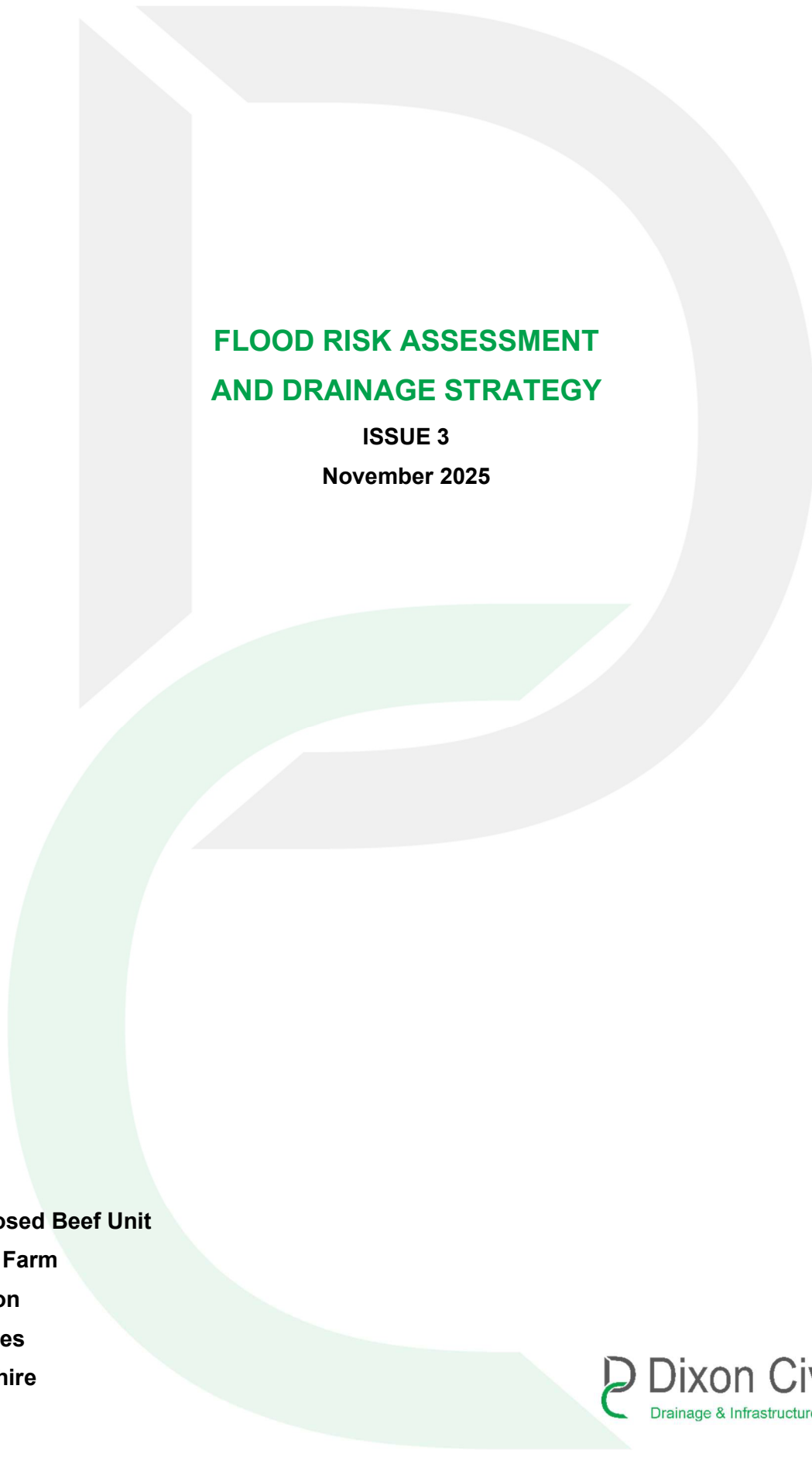




FLOOD RISK ASSESSMENT AND DRAINAGE STRATEGY

**ISSUE 3
November 2025**

**Proposed Beef Unit
Wyre Farm
Worton
Devizes
Wiltshire**

Document Control

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Prepared by	A.Dixon BSc(Hons) CEng MICE		
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Contents

1.0 Introduction	5
2.0 Referenced Policies	5
3.0 The Existing Site	6
3.1 Existing ground conditions	6
3.2 Existing Watercourses and Drainage Features	7
3.3 Existing Public Sewers	7
3.4 Existing surface water drainage arrangement	7
4.0 Evaluation of Flood Risk	7
4.1 Historic Flooding Events	7
4.2 Fluvial Flood Risk	8
4.3 Surface Water Flooding	9
4.4 Sewer Flooding	9
4.5 Groundwater	9
4.6 Flooding from reservoirs or other artificial sources	9
4.7 Summary of Flood Risk	10
5.0 Surface Water Drainage Proposals	10
5.1 Drainage Hierarchy	10
5.2 Infiltration Design	11
5.3 Attenuation Design	11
5.4 Design Exceedance	12
5.5 Water Quality	12
5.6 Adoption	13
5.7 Maintenance	13
6.0 Foul Water Drainage Proposals	14
6.1 Proposal	14
6.2 Adoption	14

6.3 Maintenance	14
7.0 Conclusion	15

APPENDIX

TITLE

PRODUCED BY

Appendix A – Drawings by DC

Drainage Strategy Layout

Dixon Civils

Appendix B – Drawings by others

Existing Site Layout

Building Design Surveys Ltd

Proposed Site Layout

Falcon Building Consultants

Appendix C – Sewer Records

Appendix D - Calculations

1.0 Introduction

This Flood Risk Assessment has been prepared on behalf of J R Bodman Ltd for the proposed beef unit and temporary agricultural workers dwelling at Wyre Farm, Worton, Devizes, Wiltshire.

This report will provide flood risk information in support of the planning application and demonstrate how the appropriate surface water drainage scheme based on Sustainable Drainage Systems (SuDS) will be achieved for the development without worsening flood risk in the area or putting itself at risk of flooding in line with national guidance.

The development proposal comprises the construction of two agricultural buildings and a temporary mobile home for use as an agricultural worker's dwelling with associated access. Access to the site is off Worton Road as existing. Refer to the Proposed Site Plan prepared by Falcon Building Consultants enclosed within appendix B of this report.

2.0 Referenced Policies

Information sources used within this report are noted below along with the date they were referred to.

- Flood Map for Planning – Environment Agency (EA, accessed September 2025);
- Flood Map for Surface Water – Environment Agency (EA, accessed September 2025);
- Geological Map - British Geological Survey (BGS, accessed September 2025);
- Groundwater Source Protection Zones Map - Environment Agency (EA, accessed September 2025);
- Wessex Water Sewer Records (September 2025); and
- Wiltshire Council Core Policy 67: Flood Risk
- Wiltshire Council Strategic Flood Risk Data Explorer.
<https://wiltscouncil.maps.arcgis.com/apps/webappviewer/index.html?id=28c00215d87b468a9a0cd79c63d3d270>
- Building Regulations 2015 – Approved Document H (Drainage and Waste Disposal);
- C753 The SuDS Manual (CIRIA, November 2015);
- National Planning Policy Framework (NPPF) (July 2021);
- Non-Statutory Technical Standards for Sustainable Drainage Systems (Defra, March 2015);
- Planning Practice Guidance (PPG) (March 2014);
- Sewerage Sector Guidance (SSG), Version 1.0 (Water UK, October 2019).

3.0 The Existing Site

Refer to the Existing Site Plan prepared by Falcon Building Consultants within appendix B this report. The application site occupies open agricultural fields located within Flood Zone 1 and is bounded to the north, east and south by open fields and to the west by the Worton Road and the village of Worton. Land use in the vicinity is predominantly agricultural.

Survey levels indicate that the site falls to the north, with existing ground levels within the application site varying from approximately 62.00mAOD along the site's southern boundary, to 56.00m at its north west extent.

3.1 Existing ground conditions

British Geological Survey (BGS) mapping indicates that the geology below the site is split between the Gault and Wardour Formations, made up of Mudstone and Sandstone.

Defra's Multi-Agency Geographic Information for the Countryside (MAGIC) map indicates that the geology underlying the site as having high groundwater vulnerability, designated as a Secondary B unproductive Aquifer. The site is not located within a Groundwater Sources Protection Zone.

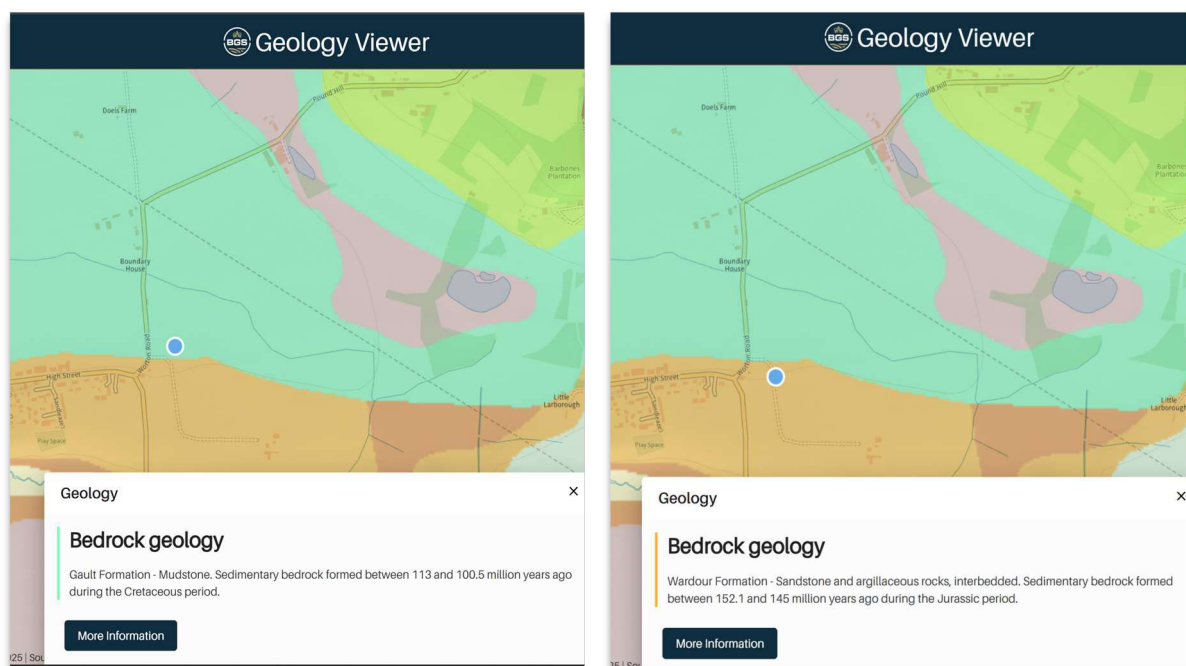


Figure 1: Bedrock Geology - Extract from British Geological Survey Geology of Britain Viewer
Source: <https://www.bgs.ac.uk/map-viewers/bgs-geology-viewer>

3.2 Existing Watercourses and Drainage Features

The site falls to the north, bounded by an un-named watercourse running east to west, becoming an Environment Agency designated main river as it passes below Worton Road to the north of the site. The river joins the Semington Brook approximately 4km to the west.

3.3 Existing Public Sewers

Asset records obtained from Wessex Water indicate the nearest foul water sewer to be located 130m from the entrance of the site at the junction of Worton Road with High Street. There are no surface water sewers within the vicinity of the site.

Refer to the Wessex Water sewer records within appendix C of this report.

3.4 Existing surface water drainage arrangement

The existing site is not formally drained and drains in part to ground and overland to the above mentioned watercourse along the northern boundary.

4.0 Evaluation of Flood Risk

This section presents an assessment of flood risk to the development and includes recommended flood risk mitigation measures if required. A summary of flood risk issues and proposed mitigation measures are recorded in Table 4.7 below.

4.1 Historic Flooding Events

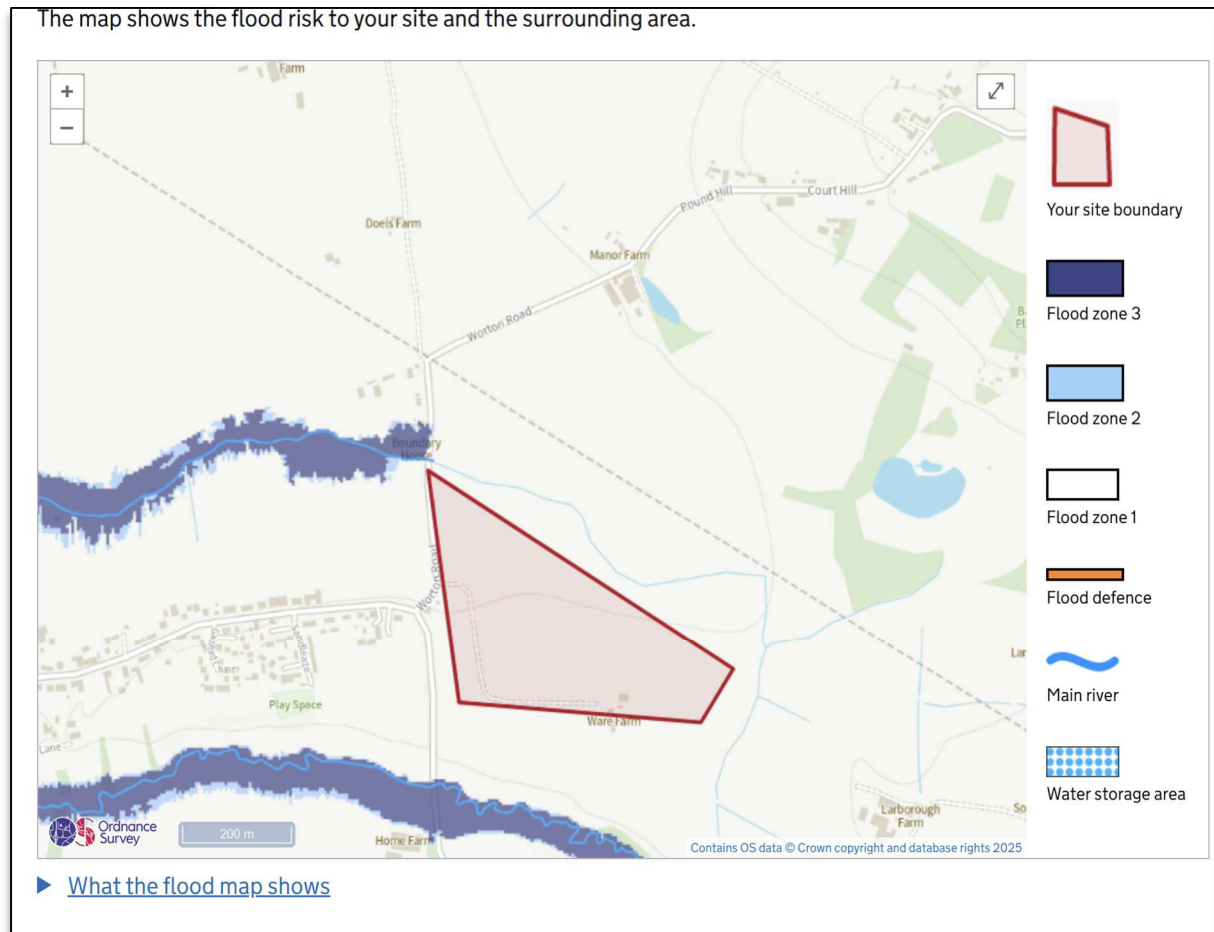
There is no record of flooding at the site on the Wiltshire Council Strategic Flood Risk Data.

<https://wiltscouncil.maps.arcgis.com/apps/webappviewer/index.html?id=28c00215d87b468a9a0cd79c63d3d270>.

4.2 Fluvial Flood Risk

The Flood Map for Planning (FMP) for the locality as produced by the EA is shown in the figure below and indicates that the site lies within Flood Zone 1 (Low Risk).

NPPF PPG Flood Risk and Coastal Change (Table 3: Flood Risk Vulnerability and Flood Zone Compatibility) confirms that all development types are appropriate within Flood Zone 1.

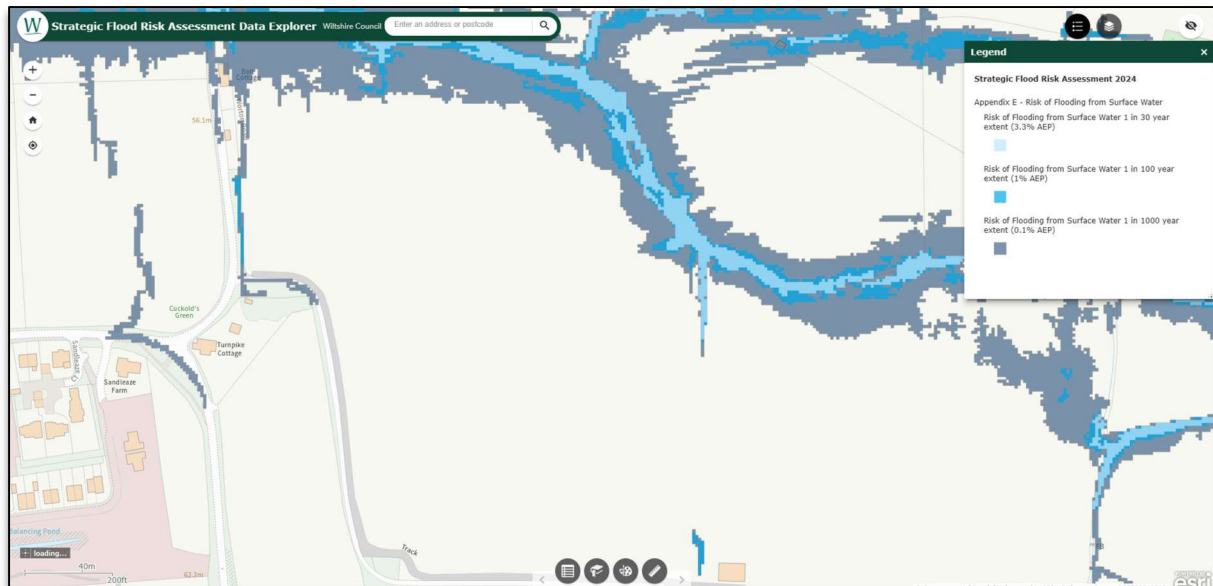


Flood Zone Map: Extract from Central Government's 'Flood Map for Planning' Mapping Portal

Source: <https://flood-map-for-planning.service.gov.uk/>

4.3 Surface Water Flooding

The Environment Agency (EA) surface water flood risk mapping and Wiltshire Council Strategic Flood Risk Data indicates that the boundary of the site is at minor risk of surface water flooding for the 0.1% AEP, associated with the boundary watercourse. The development area of the site is not affected by any surface water flooding. Refer to the figure below.



Surface Water Flood Map: Extract from Wiltshire Council's 'Strategic Flood Risk Assessment Data Explorer'

Source: <https://wiltscouncil.maps.arcgis.com/apps/webappviewer/index.html>

4.4 Sewer Flooding

Wessex Water records indicate there are no foul or surface water sewers in the vicinity of the site. Refer to the Wessex Water sewer records within appendix C. Risk of flooding from sewers is therefore low.

4.5 Groundwater

Wiltshire Council Strategic Flood Risk Data records the site as being in an area at negligible risk of groundwater flooding.

It is unlikely that groundwater levels could rise to the site surface and cause flooding, however if groundwater emergence was to occur, groundwater would follow the existing site topography, which falls to the north towards the existing watercourse and flow off site without accumulating or causing flooding at the site.

Flood risk from groundwater is therefore considered to be low.

4.6 Flooding from reservoirs or other artificial sources

The EA Map 'Extent of Flooding from Reservoirs' indicates that the site is not at risk from flooding from a reservoir breach. Therefore, flood risk from artificial sources is considered to be Very low.

4.7 Summary of Flood Risk

Source of Potential Flooding to the Development Site	Flood Risk	Mitigation/Comments
Fluvial	Low	The site is located within Flood Zone 1 (Low Risk)
Public Sewers / Drains	Low	Wessex Water records indicate there to be no sewers within the vicinity of the site.
Overland Flow	Low	EA mapping indicates the site is at Low risk of flooding from this source.
Groundwater	Low	EA mapping indicates the site is at Low risk of flooding from this source.
Artificial Sources	Very Low	EA Mapping confirms that the site is not located within an area at risk of flooding from this source.

5.0 Surface Water Drainage Proposals

Refer to DC Drawing 1179-01-01 [Proposed Drainage Strategy] in Appendix A.

This section of the report addresses how flood risk generated as a result of developing the site will be managed using sustainable control measures to ensure that flood risks are not increased on and off the site and how surface water is managed post development

5.1 Drainage Hierarchy

The surface water drainage strategy for the site has been prepared in accordance with the drainage discharge hierarchy from CIRIA C753 The SuDS Manual, where the first available option will be utilised as follows.

Technique		Summary
1	Infiltration to the maximum extent that is practical	Impermeable mudstone and sandstone soils restrict the use of infiltration solutions.
2	To surface water body	Proposed solution – Flows attenuated with a restricted discharge to an existing watercourse.
3	To a surface water sewer, highway drain or another drainage system	Not required
4	To a combined sewer	Not required

5.2 Infiltration Design

Poor infiltration properties of the Mudstone and Sandstone soils below the site precludes the use of soakaways and infiltration devices as the main source of surface water discharge. Permeable surfaces and sub-bases will however be utilised where possible to maximise any available pockets of infiltration, before overflowing to attenuation features.

5.3 Attenuation Design

In accordance with Wiltshire Council's Level 1 Strategic Flood Risk Assessment, the maximum permitted discharge rate from the proposed development should be limited to the greenfield runoff rate for equivalent rainfall events, up to and including the 1% Annual Exceedance Probability (AEP), 1 in 100-year plus climate change return period event.

Based on the existing run-off rates, as generated by Autodesk's Infodrainage software tabled below, this equates to a QBAR discharge rate of 0.4l/s for the proposed developed area.

ICP SUDS / IH 124					
Details					
Method	ICP SUDS				
Area (ha)				0.715	
SAAR (mm)				750.0	
Soil				0.1	
Region	Region 7				
Urban				0	
Return Period (years)				0	
Results					
Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)
Region 7	0.1	0.1	0.1	0.3	0.4

Calculations indicate SuDS attenuation of 970m³ will be required to accommodate the proposed development, for all storm events up to and including the 100year + 45% climate change event with a peak outflow rate restricted to 0.4 l/s.

Open space availability allows for the use of 'green' SUDS, as such a open attenuation basin is proposed to capture runoff from the proposed site and buildings, with permeable stone surfacing making up the access road and external areas, overflowing to the attenuation basin. For the purposes of design, all catchment is included within the attenuation basin.

A flow restricted outfall of 0.4 l/s discharges to the existing watercourse to the north via a Hydrobrake flow control chamber.

Refer to the Infodrainage design calculations enclosed in appendix D of this report.

5.4 Design Exceedance

Should the drainage system and SuDS fail under extreme events or blockages, floodwaters will follow the natural topography toward the watercourse to the north of the site, mirroring the existing arrangement. Exceedance routes are shown on DC drawing 1179-01-01 [Proposed Drainage Strategy] in Appendix A

5.5 Water Quality

Whilst roof water is considered fairly uncontaminated, runoff from the access road and hardstanding between buildings can be susceptible to hydrocarbon contamination, which in this instance will be intercepted within the permeable stone surfacing and attenuation basin.

Water quality has been assessed in line with the Simple Index approach from Chapter 26 of CIRIA C753 The SuDS Manual:

1. Step 1 – Allocate suitable pollution hazard indices for the proposed land use.
2. Step 2 – Select SuDS with a total pollution mitigation index that equals or exceeds the pollution hazard index.

Pollution Hazard Indices

Below figures are obtained from Table 26.2 CIRIA C753 The SuDS Manual '*Pollution hazard indices for different land use classifications*'

Land use	Pollution hazard level	Total suspended solids (TSS)	Metals	Hydrocarbons
Commercial yard and delivery areas.	Medium	0.7	0.6	0.7

Pollution Mitigation Indices

Where the mitigation index of an individual component is insufficient, two components (or more) in series will be required, where Total SuDS mitigation index = mitigation index 1 + 0.5 (mitigation index 2)

Below figures are taken from Table 26.3 & 26.4 CIRIA C753 The SuDS Manual '*Indicative SuDS mitigation indices for discharge to surface waters and groundwater*'

SuDS Feature	Total suspended solids (TSS)	Metals	Hydrocarbons
Permeable Pavement	0.7	0.6	0.7
Pond	0.35 (0.7/2)	0.35 (0.7/2)	0.25 (0.5/2)
Total	1.05	0.90	1.05

The pollution mitigation indices for the proposed drainage system therefore exceed the pollution hazard indices and provide suitable pollutant removal.

5.6 Adoption

The onsite drainage components will remain private and be the maintenance responsibility of the property owners.

5.7 Maintenance

Regular inspection and maintenance are important for the effective operation of the SuDS systems as designed. The regular maintenance of SuDS should be carried out in line with C753 The SUDS Manual (CIRIA, November 2015) or as per manufacturer's specification.

Pipework and Catchpits

It is not envisaged that silt build up within the pipework systems will require rigorous maintenance. Notwithstanding this, a suitable maintenance regime for the systems will comprise of routine inspection (every six months) and silt removal as necessary.

Permeable Paving/Surfacing

Permeable surfaces need to be regularly cleaned of silt and other sediments to preserve the infiltration capability of the surface. Suggested brushing regime would be at the end of winter to collect winter debris, mid-summer to collect dust, vegetation and grass-type deposits and after autumn leaf fall.

Attenuation Basin

It is not anticipated that the attenuation basin will require a rigorous maintenance regime, so long as silt is removed from pre-basin catchpits on a regular basis. The main maintenance requirement is mowing to retain grass lengths of 75-150mm across the treatment surface to assist in pollutant filtering and sediment retention. Other maintenance activities are likely to include removal of litter, debris and sediments as required.

6.0 Foul Water Drainage Proposals

Refer to DC Drawing 1179-01-01 [Proposed Drainage Strategy] in Appendix A.

6.1 Proposal

Asset records obtained from Wessex Water indicate there to be no public foul water sewers within the vicinity of the site.

Foul flows developed by the proposed temporary agricultural workers dwelling will discharge to a modern sewerage treatment plant, discharging to the watercourse along the eastern boundary of the site, via a sample chamber for monitoring of water quality in accordance with the Environment Agency's standard binding rules for sewerage discharge.

The foul discharge meets the Environment Agency's General Binding Rules requirements for discharge to watercourses.

Foul generated by the cows within the beef unit will be captured by straw bedding within the livestock building, with soiled bedding stored and then removed and spread on the land or taken elsewhere, depending on the time of year.

6.2 Adoption

The foul water drainage components will remain private and be the maintenance responsibility of the property owners.

6.3 Maintenance

All proposed foul drainage infrastructure will be maintained privately. The package treatment plant will be managed in accordance with the manufacturer's instructions.

7.0 Conclusion

The proposed new buildings and external areas will generate an increase in impermeable surface area when compared to existing. Surface water runoff from the new impermeable surfaces will be directed to sustainable drainage systems that dispose of water via restricted rate to the adjacent watercourse maintaining the existing greenfield runoff rates.

On site storage will be provided within an attenuation basin Sustainable Urban Drainage System (SUDS). The SUDS features have been designed to accommodate the 1 in 100 year storm, including a 45% allowance for climate change, in accordance with NPPF guidance.

Foul flow from the proposed workers dwelling will drain into a new package treatment plant, with treated water then discharging into the watercourse bounding the north east of the site.

The foul discharge meets the Environment Agency's General Binding Rules requirements for discharge to watercourses. Foul generated by the proposed beef unit will be captured separately for farming use.

The proposed SUDS and all other foul and surface water drainage components will be maintained by the owners of the property. The foul water package treatment plant will be maintained and managed in accordance with the manufacturer's instructions.

This study has been undertaken in accordance with the principles set out in NPPF and we can conclude that providing the development adheres to the conditions advised in this report, the proposals can be accommodated without increasing flood risk within the locality in accordance with objectives set by Central Government and the EA and meet the requirements set out by the Wiltshire Council Core Policy 67: Flood Risk.

Dixon Civils Ltd

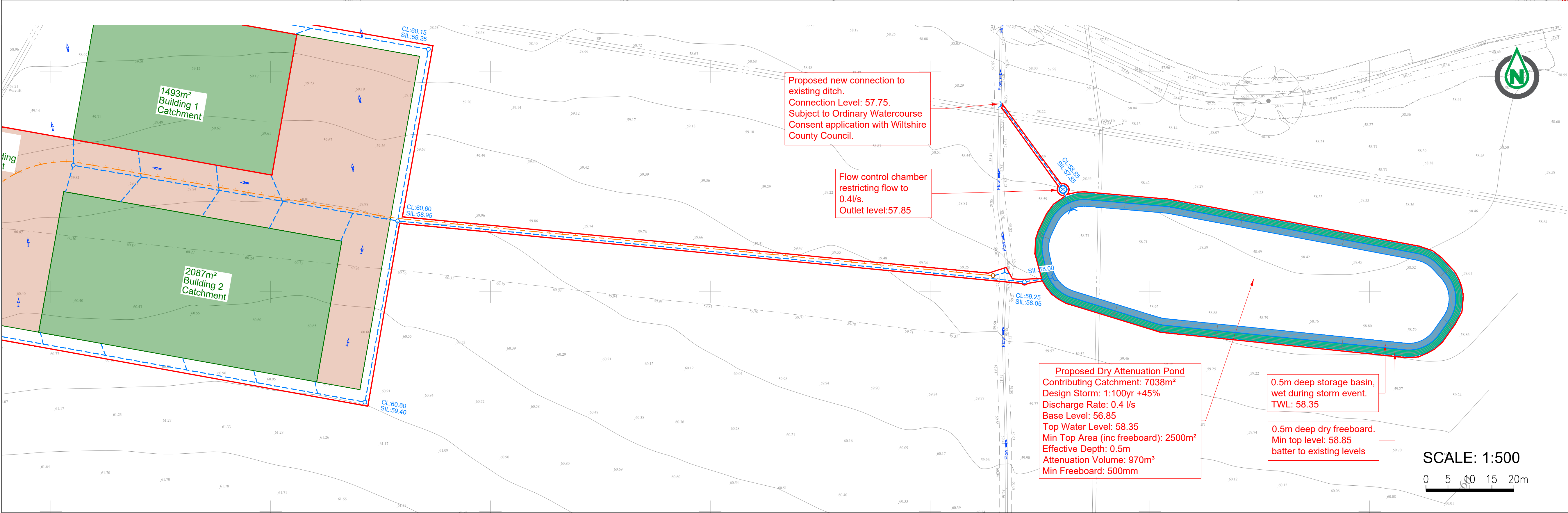
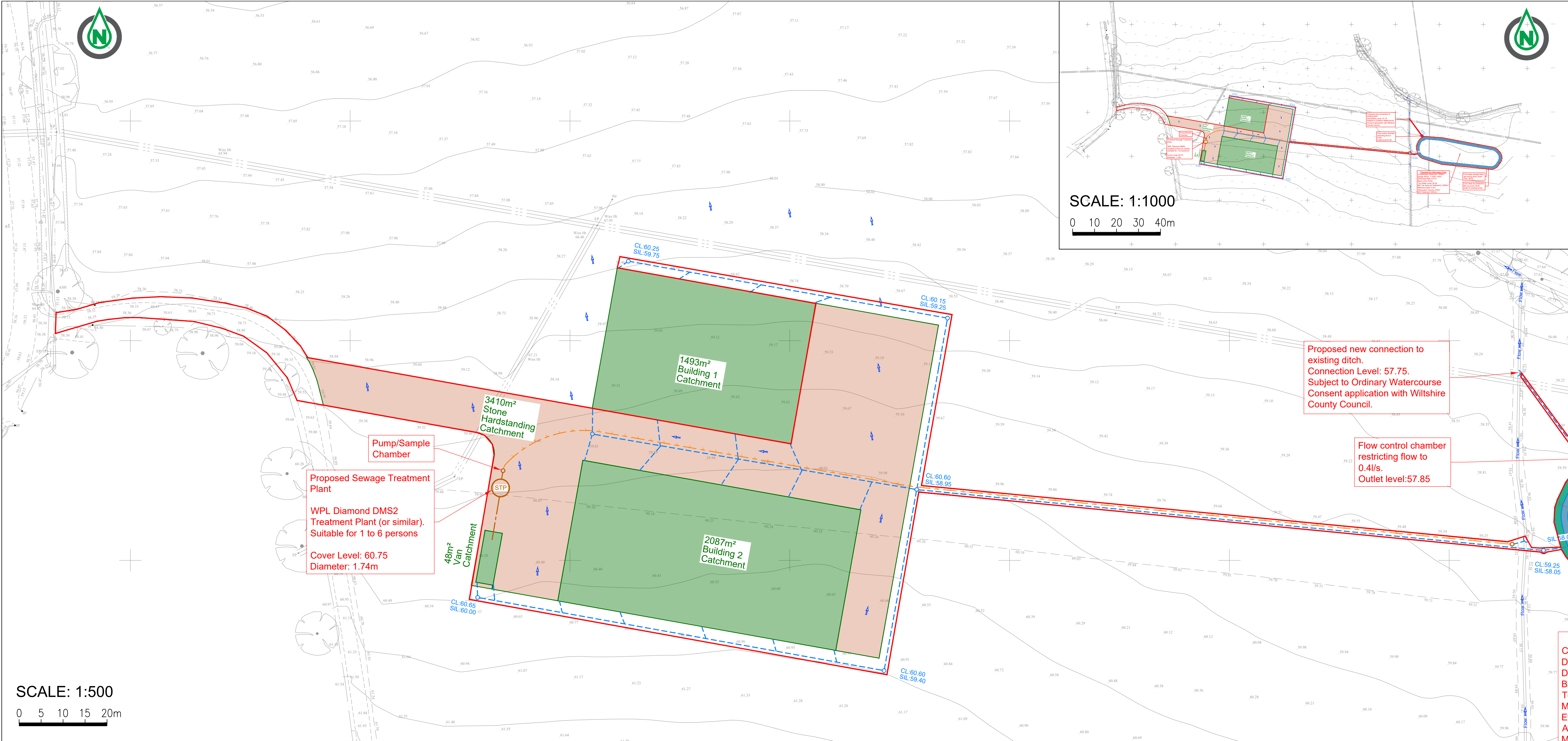
Issue 3 - November 2025

APPENDIX A

Drawings By Dixon Civils

Drawing ref. 1179-01-01

Proposed Drainage Strategy



KEY:

- Proposed Private Surface Water Drain & Inspection Chamber
- Flood Flow Route (Over 1:100yr+45% storm event)
- Proposed Surface Water Headwall
- Proposed Flow Control Chamber
- SW Catchment Areas
- Stone Surfacing
- Proposed Private Foul Water Drain & Inspection Chamber
- Proposed Private Sewage Treatment Plant
- Foul Water Rising Main
- Site Boundary

NOTES:

- Do not scale from this drawing.
- Drawing issued for planning purposes only, not for construction.
- This drawing is based on topographical survey compiled by Building Design Surveys Ltd.
- All levels are in metres and relate to the ordnance survey level.
- Do not scale from this drawing. Work only from stated dimensions or setting out details.
- This drawing shall be read in conjunction with all relevant scheme drawings and specifications.
- At the commencement of works the contractor is to carry out trial pits and liaise with Utility Companies in order to establish the exact position of all existing statutory utility plant in the vicinity of works and take adequate precautions for their protection.
- All foul and storm water drains must be constructed in accordance with the latest Building Regulations, BS8301, and, where appropriate, the relevant agreement certificates.
- 100mmØ private foul water drainage shall be laid no flatter than 1 in 80. 150mmØ pipework shall be laid no flatter than 1 in 150.

B	Site boundary and foul routing updated	AD	16.11.25
A	Drainage levels and surfacing updated	AD	13.10.25
-	FIRST ISSUE (DRAFT)	AD	01.10.25
Rev	Description	Dm	Date

REVISIONS

Dixon Civils
Drainage & Infrastructure Design
www.DixonCivils.com 01249 490813 Enquiries@DixonCivils.com

CLIENT
J R Bodman Ltd

JOB TITLE
Proposed Beef Unit
Wyre Farm
Worton, Devizes

DRAWING TITLE
Proposed Drainage Strategy

DWG	OCT 2025	DRAWN	AD
DRAWING NO.	1179-01-01	REV	B
SCALE	As Shown @ A1		

PRELIMINARY ISSUE

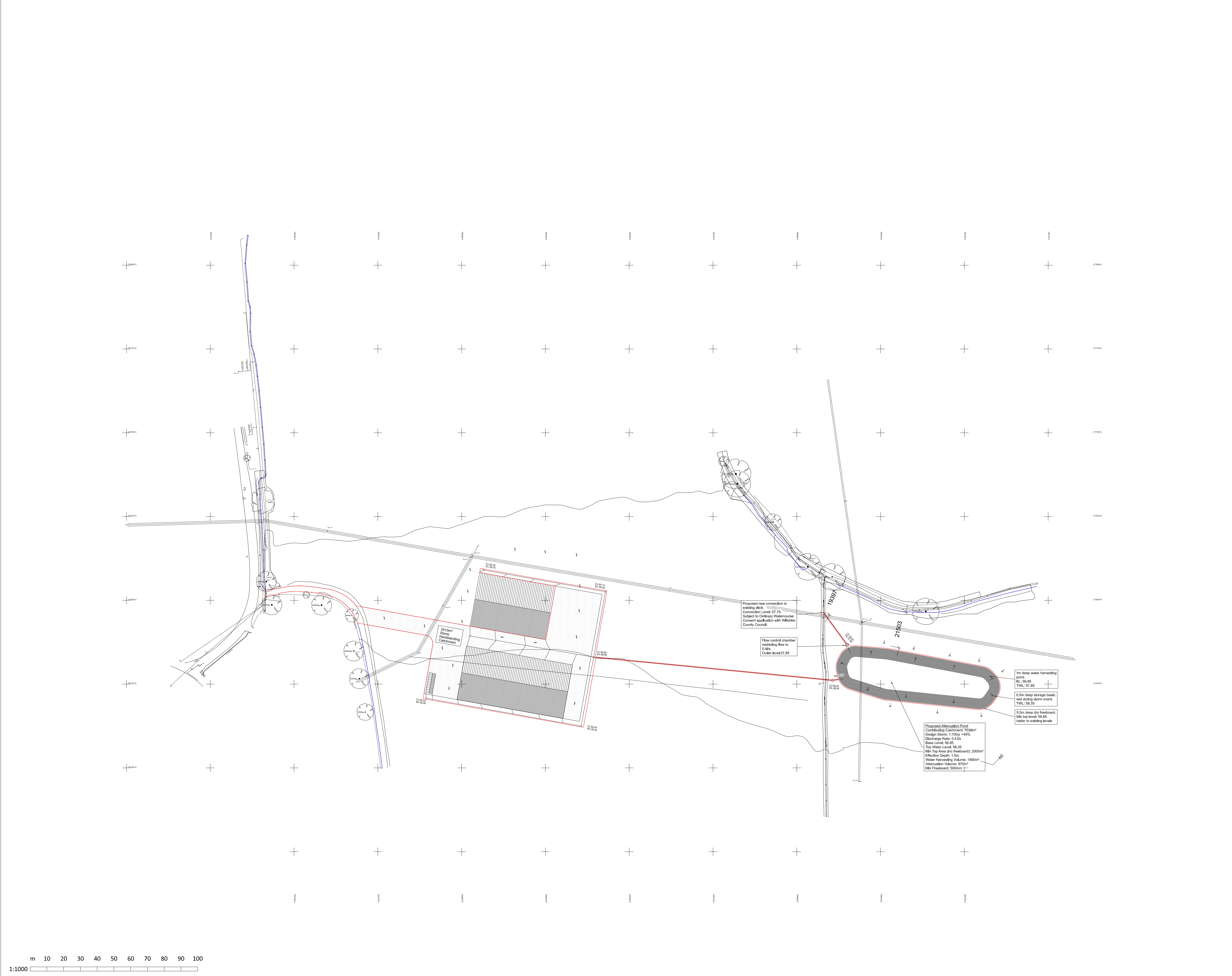
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APPENDIX B

Drawings By Others

Existing Site Layout (Building Design Surveys Ltd)

Proposed Site Layout (Falcon Building Consultants)



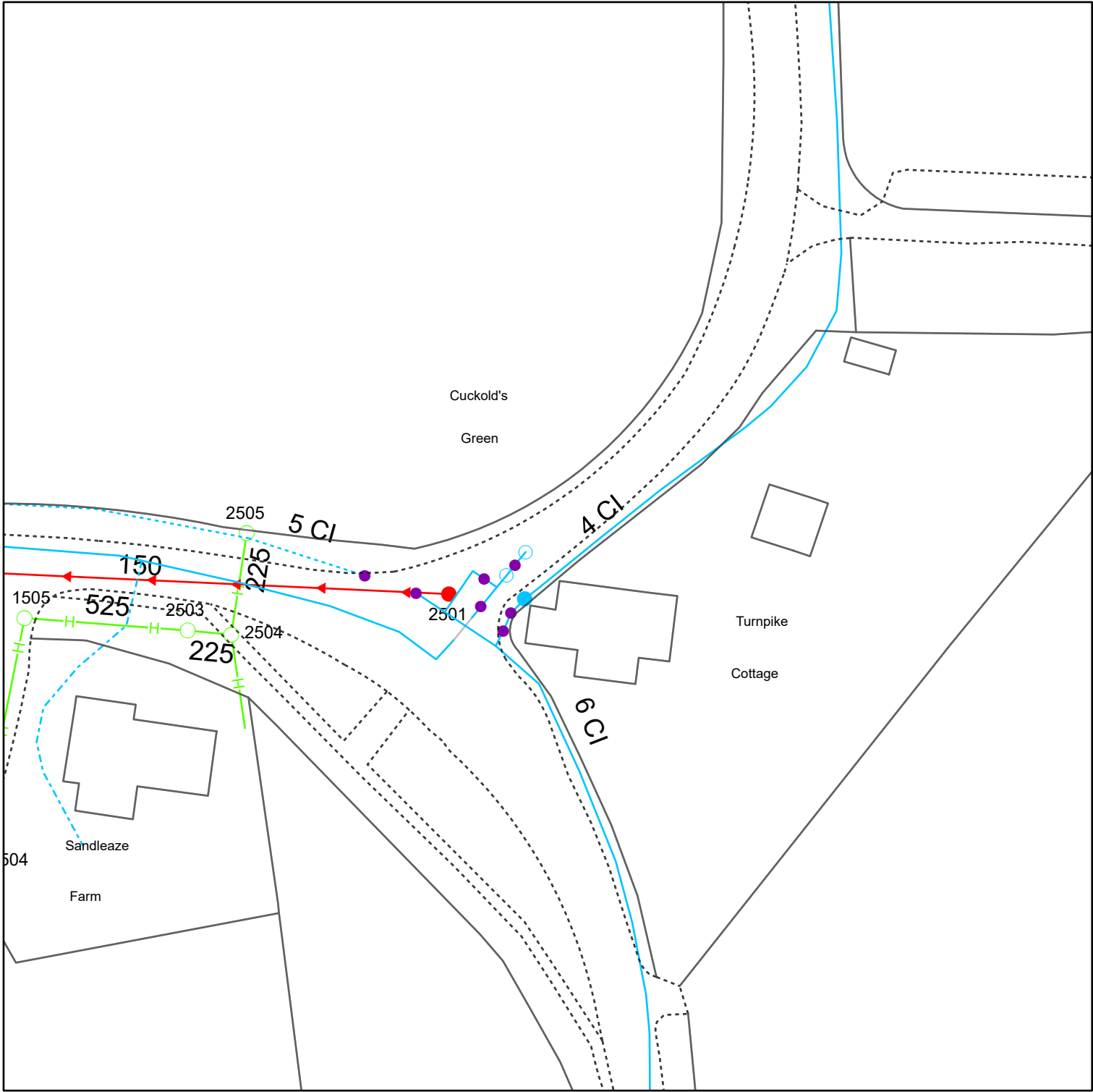
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PARTY WALL ACT
UNDER THE REQUIREMENTS OF THE PARTY WALL ACT, THE OWNER HAS A DUTY TO SERVE NOTICE ON ANY ADJOINING OWNER IF THE PROPOSEDBUILDING WORK INVOLVES WORKS ON OR TO AN EXISTING PARTY WALL. A PARTY WALL AGREEMENT IS TO BE IN PLACE PRIOR TO COMMENCING WORKS ON SITE.
CONSTRUCTION (DESIGN & MANAGEMENT) REGULATIONS 2015
FALCON BUILDING CONSULTANTS DOES NOT ACT AS PLANNING SUPERVISOR. THE ATTENTION OF THE CLIENT AND THE PRINCIPAL CONTRACTOR IS DRAWN TO THE ABOVE REGULATIONS.
IF THESE REGULATIONS ARE APPLICABLE TO THIS PROJECT THE DUTIES OF THE CLIENT AND THE PRINCIPAL CONTRACTOR MUST BE SATISFIED.
THIS DESIGN HAS BEEN UNDERTAKEN WITH REASONABLE SKILL, CARE AND DILLIGENCE TO ELIMINATE FORESEEABLE RISKS TO THE HEALTH AND SAFETY OF EXPERIENCED PERSONS INVOLVED IN THE CONSTRUCTION CLEANING OR MAINTAINING OF THE PROJECT.
THERE ARE NO SPECIAL OR EXTRAORDINARY RISKS ASSOCIATED WITH THIS DESIGN, SUBJECT TO THE CONDITIONS THAT THE PRINCIPAL CONTRACTOR IS SUITABLY EXPERIENCED IN PROJECTS OF THIS TYPE AND SIZE AND WITH THE TYPE OF CONSTRUCTION WORK WHICH WILL BE INVOLVED.

NO	DATE	REVISIONS
FALCON BUILDING CONSULTANTS 3 Creves Cottage, New Road, Studley, Wiltshire SN11 9LX Mob 07774 945251		
CLIENT MR. J. BODMAN		
JOB TITLE PROPOSED FEED/SILAGE STORE WYRE FARM WORTON		
DRAWING TITLE PROPOSED BLOCK PLAN CATTLE BUILDING © Crown copyright and database rights 2025 OS Licence 100035409		
SCALE 1:1000		A1
DATE FEB 2025		DRAWN BY KV
ORG NO 25/0100/PR2 CB		Rev

APPENDIX C

Wessex Water Sewer Records

Wessex Water Network Map



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WATER MAINS		SEWERS	STRATEGIC PUBLIC	PRIVATE	SECTION 104	OTHER WESSEX PIPES	NON-WESSEX / UNKNOWN
Distribution	Washout	Foul	Surface	Combined	Abandoned	Rising Mains	Private Rising Mains
Raw Water	Abandoned	Surface	Combined	Abandoned	Private	Standby Rising Mains	Culverted Watercourse
Private		Abandoned	Abandoned	Abandoned		EDM Effluent Disposal	Highway Drain
		Abandoned	Abandoned	Abandoned		Overflow	Use Unknown
		Abandoned	Abandoned	Abandoned		Syphon	Status Unknown
FITTINGS		STRUCTURES		OTHER STRUCTURES		OTHER STRUCTURES	
Hydrant	Other	Manhole - Foul	Manhole - Surface	Manhole - Combined	Inlet	Attenuation Tank	Chamber
		Lamphole	Outfall	Bifurcation - Foul	Bifurcation - Surface	Storage Tank	Tunnel
		Bifurcation - Combined	Combined Sewage Overflow	Air Valve	Hatch Box		Interceptor
		Non Return Valve	Washout				
		Air Valve					
		Vent Column					
		Rodding Eye					
		Catchpit					
		Flushing Chamber					
		Soakaway					
		Non Return Valve					
		Air Valve					
		Hatch Box					
		Washout					
Information in this map is provided for identification purposes only. No warranty as to accuracy is given or implied. The precise route of pipe work may not exactly match that shown. Wessex Water does not accept liability for inaccuracies. Sewers and lateral drains adopted by Wessex Water under the Water Industry (Schemes for Adoption of Private Sewers) Regulations 2011 are to be plotted over time and may not yet be shown. In carrying out any works, you accept liability for the cost of any repairs to Wessex Water apparatus damaged as a result of your works. You are advised to commence excavations using hand tools only. Mechanical digging equipment should not be used until pipe work has been precisely located. If you are considering any form of building works and pipe work is shown within the boundary of your property or a property to be purchased (or very close by) a surveyor should plot its exact position prior to commencing works or purchase. If you are proposing to build over or near Wessex Water's apparatus you should contact the Developer Services Team, tel: 01225 526333 or e-mail: developer.enquiries@wessexwater.co.uk to discuss your proposals. Details of assets within Wessex Water's land ownership are unavailable through this service.							
Date: 16/01/2025		Centre: 398250E, 157565N		Scale: 1:625		(when printed at A4 size)	

APPENDIX D

Calculations

Project: Wyre Farm Attenuation Design	Date: 30/09/2025	 Dixon Civils Drainage & Infrastructure Design	
Report Details: Type: Stormwater Controls Storm Phase: Phase	Designed by: AD <table border="1"> <tr> <td>Checked by:</td> <td>Approved By:</td> </tr> </table>		Checked by:
Checked by:	Approved By:		
Designed:: Dixon Civils Ltd			



Attenuation Pond

Type : Pond

Dimensions

Exceedance Level (m)	58.000
Depth (m)	1.000
Base Level (m)	57.000
Freeboard (mm)	500
Initial Depth (m)	0.000
Porosity (%)	100
Average Slope (1:X)	3.00
Total Volume (m³)	1058.846

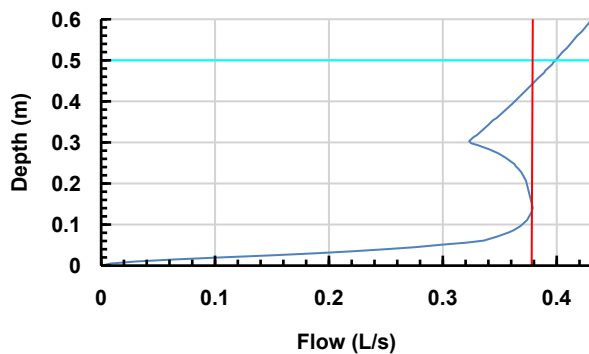
Depth (m)	Area (m²)	Volume (m³)
0.000	1996.54	0.000
1.000	2500.00	2243.557

Outlets

Outlet

Outgoing Connection	(None)
Outlet Type	Hydro-Brake®
Invert Level (m)	57.000
Design Depth (m)	0.500
Design Flow (L/s)	0.4
Objective	Minimise Upstream Storage Requirements
Application	Surface Water Only
Sump Available	☒

Unit Reference SHE-0034-4000-0500-4000



Advanced

Perimeter	Circular
Length (m)	93.791
Friction Scheme	Manning's n
n	0.03

Project: Wyre Farm Attenuation Design	Date: 30/09/2025			 Dixon Civils Drainage & Infrastructure Design
	Designed by: AD	Checked by:	Approved By:	
Report Title: Rainfall Analysis Criteria	Designed:: Dixon Civils Ltd			

Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Apply Global Value
Urban Creep Global Value (%)	0
Junction Flood Risk Margin (mm)	300
Perform No Discharge Analysis	☐

Project: Wyre Farm Attenuation Design	Date: 30/09/2025		
	Designed by: AD	Checked by:	Approved By:
Report Title: UK and Ireland Rural Runoff Calculator	Designed:: Dixon Civils Ltd		


Dixon Civils
 Drainage & Infrastructure Design

ICP SUDS / IH 124

Details

Method	ICP SUDS
Area (ha)	0.715
SAAR (mm)	750.0
Soil	0.1
Region	Region 7
Urban	0
Return Period (years)	0

Results

Region	QBAR Rural (L/s)	QBAR Urban (L/s)	Q 1 (years) (L/s)	Q 30 (years) (L/s)	Q 100 (years) (L/s)
Region 7	0.1	0.1	0.1	0.3	0.4

Project: Wyre Farm Attenuation Design	Date: 30/09/2025		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Inflows Summary Storm Phase: Phase	Designed:: Dixon Civils Ltd		



Critical Storm Per Item: Rank By: Max. Inflow

Inflow	Storm Event	Inflow Area (ha)	Max. Inflow (L/s)	Total Inflow Volume (m³)
Catchment Area	FSR: 100 years: +45 %: 15 mins: Winter	0.72	439.0	210.282

Project: Wyre Farm Attenuation Design		Date: 30/09/2025		 Dixon Civils Drainage & Infrastructure Design	
		Designed by: AD	Checked by:		Approved By:
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase		Designed:: Dixon Civils Ltd			



Critical Storm Per Item: Rank By: Max. Avg. Depth

Stormwater Control	Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Half Drain Down Time (mins)	Percentage Available (%)
Attenuation Pond	FSR: 100 years: +45 %: 10080 mins: Winter	57.460	57.460	0.460	0.460	4.8	969.660	0.000	0.000	0.4	405.386	21586	8.423

Project: Wyre Farm Attenuation Design	Date: 30/09/2025			 Dixon Civils Drainage & Infrastructure Design
	Designed by: AD	Checked by:	Approved By:	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Designed:: Dixon Civils Ltd			

Status

OK

Project: Wyre Farm Attenuation Design	Date: 30/09/2025		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Control Results Storm Phase: Phase	Designed:: Dixon Civils Ltd		

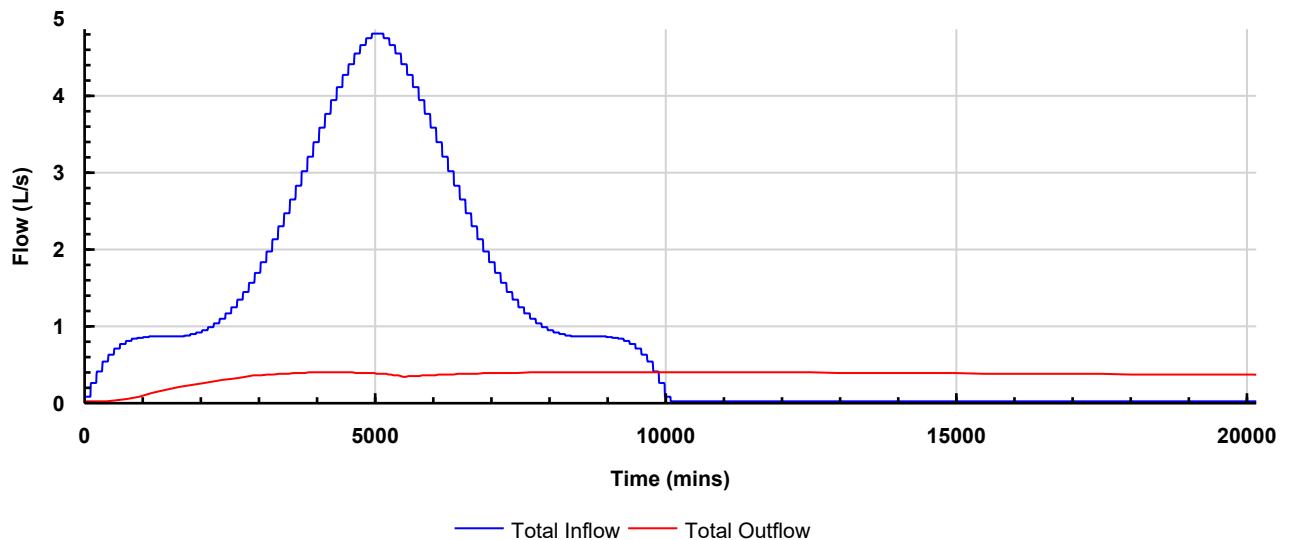


Attenuation Pond
Critical Storm: FSR: 100 years: Increase Rainfall (%): +45: 10080 mins: Winter

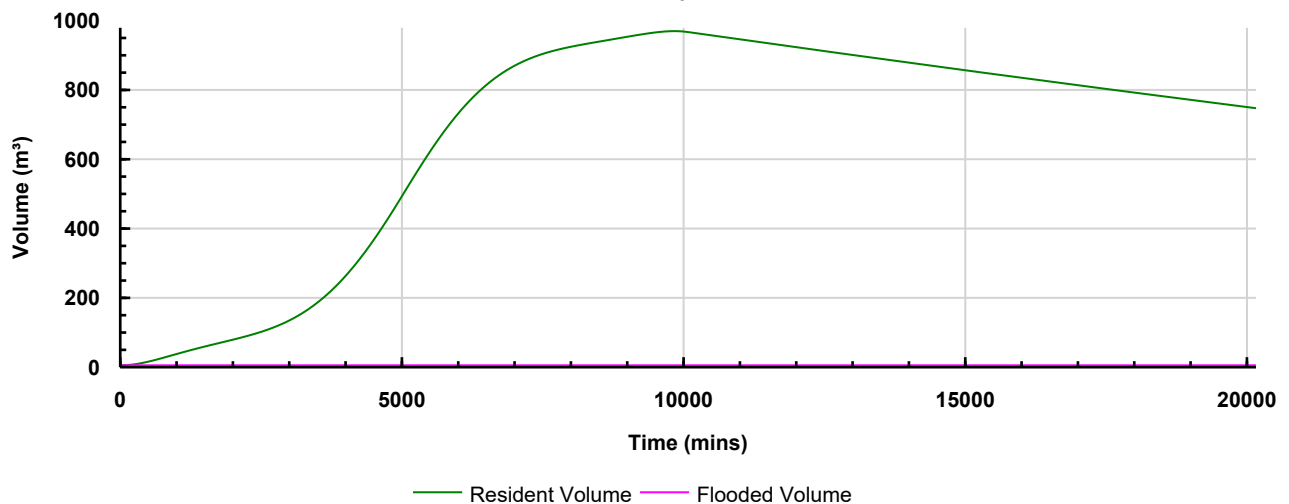
Type : Pond

Graphs

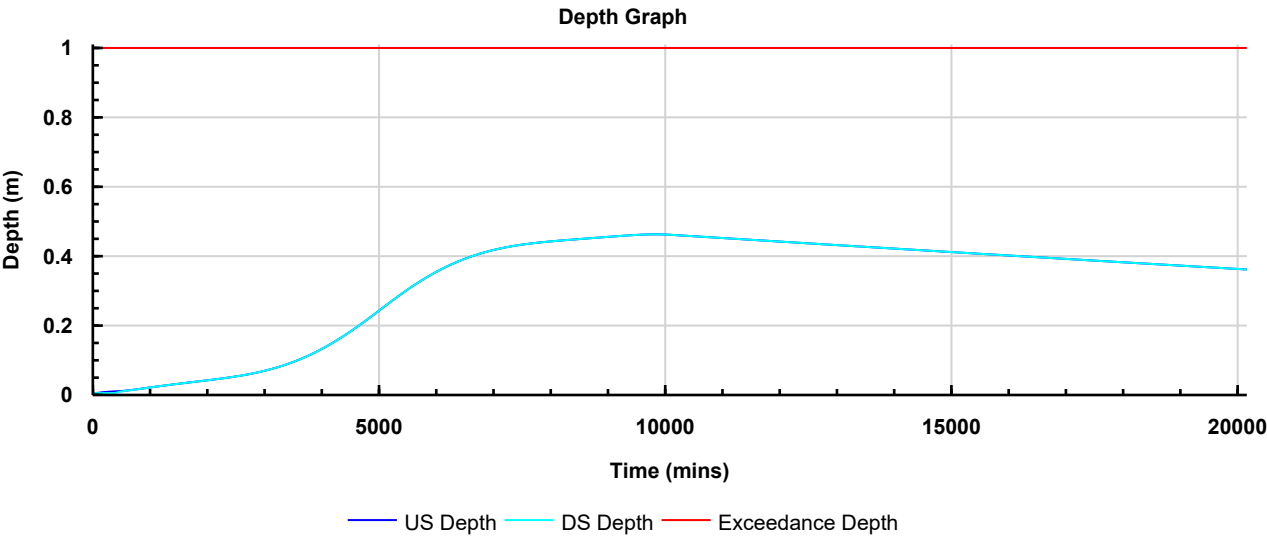
Flow Graph



Volume Graph



Project: Wyre Farm Attenuation Design	Date: 30/09/2025		
	Designed by: AD	Checked by:	Approved By:
Report Details: Type: Stormwater Control Results Storm Phase: Phase	Designed:: Dixon Civils Ltd		





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