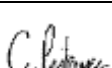


Potterne**PHASE II INTERPRETATIVE REPORT****Report Number:** R-G0807-01-P01**Date:** December 2025**Client:**

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In care of:

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1.INTRODUCTION

1.1 Instruction

Wessex and West Ground Investigation Limited (WWGI) were instructed by Attwell Projects Ltd (the Client) to carry out a Phase II Geotechnical Assessment for the site known as 'Potterne'. The instruction to proceed is contained in email correspondence dated 5th November 2025.

1.2 Proposed Development

At the time of writing, the future site use is a proposed new build residential dwelling.

1.3 Brief and Report Scope

The general specification for the works was provided by the Client and incorporated the brief to undertake a Phase II Intrusive Investigation with a geotechnical and geo-environmental assessment. The investigation comprised two window sample boreholes, three hand excavated trial pits, in situ testing, geotechnical and geo-environmental laboratory testing and the provision of a geotechnical and geo-environmental assessment report.

This report is based upon full factual records of the site work carried out, the ground conditions encountered in the exploratory holes and the laboratory test results. All information collected has been used to provide an interpretation of the ground conditions, with recommendations on potential ground contamination risks for the proposed development.

1.4 Limitations

The recommendations and opinions expressed in this report are based on the strata observed in the exploratory holes, the results of the site and laboratory tests or data provided by others. WWGI take no responsibility for conditions that have not been revealed by the exploratory holes, or which occur between them. Information provided from other sources is taken in good faith and WWGI cannot guarantee its accuracy.

The report has been prepared exclusively for the Client, for the site area indicated, and for the purpose stated. WWGI accepts no responsibility for any site, client or type of development not indicated in this report. This report should be reviewed at all stages of construction by someone familiar with the terms and assumptions it contains. It is essential that a suitably qualified and experienced engineer be appointed for the design of the works, and supervise construction.

2.THE SITE

2.1 Site Location and Description

The site is located approximately 7 km to the east of Trowbridge within the village of Potterne, Wiltshire. The National Grid Reference of the site is ST 99549 58404. A Site Location Plan is presented in the Appendix A and an aerial photograph of the site is below.



At the time of the investigation the site consisted of a car park. The site is bound to the north by a garden and residential property, to the east and west by residential property and a road to the south. The site sloped gently from north to south with an existing soakaway present at the southern boundary. It is understood that the site was previously used as a fuel filling station for an adjacent business.

2.2 Published Geology

The published geological map of the area, Sheet 282 Devizes (2008) and the British Geological Survey (BGS) online Onshore GeoIndex portal record the site to be underlain by the Upper Greensand Formation. Superficial deposits are not recorded to underly the site.

3.FIELDWORK

3.1 General

The fieldwork was carried out by WWGI on the 18th November 2025 and consisted of two window sample boreholes and three hand excavated trial pits, with associated sampling and testing.

The fieldwork was carried out generally in accordance with BS 5930:2020 *Code of Practice for Site Investigations*, and the Client's instructions, unless otherwise stated. Investigation locations were selected by WWGI based on the proposed development layout and the specification provided by the Client.

All exploratory hole locations were scanned for buried services using a Cable Avoidance Tool (CAT) to check for underground services. On completion all samples recovered from the site were transported to respective laboratories for geotechnical and geo-environmental testing. Details of the depths and types of samples recovered are indicated on the engineering logs presented within Appendix B.

3.1 Dynamic Sampling

Two dynamic sample holes, designated WS01 and WS02 were completed to maximum depths of 3.90m below existing ground level. Windowless sampling was completed by a track mounted Terrier 2000 rig driving sampling barrels by a 65kg drop weight over a 750mm fall.

Standard Penetration Tests (SPT's), were carried out in the dynamic sample boreholes in accordance with BS EN ISO 22476-3. Standard Penetration Tests (SPT's) consist of the dynamic driving of a 450mm long, 50mm diameter "split-spoon" sampler, using a standard 63.5kg trip hammer, falling over a distance of 760mm. The hammer anvil is connected to the sampler by steel boring rods. A small disturbed sample is usually recovered from the sampling barrel during the test.

Upon completion, all holes were backfilled with bentonite pellets and the surface reinstated.

3.2 Hand Excavated Trial Pits

Three hand excavated trial pits were excavated to depths of up to 0.85m bgl using hand tools. The trial pits were backfilled immediately on completion of sampling and logging.

3.3 In Situ Tests

Standard Penetration Tests (SPT), were carried out in the borehole in accordance with BS EN ISO 22476-3 and SPT calibration certificates are included in the appendices to this report. Standard Penetration Tests (SPT) consist of the dynamic driving of a 450mm long, 50mm diameter "split-spoon" sampler, using a standard 63.5kg trip hammer, falling over a distance of

760mm. The hammer anvil is connected to the sampler by steel boring rods. A small disturbed sample is usually recovered from the sampling barrel during the test. In very coarse-grained materials, or weak rocks, the sampler (or sampler shoe) is generally replaced by a 60° solid steel cone (referred to as SPT (c) tests). No samples are recovered using a solid cone.

The number of hammer blows required to drive the sampler or cone each consecutive 75mm is recorded, and is shown on the attached borehole logs. The first 150mm is regarded as the seating penetration, and the last 300mm as the test drive. The result of the test, referred to as the “N-value”, consists of the total number of blows relating to the test drive. Conventionally, the test drive is terminated after 50 blows if the full 300mm penetration has not been achieved. In this case the N-value is stated as $N > 50$, and the actual penetration of the test drive is also recorded.

Calibration certificates of in situ testing equipment are presented within Appendix C.

3.4 Soakaway Tests

Soakaway infiltration testing was carried out in within a hand pit excavated by others in general accordance with BRE Digest 365 (2016), the results of which are included in Appendix B of this report.

3.5 The Logging of Soils and Rocks

The logging of soils and rocks has been carried out in accordance with BS5930 except where superseded by the soil and rock description methodology in BS EN14688-1, BS EN 14688-2 and BS EN 14689-1.

4. LABORATORY WORK

4.1 Geotechnical Testing

A programme of laboratory testing was carried out on samples taken from the various strata to assist in classification and determine the engineering properties of the materials underlying the site. The testing was scheduled by the Client and carried out by Geo Site and Testing Limited laboratory. The test procedures used were generally in accordance with the methods described in BS1377:1990. Details of the specific tests used in each case are given below.

TEST	STANDARD (BS1377:1990)	No.
Moisture Content	Part 2, Clause 3.2	3
Atterberg testing	Part 2 Clauses 4.3 and 5.3	3

The results of the laboratory geotechnical tests are included in Appendix D of this Report.

4.1 Geo-Environmental Contamination Testing

The environmental chemistry of the samples was investigated by specialist chemical analysis, scheduled by WWGI and carried out by Chemtech Ltd.

Chemical analyses was carried out on three samples, which were submitted for the following suite of determinants:

Arsenic, Boron, Cadmium, Chromium, Chromium VI, Copper, Lead, Mercury, Nickel, Selenium, Zinc, Cyanide (free and total), Phenols, Sulphur, Sulphate, Sulphide, Soil Organic Matter, pH, speciated Polyaromatic Hydrocarbons (PAH), Asbestos ID and TPH by CWG.

The results of the laboratory contamination tests are included in in Appendix E of this report.

The range of potentially hazardous contaminants present on the site can be wide and varied, and the suite has been chosen to reflect both commonly found contaminants and others indicated by research to have a significant risk of being present. It is, however, possible that others may exist for which analyses have not been carried out. It is also possible that contaminants exist on the site but were not present at any of the exploratory hole locations.

5. GROUND CONDITIONS ENCOUNTERED

5.1 Soil Profile

The sequence of strata encountered beneath the site was:

- Made Ground. Thickness 0.40 – 0.85m (base not proven).
- Weathered Upper Greensand Formation – Very soft and soft sandy CLAY and SAND.

5.2 Obstructions

No obstructions were encountered within the boreholes during site works. A concrete obstruction was encountered within HP01 and HP02 and assumed to be the top of the historical fuel tank.

5.3 Groundwater

Groundwater was encountered at depths of between 2.00 – 3.00m bgl.

5.4 Visual / Olfactory Evidence of Contamination

Visual and olfactory evidence of contamination was not encountered in any of the exploratory holes during the investigation.

6. ENGINEERING PROPERTIES AND GEOTECHNICAL ASSESSMENT

6.1 Engineering Properties of Soils Encountered

The following sections discuss the engineering and chemical properties of the strata encountered, based on results of in situ testing and laboratory testing obtained during this investigation.

Weathered Upper Greensand Formation

The Atterberg Limit tests have given values of liquid limits of 50 - 72% and plastic limit values of between 18 - 30%, resulting in a plasticity index range of 32 - 42%. These results indicate the stratum is a clay of intermediate to very high plasticity as shown on the Plasticity Chart included in the Appendices. For design purposes, a value of plasticity index of 40% is recommended, based on the upper quartile value of the results.

In accordance with NHBC Chapter 4.2 Building Near Trees (2003), soils can be classified in terms of volume change potential, using the relationship:

$$I_p' = I_p \times \frac{\% \text{ less than } 425\mu\text{m}}{100\%}$$

.where I_p' = modified plasticity index, I_p = plasticity index.

Based on the laboratory test results, the above relationship and Table 1 of NHBC Chapter 4.2, the samples of the Upper Greensand Formation are shown to have medium volume change potential.

The SPT N values at 1.20m are in the region of 9 – 21, however, from 2m values are consistently 0 – 2. A such, an N value of 0 is recommended for depths of 2m bgl and 4m bgl and a value of 10 for 5m depth.

Shear strength may be estimated from a correlation by Stroud and Butler (1975): for a clay with a plasticity index of 40:

$$\begin{aligned} C_u &= 4.5 \times \text{SPT N value} \\ &= 4.5 \times 0 = 0 \text{ kN/m}^2 \text{ at 2m depth and;} \\ &= 4.5 \times 10 = 45 \text{ kN/m}^2 \text{ at 5m depth.} \end{aligned}$$

Based on all the above, a design value of undrained shear strength of 45 kN/m² is recommended for a depth of 5.00m.

Effective stress strength parameters may also be obtained from correlations with plasticity index. For a plasticity index of 40:

- BS8002 (1994), Table 2 gives $\phi'_{crit} \approx 24.3^\circ$;
- Gibson (1953), gives $\phi_d = 17.8^\circ$.

Based on all the above, design values of $\phi' = 17.8^\circ$ and $c' = 0 \text{ kN/m}^2$ are recommended.

6.2 Proposed Development

The proposed development comprises the construction of a new residential dwelling. At the time of writing, loadings were unknown.

6.3 Results of In Situ Testing

The results of soakaway testing are summarised in the following table:

Location	Depth	Soil Infiltration Rate (m/s)			
		Test 1	Test 2	Test 3	Recommended Design Value
SA01	0.85m	1.11×10^{-4}	5.91×10^{-5}	9.14×10^{-5}	9.00×10^{-5}

6.4 Foundations

Based on the ground conditions encountered, conventional shallow depth foundations will **not** be suitable given the thickness of extremely soft soils. Any footing set within the sands above 1.5m will highly likely suffer subsidence due to the bulb of influence into the soft soils below.

A piled solution is recommended and the advice of a specialist piling contractor should be sought.

6.5 Drainage

Based on the results of in situ testing, soakaways taken down into the natural sands will likely be suitable at this site.

Soakaways should be designed in accordance with BRE Digest 365, and for preliminary design purposes, the soil infiltration rate given in Section 7.1 can be used.

The size and required depth of soakaways will depend on factors such as area of hardstanding to be drained (and subsequent inflow to the soakaway) and outflow from the soakaway (based on soil infiltration rates).

Soakaways should be sited at least 5-10m away from structures to avoid any dissolution of the underlying soils and possible undermining of foundations.

7. CONTAMINATION ASSESSMENT

7.1 Introduction

In accordance with current best practice, the assessment of potentially contaminated sites is normally carried out by means of a risk assessment, based on a conceptual model, which examines possible sources of contamination, potential receptors, and likely links between the two. For contamination to be a hazard, it must be demonstrated that there is an identifiable source of contamination (either inside or outside the site), potential receptors that may be at risk (occupiers of the site, for example, or the environment in general), and that there are also potential pathways through which the former may affect the latter.

Potential sources of contamination can be determined from the results of the laboratory tests that have been carried out on the soil samples. Other potential sources may be evident from the information on the history of the site and its environs. Contaminants are only a hazard if they are present in suitably high concentrations.

7.2 Conceptual Model

Contaminant Sources

WWGI have not been issued the findings of any desk study that may have been undertaken by others. The below is based upon freely available information only. It is concluded that the following contaminant sources should be considered:

Historical/Current Site Sources: The site and environs appear historically linked industrial/construction activities.

Materials present on the site: No visual evidence of contamination was observed on the site surface or within arisings from the exploratory holes. However, WWGI were informed by the client two known backfilled fuel tanks are present on site.

Groundwater: Groundwater was encountered at depths of 2.00 – 3.00m during the investigation.

Surface Water: Surface water was not noted during site investigation however a known soakaway was noted at the southern boundary. WWGI were informed that this regularly becomes overwhelmed.

Asbestos: Asbestos was not encountered in any of the exploratory holes, nor within any of the samples subsequently tested.

Contamination arising from external sources: Based on freely available historical maps and current site usage, it is possible that areas near to the site are considered possible to contain elevated concentrations of some contaminants associated with farming activities.

Contaminant Pathways:

Contaminants can reach potential receptors through various routes. The following are considered to be applicable to this site:

- **Ingestion:** Some contaminants can be harmful if ingested directly after handling contaminated soils.
- **Leachate:** Soluble fractions of some contaminants can leach into the ground, contaminating groundwater.

Contaminant Receptors:

The following potential receptors are considered to be applicable to this site:

- **Future Site Occupants:** Future site occupants could be at risk from the effects of any contaminants in the soil and groundwater, and also from land gas.
- **Construction Workers:** The personnel involved in the construction of the proposed development are also at risk.
- **Groundwater / River Water:** As well as being a potential source of contamination, the groundwater also needs to be considered as a potential receptor as the bedrock is designated a Secondary A Aquifer with High groundwater vulnerability.

7.3 Soil Risk Assessment

Risk of Soil Contamination to Groundworkers During Construction

To assess the risk of soil contamination to construction and ground workers during development, guidelines from the HSE Document 'Protection of workers and the general public during development of contaminated land'(1991) are used. The document assesses soil contamination test results and classifies the site as being uncontaminated or contaminated with varying degrees of contamination from 'slight' to 'unusually heavy'.

The guideline values and laboratory test results are summarised in the following table:

**Summary of guideline values for protection of workers and the general public
during development of contaminated land**

Contaminant	Typical Values* for:					Test Results	Class
	Uncontaminated Soils	Slight Contamination	Contaminated	Heavy Contamination	Unusually Heavy Contamination		
	Class A	Class B	Class C	Class D	Class E		
Arsenic	0 - 30	30 - 50	50 - 100	100 - 500	500	7.9-10	A
Cadmium	0 - 1	1 - 3	3 - 10	10 - 50	50	<1.6 (LOD)	A
Chromium	0 - 100	100 - 200	200 - 500	500 - 2500	2500	14.6-25.6	A
Copper	0 - 100	100 - 200	200 - 500	500 - 2500	2500	<1.6-13.3	A
Lead	0 - 500	500 - 1000	1000 - 2000	2000 - 1%	1.0%	7.8-65.1	A
Mercury	0 - 1	1 - 3	3 - 10	10 - 50	50	<0.7 (LOD)	A
Nickel	0 - 20	20 - 50	50 - 200	200 - 1000	1000	7.8-13.5	A
Zinc	0 - 250	250 - 500	500 - 1000	1000 - 5000	5000	23.6-151	A
Selenium	0 - 1	1 - 3	3 - 10	10 - 50	50	<3.0 (LOD)	A

Based on results of laboratory testing, the site can generally be classified as uncontaminated.

Based on the above results there is a low potential risk from soil contamination to construction workers, ground workers and members of the public during construction, and appropriate measures, such as PPE, site health plans, appropriate disposal of material arisings will mitigate this risk. Please note, there were no soil samples taken from around the buried fuel tanks and it is recommended that additional spot tests be undertaken during excavation.

Risk of Soil Contamination to Future Site Users After Development

As part of the contamination assessment, the chemical results obtained by WWGI have been screened against accepted compliance criteria, namely:

Defra C4SL Health Criteria Values (March 2014), where available; and WWGI Tier 1 assessment values - based on LQM/CIEH Sutable 4 Use Levels (2015) (S4ULs).

The Land Quality Management (LQM)/Chartered Institute of Environmental Health (CIEH) 'Sutable 4 Use Levels' (S4ULs) have been derived for soil contaminants for which there are no C4SLs, and have been derived in accordance with UK legislation, national and Environment Agency Policy and using the Environment Agency's tools and available guidance.

The S4ULs replace the previous LQM/CIEH Generic Assessment Criteria (GAC) published in 2009. The assessment criteria have been updated in line with developments in UK human health risk assessment, in particular the additional land uses and exposure assumptions

presented in Defra's C4SL guidance. For each substance, S4ULs have been derived for six generic land uses (including the two Public Open Space land uses defined in C4SL guidance) and a range of Soil Organic Matter contents (organic contaminants only).

In March 2014 Defra published Category 4 Screening Levels (C4SLs) for assessment of land affected by contamination. C4SLs are intended for use in determining whether land is 'clearly not contaminated' as defined by the revised Part 2A Statutory Guidance, i.e., Category 4 is where there is no risk or the level of risk to human health is acceptably low. The framework developed presents a departure from the conventional approach of defining 'minimal risk' and the derivation of C4SLs has involved the development of a new toxicological criterion, the 'low level of toxicological concern' (LLTC), alongside modifications to the exposure modelling previously used to determine similar generic assessment criteria.

The S4ULs and C4SLs are intended to be used as generic assessment criteria and can be used in the preliminary evaluation of the risk to human health from long-term exposure to chemicals in soil. They represent values, which indicate to an assessor that soil concentrations above this level might present risk to the health of site-users and that further assessment, quantitative risk assessment, site investigation or remediation may be required.

The use of these reference values for initial screening purposes does not imply that they are categoric indicators of whether contamination conditions are significant, this being subject to a more detailed risk assessment.

In the case of possible receptors, one of the most significant factors is the proposed future use of the site (as some potential uses are much less sensitive to the presence of contamination than others). With regard to the assessment, at the time of writing this report, the proposed end use of the site was for residential purposes. Therefore, for the purposes of this report, the following sections compare the results of contamination analyses to residential types of end development and also commercial end development, which is considered the most appropriate.

The comparison of results is summarised in the following table:

Soil Results Comparison with Defra C4SL HCV/LLTC Values

Determinand	C4SL (mg/kg)*			Min. (mg/kg)	Max. (mg/kg)	No. of Samples with Exceedances
	Residential with home grown produce (1a)	Residential without home grown produce (1b)	Commercial (2)			
Arsenic	37	40	640	7.9	10	0
Benzo(a)pyrene	5.0	5.3	76	<0.019	0.903	0
Cadmium	26	149	410	<1.6	<1.6	0
Lead	200	310	2300	7.8	65.1	0
*Minimal risk Health Criteria Values						

The samples have shown contaminants at levels below the recommended C4SL's for residential and commercial end use.

The following contaminants were not assessed with respect to risks posed to Human Health as they are not generally considered to represent a significant risk to Human Health (CLR 8); sulphate and sulphide.

For contaminants not covered by the Defra C4SLs/CLEA SGVs, reference is made to the Suitable for Use Levels (S4ULs) derived by The Land Quality Management Ltd & Chartered Institute of Environmental Health⁽²⁰¹⁵⁾, and summarised in the following table:

Soil Results Comparison with LQM/CIEH S4ULs (Soil Organic Matter = 2.5%)

Determinand	Suitable 4 Use Levels (mg/kg)*						No. of Samples	Min. (mg/kg)	Max. (mg/kg)	No of Exceedences
	Residential		Allotment (3)	Commercial (4)	POS Res* (5)	POS Parks* (6)				
	with homegrown produce (1)	without homegrown produce (2)								
Metals										
Chromium	910	910	18000	8600	1500	33000	3	14.6	25.6	0
Copper	2400	7100	520	68000	12000	44000	3	<1.6	13.3	0
Mercury	1.2	1.2	21	58	16	30	3	<0.7	<0.7	0
Nickel	180	180	230	980	230	3400	3	7.8	13.5	0
Selenium	250	430	88	12000	1100	1800	3	<3	<3	0
Zinc	3700	4000	620	730000	81000	170000	3	23.6	151	0
Petroleum Hydrocarbons										
Aliphatic EC 5-6	42	42	730	3200	570000	95000	3	<0.1	<0.1	0
Aliphatic EC >6-8	100	100	2300	7800	600000	150000	3	<0.1	<0.1	0
Aliphatic EC >8-10	27	27	320	2000	13000	14000	3	<0.1	<0.1	0
Aliphatic EC >10-12	130	130	2200	9700	13000	21000	3	<1	<1	0
Aliphatic EC >12-16	110	1100	11000	59000	13000	25000	3	<0.5	0.8	0
Aliphatic EC >16-35	65000	65000	260000	1600000	250000	450000	3	<3.0	9.5	0
Aliphatic EC >35-44	65000	65000	260000	1600000	250000	450000	3	<0.5	0.9	0
Aromatic EC 5-7 (benzene)	70	370	13	26000	56000	76000	3	<0.01	<0.01	0
Aromatic EC >7-8 (toluene)	130	860	22	56000	56000	870000	3	<0.01	<0.01	0
Aromatic EC >8-10	34	47	8.6	3500	5000	7200	3	<0.01	<0.01	0
Aromatic EC >10-12	74	250	13	16000	5000	9200	3	<0.6	3.5	0
Aromatic EC>12-16	140	1800	23	26000	5100	10000	3	<1.0	9.2	0
Aromatic EC>16-21	280	1900	46	28000	3800	7600	3	<2.0	27.8	0
Aromatic EC>21-35	1100	1900	370	28000	3800	7800	3	<4.5	83.6	0
Aromatic EC>35-44	1100	1900	370	28000	3800	7800	3	<1.5	16.9	0
Polycyclic Aromatic Hydrocarbons										
Naphthalene	2.3	2.3	4.1	190	4900	1200	3	<0.016	0.018	0
Acenaphthylene	170	2900	28	83000	15000	29000	3	<0.015	0.053	0
Acenaphthene	210	3000	34	84000	15000	29000	3	<0.013	<0.013	0
Fluorene	170	2800	27	63000	9900	20000	3	<0.013	<0.013	0
Phenanthrene	95	1300	15	22000	3100	6200	3	<0.014	<0.014	0
Anthracene	2400	31000	380	520000	74000	150000	3	<0.017	0.054	0
Fluoranthene	280	1500	52	23000	3100	6300	3	<0.017	1.06	0
Pyrene	620	3700	110	54000	7400	15000	3	<0.016	0.963	0
Benz(a)anthracene	7.2	11	2.9	170	29	49	3	<0.02	1.04	0
Chrysene	15	30	4.1	350	57	93	3	<0.028	0.630	0
Benzo(a)pyrene	2.2	3.2	0.97	35	5.7	11	3	<0.019	0.903	0
Benzo(b)fluoranthene	2.6	3.9	0.99	44	7.1	13	3	<0.020	1.04	0
Benzo(k)fluoranthene	77	110	37	1200	190	370	3	<0.025	0.423	0
Indeno(1,2,3-cd)pyrene	27	45	9.5	500	82	150	3	<0.019	0.813	0
Dibenz(a,h)anthracene	0.24	0.31	0.14	3.5	0.57	1.1	3	<0.017	0.609	1 for (1,2,3&5)

The samples have shown contaminants at levels below the S4UIs for residential end use with the exception of Dipenzo(a,h)anthracene which was in excess of allowances for all residential end use. This only affected the location of HP01 – the area immediately above the fuel tank and is to be expected. Based on all the above, it is considered the risk from soil contamination to future site users is low in all areas away from the tank.

7.4 Groundwater Risk Assessment

Based on the ground conditions encountered, anticipated ground conditions at depth and low soil contamination levels, it is considered that the risk of contamination from groundwater is low.

7.5 Surface Water Risk Assessment

Given the levels of most metal contamination encountered, the type of development proposed and likely foundation options, it is considered the risk to surface waters is low.

7.6 Risk from Asbestos in Ground

Asbestos was not encountered in any of the exploratory holes, or within any of the samples subsequently tested. It is therefore considered the risk of asbestos being present in the ground is low.

7.7 Site Risk Assessment

The following table presents an outline summary of the contamination assessment discussed above.

Contamination Assessment Summary

Description of Receptor or Source	Risk rating
Health and safety of workers during redevelopment	Low
Current and future site users and third parties	Low
Soft cover development	Low
Risk to groundwater	Low
Risk to surface waters	Low
On-site and off-site migration of land gasses	Low
Presence of asbestos in ground	Low
OVERALL GROUND CONTAMINATION RISK RATING	Low

8. Discussion and Conclusions

The possible actions considered appropriate for the proposed development, together with other precautionary measures, are given below:

Contaminated Soils

As discussed in the above sections, the contamination tests indicate low levels of all contaminants with the exception of Dipenzo(a,h)anthracene within HP01 above the backfilled fuel tank. As no further samples could be taken from the area to the side and below the tank, it should at this stage be considered that these soils may be contaminated and adequate allowance should be made for further testing and offsite disposal of these soils during redevelopment.

The presence of higher levels of contamination on parts of the site not covered by the exploratory holes should not be discounted and additional spot checks are recommended, particularly during groundworks. Should any elevated levels of contamination be found in such areas, remedial measures, such as the replacement of the upper zone of contaminated soil with a suitable thickness of clean soil, may need to be undertaken as described above.

Contaminated Groundwater

Due to the generally low levels of contamination found at the site, it is not considered likely that there would be a hazard to the groundwater beneath the site, and no remedial action should be necessary beyond drainage to ensure off site contaminants cannot be transported and deposited.

Asbestos

Asbestos was not encountered during fieldworks nor identified during laboratory testing.

Site Personnel

As with all construction sites, personnel working on the site during the construction period should be encouraged to maintain a high standard of personal hygiene and on-site washing facilities should be available.

Other Matters

Due diligence is required during the construction period, and should any further evidence of contamination be found, appropriate investigation and / or action should be taken. The significance of any contamination not discovered by this investigation is outside the scope of this report. It is emphasised that only a very small number of tests for contamination have

been carried out, and that the possibility of further contamination existing elsewhere on the site cannot be ruled out. WWGI does not accept any liability for contamination.

9. LIST OF REFERENCES

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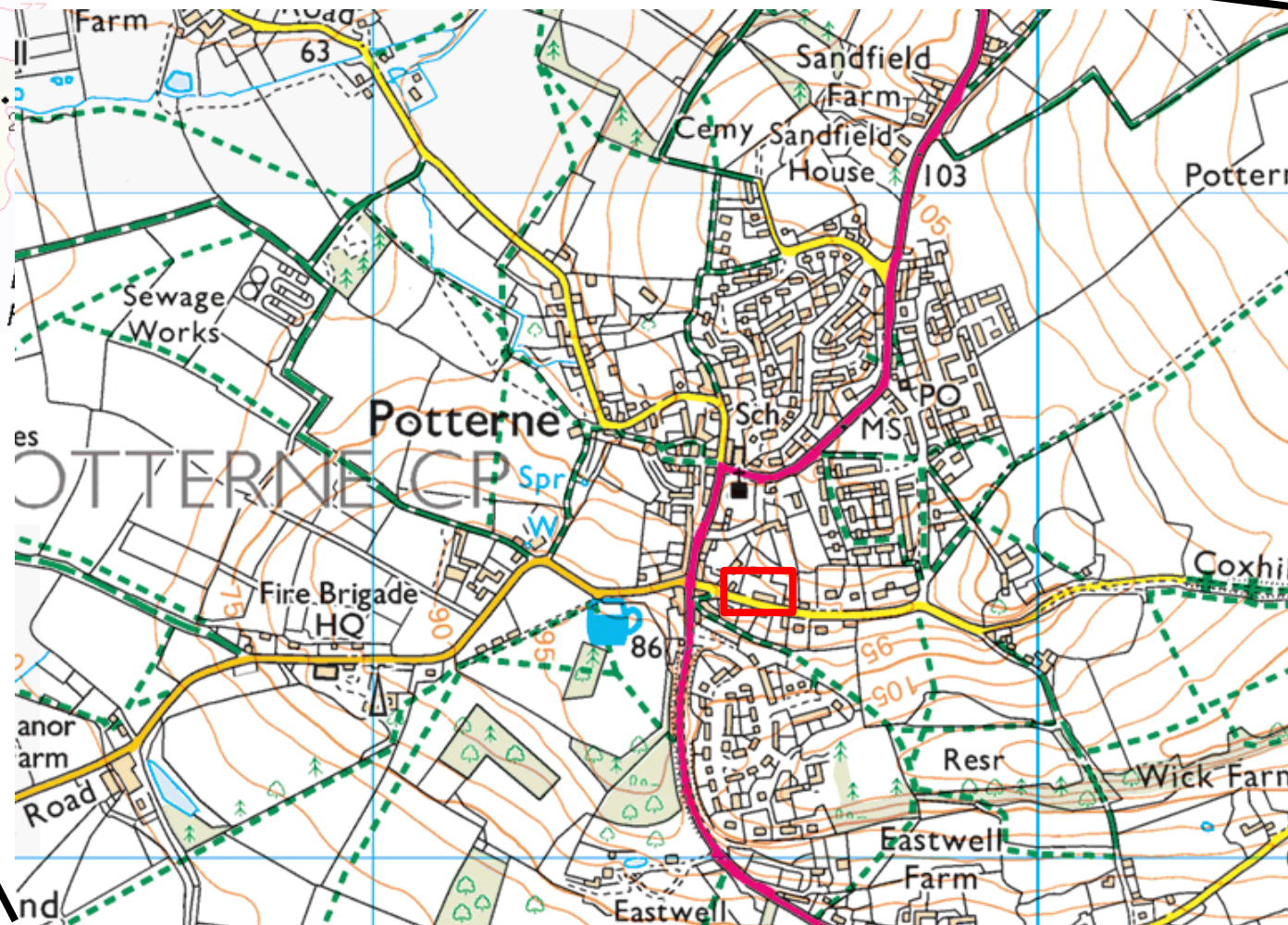
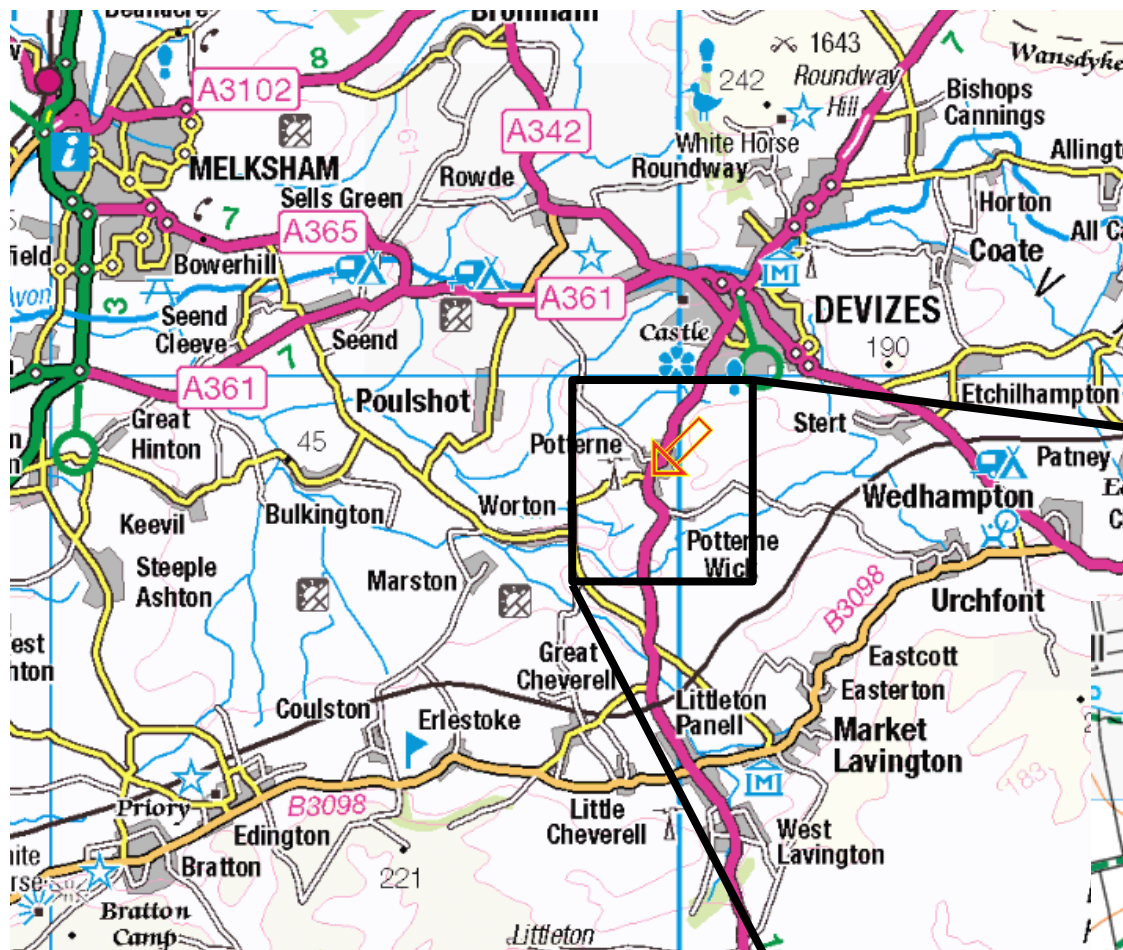
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APPENDIX A – DRAWINGS



Key



The Site

Status

Final

Scale

NTS

Current Issue Signatures

Author

C. Pristavec

Datum

N/A

Checker

I. Parsons

Grid

N/A

Approver

I. Parsons

© Copyright Reserved

Project

Potterne

Title

Site Location Plan

Drawing No.

0001

Project No.

G0807

Issue No.

01



	Key  Investigation Location	Status		Final		Project				
		Scale		Current Issue Signatures		Potterne				
		NTS		Author	C. Pristavec	Title		Exploratory Hole Location Plan		
		Datum	N/A	Checker	I. Parsons					
		Grid	N/A	Approver	I. Parsons					
		© Copyright Reserved								Drawing No.
								0002	G0807	01

APPENDIX B EXPLORATORY HOLE LOGS
SOAKAWAY TEST DATA

Key to Exploratory Hole Symbols and Abbreviations

SAMPLE TYPES

B	Bulk disturbed sample	ES	Environmental soil sample	U	Undisturbed sample
C	Core sample	EW	Environmental water sample	UT	Undisturbed thin wall sample
CBR-D	Disturbed sample from CBR test area	G	Gas sample	W	Water sample
CBR-U	Undisturbed sample from CBR test area	L	Liner sample		
D	Small disturbed sample	SPT	SPT split spoon sample		

IN-SITU TESTING

SPTs	Standard Penetration Test (using a split spoon sampler)
SPTc	Standard Penetration Test (using a solid 60 degree cone)
N	Recorded SPT 'N' Value *
-/-	Blows/Penetration (mm) after seating blows totalling 150 mm
MX	Mexi Probe Test (records CBR as %)
HV	Hand Shear Vane Test (undrained shear strength quoted in kPa)
HP	Hand Penetrometer Test (kg/m ³)
()	Denotes residual test value
PID	Photo Ionisation Detector (ppm) *
Kf/Kr	Permeability Test (f = falling head, r = rising head quoted in ms ⁻¹)
HPD	High Pressure Dilatometer Test (pressure meter)
PKR	Packer / Lugeon Permeability Test
CBR	California Bearing Ratio Test

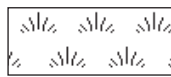
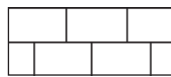
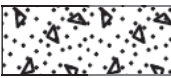
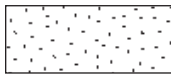
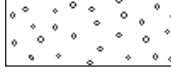
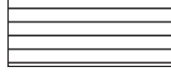
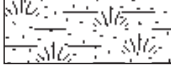
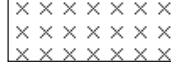
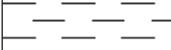
ROTARY CORE DETAILS

TCR	Total Core Recovery, %
SCR	Solid Core Recovery, %
RQD	Rock Quality Designation (% of intact core >100 mm)
FI	Fracture Spacing (average fracture spacing; in mm, over indicated length of core) **
NI	Non-Intact Core
AZCL	Assumed Zone of Core Loss

GROUNDWATER

	Groundwater strike
	Standing water level after 20 minutes; 1st, 2nd etc (number denotes level order)

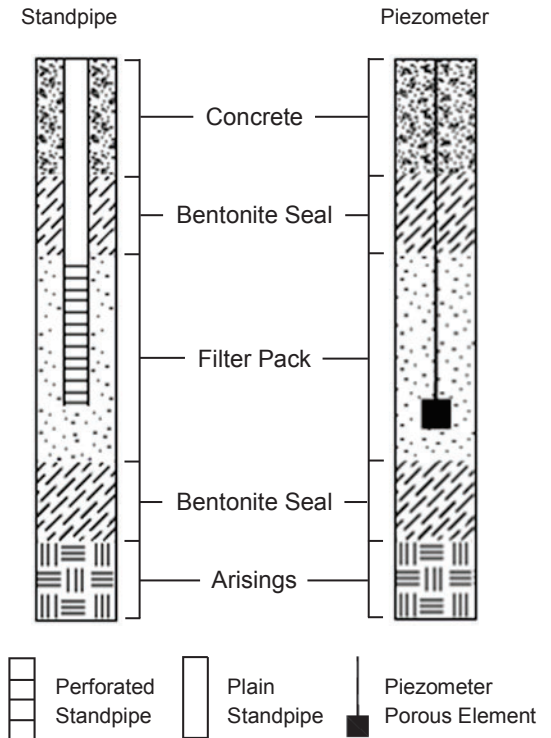
STRATA LEGENDS - Note: Composite strata types are shown by combining symbols

	Made Ground		Silt		Peat		Limestone
	Concrete		Sand		Void		Chalk
	Bituminous Bound Materials		Gravel		Mudstone		Coal
	Topsoil		Cobbles		Siltstone		Metamorphic Rock
	Clay		Boulders		Sandstone		Fine Grained Igneous Rock

* Where a single value is quoted this is the uncorrected 'N' value for a full 300 mm test drive following a seating drive of 150mm. Where the full test drive penetration is not achieved the number of blows is quoted for the penetration below the test total of 300mm, e.g.: 50/75.

** The minimum, average and maximum are shown e.g. 5/45/125.

INSTALLATION & BACKFILL DETAILS



STRATUM BOUNDARIES

	Unit boundary
--	---------------

BOREHOLE LOG

Project Potterne				BOREHOLE No WS01
Job No G0807	Date 18-11-25 18-11-25	Ground Level (m)	Co-Ordinates ()	
Contractor WWGI				Sheet 1 of 1

SAMPLES & TESTS			Water	STRATA			Geology	Instrument/ Backfill
Depth	Type No	Test Result		Reduced Level	Legend	Depth (Thick- ness)		
0,80 1,00-2,00 1,00 2,00-3,00 2,00 3,00-4,00 3,00 4,00 5,00	ES C	N21			0,30	Grass over soft brown sandy CLAY with frequent roots and rootlets. (TOPSOIL)		
					(0,70) 1,00	Greenish grey very silty SAND.		
	C	N0			(0,75) 1,75	Soft greenish grey and brown very sandy CLAY.		
					(2,90) 4,65	Very soft light grey very sandy CLAY.		
	N10			(0,80) 5,45	Firm grey silty CLAY.			

Boring Progress and Water Observations						Chiselling			Water Added		GENERAL REMARKS
Depth	Date	Time	Casing Depth	Casing Dia. mm	Water Depth	From	To	Hours	From	To	
0,00 5,45	18-11-25 18-11-25	08,45 09,45	1,00	113	2.1						
All dimensions in metres Scale 1:50		Client Peter Davies				Method/ Plant Used		Dart		Logged By IP	

BOREHOLE LOG

Project Potterne				BOREHOLE No WS02
Job No G0807	Date 18-11-25 18-11-25	Ground Level (m)	Co-Ordinates ()	
Contractor WWGI				Sheet 1 of 1

[illegible]

Boring Progress and Water Observations						Chiselling			Water Added		GENERAL REMARKS
Depth	Date	Time	Depth	Casing Dia. mm	Water Depth	From	To	Hours	From	To	
0,00 6,45	18-11-25 18-11-25	09,45 10,30	1,00	113	3.5						
All dimensions in metres Scale 1:50		Client Peter Davies				Method/ Plant Used		Dart		Logged By IP	



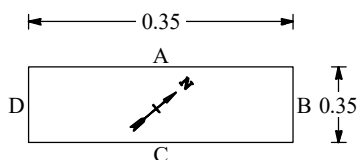
TRIAL PIT LOG

Project Potterne				TRIAL PIT No HP01
Job No G0807	Date 18-11-25 18-11-25	Ground Level (m)	Co-Ordinates ()	
Contractor WWGI				Sheet 1 of 1

A		B		C		D		Legend	
0									
1									
2									
3									
4									

STRATA				SAMPLES & TESTS		
Depth	No	DESCRIPTION		Depth	No	Remarks/Tests
0,00-0,25		MADE GROUND: Grass over soft dark brown very sandy gravelly CLAY with frequent rootlets. Gravel is angular fine brick.				

Shoring/Support: None
Stability: Stable



GENERAL REMARKS

Hole terminated upon concrete obstruction.

All dimensions in metres
Scale 1:50

Client **Peter Davies**

Method/
Plant Used

Hand tools

Logged By
IP



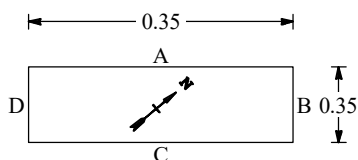
TRIAL PIT LOG

Project Potterne				TRIAL PIT No HP02
Job No G0807	Date 18-11-25 18-11-25	Ground Level (m)	Co-Ordinates ()	
Contractor WWGI				Sheet 1 of 1

A		B		C		D		Legend	
0									
1									
2									
3									
4									

STRATA				SAMPLES & TESTS		
Depth	No	DESCRIPTION		Depth	No	Remarks/Tests
0,00-0,65		MADE GROUND: Grass over soft dark brown very sandy gravelly CLAY with frequent rootlets. Gravel is angular fine brick.		0,50	ES	

Shoring/Support: None
Stability: Stable



GENERAL REMARKS

Hole terminated upon concrete obstruction.

All dimensions in metres
Scale 1:50

Client **Peter Davies**

Method/
Plant Used

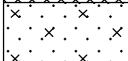
Hand tools

Logged By
IP



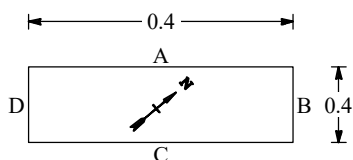
TRIAL PIT LOG

Project Potterne				TRIAL PIT No SA01
Job No G0807	Date 18-11-25 18-11-25	Ground Level (m)	Co-Ordinates ()	
Contractor WWGI				Sheet 1 of 1

A		B		C		D		Legend	
0									
									
1									
2									
3									
4									

STRATA				SAMPLES & TESTS		
Depth	No	DESCRIPTION		Depth	No	Remarks/Tests
0,00-0,40		MADE GROUND: Grass over soft dark brown very sandy gravelly CLAY with frequent rootlets. Gravel is angular fine brick.				
0,40-0,85		Greenish grey very silty SAND.				

Shoring/Support: None
Stability: Stable



GENERAL REMARKS

Soakaway test undertaken in base of hand pit in spirit of BRE365.

All dimensions in metres
Scale 1:50

Client **Peter Davies**

Method/
Plant Used

Hand tools

Logged By
IP

Trial Pit Soakaway Test



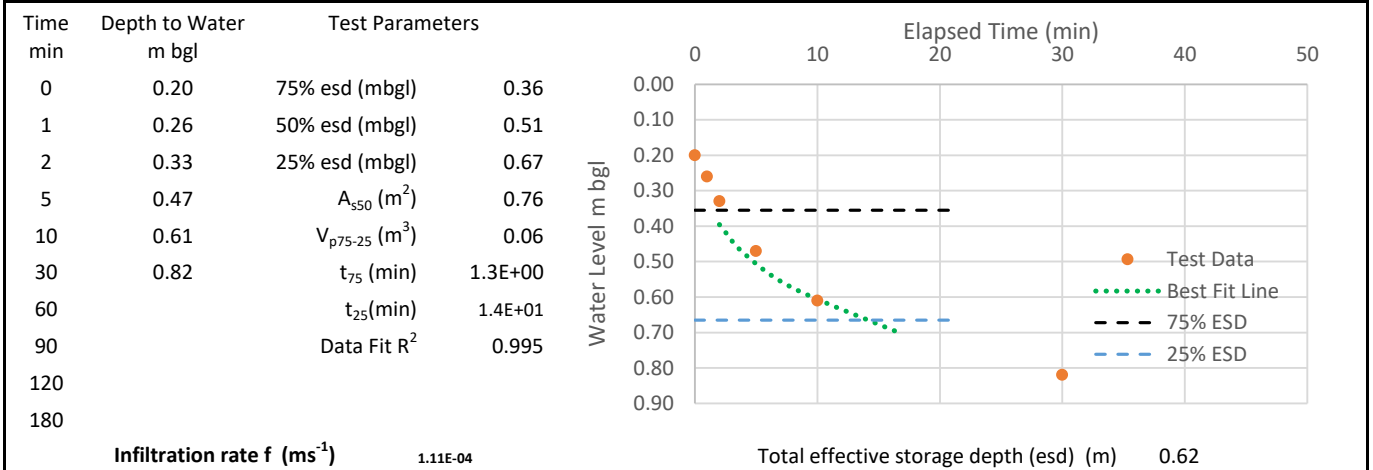
Based on BRE DG 365:2016

Project	Potterne	Status	LOCATION ID
Project ID	G0807	CHECKED	SA01

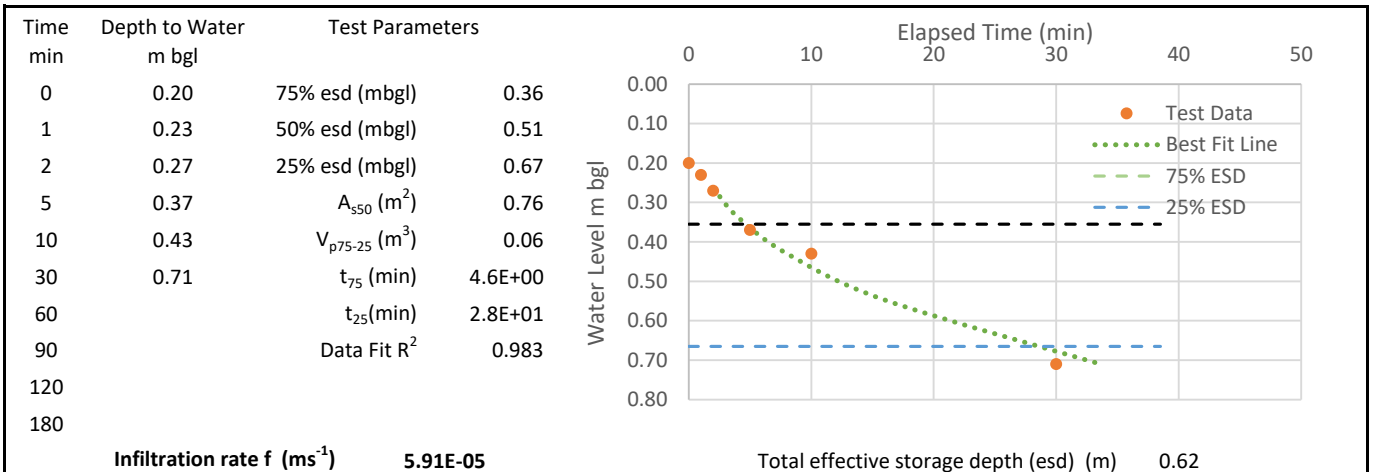
Trial Pit Details

	Test 1	Test 2	Test 3	Ground Level	mAOD	Date Excavated	18/11/2025
Depth	0.82	0.82	0.82	Coordinates	mE	Date Tested	18/11/2025
Width	0.45	0.45	0.45		mN		
Length	0.45	0.45	0.45				

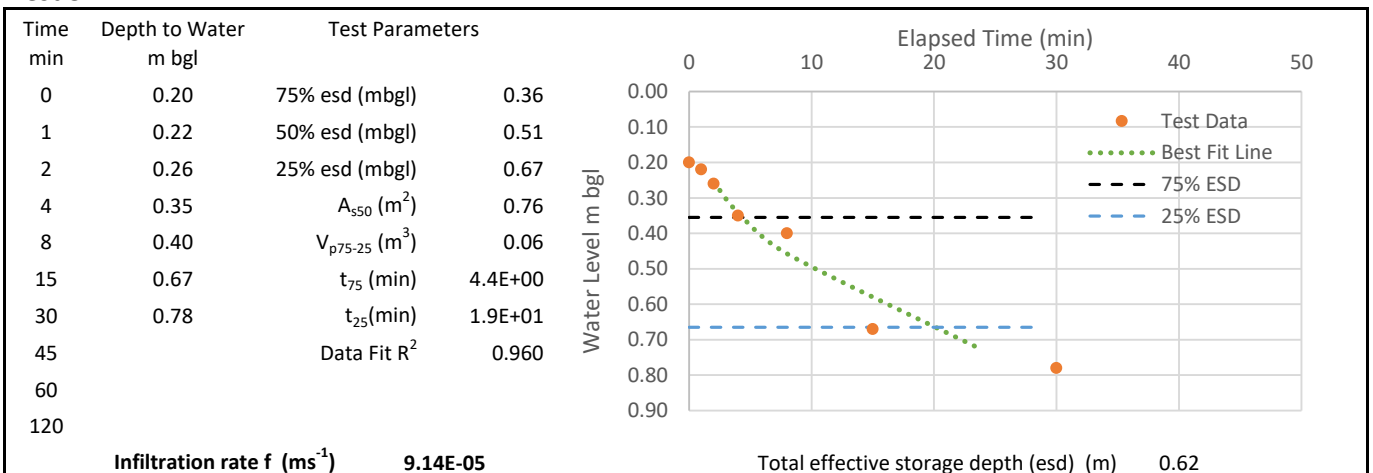
Test 1



Test 2



Test 3



Carried out by	Notes:	Logged	Checked
Wessex and West		IP	CP

APPENDIX C – CERTIFICATION

SPT Hammer Energy Test Report

in accordance with BSEN ISO 22476-3:2005

Southern Testing
Unit 11
Charlwoods Road
East Grinstead
West Sussex
RH19 2HU

SPT Hammer Ref: AR4212
Test Date: 11/02/2025
Report Date: 11/02/2025
File Name: AR4212.spt
Test Operator: BOB

Instrumented Rod Data

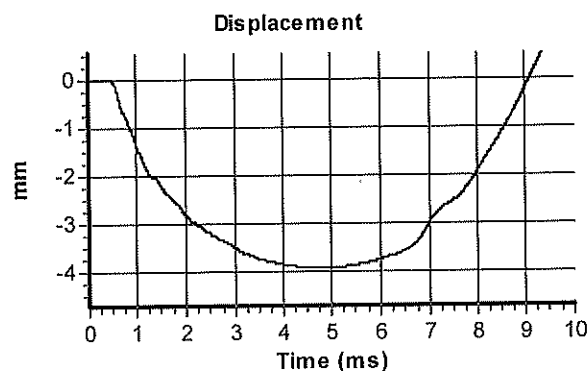
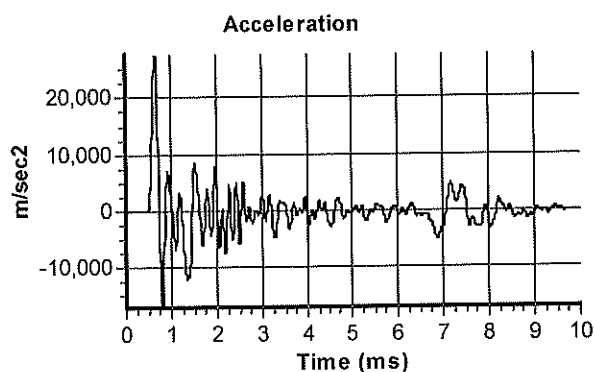
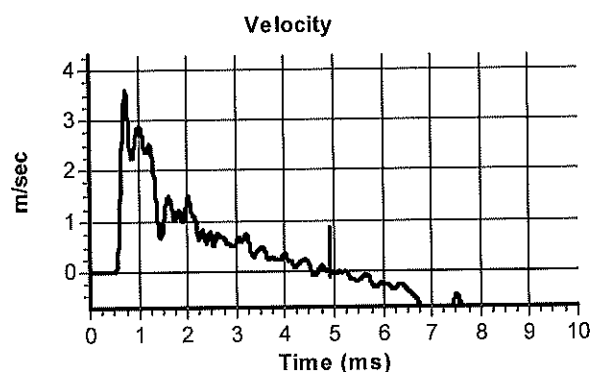
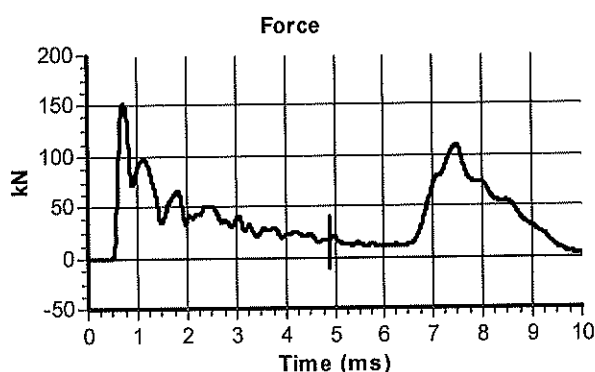
Diameter d_r (mm): 54
Wall Thickness t_r (mm): 6.7
Assumed Modulus E_a (GPa): 208
Accelerometer No.1: 73547
Accelerometer No.2: 64789

SPT Hammer Information

Hammer Mass m (kg): 63.5
Falling Height h (mm): 760
SPT String Length L (m): 14.7

Comments / Location

CHARLWOODS



Calculations

Area of Rod A (mm²): 996
Theoretical Energy E_{theor} (J): 473
Measured Energy E_{meas} (J): 290

Energy Ratio E_r (%):

61

Signed: Bob Stewart

Title: Technician

The recommended calibration interval is 6 months

APPENDIX D – GEOTECHNICAL LABORATORY TEST RESULTS



Laboratory Report



Contract Number: 82823

Client Ref: **G0807**

Client PO: **G0807**

Date Received: **25-11-2025**

Date Completed: **11-12-2025**

Report Date: **11-12-2025**

Client: **Wessex and West Ground Investigation**

This report has been checked and approved by:

Contract Title: **Potterne**

For the attention of: **Ian Parsons**

Darren Bourne

Quality Senior Technician

Description	Qty
Determination of water content BS EN ISO 17892-1:2014 - * UKAS	3
4 Point Liquid & Plastic Limit.. BS EN ISO 17892-12 - * UKAS	3

Notes: Observations and Interpretations are outside the UKAS Accreditation

* - denotes test included in laboratory scope of accreditation

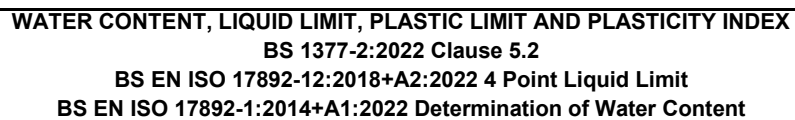
- denotes test carried out by approved contractor

@ - denotes non accredited tests

This certificate is issued in accordance with the accreditation requirements of the United Kingdom Accreditation Service. The results reported herein relate only to the material supplied to the laboratory. This test report/certificate shall not be reproduced except in full, without the approval of GEO Site & Testing Services Ltd. Any opinions or interpretations stated - within this report/certificate are excluded from the laboratories UKAS accreditation.

Approved Signatories:

Brendan Evans (Business Support Co-ordinator) - Darren Bourne (Quality Senior Technician) - Jason Jenkins (Business Development Manager)
Luke Williams (Senior Laboratory Technician) - Paul Evans (Director) - Richard John (Quality/Technical Manager)
Shaun Jones (Laboratory manager) - Shaun Thomas (Site Manager) - Vince Williams (Site Technician)
Wayne Honey (HR & HSE Manager)



Contract Number	82823	
Project Name	Potterne	
Date Tested	10/12/2025	
	DESCRIPTIONS	

[illegible]

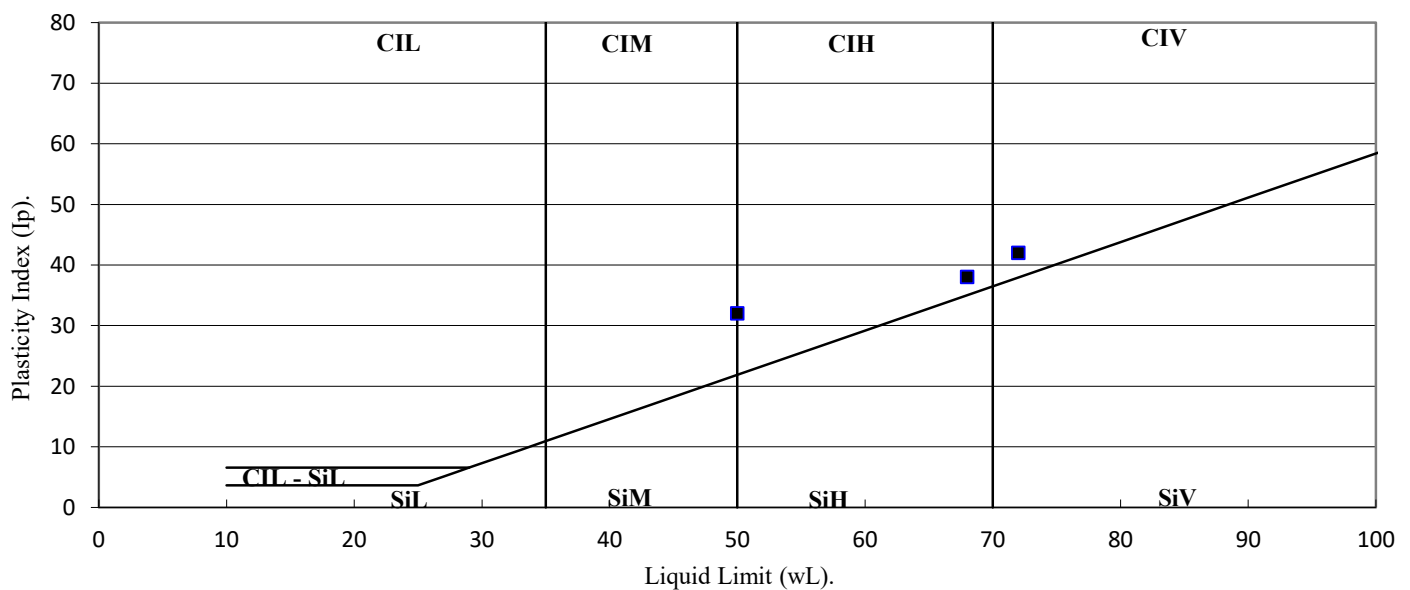
Operator
Ben

Contract Number	82823	
Project Name	Potterne	
Date Tested	10/12/2025	
Test Comments	80g/30° Fall cone used with increasing water content	

[illegible]

SYMBOLS : NP = Non Plastic NB: All liquid limits are 4 point and wet sieved

PLASTICITY CHART



***For sample descriptions please see sample descriptions sheet**

Operator
Ben

APPENDIX E – GEOENVIRONMENTAL LABORATORY TEST RESULTS

ANALYTICAL TEST REPORT

Report Number 25-11424, issue number 1

Contract name: Potterne

Client reference: G0807

Clients name: Wessex and West Ground Investigation Ltd

Clients address: Wessex and West Ground Investigation Ltd
25 Highfield Close
Semington
Trowbridge
BA14 6JZ

Samples received: 20/11/2025

Analysis started: 20/11/2025

Analysis completed: 01/12/2025

Report issued: 01/12/2025

Key

U UKAS accredited test

M MCERTS & UKAS accredited test

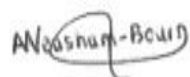
(B) Analysis performed at Southampton Site

I/S Insufficient sample to carry out test

U/S Sample not suitable for testing

NAD No Asbestos Detected

Full key available on Information page



Approved by: Abbie Neasham-Bourn
Senior Reporting Administrator

SAMPLE INFORMATION

MCERTS (Soils):

Soil descriptions are only intended to provide a log of sample matrices with respect to MCERTS validation. They are not intended as full geological descriptions. MCERTS accreditation applies for sand, clay and loam/topsoil, or combinations of these whether these are derived from naturally occurring soils or from made ground, as long as these materials constitute the major part of the sample. Other materials such as concrete, gravel and brick are not accredited if they comprise the major part of the sample.

% Moisture reported as MCERTS drying at 30°C

Lab ref	Sample ID	Depth (m)	Sample description	Material removed	% Removed	% Moisture
77420	WS01	0.80 - 0.80	Brown Clay with Gravel and Vegetation.	-	-	17.6
77421	WS02	0.80 - 0.80	Brown Clay with Gravel and Vegetation.	-	-	30.2
77422	HP01	0.80 - 0.80	Brown Clayey Loam with Gravel and Vegetation.	-	-	15.0

DEVIATING SAMPLE INFORMATION

Comments

Sample deviation is determined in accordance with the UKAS note "Guidance on Deviating Samples" and based on reference standards and laboratory trials.
For samples identified as deviating, test result(s) may be compromised and may not be representative of the sample at the time of sampling.
Chemtech Environmental Ltd cannot be held responsible for the integrity of sample(s) received if Chemtech Environmental Ltd did not undertake the sampling. Such samples may be deviating.

Key

- a Sampling date not provided
- b Sampling time not provided (waters only)
- c Sample not received in appropriate containers
- d Storage Temperature
- e Headspace present in sample container
- f Sample exceeded sampling to receipt
- g Sample exceeded holding time(s)

Lab ref	Sample ID	Depth (m)	Deviating	Tests (Reason for deviation)
77420	WS01	0.80 - 0.80	N	
77421	WS02	0.80 - 0.80	N	
77422	HP01	0.80 - 0.80	N	

SOILS

Lab Number					77420	77421	77422
Sample ID					WS01	WS02	HP01
Depth (m)					0.80 - 0.80	0.80 - 0.80	0.80 - 0.80
Sampling Date					18/11/2025	18/11/2025	18/11/2025
Sampling Time					9:30	9:45	10:00
Test	Method	Accred	LoD	Units			
Asbestos							
Asbestos Identification	SUBCO N	SU	0	-	NAD	NAD	NAD
Metals							
Water Soluble Boron	CE063	N	0.5	mg/kg	0.92	0.50	0.58
Arsenic	CE264	U	1.8	mg/kg	9.0	7.9	10.0
Beryllium	CE264	M	0.2	mg/kg	0.7	0.4	0.8
Cadmium	CE264	M	1.6	mg/kg	< 1.6	< 1.6	< 1.6
Chromium	CE264	U	2	mg/kg	25.6	14.6	19.6
Copper	CE264	U	1.6	mg/kg	< 1.6	3.9	13.3
Lead	CE264	U	2.3	mg/kg	7.8	12.3	65.1
Mercury	CE264	U	0.7	mg/kg	< 0.7	< 0.7	< 0.7
Nickel	CE264	M	2.1	mg/kg	7.8	11.4	13.5
Selenium	CE264	U	3	mg/kg	< 3.0	< 3.0	< 3.0
Vanadium	CE264	N	1.8	mg/kg	46.0	47.7	52.6
Zinc	CE264	M	4	mg/kg	28.0	23.6	151
Colourimetric							
Water Soluble Sulphate	CE261	U	1.8	mg/l	55.2	53.9	84.6
Total Monohydric Phenols	CE078	N	0.5	mg/kg	< 0.50	< 0.50	< 0.50
Total Cyanide	CE077	N	1	mg/kg	< 1.0	< 1.0	< 1.0
Free Cyanide	CE077	N	1	mg/kg	< 1.0	< 1.0	< 1.0
Thiocyanate	CE145	M	1	mg/kg	< 1.0	< 1.0	< 1.0
Total Sulphide	CE284	N	4	mg/kg	< 4	< 4	< 4
Combustion							
Moisture Content	CE001	N	0.1	%	17.6	30.2	15.0
Soil Organic Matter	CE192	N	0.1	%	1.47	2.62	7.71
Polyaromatic hydrocarbons							
Naphthalene	CE087	M	0.016	mg/kg	< 0.016	< 0.016	0.018
Acenaphthylene	CE087	M	0.015	mg/kg	< 0.015	< 0.015	0.053
Acenaphthene	CE087	M	0.013	mg/kg	< 0.013	< 0.013	< 0.013
Fluorene	CE087	U	0.013	mg/kg	< 0.013	< 0.013	< 0.013
Phenanthrene	CE087	M	0.014	mg/kg	< 0.014	< 0.014	0.185



SOILS

Lab Number					77420	77421	77422
Sample ID					WS01	WS02	HP01
Depth (m)					0.80 - 0.80	0.80 - 0.80	0.80 - 0.80
Sampling Date					18/11/2025	18/11/2025	18/11/2025
Sampling Time					9:30	9:45	10:00
Test	Method	Accred	LoD	Units			
Anthracene	CE087	U	0.017	mg/kg	< 0.017	< 0.017	0.054
Fluoranthene	CE087	M	0.017	mg/kg	< 0.017	0.019	1.06
Pyrene	CE087	M	0.016	mg/kg	< 0.016	0.020	0.963
Benzo(a)anthracene	CE087	U	0.012	mg/kg	< 0.012	0.025	0.696
Chrysene	CE087	M	0.028	mg/kg	< 0.028	< 0.028	0.630
Benzo(b)fluoranthene	CE087	M	0.02	mg/kg	< 0.020	0.025	1.04
Benzo(k)fluoranthene	CE087	M	0.025	mg/kg	< 0.025	< 0.025	0.423
Benzo(a)pyrene	CE087	U	0.019	mg/kg	< 0.019	0.022	0.903
Indeno(1,2,3-cd)pyrene	CE087	M	0.019	mg/kg	< 0.019	0.022	0.813
Dibenzo(a,h)anthracene	CE087	M	0.017	mg/kg	< 0.017	< 0.017	0.175
Benzo(g,h,i)perylene	CE087	M	0.019	mg/kg	< 0.019	< 0.019	0.609
Total PAH(16)	CE087	N	0.28	mg/kg	< 0.280	< 0.280	7.63
Total Petroleum Hydrocarbons							
>C5-C6 Aliphatic (HS_1D_AL)	CE267	N	0.1	mg/kg	< 0.10	< 0.10	< 0.10
>C6-C8 Aliphatic (HS_1D_AL)	CE267	N	0.1	mg/kg	< 0.10	< 0.10	< 0.10
>C8-C10 Aliphatic (HS_1D_AL)	CE267	N	0.1	mg/kg	< 0.10	< 0.10	< 0.10
>C10-C12 Aliphatic (EH_2D_AL)	CE250	N	1	mg/kg	< 1.0	< 1.0	< 1.0
>C12-C16 Aliphatic (EH_2D_AL)	CE250	N	0.5	mg/kg	< 0.5	< 0.5	0.8
>C16-C21 Aliphatic (EH_2D_AL)	CE250	N	0.7	mg/kg	< 0.7	< 0.7	1.2
>C21-C35 Aliphatic (EH_2D_AL)	CE250	N	4	mg/kg	< 4.0	< 4.0	9.6
>C35-C40 Aliphatic (EH_2D_AL)	CE250	N	0.5	mg/kg	< 0.5	< 0.5	0.9
>C5-C7 Aromatic (HS_1D_AR)	CE267	N	0.01	mg/kg	< 0.010	< 0.010	< 0.010
>C7-C8 Aromatic (HS_1D_AR)	CE267	N	0.01	mg/kg	< 0.010	< 0.010	< 0.010
>C8-C10 Aromatic (HS_1D_AR)	CE267	N	0.01	mg/kg	< 0.010	< 0.010	< 0.010
>C10-C12 Aromatic (EH_2D_AR)	CE250	N	0.6	mg/kg	< 0.6	< 0.6	3.5
>C12-C16 Aromatic (EH_2D_AR)	CE250	N	1	mg/kg	1.3	< 1.0	9.2
>C16-C21 Aromatic (EH_2D_AR)	CE250	N	2	mg/kg	< 2.0	< 2.0	27.8
>C21-C35 Aromatic (EH_2D_AR)	CE250	N	4.5	mg/kg	< 4.5	< 4.5	83.6
>C35-C40 Aromatic (EH_2D_AR)	CE250	N	1.5	mg/kg	< 1.5	< 1.5	16.9
Wet Chem							
pH	CE004	M	0.1	pH units	7.5	8.6	7.6

METHOD DETAILS

METHOD	TESTNAME	METHOD SUMMARY	ANALYSIS BASIS
CE267	VPH in Soil	HS-GCFID	As submitted sample
CE250	GCXGC in Soil	DCM Extraction and GCxGC-FID	As submitted sample
SUBCON	Asbestos Soil	HSG248	Air Dried Sample
CE063	Water soluble boron	ICPOES	Air dried sample
CE264	Metals by ICP in Soil	ICPOES	Air dried sample
CE261	Anions by Discrete Analyser in Soils	Gallery	Air dried sample
CE087	PAH in Soil	DCM Extraction and GCMS	As submitted sample
CE078	Phenols in Soil	Continuous Flow Analyser	As submitted sample
CE077	Cyanides in Soils	Continuous Flow Analyser	As submitted sample
CE145	Thiocyanate	UV Spectrometer	Air dried sample
CE284	Sulphide By DA in Soil	Gallery Discrete Analyser	Air dried sample

REPORT INFORMATION

Report No.:25-11424, issue number 1

Key

U	ISO17025 Accredited Result
M	ISO17025 and MCERTS Accredited Result
N	Do not currently hold accreditation
^	MCERTS accreditation not applicable for sample matrix
*	ISO17025 accreditation not applicable for sample matrix
S	Subcontracted
I/S	Insufficient Sample
U/S	Unsuitable sample
N/T	Not tested
<	Means "less than"
>	Means "greater than"

LOD refers to limit of detection, except in the case of pH soils and pH waters where it means limit of discrimination.

This report shall not be reproduced except in full, without prior written approval.

Opinions and interpretations expressed herein are outside the UKAS accreditation scope.

All testing carried out at Unit 6 Parkhead, Stanley, DH9 7YB, except for subcontracted testing.

The results relate only to the sample received.

Unless otherwise stated, sample information has been provided by the client. This may affect the validity of the results.

Moisture Content Calculated on a Wet Weight basis (at 30°C)

Unless otherwise stated, Chemtech Environmental Ltd was not responsible for sampling.

Where sampling was undertaken by Chemtech Environmental Limited it is outside the UKAS accreditation scope.

Methods, procedures and performance data are available on request.

Results reported herein relate only to the material supplied to the laboratory.

BTEX compounds are identified by retention time only and may include interference from co-eluting compounds.

For soils and solids, all results are reported on a dry basis (30°C). Samples dried at no more than 30°C in a drying cabinet.

For soils and solids, analytical results are inclusive of stones, where applicable.

'Client Reference', 'Sample ID', 'Sample Location', 'Sample Type', 'Depth', 'Sample Date' and 'Sample Time' information is provided by the customer

Sample Retention and Disposal

All soil samples will be retained for a period of 4 weeks from the point of receipt

All water samples will be retained for a period of 2 weeks from the point of Reporting

Charges may apply to extended sample storage

TPH Classification - HWOL Acronym System

HS	Headspace analysis
EH	Extractable Hydrocarbons - i.e. everything extracted by the solvent
CU	Clean-up - e.g. by florisil, silica gel
1D	GC - Single coil gas chromatography
Total	Aliphatics & Aromatics
AL	Aliphatics only
AR	Aromatics only
2D	GC-GC - Double coil gas chromatography
#1	EH_Total but with humics mathematically subtracted
#2	EH_Total but with fatty acids mathematically subtracted
_	Operator - underscore to separate acronyms (exception for +)
+	Operator to indicate cumulative e.g. EH+HS_Total or EH_CU+HS_Total
MS	Mass Spectrometry

Unless specifically identified (noted as "(B)" in analyte name) all internal analysis performed at Durham site

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