



DRAINAGE STRATEGY

ISSUE 2

January 2026

Proposed Residential Development at

5a Coxhill Lane

Potterne

Devizes

SN10 5PH

Document Control

Client	Attwell Projects Ltd on behalf of Mr and Mrs Mayland		
Report title	Surface Water Drainage Strategy		
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Prepared by	A.Dixon BSc(Hons) CEng MICE		
Document check	Z.West		
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	2	Formal Issue	19-01-2026

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APPENDIX

TITLE

PRODUCED BY

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SW Drainage Strategy

Dixon Civils

Appendix B – Drawings by others

Proposed Site Plan

Poulsom Architectural Design
and Planning Services

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1.0 Introduction

This drainage strategy report has been prepared for Attwell Projects Ltd on behalf of Mr and Mrs Mayland, for the proposed single plot residential development at 5a Coxhill Lane, Potterne, Devizes (Application Ref: PL/2024/08330). This report will demonstrate how the appropriate means of sustainable surface water drainage will be achieved for the development site, to discharge the below planning condition.

“No development hereby approved shall commence above ground floor slab level until a detailed scheme for the discharge of surface water from the site (including surface water from the access / driveway), incorporating sustainable drainage details together with permeability test results to BRE365 and including all necessary permits, consents and permissions, has been submitted to and approved in writing by the Local Planning Authority. Thereafter, the development shall not be first occupied until surface water drainage has been constructed in accordance with the approved scheme”

2.0 The Existing Site

The application site covers 540m² (0.054ha) of land occupied by a car park with macadam surfacing. Access to the site is off Coxhill Lane as existing. Refer to the Proposed Site Plan prepared by Poulsom Architectural Design and Planning Services enclosed with this report.

The site is located within flood zone 1 and is bounded to the north, east and west by housing and to the south by Coxhill Lane. Land use in the vicinity is predominantly residential.

2.1 Existing ground conditions

British Geological Survey (BGS) mapping indicates that the site is located over Upper Greensand Formation - Sandstone.

LandIS Soilscape mapping categorises the soils as slowly permeable loamy and clay soils.

Defra's Multi-Agency Geographic Information for the Countryside (MAGIC) map indicates that the geology underlying the site is a principal aquifer with high vulnerability. The site is however not located within Groundwater Source Protection Zone.

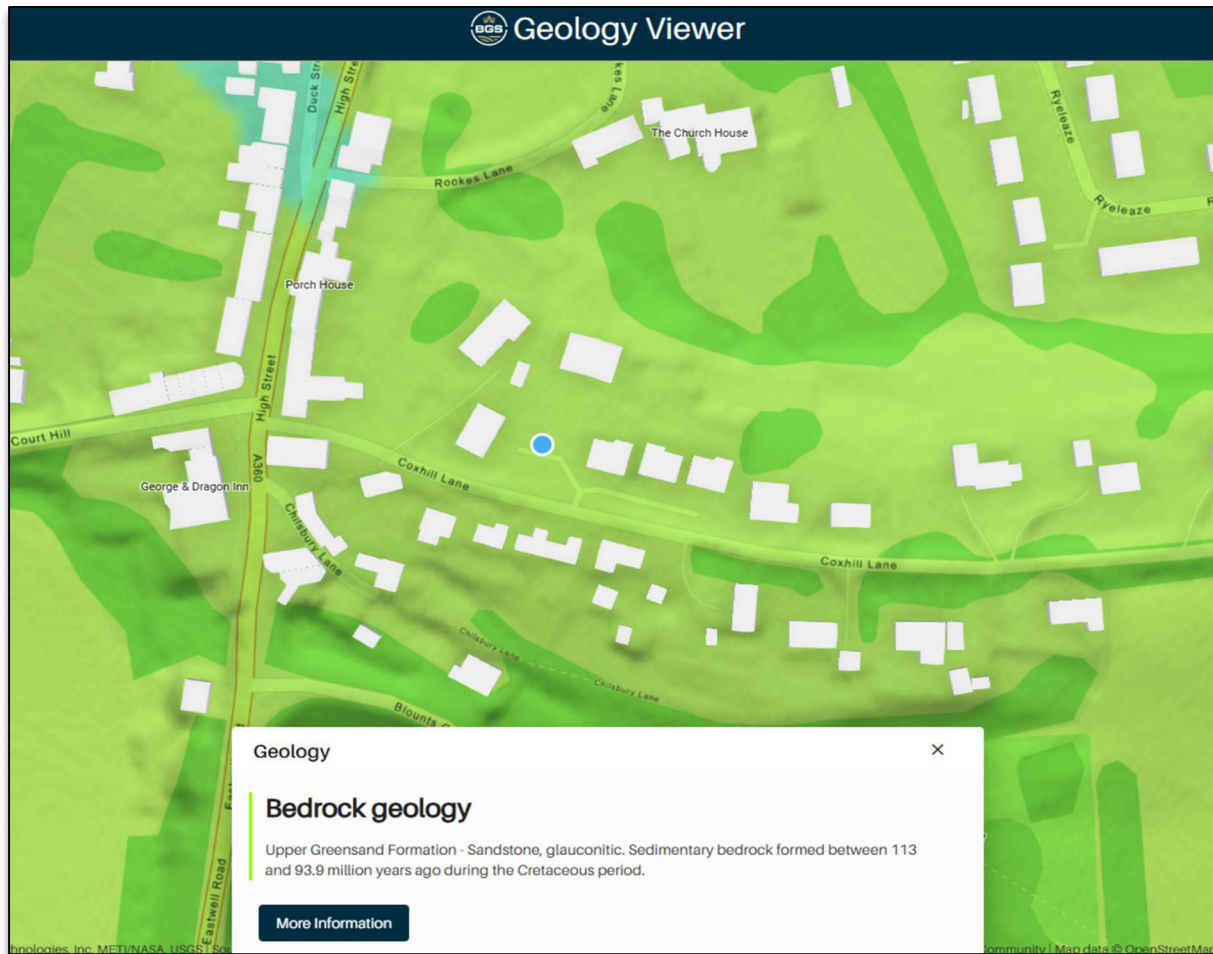


Figure 1: Bedrock Geology - Extract from British Geological Survey Geology of Britain Viewer
Source: <https://www.bgs.ac.uk/map-viewers/bgs-geology-viewer>

2.2 Intrusive Site Investigations

An intrusive site investigation has been completed by Wessex and West Ground Investigation on 18th November 2025, which included two window sample boreholes to depths of 3.9m below ground level (bgl) and three hand excavated trial pits, excavated to depths up to 0.85m bgl. Infiltration testing was completed within a test pit, at a depth of 0.83m and found viable infiltration results.

Soils were confirmed as Upper Greensand, and groundwater was encountered between depths of 2-3m bgl.

Please refer to the test results enclosed within Appendix D of this report.

2.3 Existing Watercourses and Drainage Features

There are no watercourses within the vicinity of the development. The nearest watercourse to the site is approximately 250m to the northwest, an unnamed tributary to the River Avon.

Asset records obtained from Wessex Water indicate the presence of public gravity foul and surface water sewers at the entrance of the site, in Coxhill Lane.

Refer to Wessex Water sewer records within Appendix C of this report.

2.4 Existing surface water drainage arrangement

A utilities survey reveals the presence of a soakaway below the existing car park toward the southern boundary of the application site, which is assumed to drain surface water from the existing tarmac areas.

3.0 Surface Water Drainage Proposals

Refer to DC Drawing 1212-01-01 [*Surface Water Drainage Strategy*] in Appendix A.

This section of the report addresses how flood risk generated as a result of developing the site will be managed using sustainable control measures to ensure that flood risks are not increased on and off the site and how surface water is managed post development.

3.1 Drainage Hierarchy

The surface water drainage strategy for the site has been prepared according to the drainage discharge hierarchy from CIRIA C753 The SuDS Manual, where the first available option will be utilised as follows.

Technique		Summary
1	Infiltration to the maximum extent that is practical	Proposed Solution – Infiltration testing confirms the use of soakaway devices to be feasible.
2	To surface water body	Unfeasible/Not required – No surface water body within the vicinity of the site
3	To a surface water sewer, highway drain or another drainage system	Unfeasible/Not required – No surface water sewers within the vicinity of the site.
4	To a combined sewer	Not required

3.2 Infiltration Design

Infiltration results obtained from BRE365 testing enables the use of soakaways and infiltration features for drainage of the roof and external hard surfaces.

It is therefore proposed that stormwater runoff from the new dwelling and driveway will be attenuated within the clean stone sub-base below the permeable surfaced drive and drain via infiltration through the base of the system.

Runoff from the existing impermeable access road will also be re-directed to the permeable paving sub-base to reduce the existing off-site runoff contributing to the highway network.

Calculations indicate SuDS attenuation of 10.2m³ will be required to accommodate stormwater flows from the proposed dwelling roof, driveway and access road for storm events up to and including the 100year +45% climate change event. The 160m² permeable surfaced driveway with a minimum depth of 300mm of clean stone sub-base will provide an attenuation volume of 14.4m³.

Refer to the proposed drainage calculations enclosed within Appendix E of this report, based on the slowest of the three recorded infiltration rates of 5.91x10⁻⁵ m/s.

3.3 Design Exceedance

Should the drainage system and SuDS fail under extreme events or blockages, floodwaters will follow the natural topography to the south along the access road toward Coxhill Lane, mirroring the existing development arrangement.

3.4 Water Quality

Roof water is considered fairly uncontaminated, therefore further pollution interception beyond that provided by the cellular storage is not proposed.

Runoff from the access road and parking areas can be susceptible to hydrocarbon contamination, which in this instance will be intercepted within permeable stone and textiles that make up the permeable paving sub-base construction, before discharging to ground.

3.5 Adoption

The onsite drainage components will remain private and be the maintenance responsibility of the property owners.

3.6 Maintenance

Regular inspection and maintenance are important for the effective operation of the SuDS systems as designed. The regular maintenance of SuDS should be carried out in line with C753 The SUDS Manual (CIRIA, November 2015) or as per manufacturer's specification.

Pipework and Catchpits

It is not envisaged that silt build up within the pipework systems will require rigorous maintenance. Notwithstanding this, a suitable maintenance regime for the systems will comprise of routine inspection (every six months) and silt removal as necessary.

Permeable Paving

Permeable surfaces need to be regularly cleaned of silt and other sediments to preserve the infiltration capability of the surface. Suggested brushing regime would be at the end of winter to collect winter debris, mid-summer to collect dust, vegetation and grass-type deposits and after autumn leaf fall.

4.0 Conclusions

The proposed new dwelling at 5a Coxhill Lane will increase the impermeable surface area when compared to existing. Surface water runoff from the proposed dwelling and parking will be managed by new sustainable drainage systems that dispose of all water at source via infiltration.

Runoff from the existing access will be intercepted by new drainage, directed to the proposed on site SuDS, and remove runoff to the existing highway network.

On site storage and infiltration will be provided within a permeable gravel Sustainable Urban Drainage Systems (SUDS) driveway fronting the dwelling. The SUDS features have been designed to accommodate the 1 in 100 year storm, including a 45% allowance for climate change, in accordance with NPPF guidance.

The proposed SUDS and all other surface water drainage components will remain private and be maintained by the owners of the property.

We can conclude that providing the development adheres to the conditions advised in this report, the proposals can be accommodated without increasing flood risk to the site and local area.

Dixon Civils Ltd

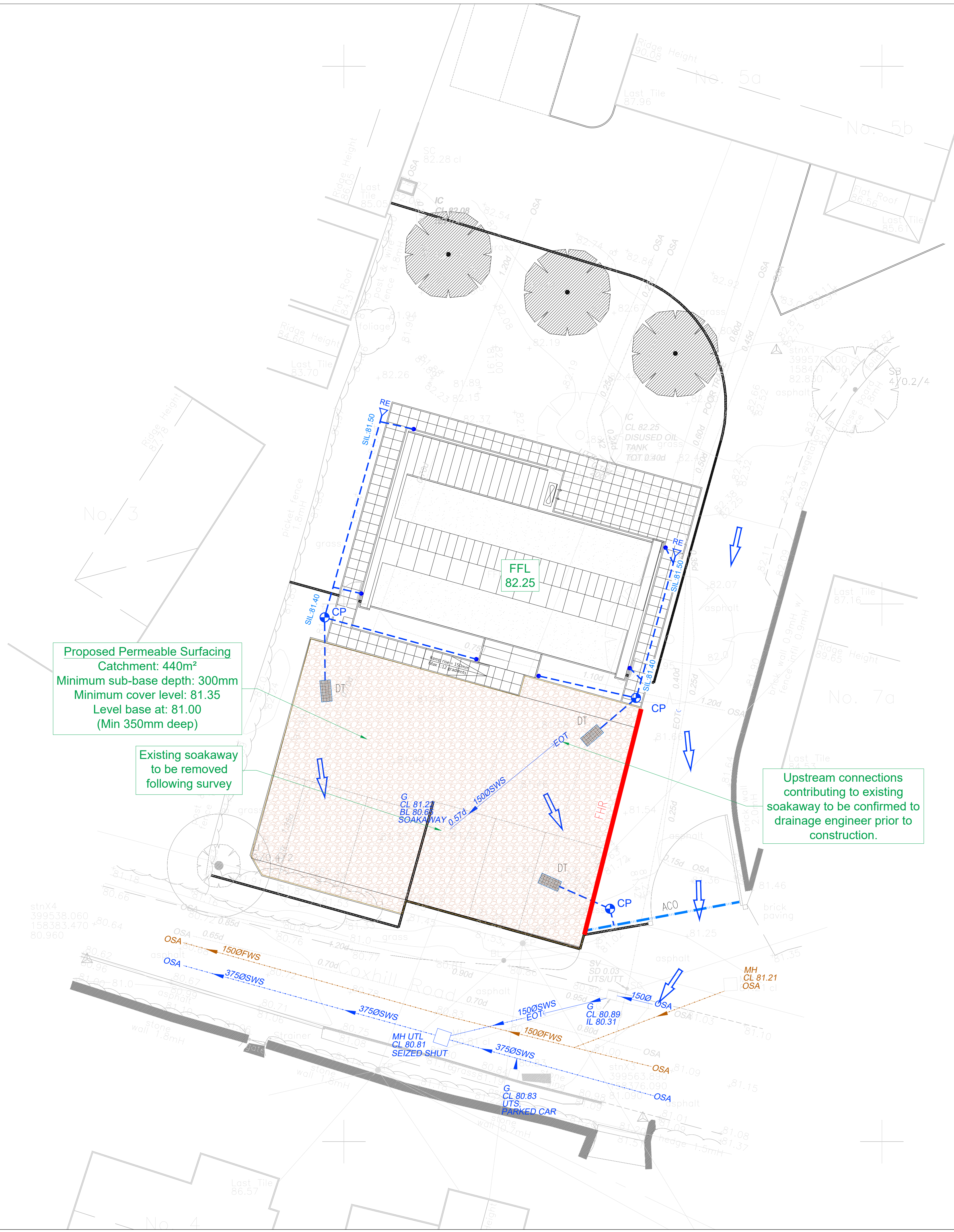
Issue 2 - January 2026

APPENDIX A

Drawings by Dixon Civils

Drawing ref. 1212-01-01

Surface Water Drainage Strategy



N158425

N158400

N158375

KEY:

- FFL 99.90 Finished Floor Level
- Proposed Private Surface Water Drain, Inspection Chamber & Invert Level
- Proposed Rodding Eye
- Proposed Surface Water Catchpit Chamber
- Channel Drain
- Flood Flow Route (Over 1:100yr+45% storm event)
- Proposed Permeable Paving Distribution Tank
- Proposed Permeable Gravel Surfacing

NOTES:

- Do not scale from this drawing.
- Drawing issued for planning purposes only, not for construction.
- This drawing is based on topographical survey compiled by Wouth West Surveys
- All levels are in metres and relate to the ordnance survey level.
- Do not scale from this drawing. Work only from stated dimensions or setting out details.
- This drawing shall be read in conjunction with all relevant scheme drawings and specifications.
- At the commencement of works the contractor is to carry out trial pits and liaise with Utility Companies in order to establish the exact position of all existing statutory utility plant in the vicinity of works and take adequate precautions for their protection.
- All foul and storm water drains which are not subject to adoption as public sewers under a Section 104 Agreement must be constructed in accordance with the latest Building Regulations, BS8301, and, where appropriate, the relevant agreement certificates.
- 100mmØ private foul water drainage shall be laid no flatter than 1 in 80. 150mmØ pipework shall be laid no flatter than 1 in 150.

A	Permeable drive surfacing updated	AD	19.01.26
-	FIRST ISSUE (DRAFT)	AD	16.01.26
Rev	Description	Dm	Date

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Drainage & Infrastructure Design
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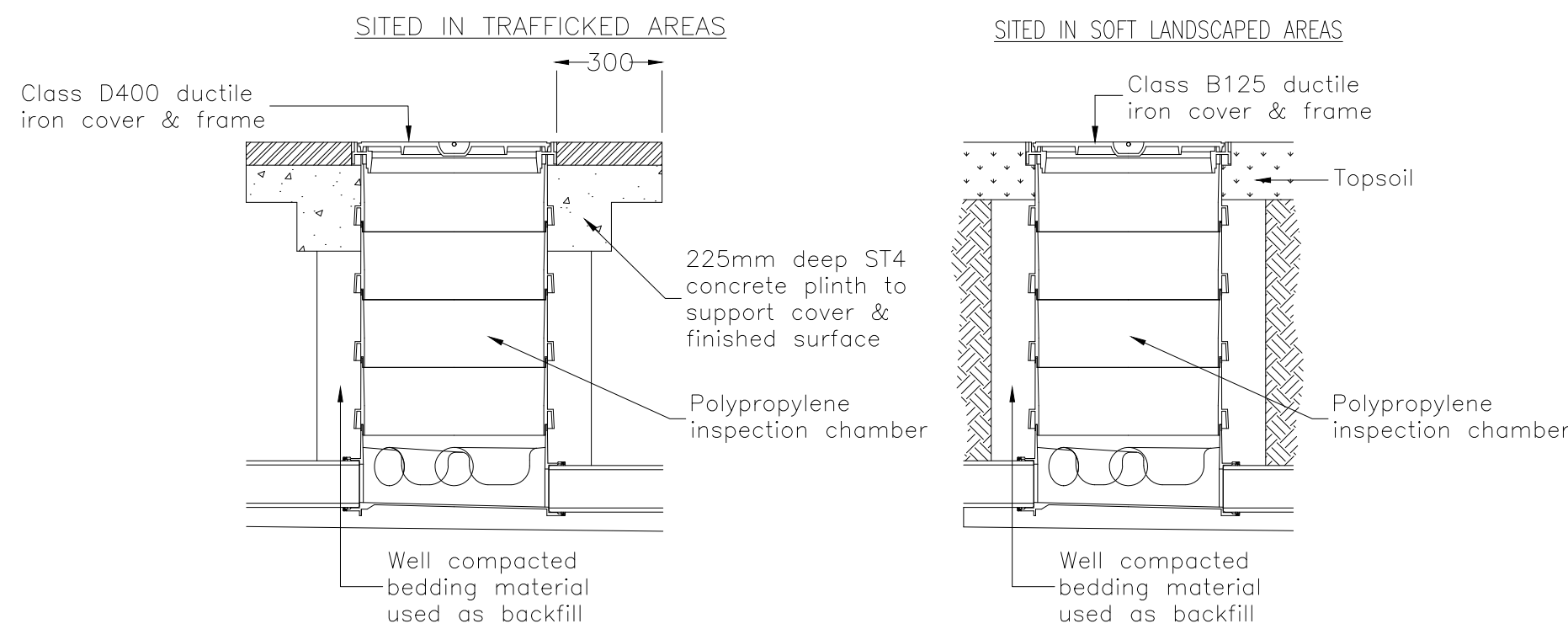
CLIENT
Attwell Projects

JOB TITLE
5a Coxhill Lane
Potterne
Devizes

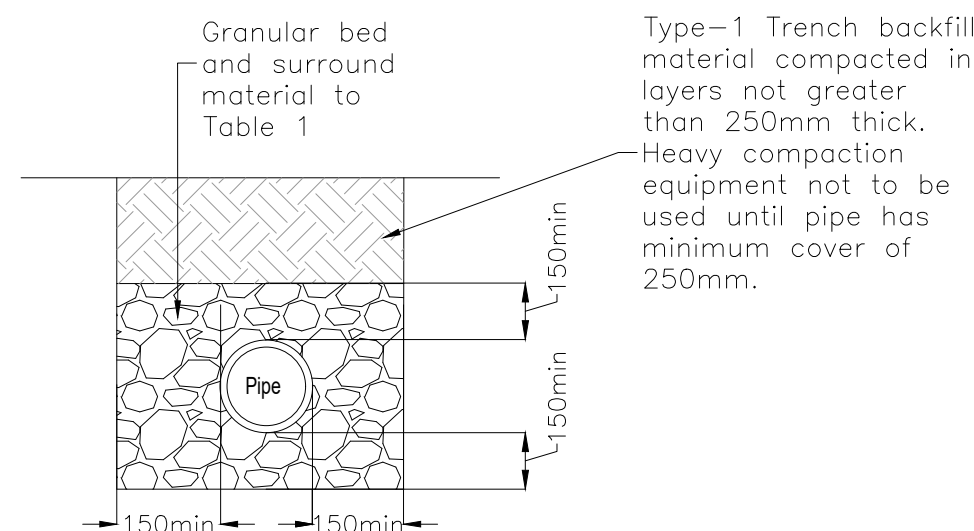
DRAWING TITLE
Surface Water Drainage
Strategy

DRAWING INFORMATION
DATE: JAN 2026 DRAWN: AD
DRAWING NO: 1212-01-01 REV: A SCALE: 1:100 @ A1
DESIGNED FOR: FOR APPROVAL

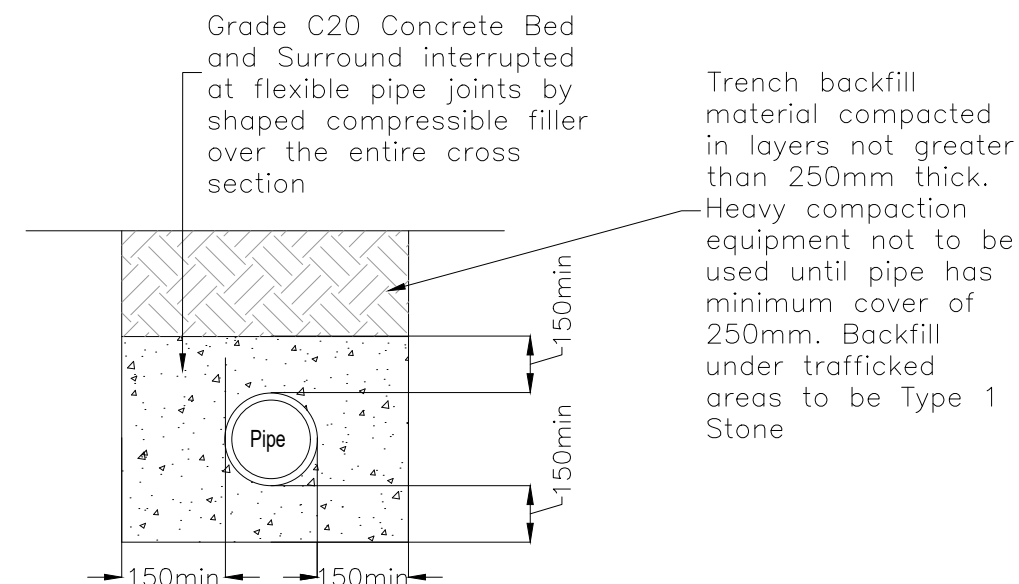
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POLYPROPYLENE INSPECTION CHAMBER



GRANULAR PIPE BEDDING AND SURROUND FOR UNLOADED APPLICATIONS AND TRAFFICKED AREAS WITH OVER 1.2M COVER

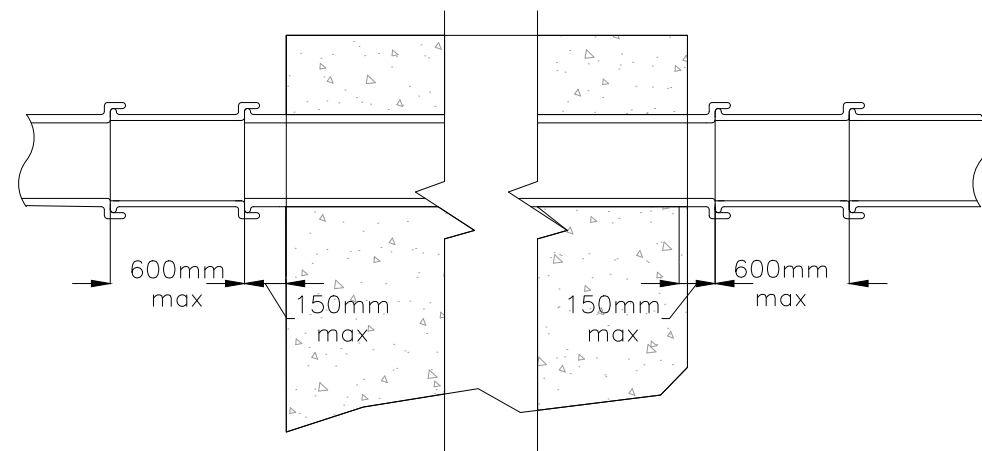


CONCRETE BEDDING AND SURROUND FOR TRAFFICKED AREAS <1.2M DEEP

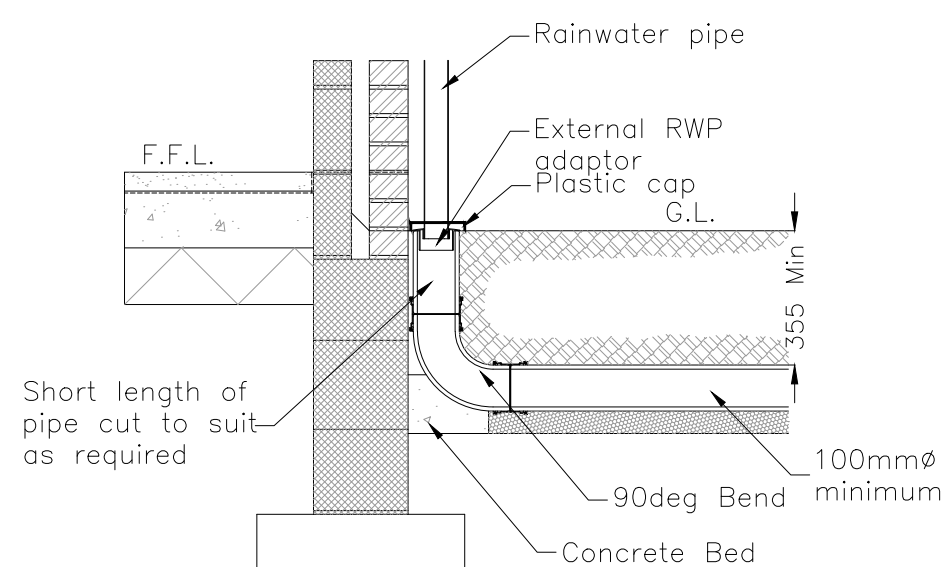
Pipe nominal size (DN)	Nominal maximum particle size (mm) See note (c)	Class of bedding See note (d)	Suitable Materials	
			EITHER: Maximum CF value for acceptability See note (b)	OR: Materials specified in British Standards See note (a)
100	10	S	0.15	10mm nominal single-size
		B	0.30	
		N	0.15	
Over 100 to 150	16	S	0.15	Fine aggregate
		B	0.30	
		N	0.15	
Over 150 to 300	20	S	0.15	10, 14 or 20mm nominal single-size or 14mm to 5mm graded or 20 to 5mm graded
		B	0.30	
		N	0.15	
Over 300 to 550	20	S	0.15	All-in aggregate or fine aggregate
		B	0.30	
		N	0.15	
Over 550	40	S	0.15	14, 20 or 40mm nominal single-size crushed rock or 14mm to 5mm, 20 to 5mm or 40 to 5mm graded
		B	0.30	
		N	0.15	

- Notes:
- (a) Processed granular materials to include aggregates to BS EN 12620.
- (b) Compaction Fraction value (CF), see Appendix B of WIs IG No. 4-06-02.
- (c) The nominal maximum particle sizes apply both to processed and as-dug materials (see Section 4 of IG No. 4-06-01 Issue 4 and No.4-06-01 Amendment (Nov 2008)).
- (d) Bedding classes are defined in BS EN 1295-1:1997 Structural Design of Buried Pipelines under various conditions of loading. TRRL - Simplified Table of External Loads on Buried Pipelines.
- (e) The sulphate content of bedding and sidefill materials for use with cementitious pipe should not be greater than 0.3% as sulphur trioxide.

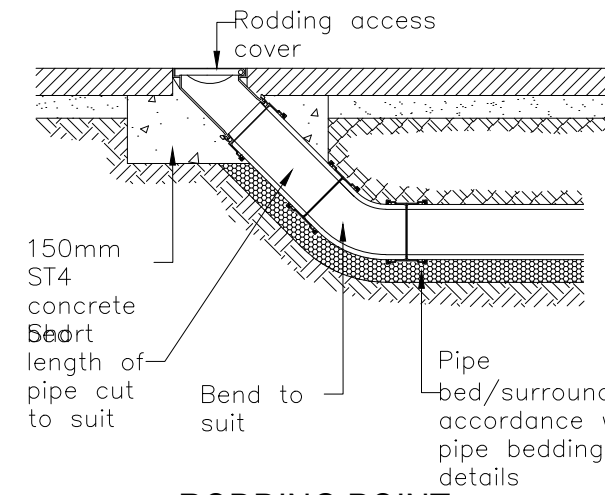
TABLE 1 - PROCESSED AND AS-DUG GRANULAR BEDDING AND SIDEFILL MATERIALS FOR RIGID PIPES



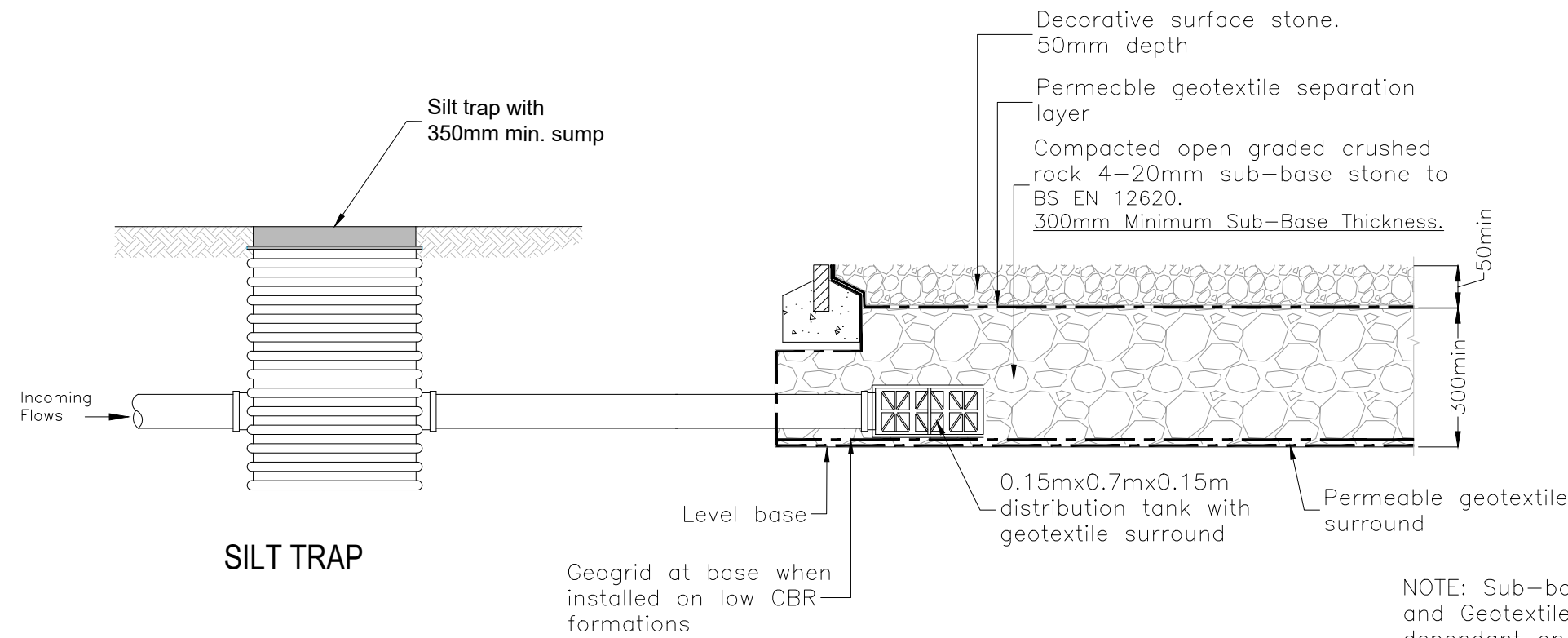
ROCKER PIPE FOR DRAINAGE PASSING THROUGH FOUNDATIONS



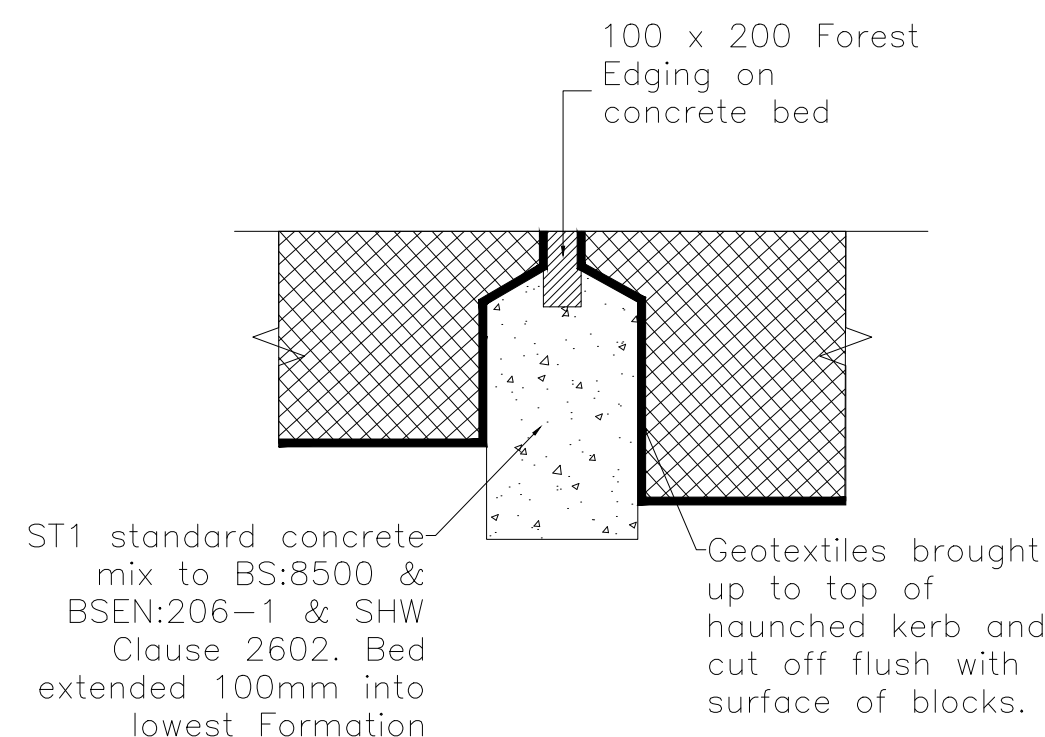
RWP DOWNPIPE CONNECTION



RODDING POINT



GRAVEL ATTENUATION SECTION AND PIPED CONNECTION



FULL HEIGHT RESTRAINT

GENERAL NOTES:

- DO NOT SCALE FROM THIS DRAWING.
- THIS DRAWING SHALL BE READ IN CONJUNCTION WITH ALL RELEVANT SCHEME DRAWINGS AND SPECIFICATIONS.

A	Permeable drive section updated	AD	19.01.26
-	FIRST ISSUE	AD	16.01.26
Rev	Description	Dm	Date

REVISIONS

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CLIENT
Attwell Projects

JOB TITLE
**5a Coxhill Lane
Potterne
Devizes**

DRAWING TITLE
Drainage Construction Details

DATE **JAN 2026** DRAWN **AD**
DRAWING No. **1212-02-01** REV **A** SCALE **NTS**

ISSUED FOR
PRELIMINARY ISSUE

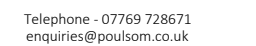
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APPENDIX B

Drawings by others

Proposed Site Plan (Poulsom Architectural Design and Planning Services)

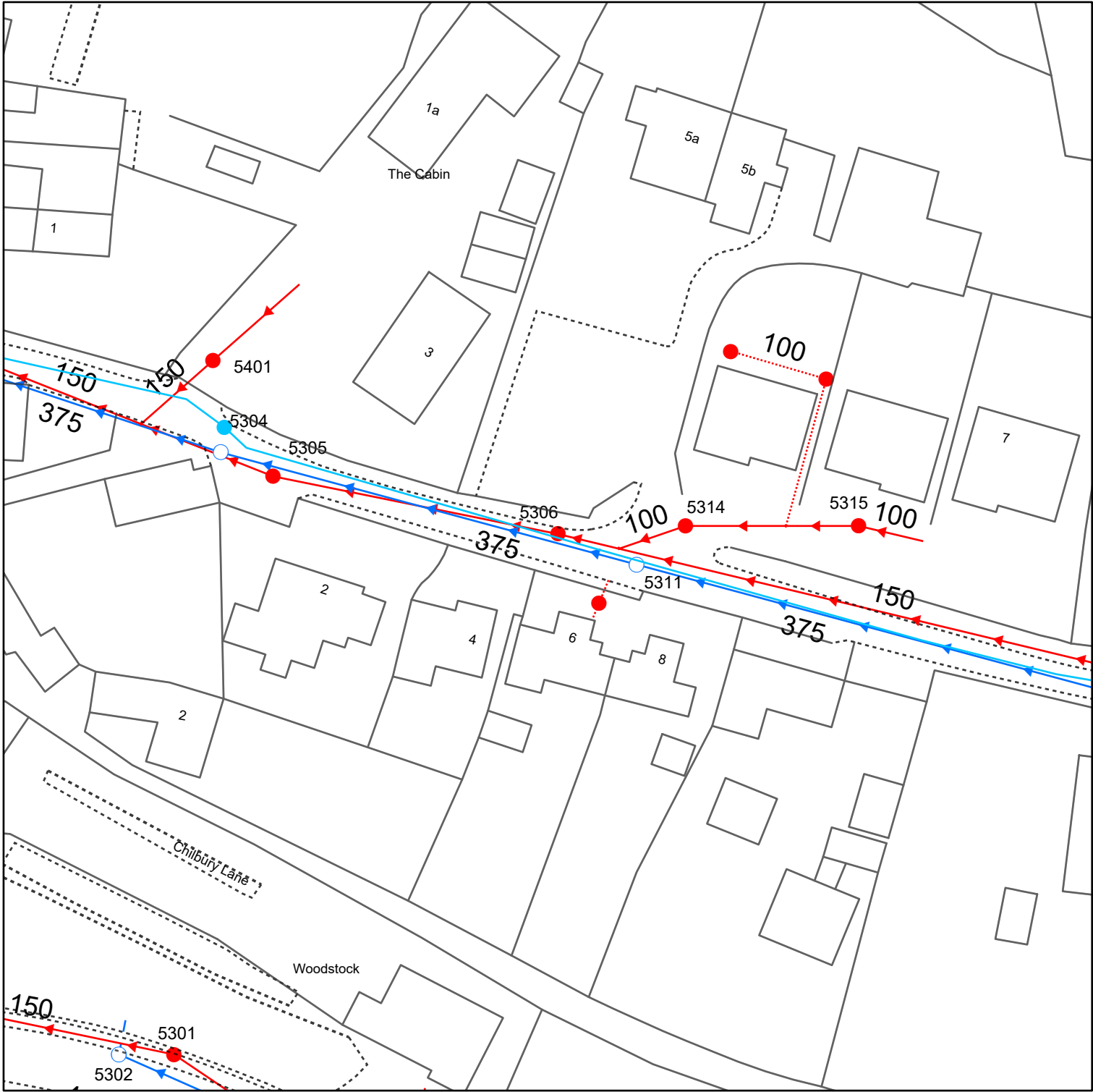
nt Name - Scale - @ A1
t = Date - November 2025



APPENDIX C

Wessex Water Sewer Records

Wessex Water Network Map



Reproduced from the Ordnance Survey map with the permission of the Controller of His Majesty's Stationery Office. © Crown copyright and database rights (2025) OS AC0000848304

WATER MAINS		SEWERS	STRATEGIC PUBLIC	PRIVATE	SECTION 104	OTHER WESSEX PIPES	NON-WESSEX / UNKNOWN
Distribution	Washout	Foul	Surface	Combined	Abandoned	Rising Mains	Private Rising Mains
Raw Water	Abandoned	Surface	Combined	Abandoned	Private	Standby Rising Mains	Culverted Watercourse
Private		Abandoned	Abandoned	Abandoned	Abandoned	EDM Effluent Disposal	Highway Drain
						Overflow	Use Unknown
						Syphon	Status Unknown
FITTINGS		STRUCTURES		OTHER STRUCTURES		OTHER STRUCTURES	
Hydrant	Other	Manhole - Foul	Manhole - Surface	Pumping Station - Surface	Pumping Stn - Foul/Combined	Attenuation Tank	Chamber
		Manhole - Combined	Inlet	Gully	Vent Column	Storage Tank	Tunnel
		Lamphole	Outfall	Rodding Eye	Catchpit		Interceptor
		Bifurcation - Foul	Bifurcation - Surface	Flushing Chamber	Soakaway		
		Bifurcation - Combined	Combined Sewage Overflow	Non Return Valve	Air Valve		
				Hatch Box	Washout		

Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526333 or e-mail: developer.enquiries@wessexwater.co.uk to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.

Date: 14/10/2025
Centre: 399554E, 158381N
Scale: 1:625
(when printed at A4 size)

APPENDIX D

BRE365 Infiltration Test Results

Trial Pit Soakaway Test



Based on BRE DG 365:2016

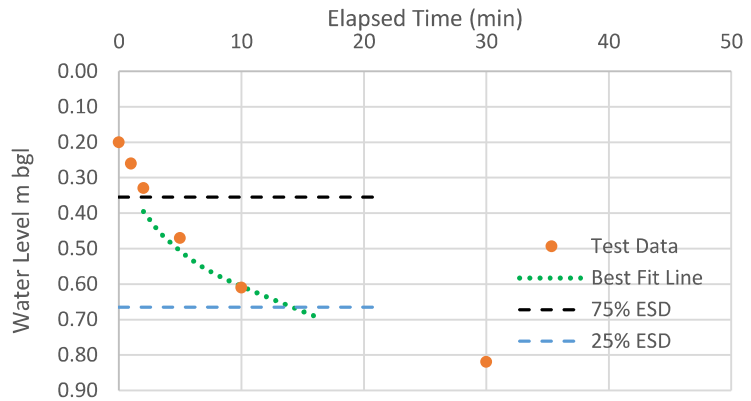
Project	Potterne	Status	LOCATION ID
Project ID	G0807	CHECKED	SA01

Trial Pit Details

	Test 1	Test 2	Test 3	Ground Level	mAOD	Date Excavated	18/11/2025
Depth	0.82	0.82	0.82	Coordinates	mE	Date Tested	18/11/2025
Width	0.45	0.45	0.45		mN		
Length	0.45	0.45	0.45				

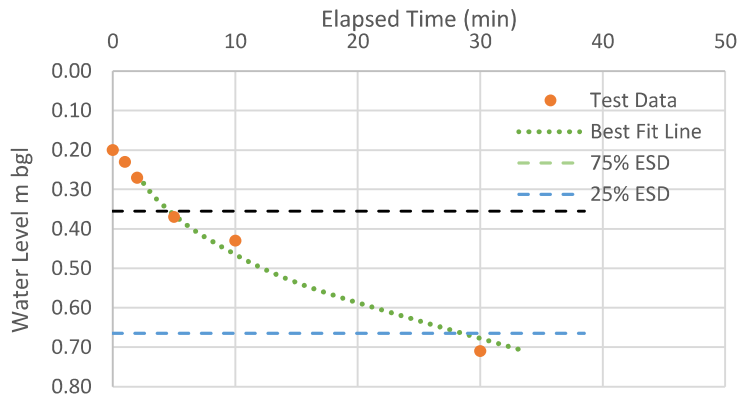
Test 1

Time min	Depth to Water m bgl	Test Parameters		Elapsed Time (min)			
0	0.20	75% esd (mbgl)	0.36	0.00			
1	0.26	50% esd (mbgl)	0.51	0.10			
2	0.33	25% esd (mbgl)	0.67	0.20			
5	0.47	A_{50} (m ²)	0.76	0.30			
10	0.61	V_{p75-25} (m ³)	0.06	0.40			
30	0.82	t_{75} (min)	1.3E+00	0.50			
60		t_{25} (min)	1.4E+01	0.60			
90		Data Fit R ²	0.995	0.70			
120				0.80			
180				0.90			
Infiltration rate f (ms ⁻¹)				Total effective storage depth (esd) (m)			
1.11E-04				0.62			



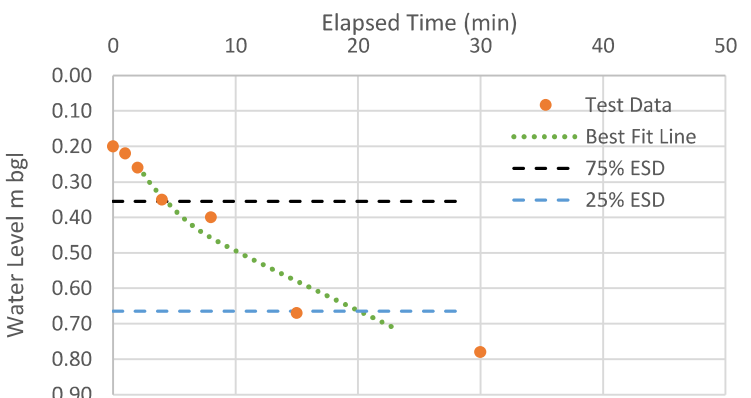
Test 2

Time min	Depth to Water m bgl	Test Parameters		Elapsed Time (min)			
0	0.20	75% esd (mbgl)	0.36	0.00			
1	0.23	50% esd (mbgl)	0.51	0.10			
2	0.27	25% esd (mbgl)	0.67	0.20			
5	0.37	A_{50} (m ²)	0.76	0.30			
10	0.43	V_{p75-25} (m ³)	0.06	0.40			
30	0.71	t_{75} (min)	4.6E+00	0.50			
60		t_{25} (min)	2.8E+01	0.60			
90		Data Fit R ²	0.983	0.70			
120				0.80			
180				0.90			
Infiltration rate f (ms ⁻¹)				Total effective storage depth (esd) (m)			
5.91E-05				0.62			



Test 3

Time min	Depth to Water m bgl	Test Parameters		Elapsed Time (min)			
0	0.20	75% esd (mbgl)	0.36	0.00			
1	0.22	50% esd (mbgl)	0.51	0.10			
2	0.26	25% esd (mbgl)	0.67	0.20			
4	0.35	A_{50} (m ²)	0.76	0.30			
8	0.40	V_{p75-25} (m ³)	0.06	0.40			
15	0.67	t_{75} (min)	4.4E+00	0.50			
30	0.78	t_{25} (min)	1.9E+01	0.60			
45		Data Fit R ²	0.960	0.70			
60				0.80			
120				0.90			
Infiltration rate f (ms ⁻¹)				Total effective storage depth (esd) (m)			
9.14E-05				0.62			



Carried out by	Notes:	Logged	Checked
Wessex and West		IP	CP

APPENDIX E

Calculations

Infodrainage Soakaway Calculations

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Inflows Storm Phase: Phase	Designed: Dixon Civils Ltd		



Dixon Civils
Drainage & Infrastructure Design



House Catchment

Type : Catchment Area

Area (ha)	0.01
-----------	------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.750
Winter Volumetric Runoff	0.840
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Parking Catchment

Type : Catchment Area

Area (ha)	0.02
-----------	------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.750
Winter Volumetric Runoff	0.840
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Inflows Storm Phase: Phase	Designed: Dixon Civils Ltd		



Ex Drive Catchment

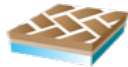
Type : Catchment Area

Area (ha)	0.014
-----------	-------

Dynamic Sizing

Runoff Method	Time of Concentration
Summer Volumetric Runoff	0.750
Winter Volumetric Runoff	0.840
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase	Designed: Dixon Civils Ltd		



Porous Paving

Type : Porous Paving

Dimensions

Exceedance Level (m)	81.300
Depth (m)	0.450
Base Level (m)	80.850
Paving Layer Depth (mm)	80
Membrane Percolation (m/hr)	1000.0
Porosity (%)	30
Length (m)	10.000
Long. Slope (1:X)	10000.00
Width (m)	16.000
Total Volume (m³)	17.760

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Parking Catchment
Bypass Destination	(None)
Capacity Type	No Restriction

Inlet (1)

Inlet Type	Point Inflow
Incoming Item(s)	Ex Drive Catchment House Catchment
Bypass Destination	(None)
Capacity Type	No Restriction

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Storm Phase: Phase	Designed: Dixon Civils Ltd		

Dixon Civils
Drainage & Infrastructure Design

Advanced

Base Infiltration Rate (m/hr)	0.213
Side Infiltration Rate (m/hr)	0.213
Safety Factor	2.0
Conductivity (m/hr)	1000.0

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Title: Rainfall Analysis Criteria	Designed: Dixon Civils Ltd		



Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Apply Global Value
Urban Creep Global Value (%)	0
Junction Flood Risk Margin (mm)	300
Perform No Discharge Analysis	☐

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed: Dixon Civils Ltd		



Dixon Civils
Drainage & Infrastructure Design



FSR: 10 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Porous Paving	FSR: 10 years: +0 %: 15 mins: Winter	80.941	80.851	0.090	0.001	12.1	2.152	0.000	5.633	0.0	0.000	3	87.883

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026			 Dixon Civils Drainage & Infrastructure Design
	Designed by: AD	Checked by:	Approved By:	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed: Dixon Civils Ltd			

Status

OK

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed: Dixon Civils Ltd		



Dixon Civils
Drainage & Infrastructure Design



FSR: 30 years: Increase Rainfall (%): +0: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Porous Paving	FSR: 30 years: +0 %: 15 mins: Winter	80.978	80.863	0.127	0.013	15.4	3.365	0.000	6.512	0.0	0.000	6	81.051

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026			
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Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed: Dixon Civils Ltd			

Status

OK

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed: Dixon Civils Ltd		



FSR: 100 years: Increase Rainfall (%): +45: Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Porous Paving	FSR: 100 years: +45 %: 30 mins: Winter	81.142	81.014	0.291	0.164	19.8	10.194	0.000	16.358	0.0	0.000	24	42.603

Project: 5a Coxhill Lane Potterne	Date: 16/01/2026			 Dixon Civils Drainage & Infrastructure Design
	Designed by: AD	Checked by:	Approved By:	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed: Dixon Civils Ltd			

Status

OK



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