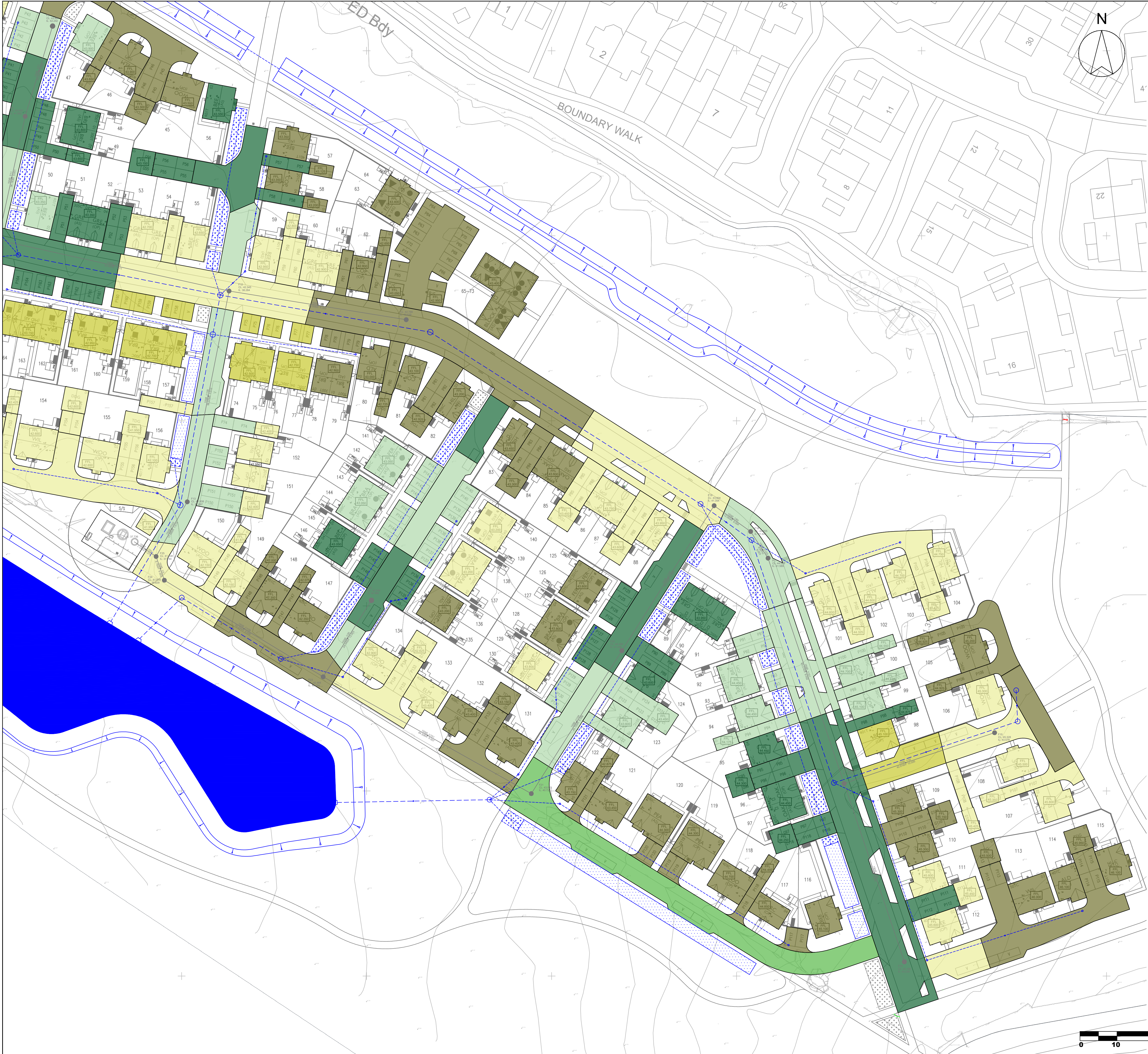


PROJECT:
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:
DRAINAGE STRATEGY FOR PLANNING
SHEET 2

PROJECT NUMBER: CLFA142	DRAWING NUMBER: CLFA142-CLFA-00-XX-DR-C-0512.3
STATUS: S2	SCALE @ A1 1:500
	REV: P04





MANHOLE CATCHMENT		
MANHOLE ID	IMPERMEABLE AREA (m²)	INCLUDING URBAN CREEP 10% (m²)
ACCESS ROAD		
SWALE	578	578
TOTAL		578
ROAD AND BRIDGE		
S81	285	285
S82	590	590
SWALE	859	859
TOTAL		1735
MAIN SITE		
P114	772	796
P112	309	323
P109	265	279
S1	493	502
S2	586	610
S3	500	507
Swale P116	1418	1435
Swale P90	1119	1147
S4	996	1022
S5	878	894
S6	2547	2629
P43	154	169
P40	96	106
P38	399	439
Swale P47	206	214
Swale P48	483	492
Swale P50	265	272
S7	1821	1859
Swale P56	489	496
Swale P55	127	127
P57	177	190
P59	0	0
S8	991	1024
S9	1758	1803
Swale P82	110	110
Swale P141	642	663
Swale P146	360	369
Swale P147	67	67
P139	208	228
P135.1	87	96
P135.2	0	0
P134	391	408
S10	538	554
S11	420	436
S12	0	0
Swale P89	685	704
Swale P122	481	487
P127	208	228
P129	87	96
P131.1	0	0
P131.2	0	0
S13	2218	2293
P179	503	503
P175	0	0
P174	0	0
Swale P169	229	229
P14	0	0
P8	300	300
S14	1041	1059
S15	214	231
S16	0	0
S17	1026	1059
S18	0	0
S19	0	0
Swale P14.1	192	198
Swale P14.2	849	861
Swale P21	0	0
P32	0	0
P28.1	183	183
P28.2	0	0
P27	0	0
S20	1151	1194
S21	510	524
S22	0	0
S23	447	447
S24	203	203
S25	389	403
P18	96	106
P5	654	663
P4.1	0	0
P4.2	0	0
S26	227	237
S27	0	0
S28	1031	1055
S29	0	0
S30	920	942
S31	798	861
TOTAL		35299



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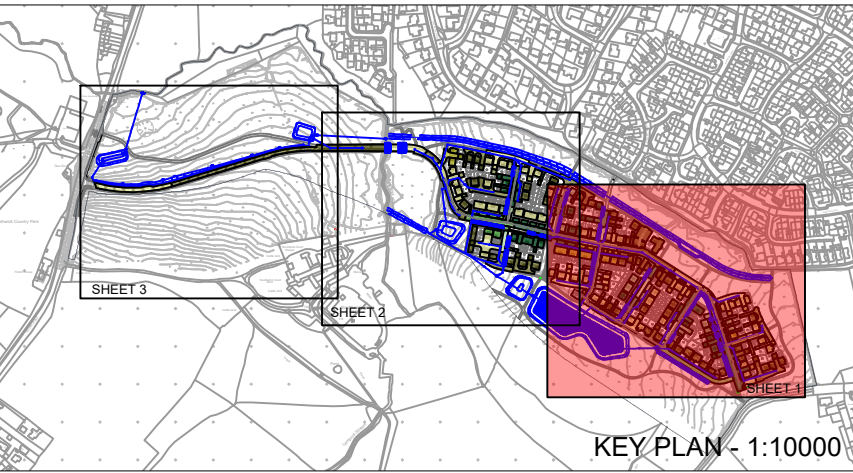
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GREEN AREAS SHOW DEVELOPMENT BEING DRAINED TO RAIN GARDENS. YELLOW AREAS SHOW DEVELOPMENT BEING DRAINED A TO CONVENTIONAL NETWORK

P04	MINOR UPDATES TO REAR FENCES	CL	FA	27/05/26
P03	MINOR SITE LAYOUT UPDATES	CL	FA	12/05/26
P02	SITE LAYOUT UPDATED	CL	FA	27/04/26
P01	FIRST ISSUE	CL	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE

CLFA DESIGN

71-75 Shelton Street, Covent Garden
London, WC2H 9JQ

CLIENT:



PROJECT:
COURT FARM
TROWBRIDGE

DRAWING:
IMPERMEABLE AREAS PLAN
SHEET 1

PROJECT NUMBER: CLFA152	DRAWING NUMBER: CLFA142-CLFA-00-XX-DR-C-0681.1	
STATUS: S2	SCALE @ A1 1:500	REV: P04

MANHOLE CATCHMENT		
MANHOLE ID	IMPERMEABLE AREA (m²)	INCLUDING URBAN CREEP 10% (m²)
ACCESS ROAD		
SWALE	578	578
TOTAL		578
ROAD AND BRIDGE		
SB1	285	285
SB2	590	590
SWALE	859	859
TOTAL		1735
MAIN SITE		
P114	772	796
P112	309	323
P109	265	279
S1	493	502
S2	586	610
S3	500	507
Swale P116	1418	1435
Swale P90	1119	1147
S4	996	1022
S5	878	894
S6	2547	2629
P43	154	169
P40	96	106
P38	399	439
Swale P47	206	214
Swale P48	483	492
Swale P50	265	272
S7	1821	1859
Swale P56	489	496
Swale P55	127	127
P57	177	190
P59	0	0
S8	991	1024
S9	1758	1803
Swale P82	110	110
Swale P141	642	663
Swale P146	360	369
Swale P147	67	67
P139	208	228
P135.1	87	96
P135.2	0	0
P134	391	408
S10	538	554
S11	420	436
S12	0	0
Swale P89	685	704
Swale P122	481	487
P127	208	228
P129	87	96
P131.1	0	0
P131.2	0	0
S13	2218	2293
P179	503	503
P175	0	0
P174	0	0
Swale P169	229	229
P14	0	0
P8	300	300
S14	1041	1059
S15	214	231
S16	0	0
S17	1026	1059
S18	0	0
S19	0	0
Swale P14.1	192	198
Swale P14.2	849	861
Swale P21	0	0
P32	0	0
P28.1	183	183
P28.2	0	0
P27	0	0
S20	1151	1194
S21	510	524
S22	0	0
S23	447	447
S24	203	203
S25	389	403
P18	96	106
P5	654	663
P4.1	0	0
P4.2	0	0
S26	227	237
S27	0	0
S28	1031	1055
S29	0	0
S30	920	942
S31	798	861
TOTAL		35299



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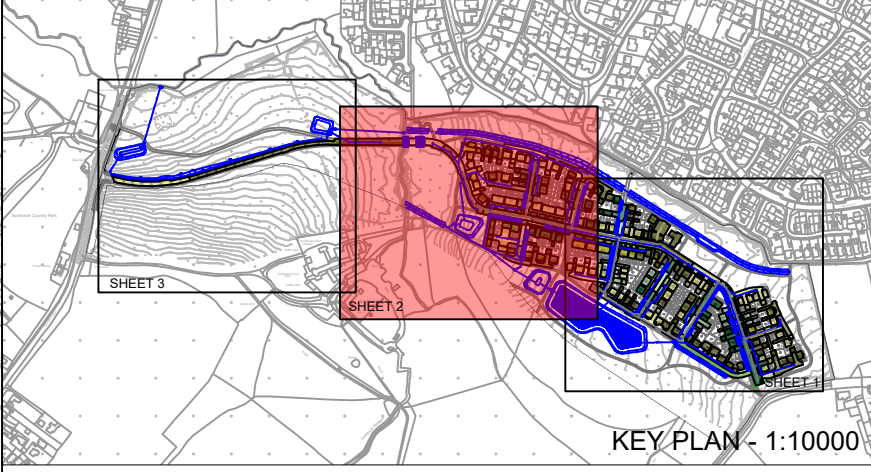
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P04	MINOR UPDATES TO REAR FENCES AND PARKING NUMBERS 173-178	CL	FA	27/05/26
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P02	SITE LAYOUT UPDATED	CL	FA	27/04/26
P01	FIRST ISSUE	CL	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE

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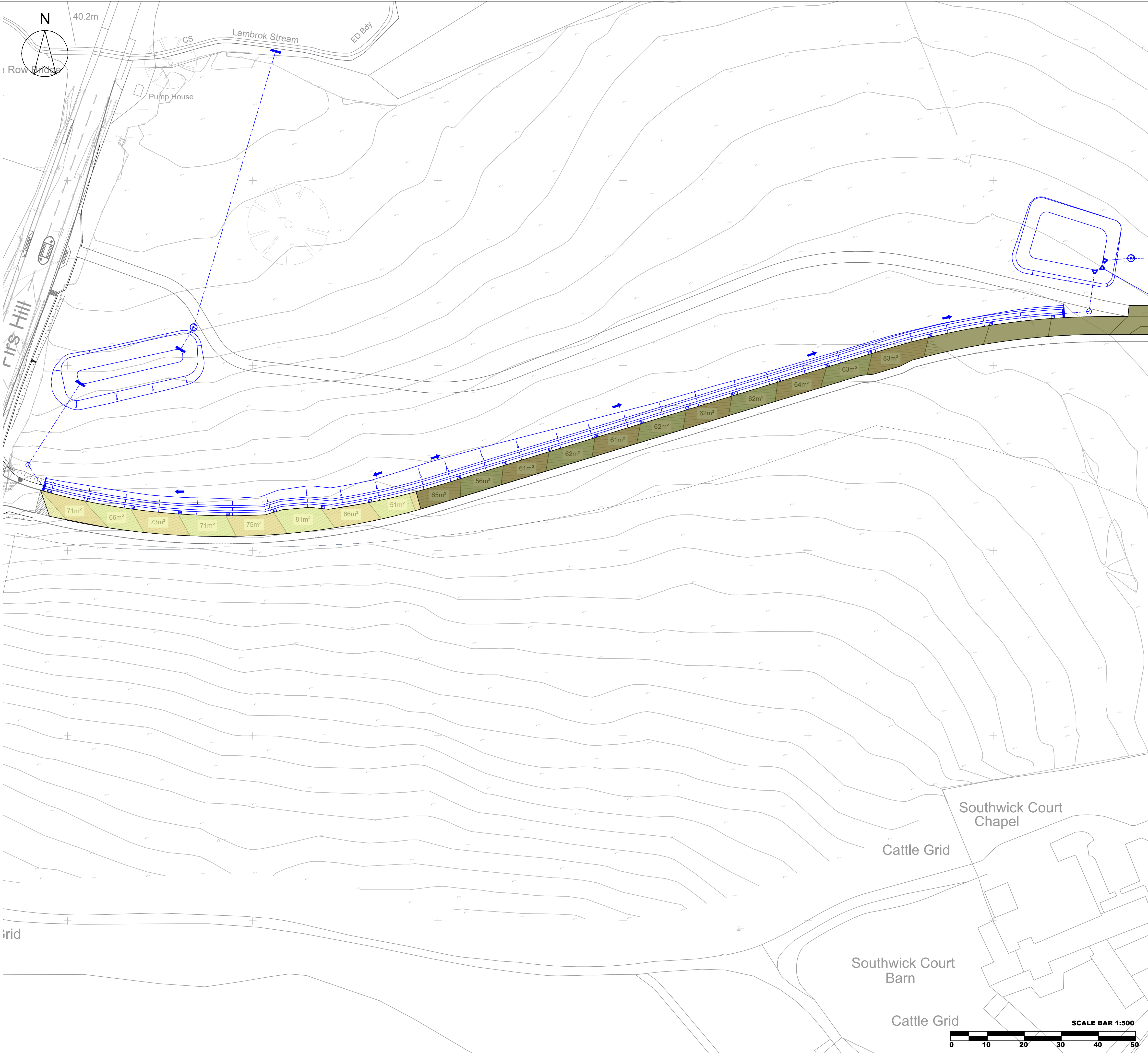


PROJECT:
COURT FARM
TROWBRIDGE

DRAWING:
IMPERMEABLE AREAS PLAN
SHEET 2

PROJECT NUMBER: CLFA152	DRAWING NUMBER: CLFA142-CLFA-00-XX-DR-C-0681.2	
STATUS: S2	SCALE @ A1 1:500	REV: P04

MANHOLE CATCHMENT		
MANHOLE ID	IMPERMEABLE AREA (m²)	INCLUDING URBAN CREEP 10% (m²)
ACCESS ROAD		
SWALE	578	578
TOTAL		578
ROAD AND BRIDGE		
SB1	285	285
SB2	590	590
SWALE	859	859
TOTAL		1735
MAIN SITE		
P114	772	796
P112	309	323
P109	265	279
S1	493	502
S2	586	610
S3	500	507
Swale P116	1418	1435
Swale P90	1119	1147
S4	996	1022
S5	878	894
S6	2547	2629
P43	154	169
P40	96	106
P38	399	439
Swale P47	206	214
Swale P48	483	492
Swale P50	265	272
S7	1821	1859
Swale P56	489	496
Swale P55	127	127
P57	177	190
P59	0	0
S8	991	1024
S9	1758	1803
Swale P82	110	110
Swale P141	642	663
Swale P146	360	369
Swale P147	67	67
P139	208	228
P135.1	87	96
P135.2	0	0
P134	391	408
S10	538	554
S11	420	436
S12	0	0
Swale P89	685	704
Swale P122	481	487
P127	208	228
P129	87	96
P131.1	0	0
P131.2	0	0
S13	2218	2293
P179	503	503
P175	0	0
P174	0	0
Swale P169	229	229
P14	0	0
P8	300	300
S14	1041	1059
S15	214	231
S16	0	0
S17	1026	1059
S18	0	0
S19	0	0
Swale P14.1	192	198
Swale P14.2	849	861
Swale P21	0	0
P32	0	0
P28.1	183	183
P28.2	0	0
P27	0	0
S20	1151	1194
S21	510	524
S22	0	0
S23	447	447
S24	203	203
S25	389	403
P18	96	106
P5	654	663
P4.1	0	0
P4.2	0	0
S26	227	237
S27	0	0
S28	1031	1055
S29	0	0
S30	920	942
S31	798	861
TOTAL		35299



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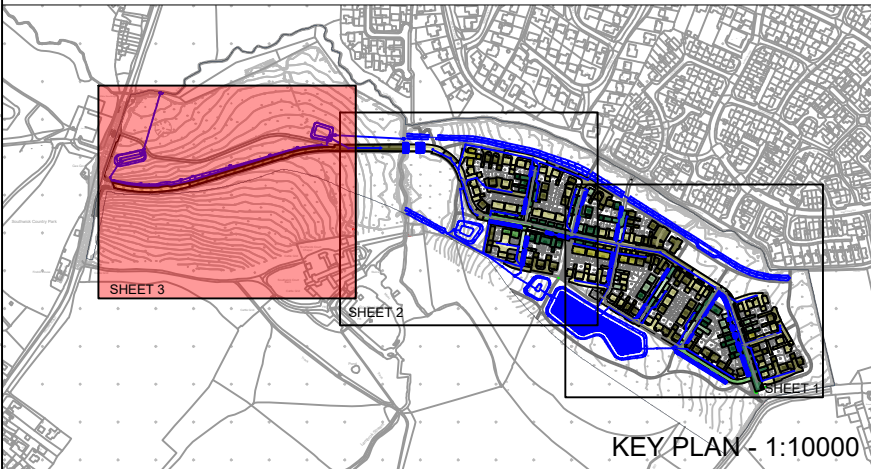
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P03	MINOR SITE LAYOUT UPDATES	CL	FA	12/05/26
P02	SITE LAYOUT UPDATED	CL	FA	27/04/26
P01	FIRST ISSUE	CL	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE

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CLIENT:



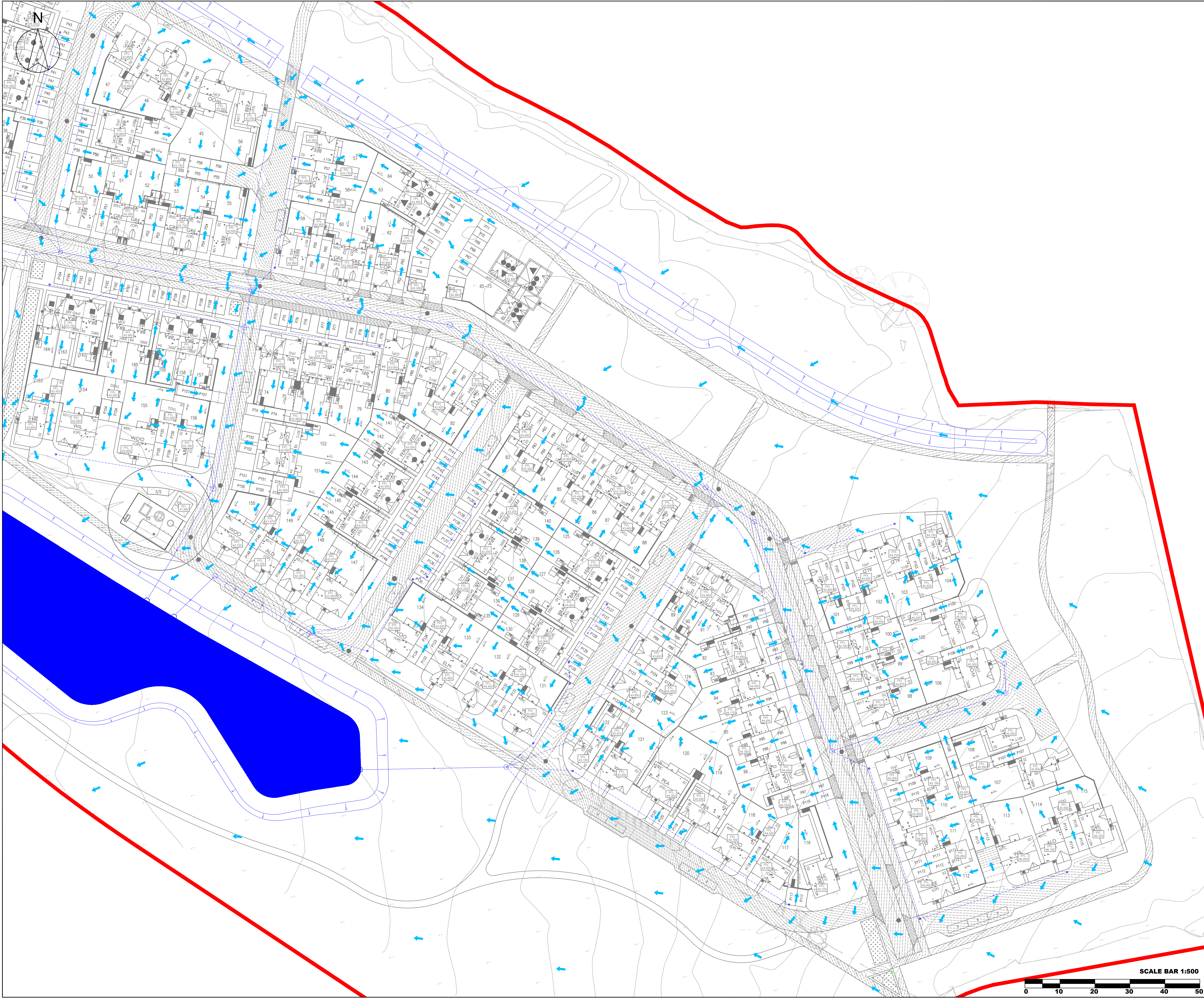
PROJECT:

COURT FARM
TROWBRIDGE

DRAWING:

IMPERMEABLE AREAS PLAN
SHEET 3

PROJECT NUMBER:	CLFA152	DRAWING NUMBER:	CLFA142-CLFA-00-XX-DR-C-0681.3
STATUS:	S2	SCALE @ A1	1:500
REV:	P04		



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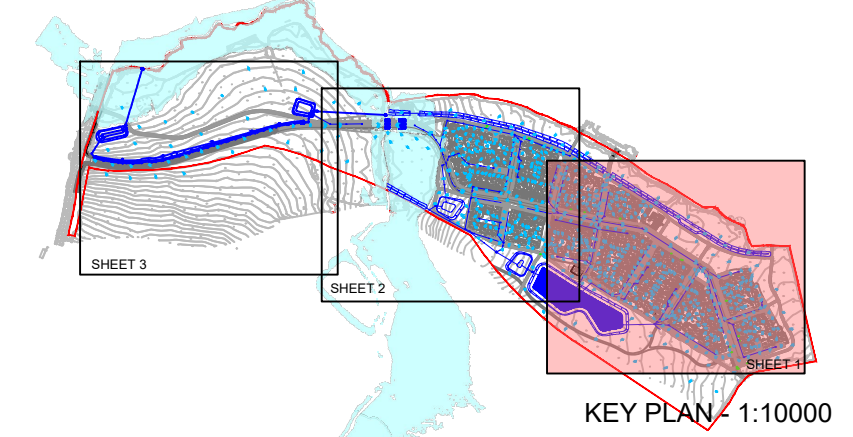
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P05	ROAD CONTOURS SHOWN	MR	FA	27/05/26
P04	MINOR SITE LAYOUT UPDATES	MR	FA	12/05/26
P03	SITE LAYOUT UPDATED	MR	FA	27/04/26
P02	SCALE BAR ADDED	CL	FA	09/12/25
P01	FIRST ISSUE	MR	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE

CLFA DESIGN

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PROJECT:

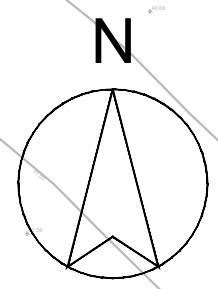
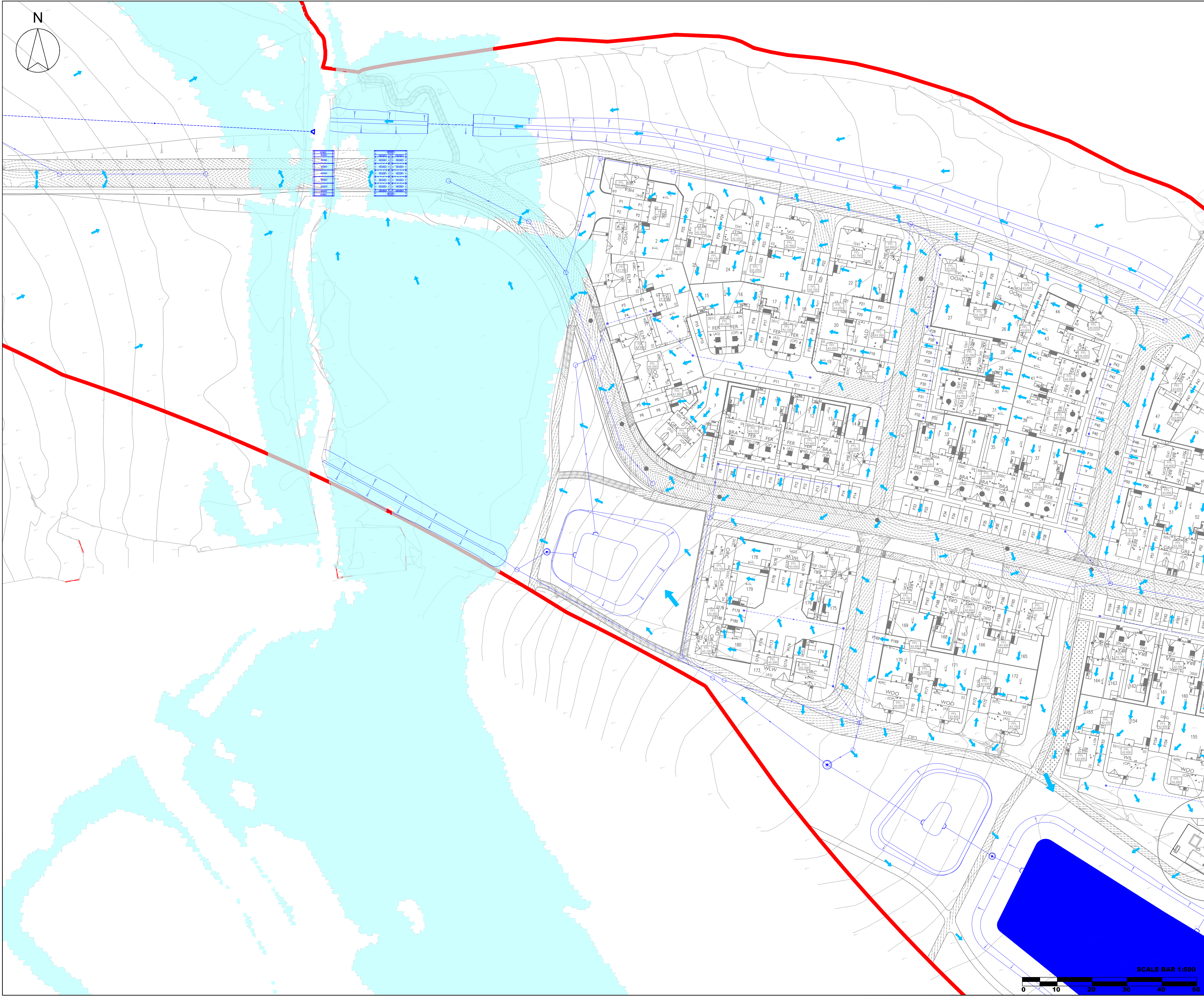
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:

DRAINAGE EXCEEDANCE PLAN
SHEET 1

PROJECT NUMBER:	CLFA142	DRAWING NUMBER:	CLFA142-CLFA-00-XX-DR-C-0505.1
STATUS:	S2	SCALE @ A1	1:500
		REV:	P05





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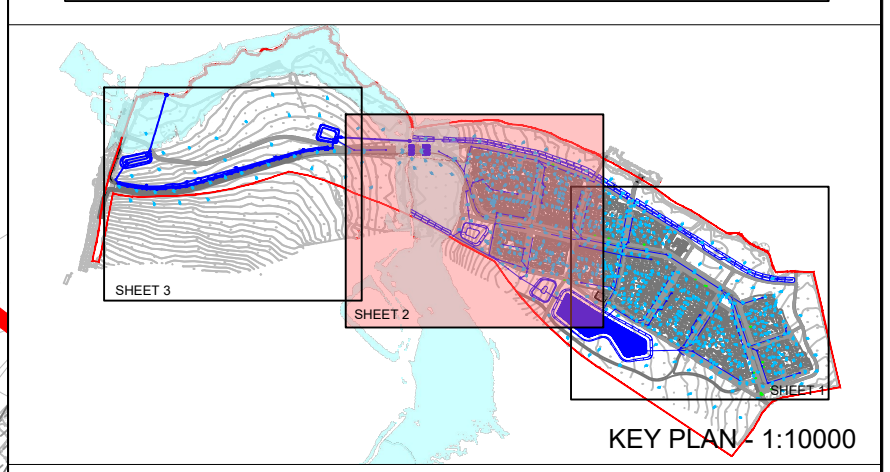
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KEY PLAN: 1:10000

P05	ROAD CONTOURS SHOWN	MR	FA	27/05/26
P04	MINOR SITE LAYOUT UPDATES	MR	FA	12/05/26
P03	SITE LAYOUT UPDATED	MR	FA	27/04/26
P02	SCALE BAR ADDED	CL	FA	09/12/25
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REV	DESCRIPTION	BY	APP	DATE



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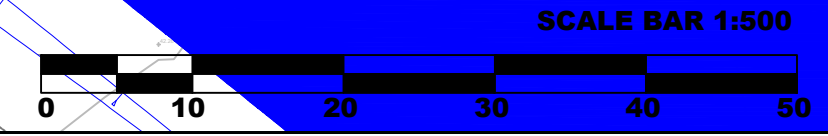
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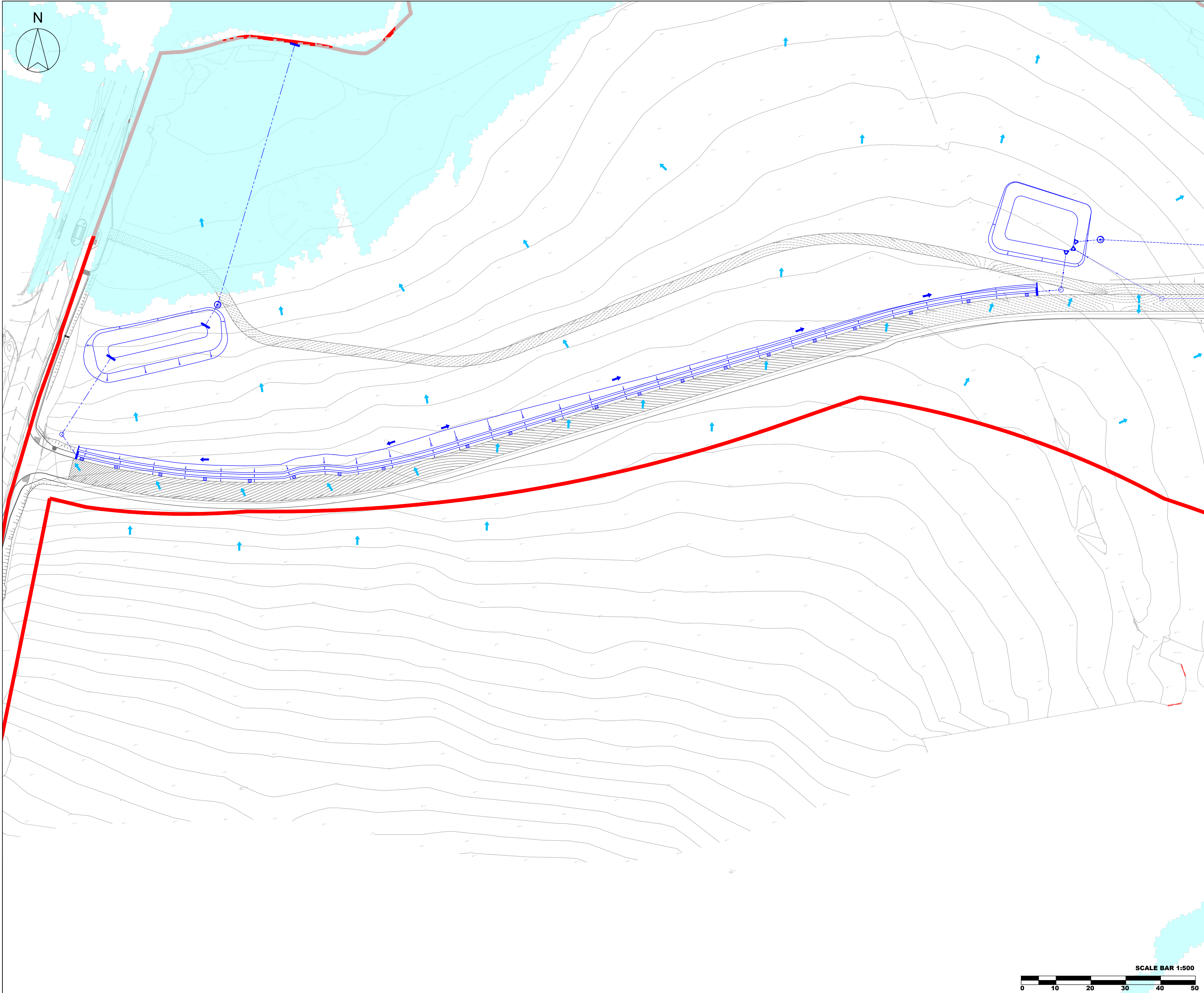


PROJECT:
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:
DRAINAGE EXCEEDANCE PLAN
SHEET 2

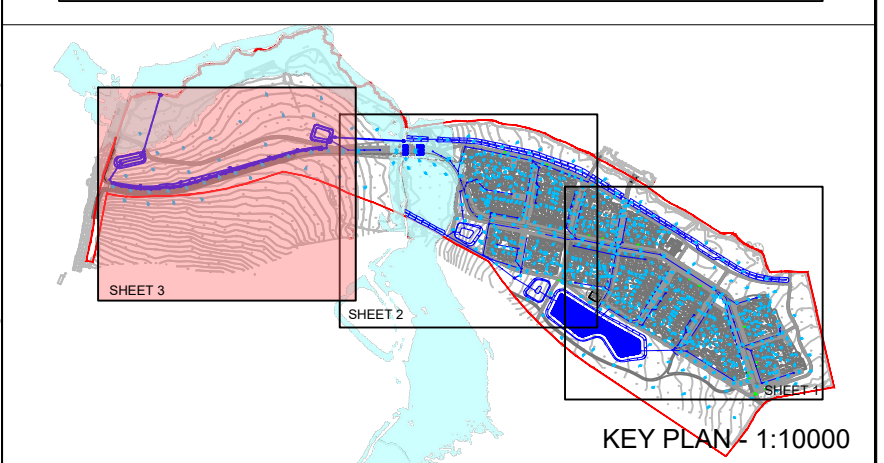
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STATUS: S2	SCALE @ A1 1:500
	REV: P05





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P05	ROAD CONTOURS SHOWN	MR	FA	27/05/26
P04	MINOR SITE LAYOUT UPDATES	MR	FA	12/05/26
P03	SITE LAYOUT UPDATED	MR	FA	27/04/26
P02	SCALE BAR ADDED	CL	FA	09/12/25
P01	FIRST ISSUE	MR	FA	10/11/25
REV	DESCRIPTION	BY	APP	DATE



71-75 Shelton Street, Covent Garden
London, WC2H 9JQ

CLIENT:


PROJECT:
SOUTHWICK COURT FARM
TROWBRIDGE

DRAWING:
DRAINAGE EXCEEDANCE PLAN
SHEET 3

PROJECT NUMBER: CLFA142	DRAWING NUMBER: CLFA142-CLFA-00-XX-DR-C-0505.3
STATUS: S2	SCALE @ A1 1:500
	REV: P05

**APPENDIX B.1 – 1 IN 1, 1 IN 100 AND 1 IN 100CC CALCULATIONS
ACCESS ROAD**

The Engineering Studio		Page 0
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 27/04/2026 08:16	Designed by Francisco	
File CLFA142-ACCESS ROAD DRA...	Checked by	
Innovyze		Network 2020.1.3

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Surface Network 1

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	21.000	Add Flow / Climate Change (%)	0
Ratio R	0.360	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Surface Network 1

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.054	4-8	0.072	8-12	0.002

Total Area Contributing (ha) = 0.128

Total Pipe Volume (m³) = 65.773

Network Design Table for Surface Network 1

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	150.863	1.536	98.2	0.058	5.00	0.0	0.600	-1	-1	Pipe/Conduit	
1.001	33.429	1.592	21.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.002	25.815	0.050	516.3	0.070	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.003	77.189	5.514	14.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	I.Area (ha)	Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	6.42	42.736	0.058	0.0	0.0	0.0	1.77	745.2	7.9
1.001	50.00	6.67	40.440	0.058	0.0	0.0	0.0	2.21	39.0	7.9
1.002	50.00	7.66	38.650	0.128	0.0	0.0	0.0	0.44	7.7«	17.3
1.003	50.00	8.13	38.600	0.128	0.0	0.0	0.0	2.71	47.8	17.3

The Engineering Studio		Page 1
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 27/04/2026 08:16 File CLFA142-ACCESS ROAD DRA...	Designed by Francisco Checked by	
Innovyze Network 2020.1.3		

Surcharged Outfall Details for Surface Network 1

Outfall Pipe Number	Outfall C. Level Name (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.003	S7	39.150	33.086	38.050	5000
Datum (m) 0.000 Offset (mins) 0					

Time (mins)	Depth (m)	Time (mins)	Depth (m)
940	40.170	1880	40.170

Simulation Criteria for Surface Network 1

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	2.000
Hot Start (mins)	0	Inlet Coefficient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1
Number of Input Hydrographs		0	Number of Storage Structures
Number of Online Controls		1	Number of Time/Area Diagrams
Number of Offline Controls		0	Number of Real Time Controls

Synthetic Rainfall Details

Rainfall Model	FSR	Profile Type	Summer
Return Period (years)	100	Cv (Summer)	0.750
Region	England and Wales	Cv (Winter)	0.840
M5-60 (mm)	21.000	Storm Duration (mins)	30
Ratio R	0.360		

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Innovyze	Network 2020.1.3	

Online Controls for Surface Network 1

Hydro-Brake® Optimum Manhole: S4, DS/PN: 1.003, Volume (m³): 2.4

Unit Reference	MD-SHE-0031-5000-1100-5000
Design Head (m)	1.100
Design Flow (l/s)	0.5
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	32
Invert Level (m)	38.600
Minimum Outlet Pipe Diameter (mm)	75
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.100	0.5
Flush-Flo™	0.141	0.3
Kick-Flo®	0.282	0.3
Mean Flow over Head Range	-	0.4

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.3	1.200	0.5	3.000	0.8	7.000	1.1
0.200	0.3	1.400	0.6	3.500	0.8	7.500	1.2
0.300	0.3	1.600	0.6	4.000	0.9	8.000	1.2
0.400	0.3	1.800	0.6	4.500	0.9	8.500	1.3
0.500	0.4	2.000	0.7	5.000	1.0	9.000	1.3
0.600	0.4	2.200	0.7	5.500	1.0	9.500	1.3
0.800	0.4	2.400	0.7	6.000	1.1		
1.000	0.5	2.600	0.7	6.500	1.1		

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Simulation Criteria

Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Rainfall Model	FSR	Ratio R	0.360
Region	England and Wales	Cv (Summer)	0.750
M5-60 (mm)	21.000	Cv (Winter)	0.840

Profile(s)	Summer and Winter
n(s) (mins)	15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080
(s) (years)	1, 100
Change (%)	0, 0

									Water
	US/MH		Return	Climate	First (X)	First (Y)	First (Z)	Overflow	Level
PN	Name	Storm	Period	Change	Surcharge	Flood	Overflow	Act.	(m)
1.000	Fake 1	15 Winter	100	+0%					42.788
1.001	SW1	15 Winter	100	+0%					40.534
1.002	Basin 1	2160 Winter	100	+0%	1/240 Winter				39.212
1.003	S4	2160 Winter	100	+0%	1/60 Winter				39.362

PN	US/MH Name	Surcharged	Flooded	Half Drain		Pipe	Status	Level Exceeded
		Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)		
1.000	Fake 1	-0.248	0.000	0.04			25.8	FLOOD RISK
1.001	SW1	-0.056	0.000	0.70			26.1	OK
1.002	Basin 1	0.412	0.000	0.06			0.4	SURCHARGED
1.003	S4	0.612	0.000	0.01			0.4	SURCHARGED

The Engineering Studio		Page 1															
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX																	
Date 27/04/2026 08:16 File CLFA142-ACCESS ROAD DRA...	Designed by Francisco Checked by																
Innovyze Network 2020.1.3																	
<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1</u>																	
	<table><thead><tr><th>PN</th><th>US/MH Name</th><th>Level Exceeded</th></tr></thead><tbody><tr><td>1.000</td><td>Fake 1</td><td></td></tr><tr><td>1.001</td><td>SW1</td><td></td></tr><tr><td>1.002</td><td>Basin 1</td><td></td></tr><tr><td>1.003</td><td>S4</td><td></td></tr></tbody></table>	PN	US/MH Name	Level Exceeded	1.000	Fake 1		1.001	SW1		1.002	Basin 1		1.003	S4		
PN	US/MH Name	Level Exceeded															
1.000	Fake 1																
1.001	SW1																
1.002	Basin 1																
1.003	S4																
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**APPENDIX B.2 – 1 IN 1, 1 IN 100 AND 1 IN 100CC CALCULATIONS
ROAD AND BRIDGE**

The Engineering Studio		Page 0
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 26/05/2026 09:15 File CLFA142-ROAD AND BRIDGE...	Designed by Francisco Checked by	
Innovyze	Network 2020.1.3	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Surface Network 2

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales			
Return Period (years)	100	PIMP (%)	100
M5-60 (mm)	21.000	Add Flow / Climate Change (%)	0
Ratio R	0.360	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Surface Network 2

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.036	4-8	0.127	8-12	0.010

Total Area Contributing (ha) = 0.173

Total Pipe Volume (m³) = 30.286

Network Design Table for Surface Network 2

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	62.815	0.155	405.3	0.043	5.00	0.0	0.600	-1	-1	Pipe/Conduit	
1.001	13.877	1.250	11.1	0.043	0.00	0.0	0.600	o	150	Pipe/Conduit	
2.000	41.829	0.403	103.8	0.028	5.00	0.0	0.600	o	150	Pipe/Conduit	
2.001	29.684	0.145	204.7	0.059	0.00	0.0	0.600	o	225	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	6.21	41.750	0.043	0.0	0.0	0.0	0.87	364.2	5.8
1.001	50.00	6.28	41.595	0.086	0.0	0.0	0.0	3.04	53.7	11.6
2.000	50.00	5.71	40.823	0.028	0.0	0.0	0.0	0.99	17.4	3.8
2.001	50.00	6.25	40.345	0.087	0.0	0.0	0.0	0.91	36.2	11.8

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Network Design Table for Surface Network 2

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.002	8.298	0.050	166.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.003	90.118	0.406	222.2	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.002	50.00	6.46	40.200	0.173	0.0	0.0	0.0	0.78	13.7«	23.4
1.003	50.00	8.70	40.150	0.173	0.0	0.0	0.0	0.67	11.8«	23.4

Surcharged Outfall Details for Surface Network 2

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,L (mm)	W (mm)
1.003	Outfall B	42.423	39.744	0.000	0	0

Datum (m) 0.000 Offset (mins) 0

Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)	Time (mins)	Depth (m)
1440	1.050	4320	1.050	7200	1.050	10080	1.050
2880	1.050	5760	1.050	8640	1.050		

Simulation Criteria for Surface Network 2

Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000
Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	2.000
Hot Start (mins)	0	Inlet Coeffiecient	0.800
Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000
Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60
Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1
Number of Input Hydrographs	0	Number of Storage Structures	1
Number of Online Controls	1	Number of Time/Area Diagrams	0
Number of Offline Controls	0	Number of Real Time Controls	0

Synthetic Rainfall Details

Rainfall Model	FSR	M5-60 (mm)	21.000
Return Period (years)	100	Ratio R	0.360
Region England and Wales Profile Type Summer			

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Synthetic Rainfall Details Cv (Summer) 0.750 Storm Duration (mins) 30 Cv (Winter) 0.840		
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Online Controls for Surface Network 2

Hydro-Brake® Optimum Manhole: SB3-FC, DS/PN: 1.003, Volume (m³): 1.9

Unit Reference	MD-SHE-0034-5000-0800-5000
Design Head (m)	0.800
Design Flow (l/s)	0.5
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	34
Invert Level (m)	40.150
Minimum Outlet Pipe Diameter (mm)	75
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	0.800	0.5
Flush-Flo™	0.151	0.4
Kick-Flo®	0.303	0.3
Mean Flow over Head Range	-	0.4

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	0.4	1.200	0.6	3.000	0.9	7.000	1.3
0.200	0.4	1.400	0.6	3.500	1.0	7.500	1.4
0.300	0.3	1.600	0.7	4.000	1.0	8.000	1.4
0.400	0.4	1.800	0.7	4.500	1.1	8.500	1.4
0.500	0.4	2.000	0.7	5.000	1.1	9.000	1.5
0.600	0.4	2.200	0.8	5.500	1.2	9.500	1.5
0.800	0.5	2.400	0.8	6.000	1.2		
1.000	0.6	2.600	0.8	6.500	1.3		

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<u>Storage Structures for Surface Network 2</u> <u>Tank or Pond Manhole: Basin 2, DS/PN: 1.002</u> Invert Level (m) 40.200 <table><tr><th>Depth (m)</th><th>Area (m²)</th><th>Depth (m)</th><th>Area (m²)</th></tr><tr><td>0.000</td><td>231.4</td><td>1.100</td><td>480.2</td></tr></table>			Depth (m)	Area (m²)	Depth (m)	Area (m²)	0.000	231.4	1.100	480.2
Depth (m)	Area (m²)	Depth (m)	Area (m²)							
0.000	231.4	1.100	480.2							
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1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)

for Surface Network 2

Simulation Criteria

Areal Reduction Factor 1.000

Hot Start (mins) 0

Hot Start Level (mm) 0

Manhole Headloss Coeff (Global) 0.500

Foul Sewage per hectare (l/s) 0.000

Additional Flow - % of Total Flow 0.000

MADD Factor * 10m³/ha Storage 2.000

Inlet Coefficient 0.800

Flow per Person per Day (l/per/day) 0.000

Number of Input Hydrographs 0

Number of Online Controls 1

Number of Offline Controls 0

Number of Storage Structures 1

Number of Time/Area Diagrams 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model

Region England and Wales

M5-60 (mm)

FSR

Cv (Summer)

21.000 Cv (Winter)

Ratio R 0.360

0.750

0.840

Margin for Flood Risk Warning (mm)

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

DVD Status

Inertia Status

300.0

ON

ON

OFF

Profile(s)

Duration(s) (mins)

Return Period(s) (years)

Climate Change (%)

Summer and Winter

15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

1, 100

0, 0

PN

US/MH Name

Storm

Return Period

Climate Change

First (X) Surge

First (Y) Flood

First (Z) Overflow

Overflow Act.

1.000

Fake 1

15 Winter

1

+0%

1.001

Fake 2

15 Winter

1

+0%

2.000

S30

15 Winter

1

+0%

2.001

S31-Catchpit

15 Winter

1

+0%

100/15 Summer

1.002

Basin 2

960 Winter

1

+0%

100/30 Summer

1.003

SB3-FC

960 Winter

1

+0%

1/180 Winter

PN

US/MH Name

Water Level (m)

Surcharged Depth (m)

Flooded Volume (m³)

Flow / Overflow Cap.

Half Drain Time (mins)

Pipe Flow (l/s)

Status

1.000

Fake 1

41.785

-0.265

0.000

0.02

5.6

FLOOD RISK

1.001

Fake 2

41.642

-0.103

0.000

0.21

10.4

FLOOD RISK

2.000

S30

40.872

-0.101

0.000

0.23

4.0

OK

2.001

S31-Catchpit

40.433

-0.137

0.000

0.32

10.9

OK

1.002

Basin 2

40.324

-0.026

0.000

0.06

0.7

OK

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<u>1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)</u> <u>for Surface Network 2</u>									
PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
1.003	SB3-FC	40.326	0.026	0.000	0.03			0.4	SURCHARGED

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100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2

PN	US/MH Name	Water	Surcharged	Flooded			Half Drain	Pipe	Status
		Level (m)	Depth (m)	Volume (m³)	Flow / Cap.	Overflow (l/s)	Time (mins)	Flow (l/s)	
1.003	SB3-FC	40.582	0.282	0.000	0.03			0.4	SURCHARGED

PN	US/MH Name	Level Exceeded
1.000	Fake 1	
1.001	Fake 2	
2.000	S30	
2.001	S31-Catchpit	
1.002	Basin 2	
1.003	SB3-FC	

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100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2

Simulation Criteria

Areal Reduction Factor 1.000

Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0

MADD Factor * 10m³/ha Storage 2.000

Hot Start Level (mm) 0

Inlet Coefficient 0.800

Manhole Headloss Coeff (Global) 0.500

Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 1

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model FSR

Ratio R 0.360

Region England and Wales Cv (Summer) 0.750

M5-60 (mm) 21.000

Cv (Winter) 0.840

Margin for Flood Risk Warning (mm) 300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status ON

DVD Status ON

Inertia Status OFF

Profile(s) Summer and Winter

Duration(s) (mins) 15, 30, 60, 120, 180, 240, 360, 480, 600, 720, 960, 1440, 2160, 2880, 4320, 5760, 7200, 8640, 10080

Return Period(s) (years) 100

Climate Change (%) 45

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.
1.000	Fake 1	15 Winter	100	+45%				
1.001	Fake 2	15 Winter	100	+45%				
2.000	S30	15 Winter	100	+45%	100/15 Summer			
2.001	S31-Catchpit	15 Winter	100	+45%	100/15 Summer			
1.002	Basin 2	2160 Winter	100	+45%	100/15 Summer			
1.003	SB3-FC	2160 Winter	100	+45%	100/15 Summer			

PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Half Drain Time (mins)	Pipe Flow (l/s)	Status
1.000	Fake 1	41.836	-0.214	0.000	0.08		26.1	FLOOD RISK
1.001	Fake 2	41.717	-0.028	0.000	1.00		49.1	FLOOD RISK
2.000	S30	41.166	0.193	0.000	0.99		16.7	SURCHARGED
2.001	S31-Catchpit	40.784	0.214	0.000	1.56		52.7	SURCHARGED
1.002	Basin 2	40.759	0.409	0.000	0.08		1.0	SURCHARGED

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APPENDIX B.3 – 1 IN 1, 1 IN 100 AND 1 IN 100CC CALCULATIONS
MAIN SITE

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STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Surface Network 2

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	2	PIMP (%)	100
M5-60 (mm)	21.000	Add Flow / Climate Change (%)	0
Ratio R	0.360	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Surface Network 2

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.011	4-8	0.753	8-12	0.276	12-16	1.574	16-20	0.902	20-24	0.024

Total Area Contributing (ha) = 3.541

Total Pipe Volume (m³) = 191.511

Network Design Table for Surface Network 2

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	44.076	0.467	94.4	0.080	5.00	0.0	0.600	o	225	Pipe/Conduit	
1.001	45.352	0.769	59.0	0.032	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.002	11.731	1.003	11.7	0.028	0.00	0.0	0.600	o	225	Pipe/Conduit	
2.000	8.434	0.056	150.6	0.050	5.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	E I.Area (ha)	E Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	5.55	44.811	0.080	0.0	0.0	0.0	1.35	53.5	10.8
1.001	50.00	5.99	44.344	0.112	0.0	0.0	0.0	1.71	67.8	15.2
1.002	50.00	6.04	43.575	0.140	0.0	0.0	0.0	3.85	153.0	19.0
2.000	50.00	5.17	43.937	0.050	0.0	0.0	0.0	0.82	14.4	6.8

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Online Controls for Surface Network 2

Hydro-Brake® Optimum Manhole: Pond 1, DS/PN: 1.010, Volume (m³): 29.6

Unit Reference	MD-SHE-0063-2000-1300-2000
Design Head (m)	1.300
Design Flow (l/s)	2.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	63
Invert Level (m)	40.200
Minimum Outlet Pipe Diameter (mm)	100
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.300	2.0
Flush-Flo™	0.280	1.7
Kick-Flo®	0.564	1.4
Mean Flow over Head Range	-	1.6

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	1.4	1.200	1.9	3.000	2.9	7.000	4.4
0.200	1.7	1.400	2.1	3.500	3.2	7.500	4.5
0.300	1.7	1.600	2.2	4.000	3.4	8.000	4.6
0.400	1.6	1.800	2.3	4.500	3.5	8.500	4.8
0.500	1.5	2.000	2.4	5.000	3.7	9.000	4.9
0.600	1.4	2.200	2.5	5.500	3.9	9.500	5.0
0.800	1.6	2.400	2.6	6.000	4.1		
1.000	1.8	2.600	2.7	6.500	4.2		

Hydro-Brake® Optimum Manhole: Basin 4, DS/PN: 1.011, Volume (m³): 15.3

Unit Reference	MD-SHE-0054-2000-2500-2000
Design Head (m)	2.500
Design Flow (l/s)	2.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	54
Invert Level (m)	40.100
Minimum Outlet Pipe Diameter (mm)	75
Suggested Manhole Diameter (mm)	1200

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<u>Hydro-Brake® Optimum Manhole: Basin 4, DS/PN: 1.011, Volume (m³): 15.3</u>																																																																										
<table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>2.500</td><td>2.0</td></tr><tr><td>Flush-Flo™</td><td>0.239</td><td>1.2</td></tr><tr><td>Kick-Flo®</td><td>0.484</td><td>1.0</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>1.4</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	2.500	2.0	Flush-Flo™	0.239	1.2	Kick-Flo®	0.484	1.0	Mean Flow over Head Range	-	1.4																																																									
Control Points	Head (m)	Flow (l/s)																																																																								
Design Point (Calculated)	2.500	2.0																																																																								
Flush-Flo™	0.239	1.2																																																																								
Kick-Flo®	0.484	1.0																																																																								
Mean Flow over Head Range	-	1.4																																																																								
The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated																																																																										
<table><tr><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td></tr><tr><td>0.100</td><td>1.0</td><td>1.200</td><td>1.4</td><td>3.000</td><td>2.2</td><td>7.000</td><td>3.2</td></tr><tr><td>0.200</td><td>1.2</td><td>1.400</td><td>1.5</td><td>3.500</td><td>2.3</td><td>7.500</td><td>3.3</td></tr><tr><td>0.300</td><td>1.2</td><td>1.600</td><td>1.6</td><td>4.000</td><td>2.5</td><td>8.000</td><td>3.4</td></tr><tr><td>0.400</td><td>1.1</td><td>1.800</td><td>1.7</td><td>4.500</td><td>2.6</td><td>8.500</td><td>3.5</td></tr><tr><td>0.500</td><td>1.0</td><td>2.000</td><td>1.8</td><td>5.000</td><td>2.8</td><td>9.000</td><td>3.6</td></tr><tr><td>0.600</td><td>1.1</td><td>2.200</td><td>1.9</td><td>5.500</td><td>2.9</td><td>9.500</td><td>3.7</td></tr><tr><td>0.800</td><td>1.2</td><td>2.400</td><td>2.0</td><td>6.000</td><td>3.0</td><td></td><td></td></tr><tr><td>1.000</td><td>1.3</td><td>2.600</td><td>2.0</td><td>6.500</td><td>3.1</td><td></td><td></td></tr></table>			Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	1.0	1.200	1.4	3.000	2.2	7.000	3.2	0.200	1.2	1.400	1.5	3.500	2.3	7.500	3.3	0.300	1.2	1.600	1.6	4.000	2.5	8.000	3.4	0.400	1.1	1.800	1.7	4.500	2.6	8.500	3.5	0.500	1.0	2.000	1.8	5.000	2.8	9.000	3.6	0.600	1.1	2.200	1.9	5.500	2.9	9.500	3.7	0.800	1.2	2.400	2.0	6.000	3.0			1.000	1.3	2.600	2.0	6.500	3.1		
Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)																																																																			
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1.000	1.3	2.600	2.0	6.500	3.1																																																																					
<u>Hydro-Brake® Optimum Manhole: Basin 3, DS/PN: 15.007, Volume (m³): 14.7</u>																																																																										
<table><tr><td>Unit Reference</td><td>MD-SHE-0096-5000-1650-5000</td></tr><tr><td>Design Head (m)</td><td>1.650</td></tr><tr><td>Design Flow (l/s)</td><td>5.0</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>96</td></tr><tr><td>Invert Level (m)</td><td>39.650</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>150</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table>			Unit Reference	MD-SHE-0096-5000-1650-5000	Design Head (m)	1.650	Design Flow (l/s)	5.0	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	96	Invert Level (m)	39.650	Minimum Outlet Pipe Diameter (mm)	150	Suggested Manhole Diameter (mm)	1200																																																		
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<table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>1.650</td><td>5.0</td></tr><tr><td>Flush-Flo™</td><td>0.419</td><td>4.6</td></tr><tr><td>Kick-Flo®</td><td>0.856</td><td>3.7</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>4.2</td></tr></table>			Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	1.650	5.0	Flush-Flo™	0.419	4.6	Kick-Flo®	0.856	3.7	Mean Flow over Head Range	-	4.2																																																									
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0.100	3.1	0.300	4.5	0.500	4.6	0.800	4.0																																																																			
0.200	4.2	0.400	4.6	0.600	4.5	1.000	4.0																																																																			
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Hydro-Brake® Optimum Manhole: Basin 3, DS/PN: 15.007, Volume (m³): 14.7

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
1.200	4.3	2.400	6.0	5.000	8.4	8.000	10.5
1.400	4.6	2.600	6.2	5.500	8.8	8.500	10.8
1.600	4.9	3.000	6.6	6.000	9.2	9.000	11.1
1.800	5.2	3.500	7.1	6.500	9.5	9.500	11.4
2.000	5.5	4.000	7.6	7.000	9.9		
2.200	5.7	4.500	8.0	7.500	10.2		

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<u>1 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2</u>									
PN	US/MH Name	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)
6.001	Swale P55		41.182	-0.168	0.000	0.15			8.6
7.000	P57		42.614	-0.068	0.000	0.21			2.7
7.001	P59		41.245	-0.055	0.000	0.41			2.6
1.007	S8		40.861	-0.314	0.000	0.56			175.3
1.008	S31		40.775	-0.366	0.000	0.41			183.7
1.009	S9		40.646	-0.346	0.000	0.48			198.9
8.000	Swale P89		43.279	-0.171	0.000	0.13			9.8
8.001	Swale P122		41.791	-0.209	0.000	0.20			15.7
9.000	P127		43.048	-0.102	0.000	0.22			3.2
9.001	P129		42.890	-0.110	0.000	0.15			4.4
9.002	P131.1		42.663	-0.114	0.000	0.13			4.4
9.003	P131.2		41.872	-0.097	0.000	0.27			4.4
8.002	S13		41.605	-0.255	0.000	0.22			47.0
10.000	Swale P82		42.729	-0.121	0.000	0.08			1.5
10.001	Swale P141		42.497	-0.153	0.000	0.22			9.4
10.002	Swale P146		42.178	-0.171	0.000	0.13			13.7
10.003	Swale P147		41.093	-0.207	0.000	0.21			14.5
11.000	P139		42.592	-0.108	0.000	0.17			3.2
11.001	P135.1		42.290	-0.110	0.000	0.16			4.4
11.002	P135.2		42.088	-0.112	0.000	0.15			4.4
11.003	P134		41.272	-0.160	0.000	0.18			9.3
10.004	S10		41.021	-0.201	0.000	0.24			30.3
10.005	S11		40.492	-0.228	0.000	0.00			0.7
1.010	Pond 1		40.492	0.142	0.000	0.21			1.1
12.000	P14		42.900	-0.100	0.000	0.00			0.0
12.001	P8		41.296	-0.104	0.000	0.20			3.6
12.002	S14		41.002	-0.217	0.000	0.17			16.2
12.003	S15		40.667	-0.198	0.000	0.25			19.0
12.004	S16		40.572	-0.200	0.000	0.06			5.0
13.000	P179		42.387	-0.082	0.000	0.41			6.9
13.001	P175		42.093	-0.086	0.000	0.38			7.0
13.002	P174		41.857	-0.115	0.000	0.13			7.0
14.000	Swale P169		43.127	-0.123	0.000	0.08			3.2
12.005	S17		40.570	0.069	0.000	0.16			9.6
12.006	S18		40.569	0.117	0.000	0.15			9.4
1.011	Basin 4		40.567	0.167	0.000	0.02			1.2
1.012	S19		39.935	-0.277	0.000	0.02			1.2
15.000	S30		42.510	-0.237	0.000	0.10			13.0
16.000	Swale P14.1		43.186	-0.114	0.000	0.13			2.8
16.001	Swale 14.2		42.886	-0.166	0.000	0.15			13.1
16.002	Swale P21		41.463	-0.137	0.000	0.32			13.1
17.000	P32		43.100	-0.100	0.000	0.00			0.0
17.001	P28.1		42.878	-0.072	0.000	0.17			2.2
17.002	P28.2		42.589	-0.072	0.000	0.17			2.1
17.003	P27		41.676	-0.064	0.000	0.28			2.2
15.001	S20		41.273	-0.244	0.000	0.26			41.0
15.002	S21		40.794	-0.223	0.000	0.34			47.1
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<u>1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)</u> <u>for Surface Network 2</u>			
	US/MH		Level
PN	Name	Status	Exceeded
17.000	P32	OK	
17.001	P28.1	OK	
17.002	P28.2	OK	
17.003	P27	OK	
15.001	S20	OK	
15.002	S21	OK	
15.003	S22	OK	
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<u>1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)</u> <u>for Surface Network 2</u>										
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow			
18.000	S23	15 Winter	1	+0%	100/15 Summer					
18.001	S24	15 Winter	1	+0%	100/15 Summer					
15.004	S25	15 Winter	1	+0%	100/15 Summer	100/720 Winter				
19.000	P18	15 Winter	1	+0%						
19.001	P5	15 Winter	1	+0%						
19.002	P4.1	15 Winter	1	+0%						
19.003	P4.2	15 Winter	1	+0%	100/15 Summer	100/720 Winter				
20.000	S26	15 Winter	1	+0%						
20.001	S27	15 Winter	1	+0%						
15.005	S28	15 Winter	1	+0%	100/15 Summer	100/720 Winter				
15.006	S29	15 Winter	1	+0%	100/30 Winter					
15.007	Basin 3	240 Winter	1	+0%	1/60 Summer					
1.013	Swale Inlet	480 Winter	1	+0%						
PN	US/MH Name	Overflow Act.	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	
18.000	S23		41.018	-0.091	0.000	0.32			6.3	
18.001	S24		40.710	-0.062	0.000	0.64			8.7	
15.004	S25		40.359	-0.292	0.000	0.26			58.4	
19.000	P18		42.276	-0.128	0.000	0.05			1.5	
19.001	P5		41.241	-0.169	0.000	0.14			9.4	
19.002	P4.1		40.861	-0.174	0.000	0.11			9.4	
19.003	P4.2		40.444	-0.156	0.000	0.20			9.5	
20.000	S26		40.855	-0.181	0.000	0.08			3.4	
20.001	S27		40.738	-0.191	0.000	0.05			3.4	
15.005	S28		40.262	-0.218	0.000	0.52			80.7	
15.006	S29		40.173	-0.269	0.000	0.34			80.6	
15.007	Basin 3		40.054	0.104	0.000	0.11			4.6	
1.013	Swale Inlet		39.702	-0.048	0.000	0.80			5.6	
PN	US/MH Name	Status	Level Exceeded							
18.000	S23	OK								
18.001	S24	OK								
15.004	S25	OK								
19.000	P18	OK								
19.001	P5	OK								
19.002	P4.1	OK								
19.003	P4.2	OK								
20.000	S26	OK								
20.001	S27	OK								
15.005	S28	OK								

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<u>1 year Return Period Summary of Critical Results by Maximum Level (Rank 1)</u> <u>for Surface Network 2</u>		

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<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2</u>			
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<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2</u>									
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surcharge	First (Y) Flood	First (Z) Overflow	Overflow Act.	
18.000	S23	15 Winter	100	+45%	100/15 Summer				
18.001	S24	15 Winter	100	+45%	100/15 Summer				
15.004	S25	240 Winter	100	+45%	100/15 Summer				
19.000	P18	15 Winter	100	+45%					
19.001	P5	15 Winter	100	+45%					
19.002	P4.1	15 Winter	100	+45%	100/15 Summer				
19.003	P4.2	240 Winter	100	+45%	100/15 Summer				
20.000	S26	240 Winter	100	+45%	100/15 Winter				
20.001	S27	240 Winter	100	+45%	100/15 Summer				
15.005	S28	240 Winter	100	+45%	100/15 Summer				
15.006	S29	600 Winter	100	+45%	100/15 Summer				
15.007	Basin 3	600 Winter	100	+45%	100/15 Summer				
1.013	Swale Inlet	600 Winter	100	+45%					
PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
18.000	S23	42.049	0.940	0.000	1.04			20.1	FLOOD RISK
18.001	S24	41.680	0.908	0.000	2.02			27.3	FLOOD RISK
15.004	S25	41.265	0.614	0.000	0.27			59.1	SURCHARGED
19.000	P18	42.304	-0.100	0.000	0.24			7.1	OK
19.001	P5	41.357	-0.053	0.000	0.78			52.8	OK
19.002	P4.1	41.204	0.169	0.000	0.59			48.5	SURCHARGED
19.003	P4.2	41.189	0.589	0.000	0.21			9.8	SURCHARGED
20.000	S26	41.188	0.152	0.000	0.08			3.1	SURCHARGED
20.001	S27	41.186	0.257	0.000	0.05			3.1	SURCHARGED
15.005	S28	41.184	0.704	0.000	0.54			83.5	FLOOD RISK
15.006	S29	41.165	0.723	0.000	0.18			42.1	FLOOD RISK
15.007	Basin 3	41.163	1.213	0.000	0.11			4.7	SURCHARGED
1.013	Swale Inlet	39.710	-0.040	0.000	0.89			6.3	OK
PN	US/MH Name	Level Exceeded							
18.000	S23								
18.001	S24								
15.004	S25								
19.000	P18								
19.001	P5								
19.002	P4.1								
19.003	P4.2								
20.000	S26								
20.001	S27								
15.005	S28								

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<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2</u>		

APPENDIX C.1 – GREENFIELD RUNOFF CALCULATIONS ACCESS ROAD

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	27/04/2026
Calculated by	
Reference	
Model version	2.2.3

Location

Site name	Court Farm
Site location	Trowbridge



Site easting (British National Grid)	384673
Site northing (British National Grid)	155948

Site details

Total site area (ha)	0.0578	ha
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Greenfield runoff

Method

Method	IH124
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IH124

SAAR (mm)	738	mm		738
How should SPR be derived?	WRAP soil type			
WRAP soil type	4		4	
SPR	0.47			
QBar (IH124) (l/s)	0.3	l/s		

Growth curve factors

Hydrological region	8		8
1 year growth factor	0.78		
2 year growth factor	0.93		
10 year growth factor	1.49		
30 year growth factor	1.95		
100 year growth factor	2.43		
200 year growth factor	2.78		

Results

Method	IH124	
Flow rate 1 year (l/s)	0.2	l/s
Flow rate 2 year (l/s)	0.3	l/s
Flow rate 10 years (l/s)	0.4	l/s
Flow rate 30 years (l/s)	0.6	l/s
Flow rate 100 years (l/s)	0.7	l/s
Flow rate 200 years (l/s)	0.8	l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent 'zero' figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.3) developed by HR Wallingford and available at uksuds.com (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at uksuds.com/terms-conditions (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

APPENDIX C.2 – GREENFIELD RUNOFF CALCULATIONS ROAD AND BRIDGE

Results

Method	IH124	
Flow rate 1 year (l/s)	0.7	l/s
Flow rate 2 year (l/s)	0.8	l/s
Flow rate 10 years (l/s)	1.3	l/s
Flow rate 30 years (l/s)	1.7	l/s
Flow rate 100 years (l/s)	2.2	l/s
Flow rate 200 years (l/s)	2.5	l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent 'zero' figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

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This report was produced using the Greenfield runoff rate estimation tool (2.2.3) developed by HR Wallingford and available at uksuds.com (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at uksuds.com/terms-conditions (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

APPENDIX C.3 – GREENFIELD RUNOFF CALCULATIONS
MAIN SITE

This is an estimation of the greenfield runoff rates that are used to meet normal best practice criteria in line with Environment Agency guidance “Rainfall runoff management for developments”, SC030219 (2013), the SuDS Manual C753 (CIRIA, 2015) and the non-statutory standards for SuDS (Defra, 2015). This information on greenfield runoff rates may be the basis for setting consents for the drainage of surface water runoff from sites.

Project details

Date	27/04/2026
Calculated by	
Reference	
Model version	2.2.3

Location

Site name	Court Farm
Site location	Trowbridge



Site easting (British National Grid)	384673
Site northing (British National Grid)	155948

Site details

Total site area (ha)	3.5299	ha
----------------------	-------------------	----

Greenfield runoff

Method

Method	IH124
--------	------------------

IH124

SAAR (mm)	738	mm		738
How should SPR be derived?	WRAP soil type			
WRAP soil type	4		4	
SPR	0.47			
QBar (IH124) (l/s)	18.1	l/s		

Growth curve factors

Hydrological region	8		8
1 year growth factor	0.78		
2 year growth factor	0.93		
10 year growth factor	1.49		
30 year growth factor	1.95		
100 year growth factor	2.43		
200 year growth factor	2.78		

Results

Method	IH124	
Flow rate 1 year (l/s)	14.1	l/s
Flow rate 2 year (l/s)	16.9	l/s
Flow rate 10 years (l/s)	27.0	l/s
Flow rate 30 years (l/s)	35.4	l/s
Flow rate 100 years (l/s)	44.1	l/s
Flow rate 200 years (l/s)	50.4	l/s

Please note runoff estimation is subject to significant uncertainty. Results are therefore normally reported to only 1 decimal place. Where 2 decimal places are provided, this does not indicate accuracy to this level, it has been adopted to prevent 'zero' figures from being reported. Outputs less than 0.01 l/s are reported as 0.01 l/s.

Disclaimer

This report was produced using the Greenfield runoff rate estimation tool (2.2.3) developed by HR Wallingford and available at uksuds.com (<https://www.uksuds.com/>). The use of this tool is subject to the UK SuDS terms and conditions and licence agreement, which can both be found at uksuds.com/terms-conditions (<https://www.uksuds.com/terms-conditions>). The outputs from this tool have been used to estimate Greenfield runoff rates. The use of these results is the responsibility of the users of this tool. No liability will be accepted by HR Wallingford, the Environment Agency, Centre for Ecology and Hydrology, Wallingford Hydrosolutions or any other organisation for the use of these data in the design or operational characteristics of any drainage scheme.

**APPENDIX D.1 – GREENFIELD VOLUME CALCULATIONS
ACCESS ROAD**

The Engineering Studio		Page 1
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 27/04/2026 07:11 File	Designed by Francisco Checked by	
Innovyze Source Control 2020.1.3		

Greenfield Runoff Volume

FSR Data

Return Period (years)	100
Storm Duration (mins)	360
Region	England and Wales
M5-60 (mm)	20.900
Ratio R	0.350
Areal Reduction Factor	1.00
Area (ha)	0.058
SAAR (mm)	733
CWI	109.258
Urban	0.000
SPR	10.000

Results

Percentage Runoff (%)	10.74
Greenfield Runoff Volume (m ³)	4.259

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APPENDIX D.2 – GREENFIELD VOLUME CALCULATIONS ROAD AND BRIDGE

The Engineering Studio		Page 1
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 27/04/2026 07:11 File	Designed by Francisco Checked by	
Innovyze Source Control 2020.1.3		

Greenfield Runoff Volume

FSR Data

Return Period (years)	100
Storm Duration (mins)	360
Region	England and Wales
M5-60 (mm)	20.900
Ratio R	0.350
Areal Reduction Factor	1.00
Area (ha)	0.174
SAAR (mm)	733
CWI	109.258
Urban	0.000
SPR	10.000

Results

Percentage Runoff (%)	10.74
Greenfield Runoff Volume (m³)	12.778

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APPENDIX D.3 – GREENFIELD VOLUME CALCULATIONS
MAIN SITE

The Engineering Studio		Page 1
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 27/04/2026 07:12 File	Designed by Francisco Checked by	
Innovyze Source Control 2020.1.3		

Greenfield Runoff Volume

FSR Data

Return Period (years)	100
Storm Duration (mins)	360
Region	England and Wales
M5-60 (mm)	20.900
Ratio R	0.350
Areal Reduction Factor	1.00
Area (ha)	3.530
SAAR (mm)	733
CWI	109.258
Urban	0.000
SPR	10.000

Results

Percentage Runoff (%)	10.74
Greenfield Runoff Volume (m³)	259.229

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**APPENDIX E.1 – 1 IN 100 YEAR, 6 HOUR EVENT CALCULATIONS
ACCESS ROAD**

The Engineering Studio						Page 0			
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX									
Date 27/04/2026 08:30 File CLFA142-ACCESS ROAD DRA...				Designed by Francisco Checked by					
Innovyze				Network 2020.1.3					
<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1</u>									
<u>Simulation Criteria</u>									
Areal Reduction Factor		1.000		Additional Flow - % of Total Flow		0.000			
Hot Start (mins)		0		MADD Factor * 10m³/ha Storage		2.000			
Hot Start Level (mm)		0		Inlet Coefficient		0.800			
Manhole Headloss Coeff (Global)		0.500		Flow per Person per Day (l/per/day)		0.000			
Foul Sewage per hectare (l/s)		0.000							
Number of Input Hydrographs				0		Number of Storage Structures		1	
Number of Online Controls				1		Number of Time/Area Diagrams		0	
Number of Offline Controls				0		Number of Real Time Controls		0	
<u>Synthetic Rainfall Details</u>									
Rainfall Model		FSR		Ratio R		0.360			
Region England and Wales Cv (Summer)		0.750							
M5-60 (mm)		21.000		Cv (Winter)		0.840			
Margin for Flood Risk Warning (mm)				300.0		DVD Status		OFF	
Analysis Timestep				Fine Inertia		Status		OFF	
DTS Status				ON					
Profile(s) Summer and Winter									
Duration(s) (mins)		360							
Return Period(s) (years)		100							
Climate Change (%)		0							
PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.	
1.000	Fake 1	360 Summer	100	+0%					
1.001	SW1	360 Summer	100	+0%					
1.002	Basin 1	360 Winter	100	+0%	100/360 Summer				
1.003	S4	360 Winter	100	+0%	100/360 Summer				
PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
1.000	Fake 1	42.753	-0.283	0.000	0.01			5.2	FLOOD RISK
1.001	SW1	40.477	-0.113	0.000	0.14			5.2	OK
1.002	Basin 1	39.097	0.297	0.000	0.07			0.5	SURCHARGED
1.003	S4	39.106	0.356	0.000	0.01			0.4	SURCHARGED
PN	US/MH Name	Level Exceeded							
1.000	Fake 1								
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The Engineering Studio		Page 1															
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX																	
Date 27/04/2026 08:30 File CLFA142-ACCESS ROAD DRA...	Designed by Francisco Checked by																
Innovyze	Network 2020.1.3																
<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 1</u>																	
	<table><tr><td></td><td>US/MH</td><td>Level</td></tr><tr><td>PN</td><td>Name</td><td>Exceeded</td></tr><tr><td>1.001</td><td>SW1</td><td></td></tr><tr><td>1.002</td><td>Basin 1</td><td></td></tr><tr><td>1.003</td><td>S4</td><td></td></tr></table>		US/MH	Level	PN	Name	Exceeded	1.001	SW1		1.002	Basin 1		1.003	S4		
	US/MH	Level															
PN	Name	Exceeded															
1.001	SW1																
1.002	Basin 1																
1.003	S4																
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**APPENDIX E.2 – 1 IN 100 YEAR, 6 HOUR EVENT CALCULATIONS
ROAD AND BRIDGE**

The Engineering Studio

Generator Building, Counterslip
Redcliffe
Bristol, BS1 6BX

Date 26/05/2026 09:19
File CLFA142-ROAD AND BRIDGE...

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Page 0



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Network 2020.1.3

100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2

Simulation Criteria

Areal Reduction Factor 1.000

Additional Flow - % of Total Flow 0.000

Hot Start (mins) 0

MADD Factor * 10m³/ha Storage 2.000

Hot Start Level (mm) 0

Inlet Coefficient 0.800

Manhole Headloss Coeff (Global) 0.500

Flow per Person per Day (l/per/day) 0.000

Foul Sewage per hectare (l/s) 0.000

Number of Input Hydrographs 0

Number of Storage Structures 1

Number of Online Controls 1

Number of Time/Area Diagrams 0

Number of Offline Controls 0

Number of Real Time Controls 0

Synthetic Rainfall Details

Rainfall Model

FSR

Ratio R 0.360

Region England and Wales Cv (Summer) 0.750

M5-60 (mm)

21.000 Cv (Winter) 0.840

Margin for Flood Risk Warning (mm)

300.0

Analysis Timestep 2.5 Second Increment (Extended)

DTS Status

ON

DVD Status

ON

Inertia Status

OFF

Profile(s) Summer and Winter

Duration(s) (mins)

360

Return Period(s) (years)

100

Climate Change (%)

0

PN	US/MH Name	Storm	Return Period	Climate Change	First (X) Surge	First (Y) Flood	First (Z) Overflow	Overflow Act.
1.000	Fake 1	360 Summer	100	+0%				
1.001	Fake 2	360 Summer	100	+0%				
2.000	S30	360 Summer	100	+0%				
2.001	S31-Catchpit	360 Winter	100	+0%				
1.002	Basin 2	360 Winter	100	+0%	100/360 Summer			
1.003	SB3-FC	360 Winter	100	+0%	100/360 Summer			

PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
1.000	Fake 1	41.778	-0.272	0.000	0.01			4.0	FLOOD RISK
1.001	Fake 2	41.635	-0.110	0.000	0.16			7.9	FLOOD RISK
2.000	S30	40.862	-0.111	0.000	0.15			2.6	OK
2.001	S31-Catchpit	40.538	-0.032	0.000	0.17			5.8	OK
1.002	Basin 2	40.537	0.187	0.000	0.04			0.5	SURCHARGED
1.003	SB3-FC	40.537	0.237	0.000	0.03			0.4	SURCHARGED

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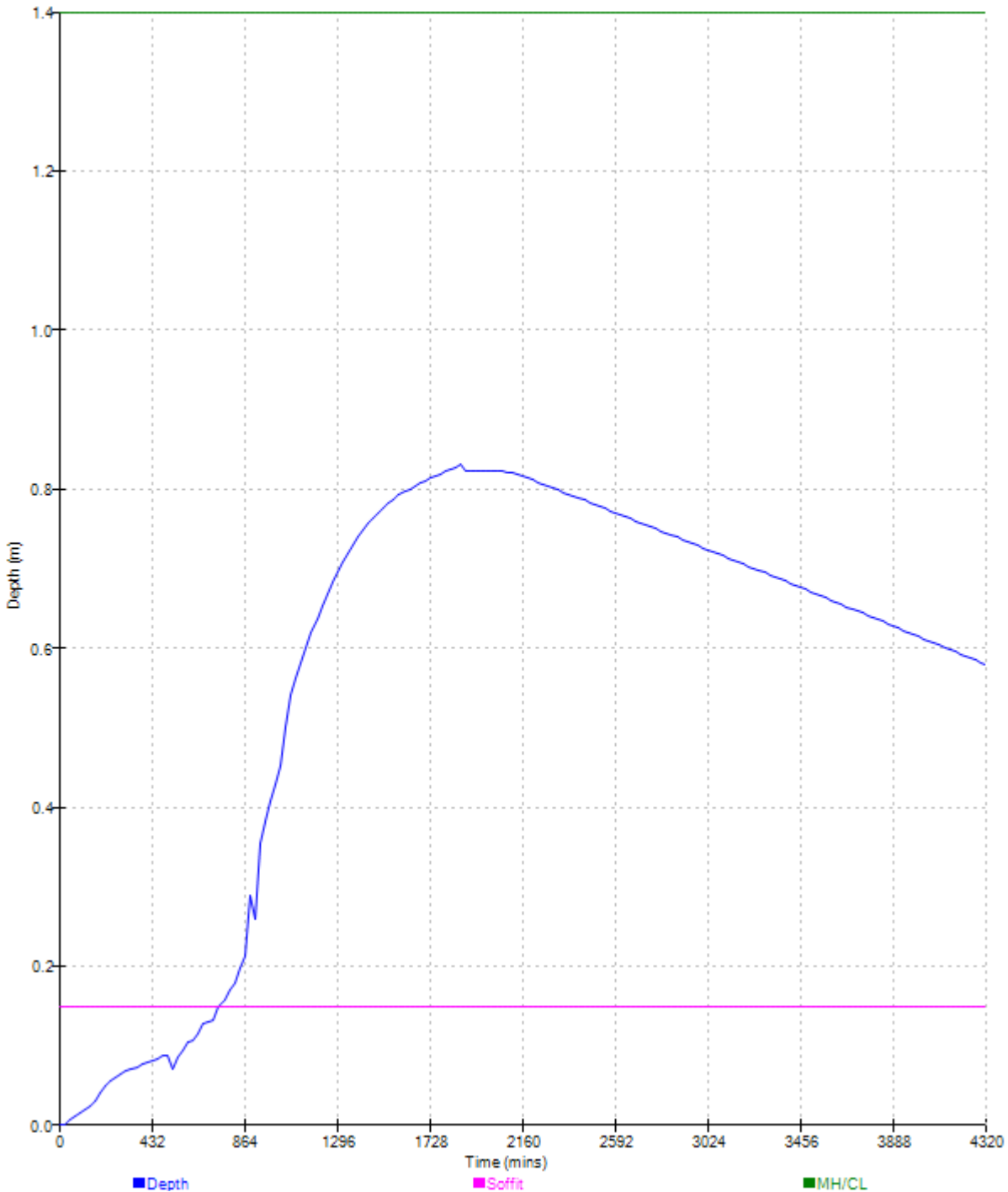
The Engineering Studio		Page 1
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX		
Date 26/05/2026 09:19 File CLFA142-ROAD AND BRIDGE...	Designed by Francisco Checked by	
Innovyze	Network 2020.1.3	
<u>100 year Return Period Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2</u>		

APPENDIX E.3– 1 IN 100 YEAR, 6 HOUR EVENT CALCULATIONS
MAIN SITE

The Engineering Studio								Page 3	
Generator Building, Counterslip Redcliffe Bristol, BS1 6BX									
Date 26/05/2026 10:53 File CLFA142-MAIN SITE - NEW...				Designed by Francisco Checked by					
Innovyze				Network 2020.1.3					
Summary of Critical Results by Maximum Level (Rank 1) for Surface Network 2									
PN	US/MH Name	Water Level (m)	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status
1.007	S8	40.812	-0.363	0.000	0.44			138.6	OK
1.008	S31	40.740	-0.401	0.000	0.33			146.9	OK
1.009	S9	40.609	-0.383	0.000	0.39			162.6	OK
8.000	Swale P89	43.268	-0.182	0.000	0.08			6.2	OK
8.001	Swale P122	41.772	-0.228	0.000	0.13			10.6	OK
9.000	P127	43.037	-0.113	0.000	0.14			2.1	OK
9.001	P129	42.882	-0.118	0.000	0.10			2.9	OK
9.002	P131.1	42.657	-0.120	0.000	0.09			2.9	OK
9.003	P131.2	41.862	-0.107	0.000	0.18			2.9	OK
8.002	S13	41.585	-0.275	0.000	0.16			34.0	OK
10.000	Swale P82	42.722	-0.128	0.000	0.05			1.0	OK
10.001	Swale P141	42.485	-0.165	0.000	0.16			6.9	OK
10.002	Swale P146	42.170	-0.179	0.000	0.10			10.2	OK
10.003	Swale P147	41.078	-0.222	0.000	0.15			10.8	OK
11.000	P139	42.583	-0.117	0.000	0.11			2.1	OK
11.001	P135.1	42.282	-0.118	0.000	0.11			2.9	OK
11.002	P135.2	42.081	-0.119	0.000	0.10			2.9	OK
11.003	P134	41.261	-0.171	0.000	0.13			6.6	OK
10.004	S10	41.006	-0.216	0.000	0.17			22.3	OK
10.005	S11	40.602	-0.118	0.000	0.14			19.5	OK
1.010	Pond 1	40.602	0.252	0.000	0.01			0.1	SURCHARGED
12.000	P14	42.900	-0.100	0.000	0.00			0.0	OK
12.001	P8	41.289	-0.111	0.000	0.15			2.7	OK
12.002	S14	41.270	0.051	0.000	0.09			9.0	SURCHARGED
12.003	S15	41.268	0.403	0.000	0.13			9.5	SURCHARGED
12.004	S16	41.267	0.495	0.000	0.11			9.2	SURCHARGED
13.000	P179	42.371	-0.098	0.000	0.26			4.5	OK
13.001	P175	42.079	-0.100	0.000	0.24			4.5	OK
13.002	P174	41.850	-0.122	0.000	0.08			4.5	OK
14.000	Swale P169	43.121	-0.129	0.000	0.05			2.1	OK
12.005	S17	41.265	0.764	0.000	0.34			20.7	SURCHARGED
12.006	S18	41.263	0.811	0.000	0.33			20.5	SURCHARGED
1.011	Basin 4	41.261	0.861	0.000	0.02			1.4	SURCHARGED
1.012	S19	39.940	-0.272	0.000	0.02			1.4	OK
15.000	S30	42.495	-0.252	0.000	0.06			8.4	OK
16.000	Swale P14.1	43.179	-0.121	0.000	0.08			1.8	OK
16.001	Swale 14.2	42.877	-0.175	0.000	0.11			9.5	OK
16.002	Swale P21	41.448	-0.152	0.000	0.23			9.5	OK
17.000	P32	43.100	-0.100	0.000	0.00			0.0	OK
17.001	P28.1	42.874	-0.076	0.000	0.13			1.6	OK
17.002	P28.2	42.585	-0.076	0.000	0.13			1.6	OK
17.003	P27	41.671	-0.069	0.000	0.21			1.6	OK
15.001	S20	41.251	-0.266	0.000	0.19			30.1	OK

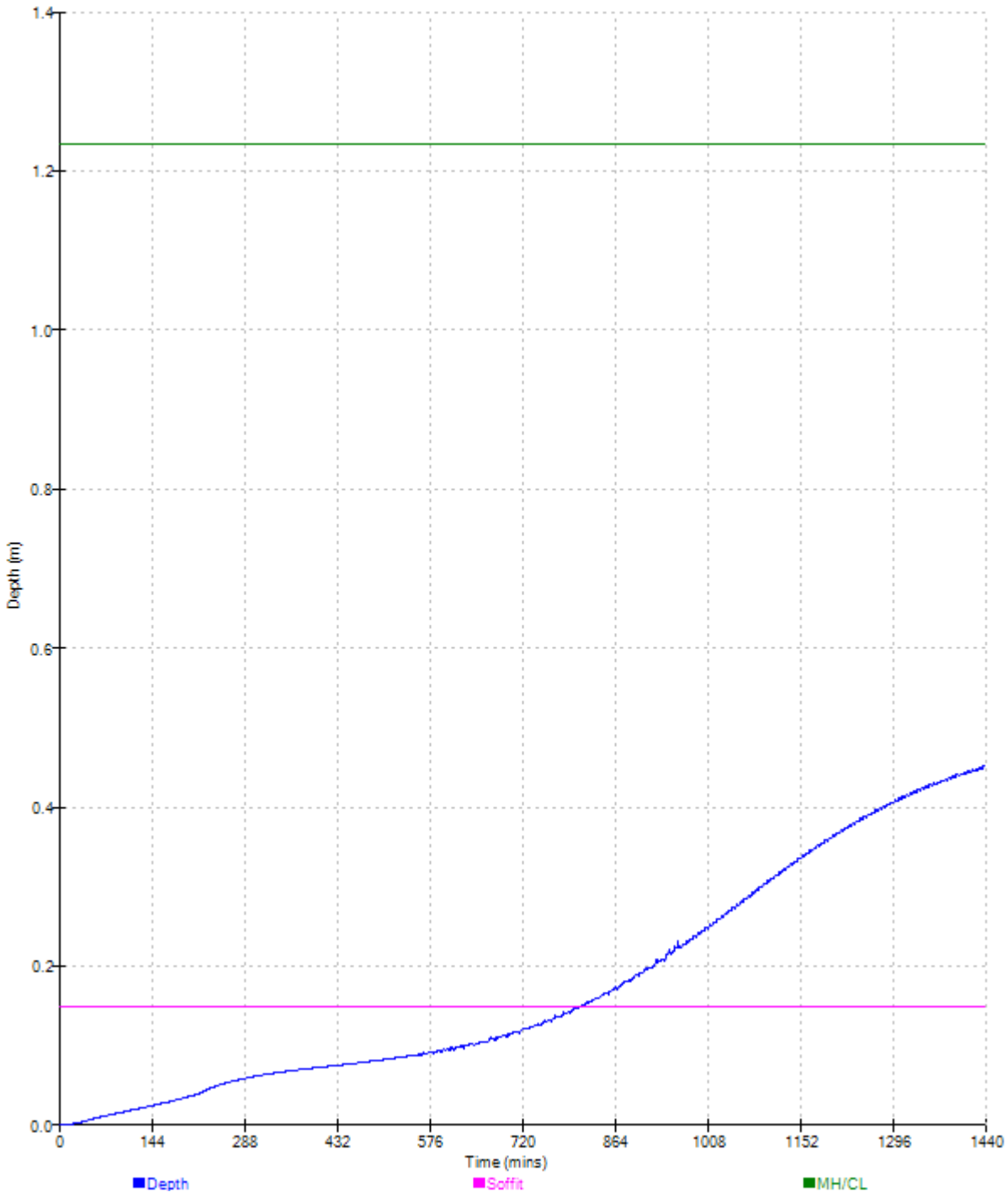
**APPENDIX F.1 – 1 IN 30CC CALCULATIONS
ACCESS ROAD**

Graphs for Pipe 1.003 US/MH S4 (Surface Network 1)
2160 minute 30 year Winter I+45%
Status: SURCHARGED

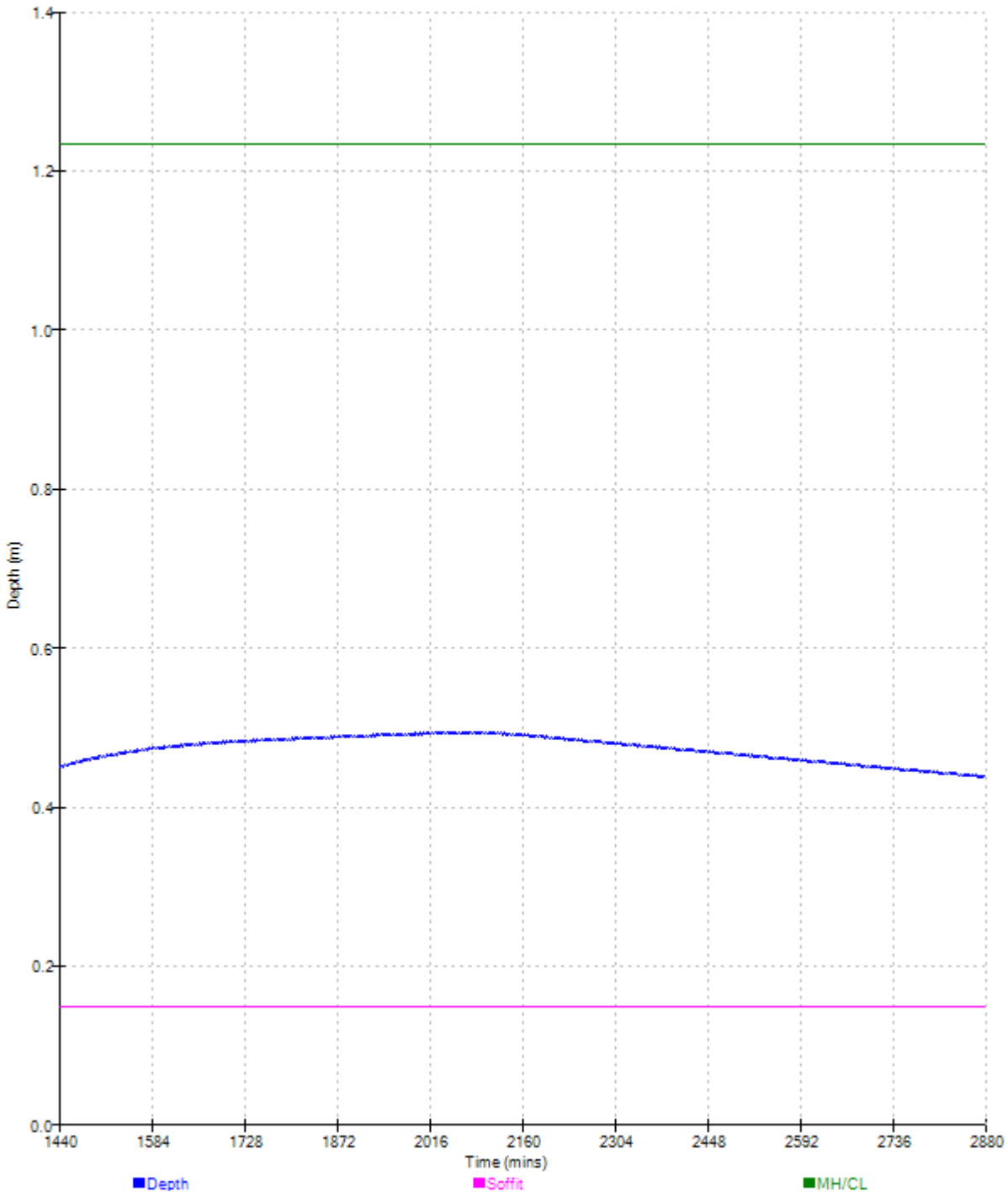


**APPENDIX F.2 – 1 IN 30CC CALCULATIONS
ROAD AND BRIDGE**

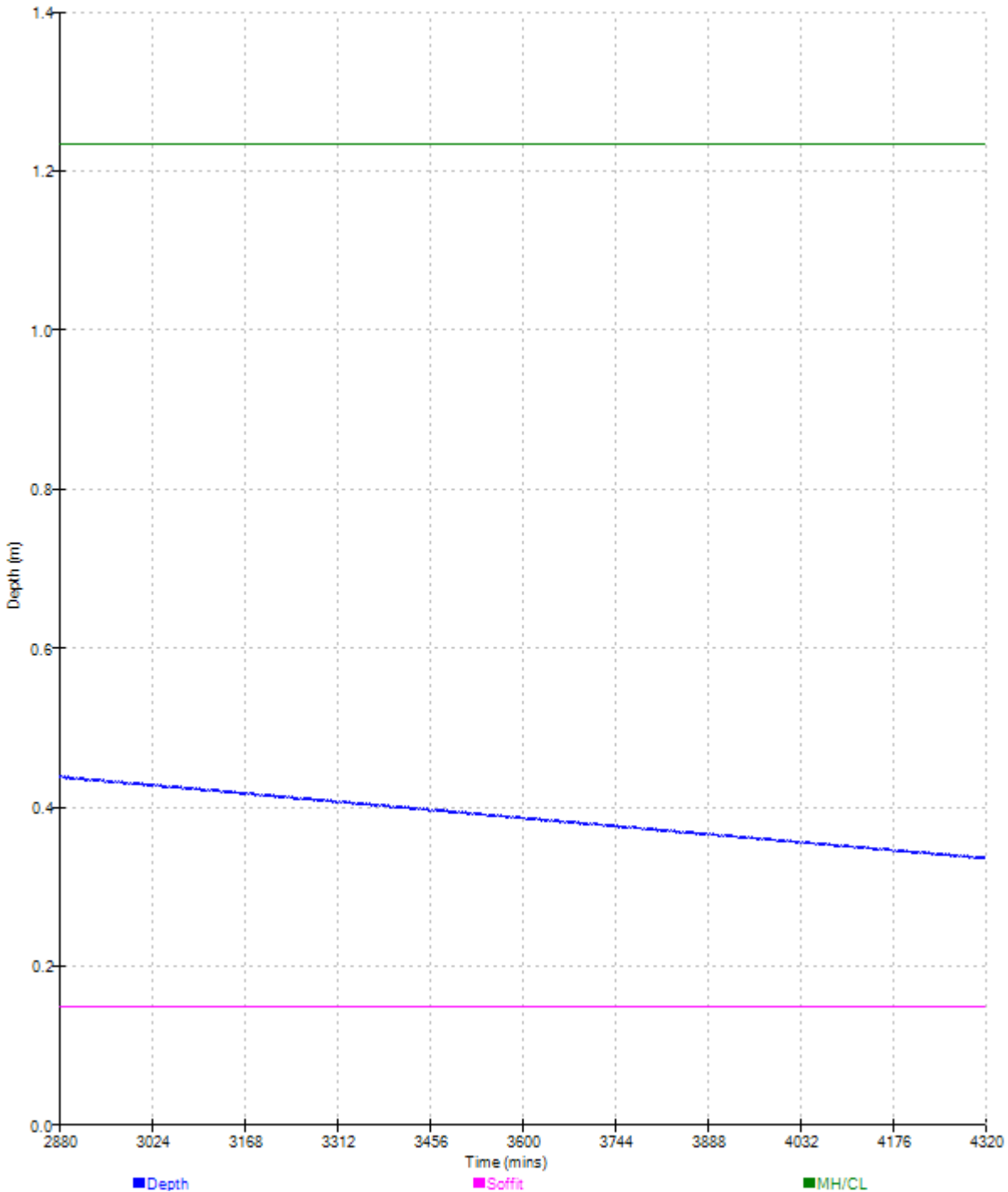
Graphs for Pipe 1.003 US/MH SB3-FC (Surface Network 2)
2160 minute 30 year Winter I+45%
Status: SURCHARGED (1 of 3)



Graphs for Pipe 1.003 US/MH SB3-FC (Surface Network 2)
2160 minute 30 year Winter I+45%
Status: SURCHARGED (2 of 3)

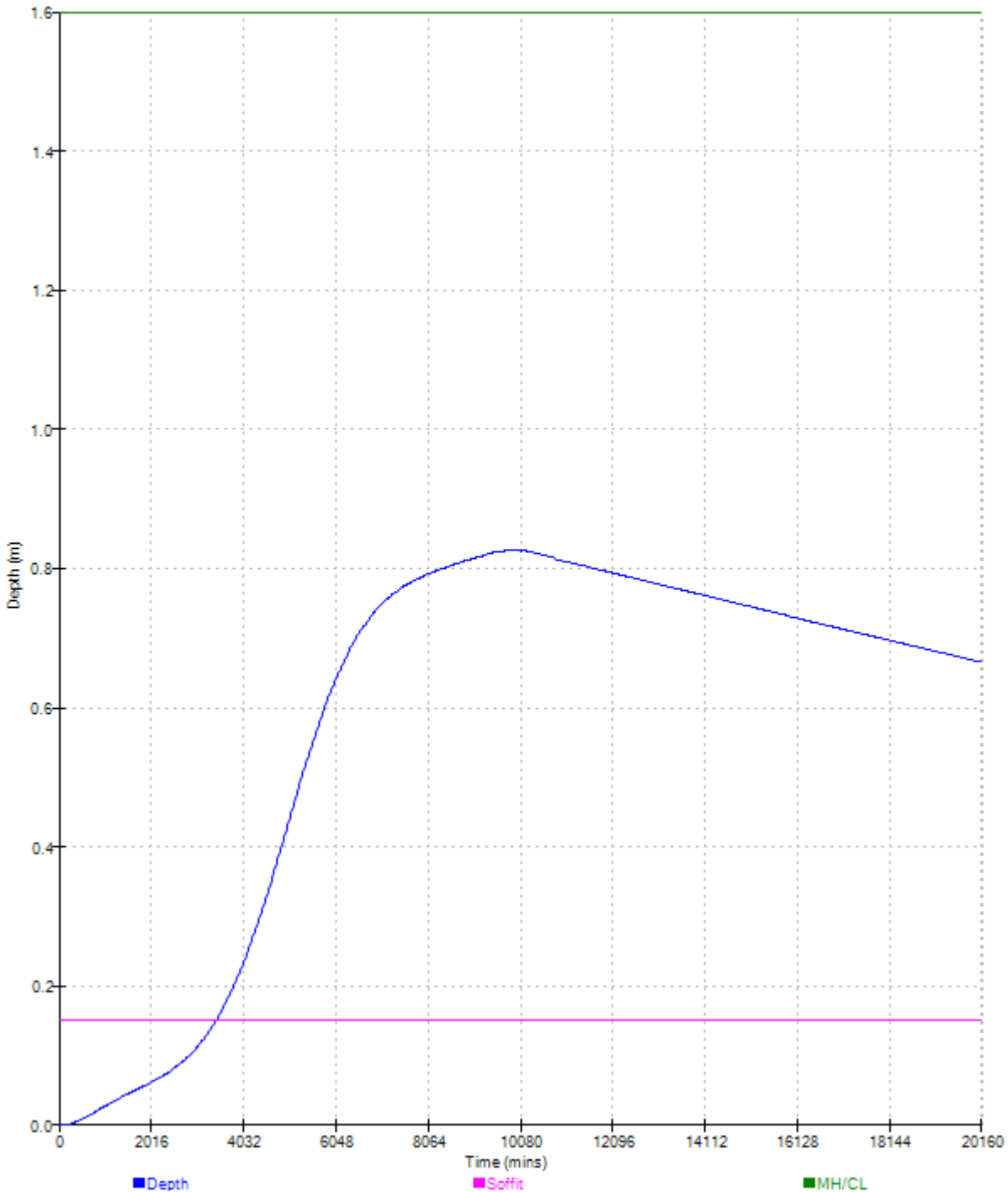


Graphs for Pipe 1.003 US/MH SB3-FC (Surface Network 2)
2160 minute 30 year Winter I+45%
Status: SURCHARGED (3 of 3)



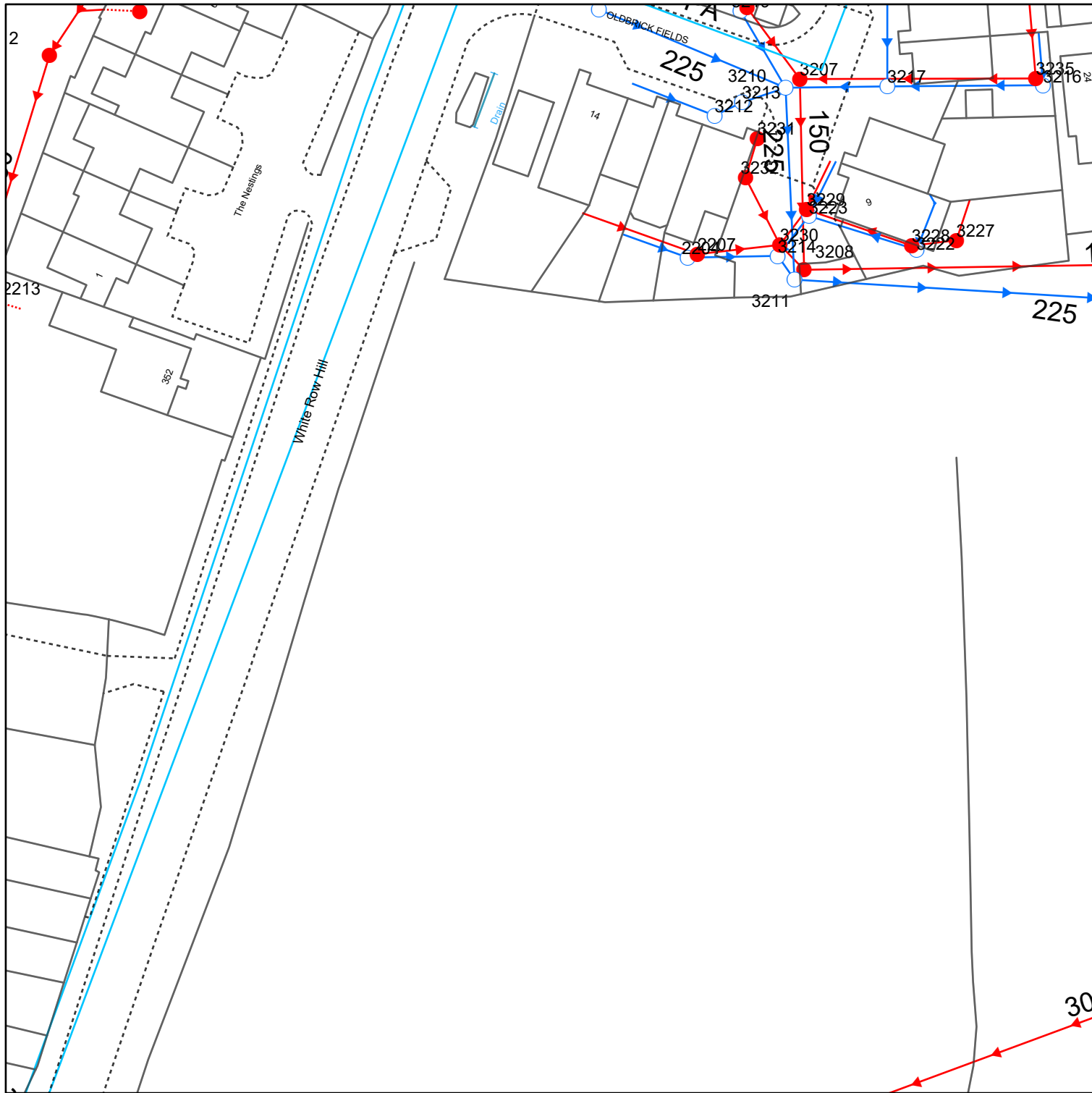
APPENDIX F.3 – 1 IN 30CC CALCULATIONS
MAIN SITE

Graphs for Pipe 1.010 US/MH Pond 1 (Surface Network 2)
10080 minute 30 year Winter I+45%
Status: SURCHARGED



APPENDIX G – WESSEX WATER ASSET MAPS

Wessex Water Network Map



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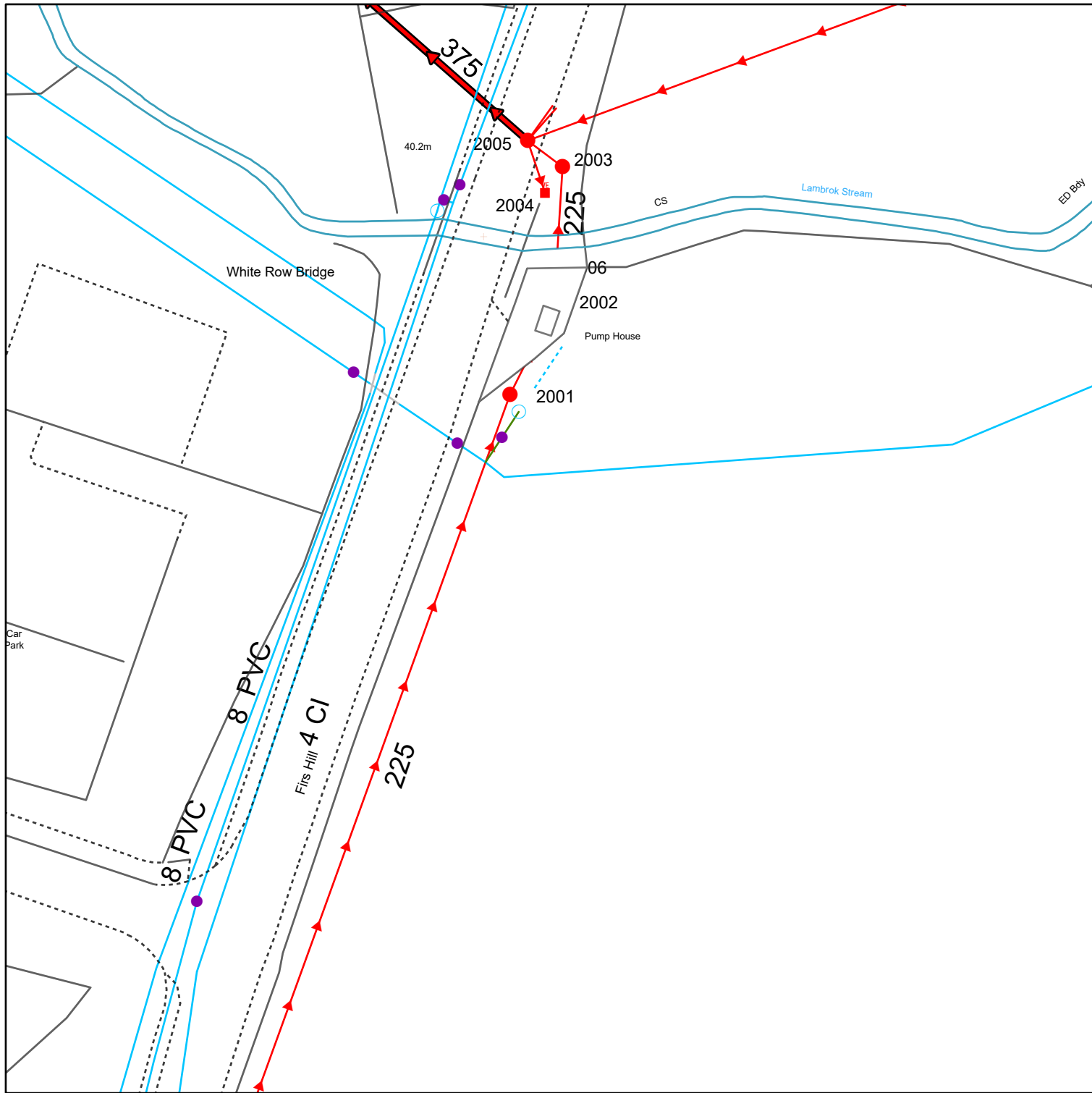
WATER MAINS		SEWERS		STRATEGIC		PUBLIC		PRIVATE		SECTION 104		OTHER WESSEX PIPES		NON-WESSEX / UNKNOWN	
	Distribution		Foul		Surface								Rising Mains		Private Rising Mains
	Washout		Combined										Standby Rising Mains		Culverted Watercourse
	Raw Water		Abandoned										EDM		Highway Drain
	Abandoned												Overflow		Use Unknown
	Private												Syphon		Status Unknown
FITTINGS		STRUCTURES													
	Hydrant		Manhole - Foul		Manhole - Surface		Pumping Station - Surface		Pumping Stn - Foul/Combined		Gully		Attenuation Tank		Tunnel
	Other		Manhole - Combined		Vent Column		Rodding Eye		Catchpit		Flushing Chamber		Storage Tank		Interceptor
			Inlet		Outfall		Soakaway		Non Return Valve		Air Valve		Hatch Box		Washout
			Lamphole		Bifurcation - Foul		Bifurcation - Surface		Bifurcation - Combined		Combined Sewage Overflow				

Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526633 or e-mail: developer.enquiries@wessexwater.co.uk to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.

Date: 06/10/2025
Centre: 384283E, 156196N
Scale: 1:625
(when printed at A4 size)



Wessex Water Network Map



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WATER MAINS		SEWERS		STRATEGIC		PUBLIC		PRIVATE		SECTION 104		OTHER WESSEX PIPES		NON-WESSEX / UNKNOWN	
	Distribution		Foul		Surface								Rising Mains		Private Rising Mains
	Washout		Combined										Standby Rising Mains		Culverted Watercourse
	Raw Water		Abandoned										Effluent Disposal		Highway Drain
	Abandoned												Overflow		Use Unknown
	Private												Syphon		Status Unknown
Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow).															
Some styles of line and symbol are shown on the key in sample/typical colours.															
FITTINGS		STRUCTURES													
	Hydrant		Manhole - Foul		Pumping Station - Surface										
	Other		Manhole - Surface		Pumping Stn - Foul/Combined										
			Manhole - Combined		Gully										
			Inlet		Vent Column										
			Outfall		Rodding Eye										
			Lamphole		Catchpit										
			Bifurcation - Foul		Flushing Chamber										
			Bifurcation - Surface		Soakaway										
			Bifurcation - Combined		Non Return Valve										
			Combined Sewage Overflow		Air Valve										
					Hatch Box		Washout								
												OTHER STRUCTURES			Chamber
													Attenuation Tank		Tunnel
													Storage Tank		Interceptor

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OTHER STRUCTURES

	Attenuation Tank		Tunnel
	Storage Tank		Interceptor

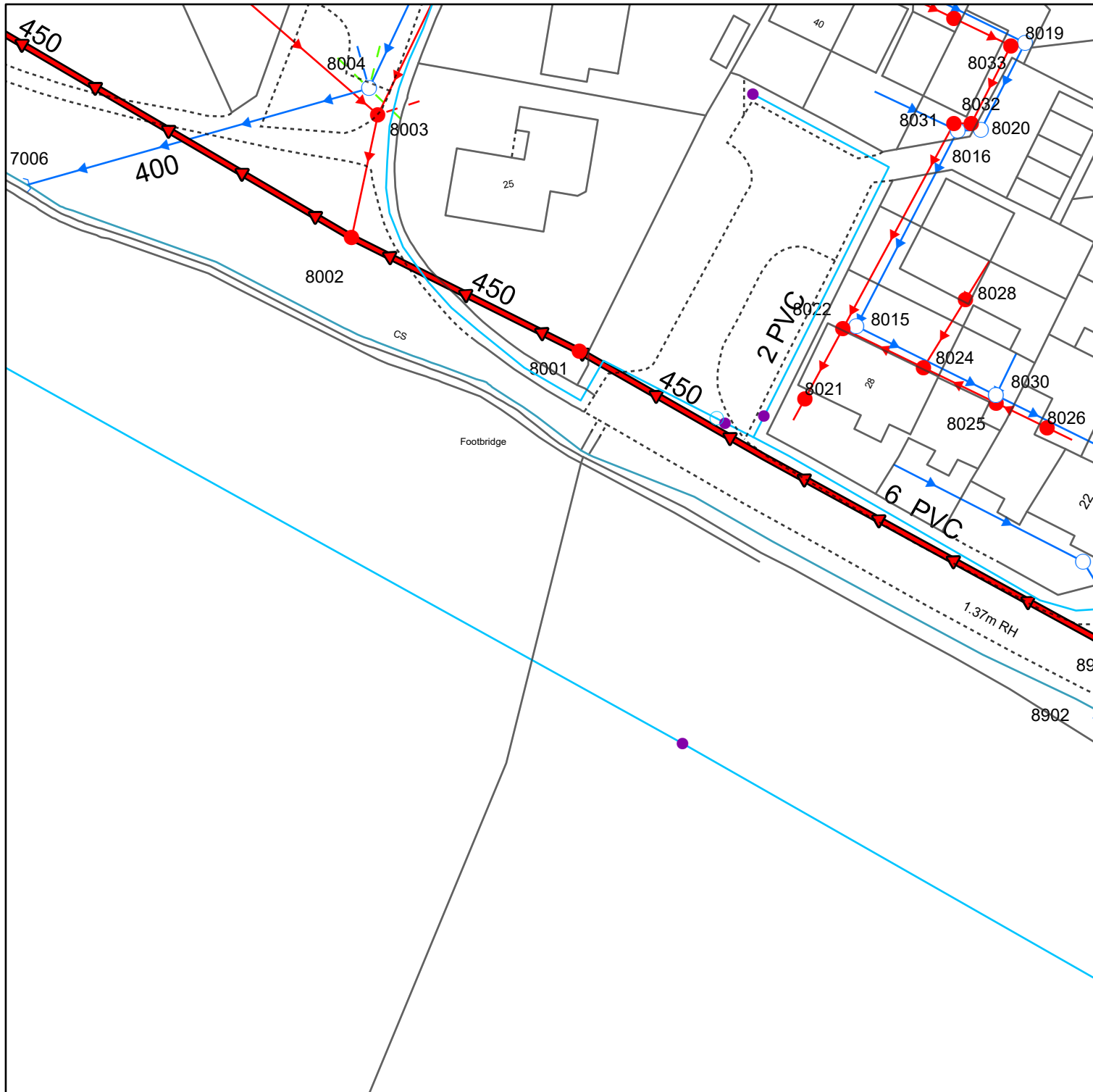
Wessex Water
YTL GROUP



Date: 06/10/2025
Centre: 384220E, 156048N
Scale: 1:625
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Wessex Water Network Map



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WATER MAINS		SEWERS	STRATEGIC	PUBLIC	PRIVATE	SECTION 104	OTHER WESSEX PIPES		NON-WESSEX / UNKNOWN	
	Distribution	Foul						Rising Mains		Private Rising Mains
	Washout	Surface						Standby Rising Mains		Culverted Watercourse
	Raw Water	Combined						EDM Effluent Disposal		Highway Drain
	Abandoned	Abandoned						Overflow		Use Unknown
	Private							Syphon		Status Unknown
		Colours generally indicate the use of the sewer/drain (i.e Red - Foul, Dark Blue - Surface, Magenta - Combined/Dual Use, Light Green - Highway Drain, Mid Green - Overflow).								
		Some styles of line and symbol are shown on the key in sample/typical colours.								
FITTINGS		STRUCTURES								
	Hydrant		Manhole - Foul		Pumping Station - Surface					
	Other		Manhole - Surface		Pumping Stn - Foul/Combined					
			Manhole - Combined		Gully					
			Inlet		Outfall					
			Lamphole		Vent Column					
			Bifurcation - Foul		Rodding Eye					
			Bifurcation - Surface		Catchpit					
			Bifurcation - Combined		Flushing Chamber					
			Combined Sewage Overflow		Soakaway					
					Non Return Valve					
					Air Valve					
					Hatch Box					
					Washout					
		OTHER STRUCTURES								
			Attenuation Tank		Tunnel					
			Storage Tank		Interceptor					
		Wessex Water YTL GROUP								

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