

DRAINAGE STRATEGY REPORT

*PREMIER INN SALISBURY NORTH
PEARCE WAY
SALISBURY
SP1 3GU*

PREPARED FOR:



JOB NO: P24-0132
DATE: 13th June 2024
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CONTENTS

1. INTRODUCTION.....	3
2. SITE LOCATION AND TOPOGRAPHY	3
3. SURFACE WATER MANAGEMENT AND DRAINAGE STRATEGY	5
4. FOUL WATER DRAINAGE STRATEGY	10
5. SUMMARY & CONCLUSIONS	11

APPENDICES

APPENDIX A:	TOPOGRAPHIC SURVEY
APPENDIX B:	DRAINAGE STRATEGY PLAN
APPENDIX C:	MICRODRAINAGE ATTENUATION TANK DESIGN RESULTS
APPENDIX D:	WESSEX WATER PRE-DEV ENQUIRY CORRESPONDANCE

1. INTRODUCTION

- 1.1 This report has been prepared on behalf of Premier Inn Hotels Ltd. to accompany a planning application for a proposed hotel extension comprising additional bedrooms, parking alterations and associated hard and soft landscaping.
- 1.2 The report details a strategy for the disposal and management of surface and foul water runoff that will be generated by the development.

2. SITE LOCATION AND TOPOGRAPHY

Site Location

- 2.1 The site is located on Pearce Way, Salisbury as shown in *Figure 1* below. The site is centred on Ordnance Survey grid reference SU 16113 32281 and co-ordinates X: 416113; Y: 132281. The site's post code is SP1 3GU.

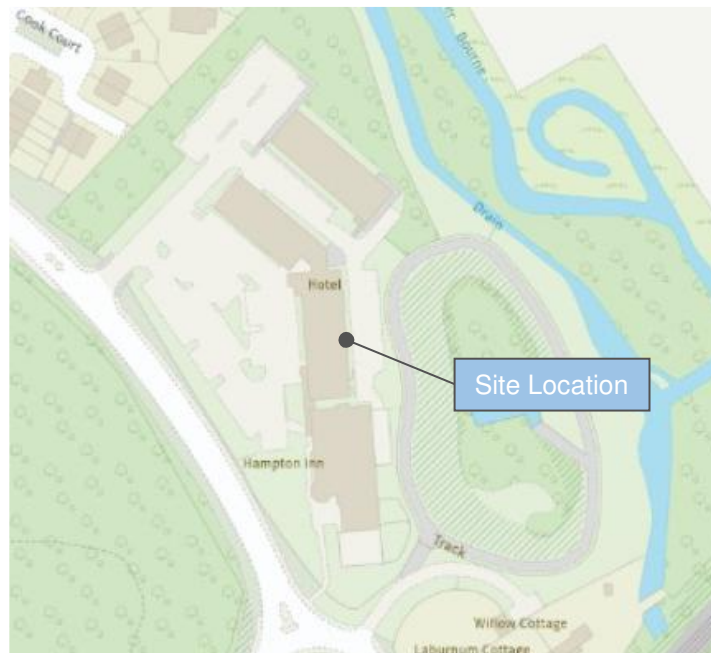


Figure 1: Site Location

Site Description

- 2.2 The existing site comprises a Premier Inn Hotel, annex and Table Table restaurant with associated parking, hard landscaping and soft landscaping. Pedestrian and vehicular access is gained from Pearce Way to the West. An additional pedestrian access route is located to the south of the site off Hampton Park Roundabout.

Topography

- 2.3 The topographical survey included in *Appendix A*, shows that the site generally falls in a North-Easterly direction with the topography of the site ranging between 58.90m AOD at the vehicular access off Pearce Way to 55.60m AOD in the northern corner of the car park. Artificial falls have been created within the existing car parking for drainage purposes.

JOB NO:	P24-0132	ISSUE NO:	1	ISSUE DATE:	13/06/2024	Page 3
AUTHOR:	JJH	OFFICE:	HENLEY	CHECKED BY:	PB	

Existing Drainage Characteristics

- 2.4 Wessex Water sewer records shown in *Figure 2* below shows the site to be drained by foul and surface water sewer networks within the local area.

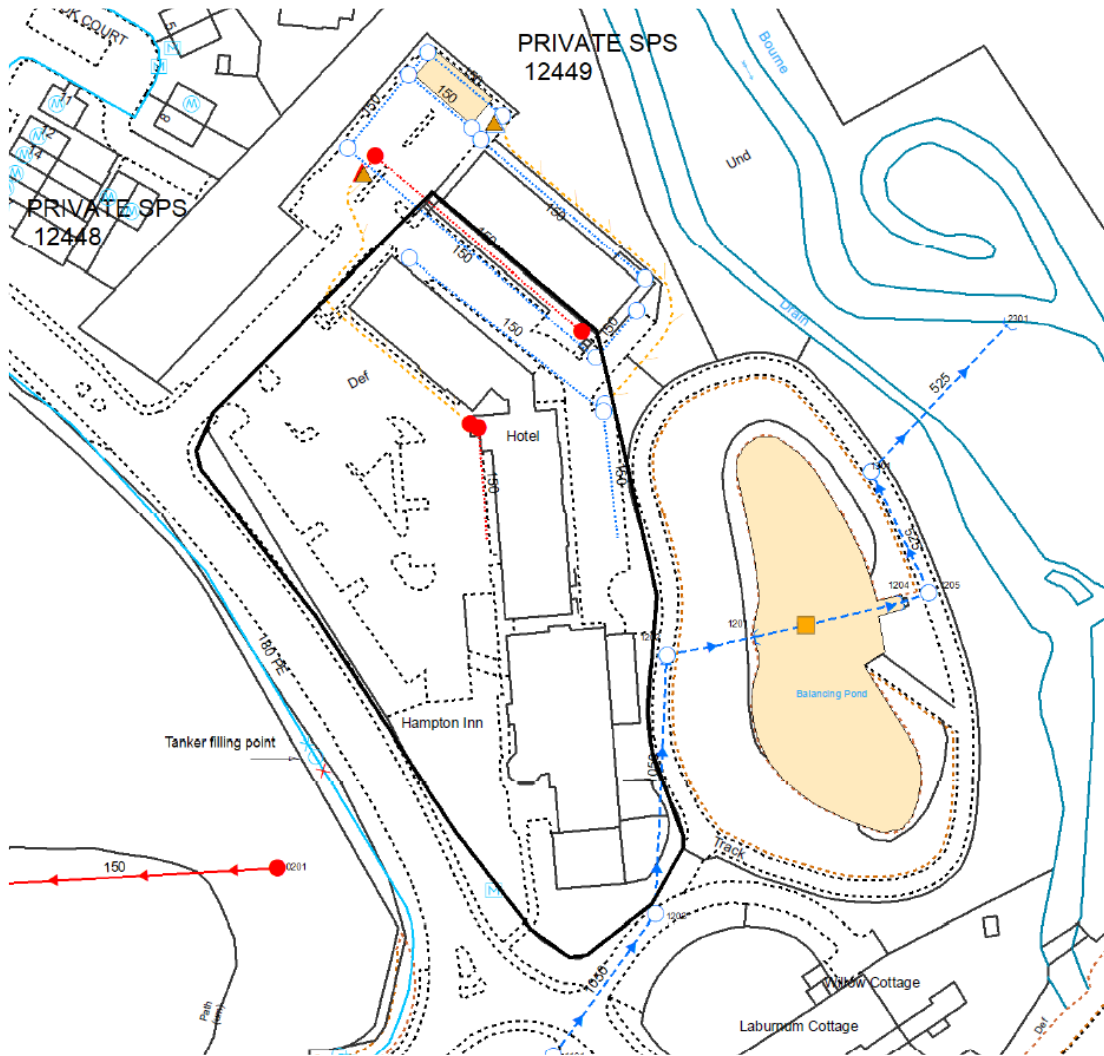


Figure 2: Wessex Water Sewer Records

- 2.5 The topographical survey concurs with this and identifies the site to be drained by separate traditional networks of below ground pipework, with an existing surface water gravity outfall to the Wessex Water surface water sewer network located to the south within a private access road where it then discharges into the River Bourne via a balancing pond. Prior to discharging into the gravity connection to the sewer, surface water flows are pumped from the northern corner of the site where the annex is located. The survey also identifies a foul water gravity outfall to the Wessex Water foul water sewer network located to the west within undeveloped land. Prior to discharging into the gravity connection to the sewer, foul water flows are pumped from the annex.

JOB NO:	P24-0132	ISSUE NO:	1	ISSUE DATE:	13/06/2024	Page 4
AUTHOR:	JJH	OFFICE:	HENLEY	CHECKED BY:	PB	

Flood Risk

- 2.6 The Flood Zone map in *Figure 3* below has been taken from the 'gov.uk' website and shows the site to be located within Flood Zone 1 which is representative of land that has a low probability of flooding from rivers and the sea. The site is also considered to be at low risk from all other sources of flooding.

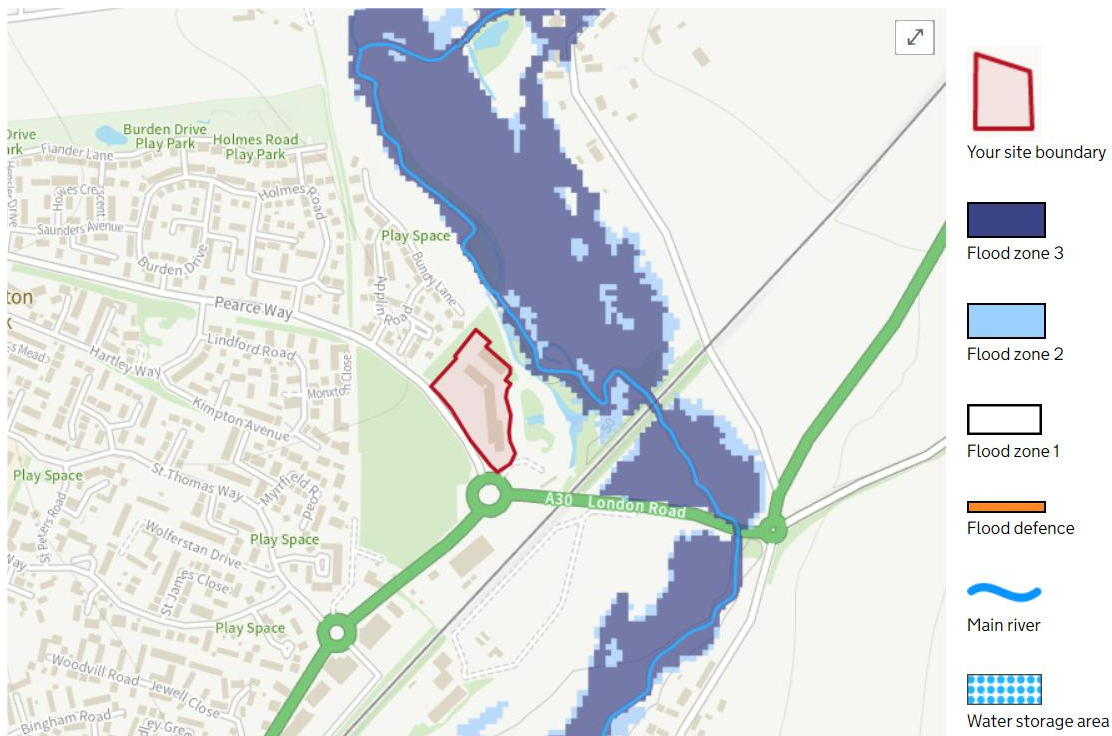


Figure 3: EA Flood Zone Map for Planning

3. SURFACE WATER MANAGEMENT AND DRAINAGE STRATEGY

Surface Water Drainage Policy & Guidance

- 3.4 In section 2 it was established that the site is drained by separate private foul and surface water networks, with connections to the Wessex Water sewer networks serving the local area.
- 3.5 It is proposed to replicate this approach for the proposed extension and a drainage strategy has been developed for the scheme on this basis. A copy of the drainage strategy plan is included in *Appendix B* along with a description of the main principles below:
- Roof water runoff from the proposed extension would be captured by a combination of rainwater gutters and downpipes that would discharge directly into a below ground network comprising traditional pipework.
 - Surface water runoff from the car park would be captured by a series of gullies and channel drains that would discharge directly into a below ground network comprising traditional pipework.

- Prior to discharging into the Wessex Water surface water sewer network, surface water runoff generated from the development would discharge into the existing private surface water package pumping station serving the Annex and northern parking area which shall remain and continue to discharge at a rate of 2.6 l/s.
- To accommodate the surface water runoff generated from the additional impermeable area, the existing below ground attenuation tank would be increased in size such that it provides a total storage volume of 121.6m³.
- The increased size of the attenuation tank not only accommodates the additional flows but has been sized in accordance with modern standards such that excess flows from the annex and the proposed extension will be stored and attenuated within the below ground geocellular attenuation tank for all return periods up to and including the 1 in 100-year event with an allowance of 45% for future climate change.

Hydraulic Analysis

3.6 The Source Control facility in the MicroDrainage software package by 'Innovyze' has been used to hydraulically design the geocellular attenuation tank based on the following parameters:

- A depth/flow relationship outflow control has been used to mimic the package pumping station, which discharges at a rate of 2.6 l/s.
- Total drained area of 1981m², which is based on the existing drained area plus the increase in impermeable area as a result of the extension (31m²).
- Return periods up to and including the 1 in 100 year event with an allowance of 45% for potential future increases in rainfall intensities associated with climate change in accordance with the climate change allowance map found on the 'gov.uk' website. An extract is included in *Figure 4* below.

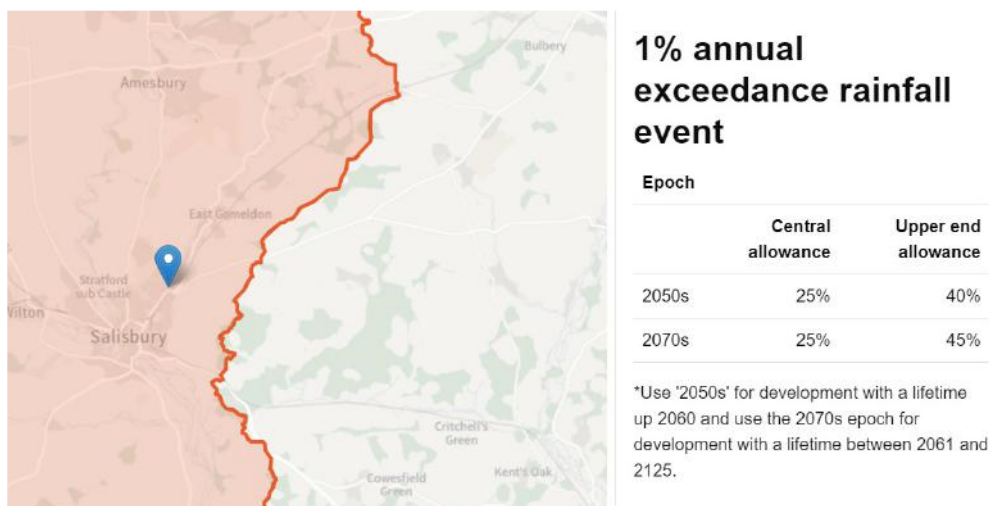


Figure 4: Avon Hampshire Management Catchment peak rainfall allowances

- Cv values of 0.95
- FEH rainfall point data (2013) GB 416091 132356.

- 3.7 The design results for the geocellular attenuation tank are included in *Appendix C*. The results demonstrate that the proposed attenuation tank would have sufficient capacity to store runoff for all rainfall events up to and including the required 1 in 100-year event with 45% allowance for climate change.

Exceedance

- 3.8 If the capacity of the proposed surface water drainage network was exceeded, site levels would allow surface water to be channelled off-site in a north-easterly direction.
- 3.9 The building's finished floor level would generally be raised above surrounding levels to minimise the risk from overland flows. Exceedance flood flow route arrows have been shown to demonstrate this on the drainage strategy drawing included in *Appendix B*.

Implementation and Maintenance

- 3.10 During construction, it is normal practice for a drainage system to be installed at an early stage in the programme. As such, runoff from the construction site, which can be heavily laden with silt, are likely to restrict flow within the drainage system. Therefore, the following measures will be considered to address this issue:

- A catchpit chamber will be installed upstream of the attenuation tank to prevent sediments such as soil and mud from transferring into the drainage system downstream. The catchpit is to be regularly inspected throughout construction.
- Protective coverings will be used to help prevent runoff stripping material stockpiles.
- Surfaces used as access roads and storage areas during construction will be swept regularly to prevent the accumulation of dust and mud.
- Should groundwater be encountered in excavations, the water will not be discharged to the drainage systems until the amount of suspended solids has been reduced through the controlled use of skips or tanks, which will act as stilling basins.
- To prevent contamination associated with the use of oils and hydrocarbons during construction, the contractor shall ensure that the following precautionary measures are employed during construction:
 - Regular maintenance of machinery and plant.
 - Use of drip trays.
 - Regular checking of machinery and plant for oil leaks.
 - Use of correct storage facilities.
 - Regular checks for signs of wear and tear on tanks.
 - Specific procedures are followed when refuelling.
 - Emergency spill kit to be located near refuelling area.
 - Regular emptying of bunds.
 - Tanks should be located in secure areas to stop vandalism.

- 3.11 On occupation of the development, it is recommended that each element of the as-built drainage system is maintained by Premier Inn Hotels appointed management company in accordance with the regime set out in the tables below.

JOB NO:	P24-0132	ISSUE NO:	1	ISSUE DATE:	13/06/2024	Page 7
AUTHOR:	JJH	OFFICE:	HENLEY	CHECKED BY:	PB	

Table 1: Below Ground Drainage System - Operation and Maintenance Requirements

Maintenance schedule	Required action	Frequency
Regular maintenance	Remove all litter and debris from external hard landscaped areas and adjacent landscaping, which may pose a risk to the performance of the system.	Monthly.
	Remove build-up of sediment / silt in catch-pits and dispose of oils / petrol residues using safe standard practices.	As required.
	Stabilise and mow adjacent landscaped areas and remove weeds.	
Remedial actions	Repair or rehabilitate inlet and outlets to ensure they are in good condition and operating as designed.	
	Remediate any landscaping, which has raised to within 50mm of the level of adjacent hard landscaping.	
Monitoring	Check of all inlets / outlets for blockages or evidence of physical damage with any necessary remedial action or clearance carried out if required.	On a monthly basis for the first 3 months of operation, thereafter every 6 months & following severe rainfall events.
	Inspect all surfaces for ponding, or silt accumulation. Record areas where water is ponding for more than 48 hours and carry out any remedial work deemed necessary.	After severe storms.

Table 2: Existing Bypass Separator - Operation and Maintenance Requirements

Maintenance schedule	Required action	Frequency
Regular maintenance	Cleaning of interceptor, inlets, and outlets of retained pollutants & silt.	Every 3, 6 or 12 months depending on site conditions (or following poor performance).
Remedial actions	Repair of erosion damage, or damage to tank.	As required.
	Interceptor repairs.	
Monitoring	Inspection of the tank for pollutants and silt.	Every 3, 6 or 12 months depending on site conditions (or following poor performance).
	Inspection of inlets and outlets.	
	Inspection of areas receiving overflow, for evidence of erosion.	

Table 3: Proposed/Existing Geocellular Storage Tank - Operation and Maintenance Requirements

Maintenance schedule	Required action	Frequency
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action.	Monthly for first 3 months of operation, then every 6 months.
	Debris removal from catchment surface (where may cause risks to performance).	Monthly.
	Where rainfall infiltrates into blocks from above, check surface of filter for blockage by silt, algae or other matter. Remove and replace surface infiltration medium as necessary.	Monthly / after severe storms.
	Remove sediment from pre-treatment structures.	Annually, or as required.
Remedial actions	Repair/rehabilitation of inlets, outlet, overflows and vents.	As required.
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed.	Annually and after large storms.

Table 4: Existing Package Pumping Station - Operation and Maintenance Requirements

Maintenance schedule	Required action	Frequency
Regular maintenance	Cleaning off the pumps and floats of any debris/ sediment	As required
Remedial Actions	Pump repairs.	As required
	Repair of erosion damage, or damage to tank.	
Monitoring	Checking current resistance to see if there is any indication of motor degradation.	Every 6 months (Unless poor site conditions, therefore every 3 months)
	Checking the impellers to see if they require replacing due to wear.	
	Checking the float switches are set and operating correctly.	
	Inspection of the tank for debris and sediment build up.	
	Inspection of inlets & outlets.	
	Inspection of areas receiving overflow, for evidence of erosion.	

4. FOUL WATER DRAINAGE STRATEGY

- 4.1 This section of the report outlines an approach for the disposal and management of foul water runoff from the development.
- 4.2 A pre-planning enquiry was sent to Wessex Water to confirm that the existing public sewer network can accommodate foul water flows from the development. A definitive response has been received from Wessex Water confirming there is sufficient sewerage capacity, and their response can be found in *Appendix D*.
- 4.3 A foul water drainage scheme has been developed for the site on this basis and is shown on the drainage strategy plan included in *Appendix B*. The drawing shows foul water runoff from the development to be discharged into the existing package foul water pumping station serving the existing annex, where it is pumped back into the main hotels foul water drainage network before discharging into the existing Wessex Water foul water sewer located to the west via a gravity connection.
- 4.4 To accommodate the additional bedrooms from the proposed extension a 2.1mØ by 1.02m deep chamber is proposed to be installed upstream of the pump station which has been sized to ensure the system continues to provide 24-hour emergency storage in the event of a pump failure.

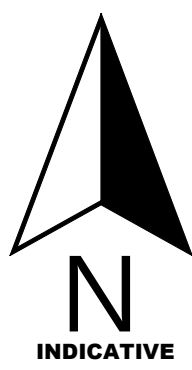
JOB NO:	P24-0132	ISSUE NO:	1	ISSUE DATE:	13/06/2024	Page 10
AUTHOR:	JJH	OFFICE:	HENLEY	CHECKED BY:	PB	

5. SUMMARY & CONCLUSIONS

- 5.1 It has been established that the site is located in Flood Zone 1, which is land assessed to not be at risk of fluvial flooding. In addition, the site has been established to be at low risk of flooding from all other sources.
- 5.1 It has been established to be appropriate to discharge surface water runoff from the development to the Wessex Water surface water sewer via the sites existing private drainage network. A surface water drainage scheme has been developed, which demonstrates that the existing private surface water package pumping station would be retained to pump surface water runoff from the existing annex, proposed extension and associated parking areas with additional storage added to the existing attenuation tank to accommodate the increase in impermeable area. Furthermore, the upgrades to the existing attenuation tank brings the existing system in line with current design standards and therefore providing a betterment to the existing drainage system.
- 5.1 It has been established to be appropriate to discharge foul water runoff to the existing Wessex Water foul sewer via the sites existing private drainage network. A foul drainage scheme has been developed, which demonstrates that the existing private foul water package pumping station would be retained to pump foul water flows from the existing annex and proposed extension with additional emergency storage added to accommodate the proposed extension.
- 5.2 In terms of flood risk and drainage strategy, it is concluded that the development can be occupied and operated safely and that there will be no increase in the level of flood risk to the site or neighbouring sites thus fulfilling the aims of national and local planning policy.

JOB NO:	P24-0132	ISSUE NO:	1	ISSUE DATE:	13/06/2024	Page 11
AUTHOR:	JJH	OFFICE:	HENLEY	CHECKED BY:	PB	

APPENDIX A
TOPOGRAPHIC SURVEY



No.	REVISION NOTE	DATE	SURVEYED	DRAWN	CHECKED	APPROVED

A range of detection methods are used to obtain the underground information produced in this drawing. Variations in the ground conditions can affect the quality of the data and therefore an absolute affirmation cannot be guaranteed.

The identification of any located service should not be treated as infallible. The originating source may not have been traced.

The location and/or identification of a service will not necessarily indicate whether it is live or dead or identify whether it is a single or multiple service where one or more services may be situated above, below or in close proximity of the indicated line. Where possible, this will be denoted as 'trapped'.

All depths of metallic features are to centre of the service, depths on drainage runs are to the pipe invert at the manhole and should be treated as an approximate indication.

Underground drainage has been proven for position using traceable sonde method, unless otherwise stated. No allowance has been made for sub surface entry into manholes or other chambers or voids below ground level. Therefore any details relating to depths, sizes etc. are taken from above ground and as such will be approximate only.

Features not traceable by electromagnetic or sonde techniques, such as polyethylene water/gas pipes have been located using Ground Penetrating Radar (GPR). Successful GPR detection is based on a number of factors, clear access, ground/physical conditions and size/type of physical target. All GPR work, unless verified (Q.A.) is interpretive. Trial holes should be carried out to confirm service identification, position and particularly depths, where these are critical, in accordance with HSG47 - "Avoiding Danger from Underground Services".

Premier Surveys use skilled professional surveyors and modern, calibrated location equipment. However, the completeness of any underground utility survey should not be 100% guaranteed. We cannot be held liable for inaccuracies beyond those that could be reasonably expected of a professionally skilled company. Any information taken from these drawings, site locations etc. cannot be 100% guaranteed.

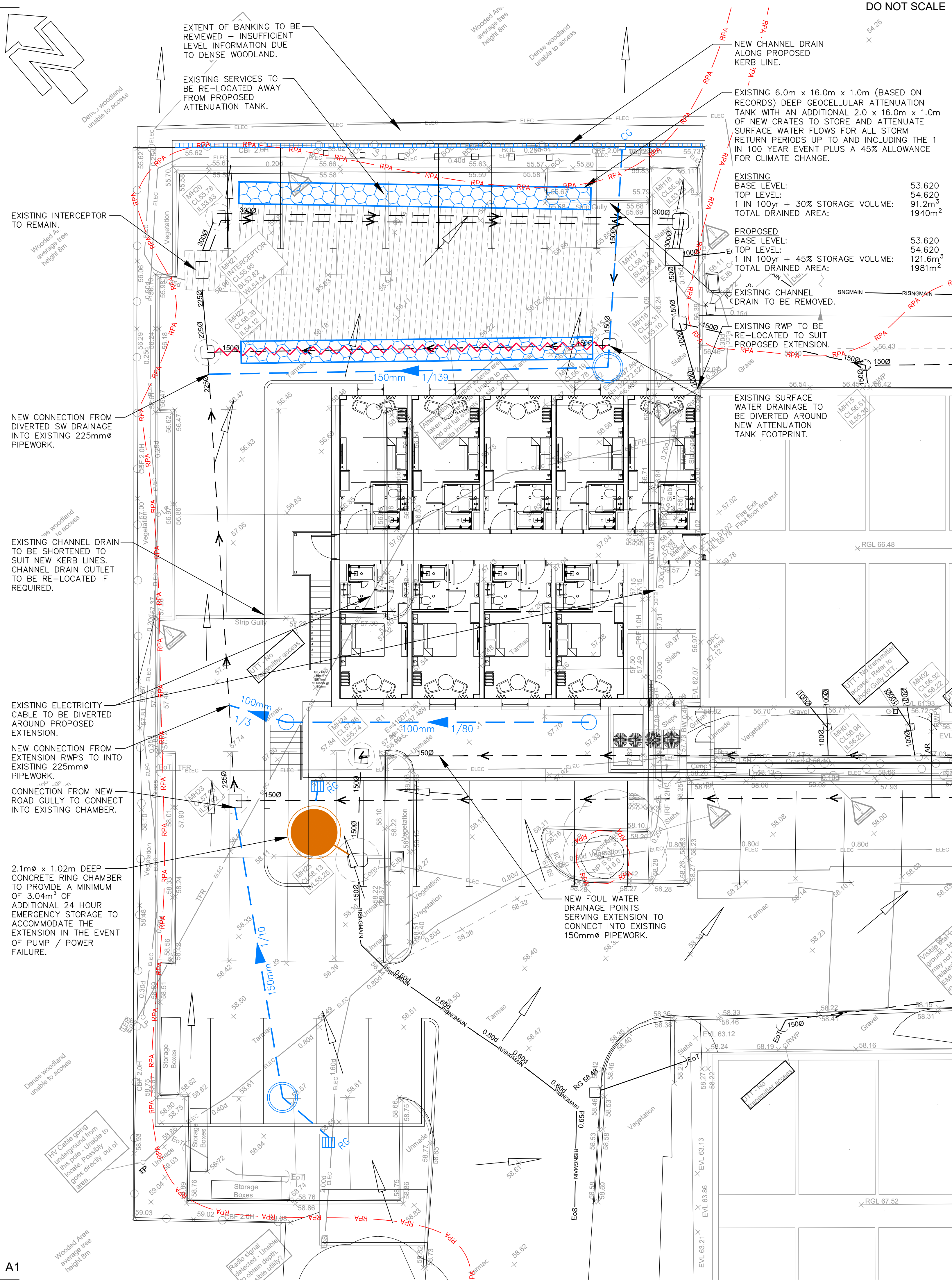
These results are not finalities and trial holes should be carried out to confirm service identification, position and particularly depths, where these are critical, in accordance with HSG47 - "Avoiding Danger from Underground Services".

This drawing must be read in conjunction with all available statutory undertakers record information. Record information is often incomplete and should be used for reference only and combined with detailed survey work/verification.

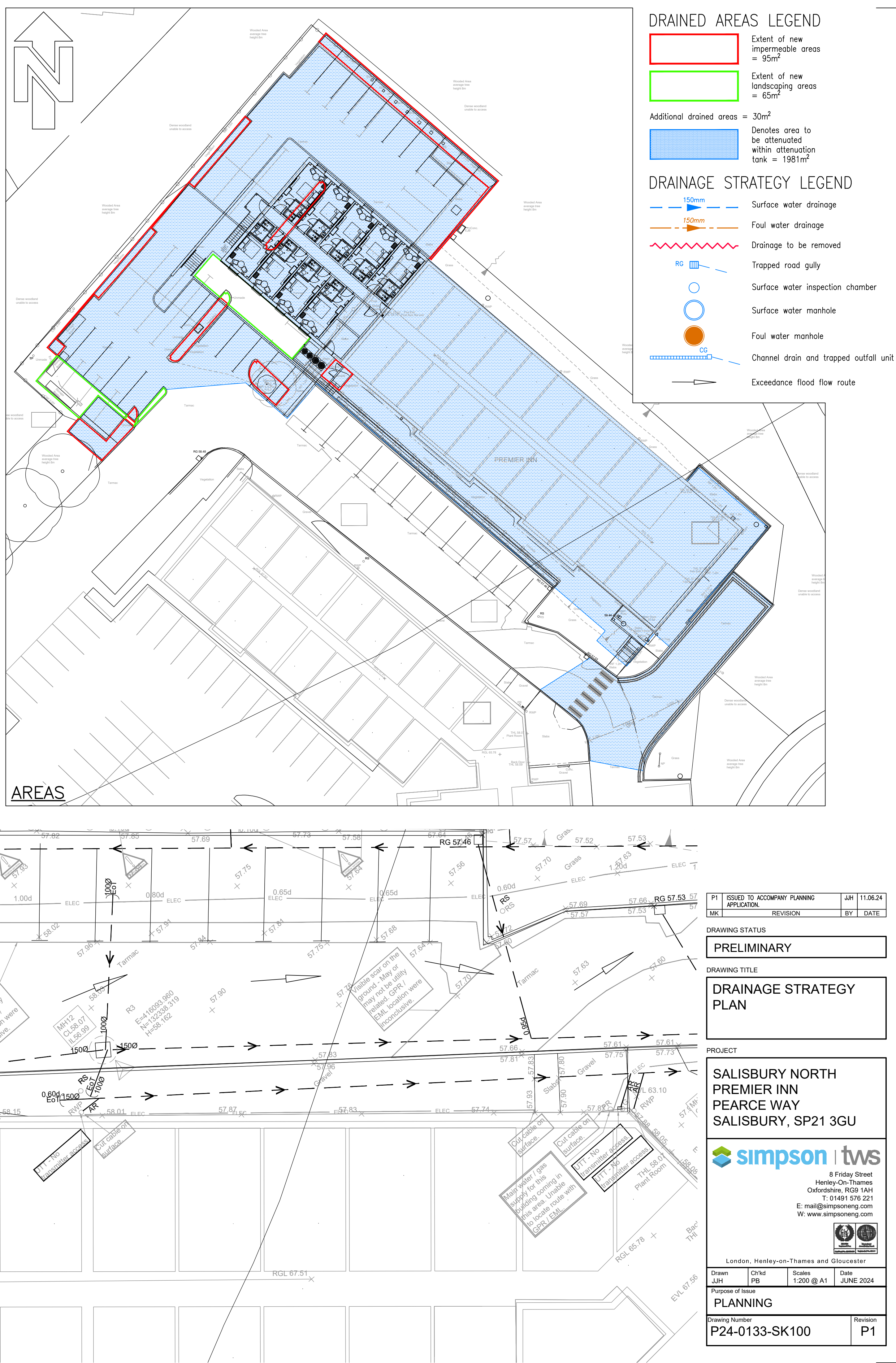
LEGEND

Red line	Power Carrying Feature	End of Trench Scar
Blue line	Water Main	End of Trace
Orange line	Gas Main	End of Survey
Green line	Sewer Four Water	Depth To Service (Dependent on All Types)
Yellow line	Sewer Rising Man	Taken From Records
Purple line	Sewer Surface Water	Untraceable Areas (Obstructions)
Light blue line	Sewer Surface Water	Untraceable Areas Due To Vegetation
Dark blue line	Sewer Four Water	
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APPENDIX B
DRAINAGE STRATEGY PLAN



DO NOT SCALE



DRAINED AREAS LEGEND

- Extent of new impermeable areas = 95m²
- Extent of new landscaping areas = 65m²
- Additional drained areas = 30m²
- Denotes area to be attenuated within attenuation tank = 1981m²

DRAINAGE STRATEGY LEGEND

- 150mm Surface water drainage
- 150mm Foul water drainage
- Drainage to be removed
- RG Trapped road gully
- Surface water inspection chamber
- Surface water manhole
- Foul water manhole
- CG Channel drain and trapped outfall unit
- Exceedance flood flow route

P1	ISSUED TO ACCOMPANY PLANNING APPLICATION	JWH	11.06.24
MK	REVISION	BY	DATE

DRAWING STATUS: **PRELIMINARY**

DRAWING TITLE: **DRAINAGE STRATEGY PLAN**

PROJECT: **SALISBURY NORTH PREMIER INN PEARCE WAY SALISBURY, SP21 3GU**

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Drawn	Chkd	Scales	Date
JWH	PS	1:200 @ A1	JUNE 2024

Purpose of Issue: **PLANNING**

Drawing Number	Revision
P24-0133-SK100	P1

APPENDIX C
MICRODRAINAGE ATTENUATION TANK DESIGN RESULTS

Simpson Associates		Page 1
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:45 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		Source Control 2019.1

Summary of Results for 2 year Return Period

Half Drain Time : 75 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	53.730	0.110	0.0	2.6	2.6	13.4	O K
30 min Summer	53.752	0.132	0.0	2.6	2.6	16.1	O K
60 min Summer	53.761	0.141	0.0	2.6	2.6	17.1	O K
120 min Summer	53.782	0.162	0.0	2.6	2.6	19.7	O K
180 min Summer	53.788	0.168	0.0	2.6	2.6	20.4	O K
240 min Summer	53.787	0.167	0.0	2.6	2.6	20.3	O K
360 min Summer	53.774	0.154	0.0	2.6	2.6	18.8	O K
480 min Summer	53.757	0.137	0.0	2.6	2.6	16.6	O K
600 min Summer	53.738	0.118	0.0	2.6	2.6	14.4	O K
720 min Summer	53.720	0.100	0.0	2.6	2.6	12.2	O K
960 min Summer	53.688	0.068	0.0	2.6	2.6	8.3	O K
1440 min Summer	53.644	0.024	0.0	2.6	2.6	2.9	O K
2160 min Summer	53.620	0.000	0.0	2.5	2.5	0.0	O K
2880 min Summer	53.620	0.000	0.0	2.1	2.1	0.0	O K
4320 min Summer	53.620	0.000	0.0	1.6	1.6	0.0	O K
5760 min Summer	53.620	0.000	0.0	1.3	1.3	0.0	O K
7200 min Summer	53.620	0.000	0.0	1.2	1.2	0.0	O K
8640 min Summer	53.620	0.000	0.0	1.1	1.1	0.0	O K
10080 min Summer	53.620	0.000	0.0	1.0	1.0	0.0	O K
15 min Winter	53.731	0.111	0.0	2.6	2.6	13.5	O K
30 min Winter	53.753	0.133	0.0	2.6	2.6	16.2	O K
60 min Winter	53.763	0.143	0.0	2.6	2.6	17.4	O K
120 min Winter	53.782	0.162	0.0	2.6	2.6	19.6	O K
180 min Winter	53.784	0.164	0.0	2.6	2.6	19.9	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15 min Summer	34.440	0.0	16.0	17
30 min Summer	22.392	0.0	20.9	31
60 min Summer	14.115	0.0	26.3	58
120 min Summer	9.703	0.0	36.2	94
180 min Summer	7.598	0.0	42.5	128
240 min Summer	6.313	0.0	47.4	164
360 min Summer	4.775	0.0	53.5	232
480 min Summer	3.875	0.0	58.0	300
600 min Summer	3.282	0.0	61.3	364
720 min Summer	2.859	0.0	64.2	428
960 min Summer	2.295	0.0	68.7	550
1440 min Summer	1.685	0.0	75.6	778
2160 min Summer	1.250	0.0	84.2	0
2880 min Summer	1.021	0.0	91.7	0
4320 min Summer	0.782	0.0	105.4	0
5760 min Summer	0.657	0.0	118.1	0
7200 min Summer	0.581	0.0	130.5	0
8640 min Summer	0.529	0.0	142.6	0
10080 min Summer	0.491	0.0	154.5	0
15 min Winter	34.440	0.0	16.0	17
30 min Winter	22.392	0.0	20.9	31
60 min Winter	14.115	0.0	26.5	58
120 min Winter	9.703	0.0	36.3	96
180 min Winter	7.598	0.0	42.7	136

Simpson Associates		Page 2
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:45 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage Source Control 2019.1		

Summary of Results for 2 year Return Period

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
240 min Winter	53.777	0.157	0.0	2.6	2.6	19.1	O K
360 min Winter	53.754	0.134	0.0	2.6	2.6	16.2	O K
480 min Winter	53.725	0.105	0.0	2.6	2.6	12.8	O K
600 min Winter	53.698	0.078	0.0	2.6	2.6	9.5	O K
720 min Winter	53.673	0.053	0.0	2.6	2.6	6.5	O K
960 min Winter	53.635	0.015	0.0	2.6	2.6	1.8	O K
1440 min Winter	53.620	0.000	0.0	2.2	2.2	0.0	O K
2160 min Winter	53.620	0.000	0.0	1.6	1.6	0.0	O K
2880 min Winter	53.620	0.000	0.0	1.3	1.3	0.0	O K
4320 min Winter	53.620	0.000	0.0	1.0	1.0	0.0	O K
5760 min Winter	53.620	0.000	0.0	0.9	0.9	0.0	O K
7200 min Winter	53.620	0.000	0.0	0.8	0.8	0.0	O K
8640 min Winter	53.620	0.000	0.0	0.7	0.7	0.0	O K
10080 min Winter	53.620	0.000	0.0	0.6	0.6	0.0	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
240 min Winter	6.313	0.0	47.3	174
360 min Winter	4.775	0.0	53.5	248
480 min Winter	3.875	0.0	57.9	316
600 min Winter	3.282	0.0	61.4	380
720 min Winter	2.859	0.0	64.2	440
960 min Winter	2.295	0.0	68.7	540
1440 min Winter	1.685	0.0	75.7	0
2160 min Winter	1.250	0.0	84.2	0
2880 min Winter	1.021	0.0	91.7	0
4320 min Winter	0.782	0.0	105.4	0
5760 min Winter	0.657	0.0	118.1	0
7200 min Winter	0.581	0.0	130.5	0
8640 min Winter	0.529	0.0	142.6	0
10080 min Winter	0.491	0.0	154.5	0

Simpson Associates		Page 3
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:45 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		
Source Control 2019.1		

Rainfall Details

Rainfall Model	FEH	Winter Storms	Yes
Return Period (years)	2	Cv (Summer)	0.950
FEH Rainfall Version	2013	Cv (Winter)	0.950
Site Location	GB 416091 132356 SU 16091 32356	Shortest Storm (mins)	15
Data Type		Point Longest Storm (mins)	10080
Summer Storms	Yes	Climate Change %	+0

Time Area Diagram

Total Area (ha) 0.197

Time (mins)		Area
From:	To:	(ha)
0	4	0.197

Time Area Diagram

Total Area (ha) 0.000

Time (mins)		Area
From:	To:	(ha)
0	4	0.000

Model Details

Storage is Online Cover Level (m) 54.620

Cellular Storage Structure

Invert Level (m) 53.620 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)
0.000	128.0	128.0	1.000	128.0	176.0	1.001	0.0	176.0

Depth/Flow Relationship Outflow Control

Invert Level (m) 53.060

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.200	2.6000	1.400	2.6000	2.600	2.6000	3.800	2.6000	5.000	2.6000
0.400	2.6000	1.600	2.6000	2.800	2.6000	4.000	2.6000	5.200	2.6000
0.600	2.6000	1.800	2.6000	3.000	2.6000	4.200	2.6000	5.400	2.6000
0.800	2.6000	2.000	2.6000	3.200	2.6000	4.400	2.6000	5.600	2.6000
1.000	2.6000	2.200	2.6000	3.400	2.6000	4.600	2.6000	5.800	2.6000
1.200	2.6000	2.400	2.6000	3.600	2.6000	4.800	2.6000	6.000	2.6000

Simpson Associates		Page 1
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:44 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		Source Control 2019.1

Summary of Results for 30 year Return Period

Half Drain Time : 192 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	53.887	0.267	0.0	2.6	2.6	32.4	O K
30 min Summer	53.962	0.342	0.0	2.6	2.6	41.6	O K
60 min Summer	54.028	0.408	0.0	2.6	2.6	49.6	O K
120 min Summer	54.062	0.442	0.0	2.6	2.6	53.8	O K
180 min Summer	54.058	0.438	0.0	2.6	2.6	53.3	O K
240 min Summer	54.047	0.427	0.0	2.6	2.6	51.9	O K
360 min Summer	54.023	0.403	0.0	2.6	2.6	49.0	O K
480 min Summer	53.997	0.377	0.0	2.6	2.6	45.8	O K
600 min Summer	53.971	0.351	0.0	2.6	2.6	42.6	O K
720 min Summer	53.945	0.325	0.0	2.6	2.6	39.5	O K
960 min Summer	53.895	0.275	0.0	2.6	2.6	33.4	O K
1440 min Summer	53.807	0.187	0.0	2.6	2.6	22.8	O K
2160 min Summer	53.710	0.090	0.0	2.6	2.6	11.0	O K
2880 min Summer	53.653	0.033	0.0	2.6	2.6	4.0	O K
4320 min Summer	53.620	0.000	0.0	2.4	2.4	0.0	O K
5760 min Summer	53.620	0.000	0.0	2.0	2.0	0.0	O K
7200 min Summer	53.620	0.000	0.0	1.7	1.7	0.0	O K
8640 min Summer	53.620	0.000	0.0	1.5	1.5	0.0	O K
10080 min Summer	53.620	0.000	0.0	1.4	1.4	0.0	O K
15 min Winter	53.887	0.267	0.0	2.6	2.6	32.5	O K
30 min Winter	53.963	0.343	0.0	2.6	2.6	41.7	O K
60 min Winter	54.030	0.410	0.0	2.6	2.6	49.8	O K
120 min Winter	54.068	0.448	0.0	2.6	2.6	54.5	O K
180 min Winter	54.067	0.447	0.0	2.6	2.6	54.4	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15 min Summer	75.366	0.0	35.3	18
30 min Summer	49.868	0.0	46.6	33
60 min Summer	31.679	0.0	59.3	62
120 min Summer	19.414	0.0	72.6	120
180 min Summer	14.475	0.0	81.2	172
240 min Summer	11.689	0.0	87.5	200
360 min Summer	8.556	0.0	96.1	262
480 min Summer	6.811	0.0	101.9	330
600 min Summer	5.693	0.0	106.4	398
720 min Summer	4.910	0.0	110.1	464
960 min Summer	3.883	0.0	116.2	598
1440 min Summer	2.786	0.0	125.1	852
2160 min Summer	2.006	0.0	135.0	1208
2880 min Summer	1.599	0.0	143.7	1528
4320 min Summer	1.185	0.0	159.6	0
5760 min Summer	0.970	0.0	174.4	0
7200 min Summer	0.839	0.0	188.4	0
8640 min Summer	0.750	0.0	202.1	0
10080 min Summer	0.685	0.0	215.5	0
15 min Winter	75.366	0.0	35.2	18
30 min Winter	49.868	0.0	46.5	32
60 min Winter	31.679	0.0	59.1	60
120 min Winter	19.414	0.0	72.6	118
180 min Winter	14.475	0.0	81.3	172

Simpson Associates		Page 2
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:44 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage Source Control 2019.1		

Summary of Results for 30 year Return Period

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
240 min Winter	54.050	0.430	0.0	2.6	2.6	52.3	O K
360 min Winter	54.018	0.398	0.0	2.6	2.6	48.4	O K
480 min Winter	53.982	0.362	0.0	2.6	2.6	44.0	O K
600 min Winter	53.943	0.323	0.0	2.6	2.6	39.3	O K
720 min Winter	53.905	0.285	0.0	2.6	2.6	34.6	O K
960 min Winter	53.832	0.212	0.0	2.6	2.6	25.8	O K
1440 min Winter	53.714	0.094	0.0	2.6	2.6	11.4	O K
2160 min Winter	53.621	0.001	0.0	2.6	2.6	0.1	O K
2880 min Winter	53.620	0.000	0.0	2.1	2.1	0.0	O K
4320 min Winter	53.620	0.000	0.0	1.6	1.6	0.0	O K
5760 min Winter	53.620	0.000	0.0	1.3	1.3	0.0	O K
7200 min Winter	53.620	0.000	0.0	1.1	1.1	0.0	O K
8640 min Winter	53.620	0.000	0.0	1.0	1.0	0.0	O K
10080 min Winter	53.620	0.000	0.0	0.9	0.9	0.0	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
240 min Winter	11.689	0.0	87.3	222
360 min Winter	8.556	0.0	96.1	276
480 min Winter	6.811	0.0	101.9	352
600 min Winter	5.693	0.0	106.5	426
720 min Winter	4.910	0.0	110.1	498
960 min Winter	3.883	0.0	116.3	632
1440 min Winter	2.786	0.0	125.1	868
2160 min Winter	2.006	0.0	135.2	1120
2880 min Winter	1.599	0.0	143.7	0
4320 min Winter	1.185	0.0	159.6	0
5760 min Winter	0.970	0.0	174.4	0
7200 min Winter	0.839	0.0	188.4	0
8640 min Winter	0.750	0.0	202.1	0
10080 min Winter	0.685	0.0	215.5	0

Simpson Associates		Page 3
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:44 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		
Source Control 2019.1		

Rainfall Details

Rainfall Model	FEH	Winter Storms	Yes
Return Period (years)	30	Cv (Summer)	0.950
FEH Rainfall Version	2013	Cv (Winter)	0.950
Site Location	GB 416091 132356 SU 16091 32356	Shortest Storm (mins)	15
Data Type		Point Longest Storm (mins)	10080
Summer Storms	Yes	Climate Change %	+0

Time Area Diagram

Total Area (ha) 0.197

Time (mins) Area
From: To: (ha)

0 4 0.197

Time Area Diagram

Total Area (ha) 0.000

Time (mins) Area
From: To: (ha)

0 4 0.000

Model Details

Storage is Online Cover Level (m) 54.620

Cellular Storage Structure

Invert Level (m) 53.620 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)
0.000	128.0	128.0	1.000	128.0	176.0	1.001	0.0	176.0

Depth/Flow Relationship Outflow Control

Invert Level (m) 53.060

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.200	2.6000	1.400	2.6000	2.600	2.6000	3.800	2.6000	5.000	2.6000
0.400	2.6000	1.600	2.6000	2.800	2.6000	4.000	2.6000	5.200	2.6000
0.600	2.6000	1.800	2.6000	3.000	2.6000	4.200	2.6000	5.400	2.6000
0.800	2.6000	2.000	2.6000	3.200	2.6000	4.400	2.6000	5.600	2.6000
1.000	2.6000	2.200	2.6000	3.400	2.6000	4.600	2.6000	5.800	2.6000
1.200	2.6000	2.400	2.6000	3.600	2.6000	4.800	2.6000	6.000	2.6000

Simpson Associates		Page 1
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:43 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage	Source Control 2019.1	

Summary of Results for 100 year Return Period

Half Drain Time : 253 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	53.963	0.343	0.0	2.6	2.6	41.7	O K
30 min Summer	54.068	0.448	0.0	2.6	2.6	54.5	O K
60 min Summer	54.167	0.547	0.0	2.6	2.6	66.5	O K
120 min Summer	54.212	0.592	0.0	2.6	2.6	72.0	O K
180 min Summer	54.219	0.599	0.0	2.6	2.6	72.8	O K
240 min Summer	54.205	0.585	0.0	2.6	2.6	71.2	O K
360 min Summer	54.174	0.554	0.0	2.6	2.6	67.3	O K
480 min Summer	54.147	0.527	0.0	2.6	2.6	64.1	O K
600 min Summer	54.120	0.500	0.0	2.6	2.6	60.8	O K
720 min Summer	54.091	0.471	0.0	2.6	2.6	57.3	O K
960 min Summer	54.031	0.411	0.0	2.6	2.6	50.0	O K
1440 min Summer	53.931	0.311	0.0	2.6	2.6	37.8	O K
2160 min Summer	53.809	0.189	0.0	2.6	2.6	23.0	O K
2880 min Summer	53.722	0.102	0.0	2.6	2.6	12.4	O K
4320 min Summer	53.632	0.012	0.0	2.6	2.6	1.4	O K
5760 min Summer	53.620	0.000	0.0	2.3	2.3	0.0	O K
7200 min Summer	53.620	0.000	0.0	2.0	2.0	0.0	O K
8640 min Summer	53.620	0.000	0.0	1.7	1.7	0.0	O K
10080 min Summer	53.620	0.000	0.0	1.6	1.6	0.0	O K
15 min Winter	53.964	0.344	0.0	2.6	2.6	41.8	O K
30 min Winter	54.069	0.449	0.0	2.6	2.6	54.6	O K
60 min Winter	54.170	0.550	0.0	2.6	2.6	66.8	O K
120 min Winter	54.216	0.596	0.0	2.6	2.6	72.5	O K
180 min Winter	54.227	0.607	0.0	2.6	2.6	73.8	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15 min Summer	95.343	0.0	44.5	18
30 min Summer	63.701	0.0	59.6	33
60 min Summer	40.767	0.0	76.1	62
120 min Summer	24.289	0.0	90.7	122
180 min Summer	17.985	0.0	101.0	180
240 min Summer	14.492	0.0	108.4	230
360 min Summer	10.588	0.0	118.9	286
480 min Summer	8.452	0.0	126.5	348
600 min Summer	7.069	0.0	132.3	416
720 min Summer	6.090	0.0	136.6	484
960 min Summer	4.771	0.0	142.7	616
1440 min Summer	3.409	0.0	153.2	880
2160 min Summer	2.442	0.0	164.6	1252
2880 min Summer	1.933	0.0	173.5	1588
4320 min Summer	1.404	0.0	189.3	2244
5760 min Summer	1.131	0.0	203.2	0
7200 min Summer	0.966	0.0	216.9	0
8640 min Summer	0.855	0.0	230.4	0
10080 min Summer	0.776	0.0	243.9	0
15 min Winter	95.343	0.0	44.5	18
30 min Winter	63.701	0.0	59.5	32
60 min Winter	40.767	0.0	76.3	62
120 min Winter	24.289	0.0	90.7	118
180 min Winter	17.985	0.0	100.8	176

Simpson Associates		Page 2
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:43 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage Source Control 2019.1		

Summary of Results for 100 year Return Period

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max E Outflow (l/s)	Max Volume (m³)	Status
240 min Winter	54.218	0.598	0.0	2.6	2.6	72.7	O K
360 min Winter	54.174	0.554	0.0	2.6	2.6	67.4	O K
480 min Winter	54.141	0.521	0.0	2.6	2.6	63.3	O K
600 min Winter	54.102	0.482	0.0	2.6	2.6	58.7	O K
720 min Winter	54.062	0.442	0.0	2.6	2.6	53.7	O K
960 min Winter	53.976	0.356	0.0	2.6	2.6	43.3	O K
1440 min Winter	53.834	0.214	0.0	2.6	2.6	26.0	O K
2160 min Winter	53.681	0.061	0.0	2.6	2.6	7.4	O K
2880 min Winter	53.620	0.000	0.0	2.5	2.5	0.0	O K
4320 min Winter	53.620	0.000	0.0	1.8	1.8	0.0	O K
5760 min Winter	53.620	0.000	0.0	1.5	1.5	0.0	O K
7200 min Winter	53.620	0.000	0.0	1.3	1.3	0.0	O K
8640 min Winter	53.620	0.000	0.0	1.1	1.1	0.0	O K
10080 min Winter	53.620	0.000	0.0	1.0	1.0	0.0	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
240 min Winter	14.492	0.0	108.5	230
360 min Winter	10.588	0.0	118.9	292
480 min Winter	8.452	0.0	126.6	366
600 min Winter	7.069	0.0	132.1	442
720 min Winter	6.090	0.0	136.8	516
960 min Winter	4.771	0.0	142.9	656
1440 min Winter	3.409	0.0	153.1	922
2160 min Winter	2.442	0.0	164.6	1252
2880 min Winter	1.933	0.0	173.7	0
4320 min Winter	1.404	0.0	189.3	0
5760 min Winter	1.131	0.0	203.2	0
7200 min Winter	0.966	0.0	216.9	0
8640 min Winter	0.855	0.0	230.4	0
10080 min Winter	0.776	0.0	243.9	0

Simpson Associates		Page 3
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:43 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		
Source Control 2019.1		

Rainfall Details

Rainfall Model	FEH	Winter Storms	Yes
Return Period (years)	100	Cv (Summer)	0.950
FEH Rainfall Version	2013	Cv (Winter)	0.950
Site Location	GB 416091 132356 SU 16091 32356	Shortest Storm (mins)	15
Data Type		Point Longest Storm (mins)	10080
Summer Storms	Yes	Climate Change %	+0

Time Area Diagram

Total Area (ha) 0.197

Time (mins)		Area
From:	To:	(ha)
0	4	0.197

Time Area Diagram

Total Area (ha) 0.000

Time (mins)		Area
From:	To:	(ha)
0	4	0.000

Model Details

Storage is Online Cover Level (m) 54.620

Cellular Storage Structure

Invert Level (m) 53.620 Safety Factor 2.0
Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)
0.000	128.0	128.0	1.000	128.0	176.0	1.001	0.0	176.0

Depth/Flow Relationship Outflow Control

Invert Level (m) 53.060

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.200	2.6000	1.400	2.6000	2.600	2.6000	3.800	2.6000	5.000	2.6000
0.400	2.6000	1.600	2.6000	2.800	2.6000	4.000	2.6000	5.200	2.6000
0.600	2.6000	1.800	2.6000	3.000	2.6000	4.200	2.6000	5.400	2.6000
0.800	2.6000	2.000	2.6000	3.200	2.6000	4.400	2.6000	5.600	2.6000
1.000	2.6000	2.200	2.6000	3.400	2.6000	4.600	2.6000	5.800	2.6000
1.200	2.6000	2.400	2.6000	3.600	2.6000	4.800	2.6000	6.000	2.6000

Simpson Associates		Page 1
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:41 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		Source Control 2019.1

Summary of Results for 100 year Return Period (+45%)

Half Drain Time : 387 minutes.

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
15 min Summer	54.128	0.508	0.0	2.6	2.6	61.8	O K
30 min Summer	54.288	0.668	0.0	2.6	2.6	81.2	O K
60 min Summer	54.448	0.828	0.0	2.6	2.6	100.7	Flood Risk
120 min Summer	54.547	0.927	0.0	2.6	2.6	112.7	Flood Risk
180 min Summer	54.591	0.971	0.0	2.6	2.6	118.1	Flood Risk
240 min Summer	54.604	0.984	0.0	2.6	2.6	119.7	Flood Risk
360 min Summer	54.576	0.956	0.0	2.6	2.6	116.3	Flood Risk
480 min Summer	54.536	0.916	0.0	2.6	2.6	111.4	Flood Risk
600 min Summer	54.499	0.879	0.0	2.6	2.6	106.9	Flood Risk
720 min Summer	54.464	0.844	0.0	2.6	2.6	102.7	Flood Risk
960 min Summer	54.393	0.773	0.0	2.6	2.6	94.0	Flood Risk
1440 min Summer	54.278	0.658	0.0	2.6	2.6	80.0	O K
2160 min Summer	54.128	0.508	0.0	2.6	2.6	61.7	O K
2880 min Summer	54.001	0.381	0.0	2.6	2.6	46.3	O K
4320 min Summer	53.813	0.193	0.0	2.6	2.6	23.5	O K
5760 min Summer	53.699	0.079	0.0	2.6	2.6	9.6	O K
7200 min Summer	53.639	0.019	0.0	2.6	2.6	2.3	O K
8640 min Summer	53.620	0.000	0.0	2.5	2.5	0.0	O K
10080 min Summer	53.620	0.000	0.0	2.3	2.3	0.0	O K
15 min Winter	54.128	0.508	0.0	2.6	2.6	61.8	O K
30 min Winter	54.289	0.669	0.0	2.6	2.6	81.4	O K
60 min Winter	54.451	0.831	0.0	2.6	2.6	101.0	Flood Risk
120 min Winter	54.552	0.932	0.0	2.6	2.6	113.3	Flood Risk
180 min Winter	54.598	0.978	0.0	2.6	2.6	118.9	Flood Risk

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
15 min Summer	138.247	0.0	64.6	19
30 min Summer	92.366	0.0	86.3	33
60 min Summer	59.112	0.0	110.4	62
120 min Summer	35.218	0.0	131.7	122
180 min Summer	26.078	0.0	146.3	182
240 min Summer	21.014	0.0	157.1	242
360 min Summer	15.353	0.0	172.4	358
480 min Summer	12.256	0.0	183.5	408
600 min Summer	10.250	0.0	191.7	468
720 min Summer	8.830	0.0	198.3	528
960 min Summer	6.918	0.0	207.1	658
1440 min Summer	4.944	0.0	221.9	926
2160 min Summer	3.541	0.0	238.4	1320
2880 min Summer	2.803	0.0	251.8	1700
4320 min Summer	2.036	0.0	274.3	2420
5760 min Summer	1.640	0.0	294.4	3064
7200 min Summer	1.401	0.0	314.6	3744
8640 min Summer	1.240	0.0	334.2	0
10080 min Summer	1.125	0.0	353.6	0
15 min Winter	138.247	0.0	64.6	18
30 min Winter	92.366	0.0	86.4	33
60 min Winter	59.112	0.0	110.7	62
120 min Winter	35.218	0.0	131.7	120
180 min Winter	26.078	0.0	146.5	178

Simpson Associates		Page 2
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:41 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage Source Control 2019.1		

Summary of Results for 100 year Return Period (+45%)

Storm Event	Max Level (m)	Max Depth (m)	Max Infiltration (l/s)	Max Control (l/s)	Max Σ Outflow (l/s)	Max Volume (m³)	Status
240 min Winter	54.614	0.994	0.0	2.6	2.6	120.9	Flood Risk
360 min Winter	54.595	0.975	0.0	2.6	2.6	118.5	Flood Risk
480 min Winter	54.548	0.928	0.0	2.6	2.6	112.8	Flood Risk
600 min Winter	54.499	0.879	0.0	2.6	2.6	106.9	Flood Risk
720 min Winter	54.457	0.837	0.0	2.6	2.6	101.8	Flood Risk
960 min Winter	54.363	0.743	0.0	2.6	2.6	90.4	Flood Risk
1440 min Winter	54.198	0.578	0.0	2.6	2.6	70.3	O K
2160 min Winter	53.983	0.363	0.0	2.6	2.6	44.1	O K
2880 min Winter	53.813	0.193	0.0	2.6	2.6	23.5	O K
4320 min Winter	53.625	0.005	0.0	2.6	2.6	0.6	O K
5760 min Winter	53.620	0.000	0.0	2.2	2.2	0.0	O K
7200 min Winter	53.620	0.000	0.0	1.8	1.8	0.0	O K
8640 min Winter	53.620	0.000	0.0	1.6	1.6	0.0	O K
10080 min Winter	53.620	0.000	0.0	1.5	1.5	0.0	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m³)	Discharge Volume (m³)	Time-Peak (mins)
240 min Winter	21.014	0.0	157.2	234
360 min Winter	15.353	0.0	172.5	346
480 min Winter	12.256	0.0	183.4	448
600 min Winter	10.250	0.0	191.8	480
720 min Winter	8.830	0.0	198.2	554
960 min Winter	6.918	0.0	207.0	702
1440 min Winter	4.944	0.0	222.1	994
2160 min Winter	3.541	0.0	238.3	1388
2880 min Winter	2.803	0.0	251.8	1732
4320 min Winter	2.036	0.0	274.4	2252
5760 min Winter	1.640	0.0	294.6	0
7200 min Winter	1.401	0.0	314.5	0
8640 min Winter	1.240	0.0	334.2	0
10080 min Winter	1.125	0.0	353.6	0

Simpson Associates		Page 3
1 Market Place Mews Henley-on-Thames RG9 2AH	Premier Inn Extension Salisbury (1 in 100 yr + 45%, 2.6 l/s)	
Date 12/06/2024 11:41 File P24-0132 - PROPOSED.SRCX	Designed by JJH Checked by PB	
Micro Drainage		
Source Control 2019.1		

Rainfall Details

Rainfall Model	FEH	Winter Storms	Yes
Return Period (years)	100	Cv (Summer)	0.950
FEH Rainfall Version	2013	Cv (Winter)	0.950
Site Location	GB 416091 132356 SU 16091 32356	Shortest Storm (mins)	15
Data Type		Point Longest Storm (mins)	10080
Summer Storms	Yes	Climate Change %	+45

Time Area Diagram

Total Area (ha) 0.197

Time (mins)		Area
From:	To:	(ha)
0	4	0.197

Time Area Diagram

Total Area (ha) 0.000

Time (mins)		Area
From:	To:	(ha)
0	4	0.000

Model Details

Storage is Online Cover Level (m) 54.620

Cellular Storage Structure

Invert Level (m) 53.620 Safety Factor 2.0
 Infiltration Coefficient Base (m/hr) 0.00000 Porosity 0.95
 Infiltration Coefficient Side (m/hr) 0.00000

Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)	Depth (m)	Area (m²)	Inf. Area (m²)
0.000	128.0	128.0	1.000	128.0	176.0	1.001	0.0	176.0

Depth/Flow Relationship Outflow Control

Invert Level (m) 53.060

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.200	2.6000	1.400	2.6000	2.600	2.6000	3.800	2.6000	5.000	2.6000
0.400	2.6000	1.600	2.6000	2.800	2.6000	4.000	2.6000	5.200	2.6000
0.600	2.6000	1.800	2.6000	3.000	2.6000	4.200	2.6000	5.400	2.6000
0.800	2.6000	2.000	2.6000	3.200	2.6000	4.400	2.6000	5.600	2.6000
1.000	2.6000	2.200	2.6000	3.400	2.6000	4.600	2.6000	5.800	2.6000
1.200	2.6000	2.400	2.6000	3.600	2.6000	4.800	2.6000	6.000	2.6000

APPENDIX D
WESSEX WATER PRE-DEV ENQUIRY CORRESPONDANCE

Response to: [REDACTED] (Developer Response)			
Planning Ref:	N/A	Email:	Planning.liaison@wessexwater.co.uk
Proposal:	12 Bedroom extension to existing Premier Inn Hotel	Our Ref:	SU13SE/ 49
Location:	Premier Inn North Bishopsdown Salisbury	Date:	24/04/2024

Existing Services

There are no known Wessex Water assets within the proposed site boundary. However, our records indicate a private rising main, a private foul sewer and private surface water drainage sewers.

Foul Drainage

Capacity is available within the existing network to accommodate foul flows from the proposed additional 21 bedrooms.

Surface Water Drainage (rainwater falling onto, and running off, impermeable surfaces)

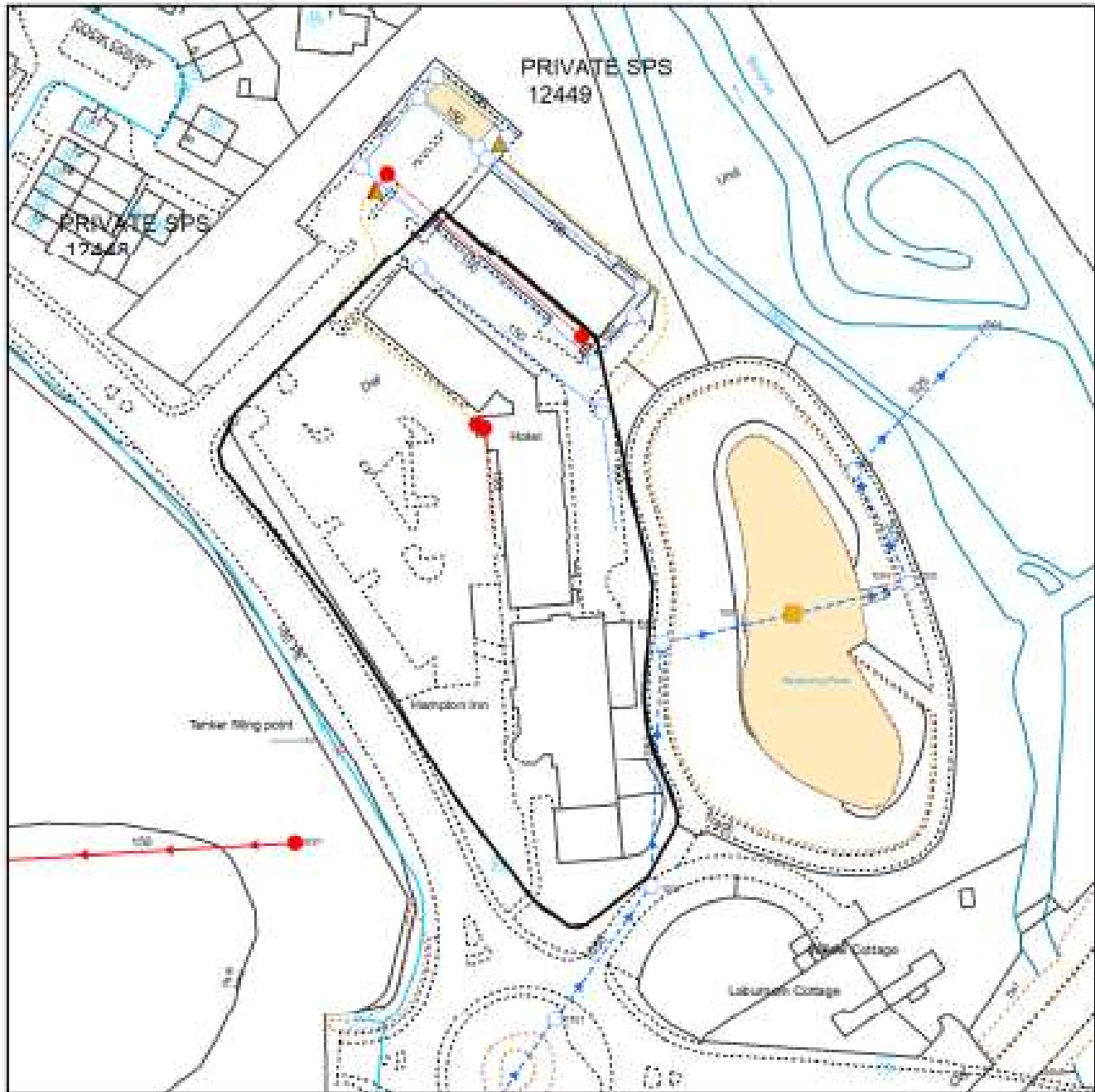
The proposed extension appears to be sited on an existing impermeable car parking area; therefore, we would not anticipate any increase in surface water runoff rates from the development. Please confirm the proposal for discharge of surface water runoff from the roof area only of the extension does not increase over existing.

Please Note: Wessex Water will not accept any surface water runoff, land drainage or ground water into the public foul sewer network either directly or indirectly.

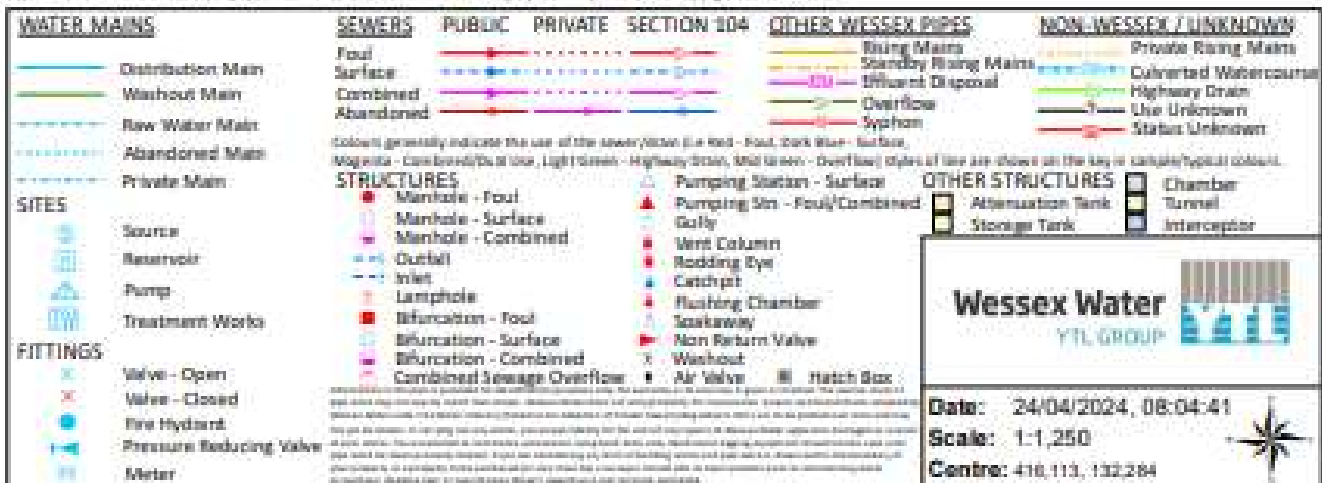
NOTE: These comments are based upon known circumstances prevailing at the time of writing. A review of the contents of this response is required where 18 months or more have elapsed since issue or in the light of significant changes likely to impact upon the response (e.g., changes in development numbers or phasing).

Please email review requests to planning.liaison@wessexwater.co.uk

SU13SE/ 49 ASSET MAP



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